



County of San Diego
Department of Environmental Health and Quality
Land and Water Quality Division

Local Agency Management Program (LAMP) for Onsite Wastewater Treatment Systems

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LAMP REVISIONS HISTORY				
Effective Date	Revision	Description	Board of Supervisor Approval Date	San Diego Regional Board Approval Resolution/Date
7/24/2015	Original Version V1	Original LAMP	Consent 6/24/2015	Executive Officer Letter 4/29/2015
	Revision V2	Revisions to original LAMP based on results of 5-Year LAMP evaluation and stakeholder feedback		

CHAPTER 1.0 INTRODUCTION

1.1 Introduction

The Local Agency Management Program (LAMP) is the culmination of the actions required by Assembly Bill 885 (AB 885). AB 885 was introduced to the California State Assembly on February 25, 1999 and approved on September 27, 2000. This legislation directed the State Water Resources Control Board (SWRCB) to develop regulations or standards for onsite wastewater treatment systems (OWTS) to be implemented by qualified local agencies. The SWRCB adopted the Water Quality Control Policy for Siting, Design, Operation, and Maintenance of Onsite Wastewater Treatment Systems on June 19, 2012 (OWTS Policy). The policy was subsequently approved by the Office of Administrative Law on November 13, 2012 and became effective on May 13, 2013. As required by the OWTS Policy, the OWTS Policy was subsequently adopted into **the California Code of Regulations, sections 2924 and 3991.1, and** the Water Quality Control Plans (Basin Plans) for the Colorado River Basin and San Diego Regional Water Quality Control Boards (**RWQCB**). In addition to local ordinance authority to permit the installation of OWTS to address health and safety concerns, the OWTS Policy provides the option for a local agency to submit a LAMP (a Tier 2 program under the OWTS Policy) to the ~~Regional Water Quality Control Board (RWQCB)~~ for approval, and, upon approval, manage the installation of new and replacement OWTS to the extent authorized under that program and local authority. LAMPs approved under Tier 2 of the OWTS Policy provide an alternative method from Tier 1 programs to achieve the same policy purpose, which is to protect water quality and public health. In order to address local conditions, LAMPs may include standards that differ from Tier 1 requirements contained in Sections 7 and 8 of the OWTS Policy for new and replacement OWTS.

The purpose of the LAMP is to **implement minimum standards** to allow the continued use of OWTS **or other sanitation facilities** while protecting water quality and public health. To address the diverse range of geological and climatic conditions in the region, the County of San Diego (COSD), Department of Environmental Health and Quality (DEHQ) developed a LAMP as an alternative method to the Tier 1 requirements for new and replacement OWTS. The COSD LAMP was approved by the ~~San Diego Regional Water Quality Control Board (RWQCB)~~ on April 27, 2015 and by the County Board of Supervisors on July 24, 2015. **This LAMP is effective unless withdrawn by DEHQ or revoked by the RWQCB and may be modified or updated upon approval by the COSD Board of Supervisors and the RWQCB.**

The OWTS installation permitting process and the prescribed standards contained in the LAMP are designed to protect groundwater sources and surface water bodies from pollution through the proper siting, design installation, operation, and maintenance of individual new and replacement OWTS **and other sanitation facilities in accordance with the provisions of the OWTS Policy and San Diego County Regulatory Code.**

1.2 State/County Coordination

As OWTS discharge wastewater to the subsurface and may impact water quality, they are discharges regulated under California Water Code (CWC) section 13260, the Waste Discharge Requirements program implemented by the SWRCB and the RWQCBs. The SWRCB's OWTS Policy is regulation that was

required pursuant to CWC section 13290 et seq. and provides statewide minimum standards for small OWTS. The OWTS Policy includes a conditional waiver from the requirement for waste discharge requirements for OWTS owners when their OWTS meet the requirements and conditions contained within the OWTS Policy. OWTS are also typically regulated and permitted by local agencies under local ordinance authority derived from the California Constitution. CWC section 13002 provides that the provisions of CWC Division 7, the Porter-Cologne Water Quality Control Act (sections 13000-16104), and any ruling of the SWRCB, does not impose limitations on a county to adopt and enforce regulations not in conflict with these rules, or to declare, prohibit, and abate nuisances.

The OWTS Policy provides a tiered approach: Tier 0-Existing OWTS. No action needed except for those OWTS requiring Tier 3 or Tier 4 corrective actions; Tier 1-Statewide minimum siting, design, and construction standards for new and replacement OWTS; Tier 2-Allows for local agencies to implement alternative minimum standards to Tier 1 for new and replacement OWTS with an equivalent level of protection through a LAMP approved by the RWQCBs; Tier 3-Provisions for OWTS near impaired water bodies; and Tier 4-Requirements for OWTS needing corrective action.

The OWTS Policy and any OWTS provisions adopted in the RWQCB's Basin Plans provide the State's requirements for small OWTS regulation. State and Regional Boards are responsible for implementation of the OWTS Policy with local implementation of minimum standards contained in Tier 1 or Tier 2 for siting and design; minimum, interim actions in Tier 3 for OWTS near impaired water bodies; and minimum requirements for OWTS needing corrective actions in Tier 4. All actions by a local agency are limited to the extent of their local authority.

The COSD DEHQ implements a local OWTS permitting program for new and replacement OWTS **and other sanitation facilities** under local ordinance and a OWTS Policy Tier 2 program LAMP approved by the **San Diego Regional Water Quality Control** Board in 2015. The local program includes provisions for corrective action when an OWTS has defective components and OWTS that are no longer containing waste to the subsurface. Except for implementing the minimum standards in the LAMP for new and replacement OWTS, the local program does not contain special provisions for addressing OWTS near impaired water bodies. OWTS near impaired water bodies are best regulated under the Federal and State programs established for these water bodies, such as the Total Maximum Daily Load and Waste Discharge Requirements programs. The local program does provide for the interim regulation of new and replacement OWTS near impaired water bodies identified in Attachment 2 of the OWTS Policy, at such time any are listed, until a **Total Maximum Daily Load (TMDL)** implementation plan is adopted by the water boards.

1.3 Definitions

"303 (d) list" means the same as **"Impaired Water Bodies."**

"Advanced Protection Management Program" or "APMP" means the minimum required management program for all OWTS located near a water body that has been listed as impaired due to nitrogen or pathogen indicators pursuant to Section 303(d) of the Clean Water Act. The geographical area of an APMP is defined by the applicable TMDL. If no TMDL has been adopted, by a LAMP only if the LAMP

contains special provisions for the impaired waterbody. If no adopted TMDL or LAMP with special provisions, then it is defined as 600 linear feet of a water body listed in Attachment 2 of the OWTS Policy, OWTS near impaired water bodies that are not listed in Attachment 2 of the OWTS Policy, and do not have a TMDL, and are not covered by a LAMP with special provisions, are not addressed by Tier 3 of the OWTS Policy.

“Alternative toilet” means a holding tank, vault, composting, chemical toilet or other approved means of sewage disposal for non-residential uses in isolated areas, such as campsites, parks or trails, when no public sewer is available and it is impracticable to connect water to the area where the toilet is to be located, or for temporary or occasional uses authorized by this Chapter or other County Code.

“Annual Operating Permit” means an annual operating permit issued to an owner of an OWTS where ongoing maintenance monitoring and report submittal is required.

“Basin Plan” means the same as “water quality control plan” as defined in Division 7 (commencing with Section 13000) of the Water Code. Basin Plans are adopted by each Regional Water Board, approved by the State Water Board and the Office of Administrative Law, and identify surface water and groundwater bodies within each Region’s boundaries and establish, for each, its respective beneficial uses and water quality objectives. Copies are available from the Regional Water Boards, electronically at each Regional Water Boards website, or at the State Water Board’s *Plans and Policies* web page (http://www.waterboards.ca.gov/plans_policies/).

“Bedrock” means the rock, usually solid, that underlies soil or other unconsolidated, surficial material.

“Cap/Cap depth” means the depth measured from natural ground surface to the top of the gravel-fill in deep bed and vertical seepage pit dispersal systems.

“CEDEN” means California Environmental Data Exchange Network and information about it is available at the State Water Boards website or <http://www.ceden.org/index.shtml>.

“Cesspool” means an excavation in the ground receiving domestic wastewater, designed to retain the organic matter and solids, while allowing the liquids to seep into the soil. Cesspools are not authorized under this LAMP. Cesspools differ from seepage pits because cesspool systems do not have septic tanks.

“Clay” means a soil particle; the term also refers to a type of soil texture. As a soil particle, clay consists of individual rock or mineral particles in soils having diameters <0.002 mm. As a soil texture, clay is the soil material that is comprised of 40 percent or more clay particles, not more than 45 percent sand and not more than 40 percent silt particles using the USDA soil classification system.

“Cobbles” means rock fragments 76 mm or larger using the USDA soil classification systems.

“Cut/Slope” means any natural slope greater than 60% or man-made contour that exposes the vertical soil profile.

“Defective system” (also referred to as a failing system) means an OWTS or other sanitation facility that allows sewage, human excrement, or other liquid wastes to be disposed of such that the waste is not confined underground, or within its tank is a defective system. or which An onsite wastewater treatment system or other sanitation facility that requires frequent pumping to remove accumulated wastes to in order to confine sewage underground is also a defective system whether or not pumping the system allows waste to be confined underground.

“Dispersal system” means a leach field, shallow or deep bed, seepage pit, subsurface drip field, or other type of dispersal system for subsurface discharge and final wastewater treatment.

“Domestic wastewater” means wastewater with a measured strength less than high-strength wastewater and is the type of wastewater normally discharged from, or similar to that discharged from, plumbing fixtures, appliances and other household devices including, but not limited to toilets, bathtubs, showers, laundry facilities, dishwashing facilities, and garbage disposals. Domestic wastewater may include wastewater from commercial buildings such as office buildings, retail stores, and some restaurants, or from industrial facilities where the domestic wastewater is segregated from the industrial wastewater. Domestic wastewater does not include wastewater consisting of RV holding tank wastewater, such as at RV dump stations. Domestic wastewater does not include wastewater from industrial processes.

“Domestic well” means a groundwater well that provides water for human consumption and is not regulated by the State Water Resources Control Board.

~~**“Drainage Course”** means a drainage consisting of native soils such as a natural swale or topographic depression which gathers or conveys runoff~~

“Dump station” means a facility intended to receive the discharge of wastewater from a holding tank installed on a RV. A dump station does not include a full hook-up sewer connection at an individual space in a recreational vehicle park.

“Effluent” means sewage, water, or other liquid, partially or completely treated or in its natural state, flowing out of a septic tank, supplemental treatment component, dispersal system, or other OWTS component.

“Electronic deliverable format” or “EDF” means the data standard adopted by the State Water Board for submittal of groundwater quality monitoring data to the State Water Board’s internet-accessible database system GeoTracker (<http://geotracker.waterboards.ca.gov/>).

“Ephemeral stream or drainage course” means a natural water course that carries only stormwater in direct response to precipitation with water flowing only during and shortly after large precipitation events. An ephemeral stream may or may not have a well-defined channel, the aquatic bed is always above the water table, and stormwater runoff is the primary source of water. An ephemeral stream typically lacks the biological, hydrological, and physical characteristics commonly associated with the continuous or intermittent conveyance of water. The term intermittent means surface water flowing

continuously during certain times of the year and more than in direct response to precipitation (e.g., seasonally when the groundwater table is elevated or when snowpack melts).

“Existing OWTS” means an OWTS that was constructed and operating prior to the effective date of the OWTS Policy, May 13, 2013, and OWTS for which a valid construction permit has been issued prior to the effective date of the OWTS Policy.

“Flowing water body” means a body of running water flowing over the earth in a natural water course, where the movement of the water is readily discernible or if water is not present it is apparent from review of the geology that when present it does flow, such as in an ephemeral drainage, creek, stream, or river.

“Groundwater” means water below the land surface that is at or above atmospheric pressure.

“Deep bed dispersal system” means a gravel filled excavation, four to six feet wide, six to seven feet deep with a cap depth of two to five feet, and length determined by the percolation rate of the soil, that receives the effluent discharge from a septic tank or other OWTS treatment unit for dispersal.

“High-strength wastewater” means wastewater having a 30-day average concentration of biochemical oxygen demand (BOD) greater than 300 milligrams-per-liter (mg/L) or of total suspended solids (TSS) greater than 330 mg/L or a fats, oil, and grease (FOG) concentration greater than 100 mg/L prior to the septic tank or other OWTS treatment component.

“IAPMO” means the International Association of Plumbing and Mechanical Officials.

“Impaired water bodies” means those surface water bodies or segments thereof that are identified on a list approved first by the State Water Board and then approved by US EPA pursuant to Section 303(d) of the federal Clean Water Act.

“Installation Permit” means a document issued by a local agency that allows the installation, construction, reconstruction, repair, addition, modification, or abandonment of an OWTS.

“Licensed installer” means a licensed General Engineering Contractor (Class A), General Building Contractor (Class B), Sanitation System Contractor (Specialty Class C-42), or Plumbing Contractor (Class C-36). Such licensed installer shall install all new and replacement OWTS in accordance with California Business and Professional Code Sections 7056, 7057, and 7058 and Article 3, Division 8, Title 16 of the California Code of Regulations.

“Local agency” means any subdivision of state government that has responsibility for permitting the installation of and regulating OWTS within its jurisdictional boundaries; typically a county, city, or special district.

“Major repair” means either: (1) for a dispersal system, repairs required for an OWTS dispersal system due to surfacing wastewater effluent from the dispersal field and/or wastewater backed up into

plumbing fixtures because the dispersal system is not able to percolate the design flow of wastewater associated with the structure served, or (2) for a septic tank, repairs required to the tank for a compartment baffle failure or tank structural integrity failure such that either wastewater is exfiltrating or groundwater is infiltrating.

“Mound system” means an aboveground dispersal system (covered sand bed with effluent leach field elevated above original ground surface inside) used to enhance soil treatment, dispersal, and absorption of effluent discharged from an OWTS treatment unit such as a septic tank. Mound systems have a subsurface discharge.

“New OWTS” means an OWTS permitted after the effective date of the OWTS Policy, May 13, 2013. that is not a repair or a replacement.

“Nonconforming OWTS” means any existing, replaced, or repaired OWTS that does not conform to the current requirements related to system sizing, setbacks, groundwater separation, or allowable cover.

“NSF” means NSF International (a.k.a. National Sanitation Foundation), a not for profit, non-governmental organization that develops health and safety standards and performs product certification.

“Oil/grease interceptor” means a passive interceptor that has a rate of flow exceeding 50 gallons-per-minute and that is located outside a building. Oil/grease interceptors are used for separating and collecting oil and grease from wastewater.

“Onsite wastewater treatment system(s)” or “OWTS” means ~~a system on a property not connected to a public sewer, that treats and disposes of sewage and other wastes produced on the property where the system is located using individual disposal systems, community collection and disposal systems, and alternative collection and disposal systems that use subsurface dispersal.~~ The short form of the term may be singular or plural. OWTS includes a standard system consisting of a septic tank with effluent discharging into a subsurface dispersal system of a leach trench dispersal system, a chamber dispersal system, or a seepage pit dispersal system. OWTS also includes an OWTS with supplemental treatment. OWTS do not include “gray water” systems pursuant to Health and Safety Code Section 17922.12.

“Owner-Builder” means an individual or group of individuals who own the property on which they plan to construct, alter, repair, improve, or remodel a building or structure or appurtenance thereto in accordance with the requirements and limitations set forth in the Business and Professions Code, Section 7044. Business and Professions Code, Section 7026.1 defines any person who acts as consultant to an owner-builder is a contractor.

“Percolation test” means a method of testing water absorption of the soil. The test is conducted with clean water and test results can be used to establish the dispersal system design taking into consideration differences between hydraulic loading rates and the infiltrative rates associated with wastewater, especially higher strength wastewater.

“Person” means any individual, firm, association, organization, partnership, business trust, corporation, company, State agency or department, or unit of local government who is, or that is, subject to this LAMP.

“Pit-privy” (outhouse, pit-toilet, vault) means self-contained waterless toilet used for disposal of non-water carried human waste; consists of a shelter built above an underground self-contained, water-tight, concrete-lined tank, pit or vault in the ground into which human waste is discharged.

“Policy” means the *State Water Resources Control Board’s Water Quality Control Policy for Siting, Design, Operation and Maintenance of Onsite Wastewater Treatment Systems*.

“Pollutant” means any substance that alters water quality of the waters of the State to a degree that it may potentially affect the beneficial uses of water, as listed in a Basin Plan.

“Potable water” means water provided from a permitted public water system, as defined in Section 116275(h) of the California Health and Safety Code, that meets state and federal standards for consumption, a state small water system, as defined in Section 116275(n) of the California Health and Safety Code, that meets state standards for consumption, or groundwater from a permitted water well that is not part of a public water system or state small water system and that meets the following minimum water quality monitoring requirements: 1) at least one water sample obtained from the well within three months of submittal to DEHQ for review shall be negative for the presence of total coliform bacteria and fecal coliforms or *Escherichia coli* (E. coli); 2) at least one water sample obtained from the well and analyzed for Nitrate (as nitrogen) shall be less than the maximum contaminant level as specified in the California Code of Regulations, Section 64431 (10 mg/L); 3) other sampling that may be required by the Director of Environmental Health based on known or suspected sources of pollution in the area that may affect the water quality of the well. The samples shall be analyzed by a laboratory certified by the State Water Resources Control Board for that analysis pursuant to California Health and Safety Code, Division 101, Part 1, Chapter 4, Article 3, commencing with Section 100825.

“Projected flows” means wastewater flows into the OWTS determined in accordance with provisions contained in this LAMP.

“Public water system” is a water system as defined in Section 116275 (h) of the California Health and Safety Code and regulated by the State Water Resources Control Board or a Local Primacy Agency pursuant to the California Safe Drinking Water Act (California Health and Safety Code, Division 104, Part 12, Chapter 4).

“Public water well” is a ground water well serving a public water system. A spring which is not subject to the California Surface Water Treatment Rule (SWTR), CCR, Title 22, sections 64650 through 64666 is a public well.

“Qualified professional” means an individual licensed or certified by a State of California agency to design OWTS and practice as professionals for other associated reports, as allowed under their license or registration. Depending on the work to be performed and various licensing and registration

requirements, this may include an individual who possesses a registered environmental health specialist certificate or is currently licensed as a professional engineer or professional geologist.

“Qualified service provider” means a person capable of operating, monitoring, and maintaining an OWTS in accordance with the OWTS Policy and local requirements. The individual must also be certified and/or competently trained by the manufacturer of an OWTS with supplemental treatment to install, maintain, service, and repair the specific model/type of OWTS.

“Regional Water Board” or “RWQCB” is any of the Regional Water Quality Control Boards designated by Water Code Section 13200. Any reference to an action of the Regional Water **Quality Control** Board in this LAMP also refers to an action of its Executive Officer, including the conducting of public hearings, pursuant to any general or specific delegation under Water Code Section 13223.

“Repair” is any action that modifies/replaces the existing dispersal system, replaces an existing septic tank, or modifies/replaces a major component of the onsite wastewater treatment system. Repairs require an Installation Permit issued by the DEHQ and must be inspected by DEHQ staff.

“Replacement OWTS” means an OWTS that has its treatment capacity expanded, or its dispersal system replaced or added onto, after the effective date of this LAMP. Replacements require an Installation Permit issued by the DEHQ and must be inspected by DEHQ staff.

“Sand” means a soil particle; this term also refers to a type of soil texture. As a soil particle, sand consists of individual rock or mineral particles in soils having diameters ranging from 0.05 to 2.0 millimeters. As a soil texture, sand is soil that is comprised of 85 percent or more sand particles, with the percentage of silt plus 1.5 times the percentage of clay particles comprising less than 15 percent using the USDA soil classification system.

“Sanitation facilities” means the method used to collect, store, or dispose of sewage, human excrement, or other liquid wastes associated with human habitation or activities. Sanitation facilities include but are not limited to holding tanks, vaulted privies, and chemical toilets.

“Septic tank” means a watertight, covered receptacle designed for primary treatment of wastewater and constructed to 1) Receive wastewater discharged from a building; 2) Separate settleable and floating solids; 3) Digest organic matter by bacterial action; 4) Store undigested solids; and 5) Clarify wastewater for further treatment.

“Silt” means a soil particle; this term also refers to a type of soil texture. As a soil particle, silt consists of individual rock or mineral particles in soils having diameters ranging from between 0.05 and 0.002 mm. As a soil texture, silt is soil that is comprised as approximately 80 percent or more silt particles and not more than 12 percent clay particles using the USDA soil classification system.

“Site” means the location of the OWTS and a reserve dispersal area capable of disposing of 100% of the design flow from all sources the OWTS is intended to serve.

“Site evaluation” means an assessment of the characteristics of the site sufficient to determine its suitability for an OWTS to meet the requirements of this LAMP.

“Soil” means the naturally occurring body of porous mineral and organic materials on the land surface, which is composed of unconsolidated materials, including sand-sized, silt-sized, and clay-sized particles mixed with varying amounts of larger fragments and organic material. The various combinations of particles differentiate specific soil textures identified in the soil textural triangle developed by the United States Department of Agriculture (USDA) as found in Soil Survey Staff, USDA; Soil Survey Manual, Handbook 18, U.S. Government Printing Office, Washington, DC, 1993, p. 138. For the purposes of this Policy, soil shall contain earthen material of particles smaller than 0.08 inches (2 mm) in size.

“Soil structure” means the arrangement of primary soil particles into compound particles, peds, or clusters that are separated by natural planes of weakness from adjoining aggregates.

“Soil texture” means the soil class that describes the relative amount of sand, clay, silt and combinations thereof as defined by the classes of the soil textural triangle developed by the USDA (see definition of “Soil”).

“Standard OWTS” means an OWTS consisting of a septic tank with effluent discharging into a subsurface dispersal system of a leach trench dispersal system, a chamber dispersal system, or a seepage pit dispersal system.

“State Water Board” or “SWRCB” is the State Water Resources Control Board.

“Stormwater feature” means a man-made private and public drainage facilities by which stormwater run-off may be conveyed to receiving waters, and includes but is not limited to, constructed channels and ponds, inlets to storm drains or pipes, or catch basins. For the purposes of this LAMP, “stormwater feature” does not include roads, streets, or street gutters.

~~**“STS”** is the acronym for Supplemental Treatment System and is used to describe an Onsite Wastewater Treatment System with Supplemental Treatment.~~

“Supplemental treatment” or “STS” means any OWTS or component of an OWTS, except a septic tank or dosing tank, that performs additional wastewater treatment so that the effluent meets a predetermined performance requirement prior to discharge of effluent into the dispersal field.

“SWAMP” means Surface Water Ambient Monitoring Program and more information is available at: http://www.waterboards.ca.gov/water_issues/programs/swamp/

“Telemetric” means the ability to automatically measure and transmit OWTS data by wire, radio, or other means.

“Total maximum daily load” or “TMDL” is a calculation of the maximum amount of a pollutant that a waterbody can accept and still meet the state’s water quality standards. A TMDL is the sum of individual

waste load allocations (WLAs) for point sources and load allocations (LAs) for nonpoint sources and natural background. ~~the acronym for "total maximum daily load."~~ Section 303(d)(1) of the Clean Water Act requires each State to establish a TMDL for each impaired water body to address the pollutant(s) causing the impairment. In California, TMDLs are usually adopted as Basin Plan amendments and contain implementation plans detailing how water quality standards will be attained.

"Unstable land mass" means areas showing evidence of mass downslope movement such as debris flow, landslides, rockfall, and hummock hill slopes with undrained depressions upslope. Examples are landforms exhibiting slip surfaces roughly parallel to the hillside; landslide scars and curving debris ridges; fences, trees, and telephone poles that appear tilted; and tree trunks that bend uniformly as they enter the ground. Active sand dunes are unstable landforms.

"Vaulted Privy" means the same as pit-privy.

"Vertical seepage pit" means a drilled excavation that is gravel filled and receives the effluent discharge from a septic tank or other OWTS treatment unit for dispersal.

"Waste discharge requirement" or "WDR" means an operation and discharge permit issued for the discharge of waste pursuant to Section 13260 of the California Water Code.

CHAPTER 2.0 LAMP SCOPE OF COVERAGE

2.1 Scope of Coverage

The scope of coverage under this LAMP encompasses the permitting of the installation of new and replacement OWTS, or one or more OWTS serving a single premise, up to a maximum of 3,500 gallons per day of low strength domestic wastewater only.

Any OWTS not regulated under the scope of the LAMP fall within the RWQCB jurisdiction for regulation. The DEHQ may issue an installation permit only after the siting and design have been approved by the RWQCB.

The scope of coverage under this LAMP for existing OWTS is limited to the permitting of OWTS for repairs to a septic tank, other structural repairs, defective component repairs, and for repairs to the dispersal system due to surfacing wastewater from the dispersal field and/or wastewater backup into plumbing fixtures because the dispersal system is not able to percolate the design flow of wastewater associated with the structure served.

2.1.1 The following OWTS are not included in the LAMP scope of coverage:

2.1.1.1 OWTS (one or more) serving a project or premise with a cumulative projected wastewater flow of over 3,500 gallons per day.

2.1.1.2 OWTS that receives high strength wastewater unless the waste stream is from a commercial food service facility.

2.1.1.3 OWTS that receives high strength wastewater from a commercial food service facility with a BOD higher than 900 mg/L or that does not have a properly sized and functioning oil/grease interceptor.

2.1.1.4 OWTS receiving discharges of waste that are not domestic wastewater.

2.1.1.5 OWTS within an incorporated city or tribal jurisdictions, unless the authority has been delegated through specific municipal ordinance, memorandum of understanding, or another appropriate mechanism.

2.1.1.6 Existing OWTS where the RWQCB has determined the discharges are impacting groundwater to an extent requiring corrective actions.

2.1.1.7 OWTS where the RWQCB has determined that the discharges require effluent limitations and ongoing sampling, monitoring, and reporting.

2.1.1.8 Approval of siting, design and construction for OWTS proposed as the means of sewage disposal for subdivisions of land of four or fewer parcels that do not meet exceed the minimum

density allowed as shown in Table 3.7-1 for standard OWTS or the requirements in Section 3.7.3.3.2 for OWTS with supplemental treatment for nitrogen reduction and drip dispersal with vegetative uptake design, ~~(and where a standard nitrate loading study shows the potential for the resultant average of nitrate nitrogen in recharge water to be over the water quality objectives established per the Basin Plan).~~

~~2.1.1.9 Review of OWTS proposed for subdivisions of land of five parcels or greater, or any development project, use permit, or building permit which may involve the discharge of wastewater from five family units or five farm labor housing units or greater.~~

2.1.1.10 New OWTS proposed for any development project or building permit that does not meet the minimum requirements of this LAMP.

2.2 Use Limitations

~~DEHQ's~~ The oversight of OWTS by DEHQ is limited to those systems as defined in this LAMP. Limitations exist for the use of OWTS related to the amount and type of wastewater flows that will be generated, types of systems, availability of public sewer and setbacks to public water supplies. The following are not allowed to be authorized by DEHQ and any such system or deviations can only be approved by the RWQCB.

2.2.1 Cesspools of any kind or size.

2.2.2 OWTS receiving a projected flow over 3,500 gallons per day of any strength or type of wastewater.

2.2.4 OWTS that utilize any form of effluent dispersal that discharges on or above the post installation ground surface such as sprinklers, exposed drip lines, free-surface wetlands, or a pond.

2.2.5 Slopes greater than ~~25~~30 percent without a slope stability screening and/or report approved by a qualified professional.

2.2.7 OWTS utilizing supplemental treatment without requirements for periodic monitoring or inspections.

2.2.8 OWTS dedicated to receiving significant amounts of wastes dumped from RV holding tanks.

2.2.9 Separation of the bottom of dispersal system to groundwater of less than two (2) feet for OWTS with supplemental treatment for nitrogen reduction, of less than three (3) feet for OWTS with supplemental treatment for pathogen reduction, and of less than ten (10) feet, ~~except for vertical seepage pits, which shall not be less than 10 feet.~~

2.2.10 Installation of new or replacement OWTS where public sewer is available. Public sewer availability is defined as follows:

2.2.10.1 The property on which the structure is located abuts a public sewer, or a public sewer is located within 200 feet of the building.

2.2.10.2 The property is within the boundaries of the sewer district or annexation to the sewer district had been completed.

2.2.10.3 No easements must be obtained to access the sewer line.

2.2.10.4 This provision does not apply to replacement OWTS where the connection fees and construction costs are greater than twice the total cost of the OWTS and an OWTS can be installed that will meet the minimum requirements of this LAMP and not affect groundwater or surface water to a degree that makes it unfit for drinking or other uses.

2.2.10.5 When a public sewer is not available, because one or more of the conditions above have not been satisfied, the drainage system of a building shall be connected to an approved and permitted OWTS.

2.2.11 Except as provided for in Item 2.2.12 and 2.2.13, new or replacement OWTS with minimum horizontal setbacks less than any of the following:

2.2.11.1 150 feet from a public water well where the depth of the effluent dispersal system does not exceed 10 feet in depth.

2.2.11.2 200 feet from a public water well where the depth of the effluent dispersal system exceeds 10 feet in depth.

2.2.11.3 Where the effluent dispersal system is within 600 feet of a public water well and exceeds 20 feet in depth, the horizontal setback required to achieve a two-year travel time for microbiological contaminants shall be evaluated by a qualified professional. In no case shall the setback be less than 200 feet.

2.2.11.4 Where the effluent dispersal system is within 1,200 feet from a public water system's surface water intake point, within the catchment of the drainage, and located such that it may impact water quality at the intake point such as upstream of the intake point for flowing water bodies, the dispersal system shall be no less than 400 feet from the high-water mark of the reservoir, lake or flowing water body.

2.2.11.5 Where the effluent dispersal system is located more than 1,200 feet but less than 2,500 feet from a public water system's surface water intake point, within the catchment of the drainage, and located such that it may impact water quality at the intake point such as upstream of the intake point for flowing water bodies, the dispersal system shall be no less than 200 feet from the high water mark of the reservoir, lake or flowing water body.

2.2.12 For replacement OWTS that do not meet the horizontal separation requirements in ~~Item 2.2.10 above~~ **Section 2.2.11**, the replacement OWTS shall meet the horizontal separation to the greatest extent practicable. In such case, the replacement OWTS shall utilize supplement treatment and other mitigation measures, unless the permitting authority finds that there is no indication that the previous system is adversely affecting the public water source, and there is limited potential that the replacement system could impact the water source based on topography, soil depth, soil texture, and groundwater separation.

2.2.13 For new OWTS, installed on parcels of record existing before May 13, 2013, which is the effective date of the State's OWTS Policy, that cannot meet the horizontal separation requirements in ~~Item 10 above~~ **Section 2.2.11**, the OWTS shall meet the horizontal separation to the greatest extent practicable and shall utilize supplemental treatment for pathogens as specified in Section 10.8 of the State's OWTS Policy and any other mitigation measures prescribed by DEHQ.

2.3 Prohibited Discharges

Prohibited Discharges – Owners and /or those who maintain any OWTS shall prohibit any of the following to flow or enter into an OWTS:

2.3.1 Automobile and Garage Waste. Wastewater from automobile washing or garage/shop floors.

2.3.2 Storm Drainage. Roof drainage or drainage waste resulting from natural runoff or irrigation.

2.3.3 Solvents and Toxics. Gasoline, cleaning solvents, paints, thinners, oils, or greases other than normal residential kitchen wastes, from hazardous materials.

2.3.4 Solids. Cloth, rope, metals, and solids of any kind.

2.3.5 Garbage. Garbage and similar waste material except when processed by approved garbage disposal units.

2.3.6 Kitchen Wastewater. Wastewater from any restaurant, bar, or other kitchen where food is prepared for public consumption unless first directed through an approved grease trap, as required by the Uniform Plumbing Code and this LAMP.

2.3.7 Air Conditioners. Waste drainage from water cooled refrigeration air conditioning.

2.3.8 Backwash. Backwash from water softeners, iron filters, and swimming pools.

2.3.9 Truck Terminal Wastes. Oil, grease, grit, and miscellaneous waste from operation of truck terminal, including wash water from trucks and garage floors.

2.3.10 Recreational Vehicle Holding Tank Waste. Wastes dumped from recreational vehicle holding tanks.

2.3.11 Industrial and Manufacturing Wastes. Wastes from non-domestic sources, including from industrial or manufacturing processes.

2.3.12 Food Processing Wastes. Wastes from commercial food manufacturing or food production processes, excluding retail food facilities.

2.4 Basin Plan Special Provisions for OWTS

The State Water Resources Control Board and Regional Water Quality Control Boards are responsible for adopting water quality policy, and any local ordinance and this LAMP must be consistent with these policies. This section reviews any special provisions contained in an applicable basin plan relating to OWTS within the scope of coverage of this LAMP.

2.4.1 Water Quality Control Plan for the Colorado River Region

The current version (including amendments effective on or before January 8, 2019) of the Water Quality Control Plan for the Colorado River Region (Colorado River Basin Plan) addresses OWTS in several areas.

In Chapter 4-Implementation, Section II-Point Source Controls, Subsection H-Septic Systems, the Colorado River Basin Plan provides for OWTS to be regulated pursuant to the OWTS Policy and includes OWTS prohibitions and exceptions for three specific areas. None of these prohibition areas are located in San Diego County. The reference in this section to the Colorado River Basin Regional Board's 1979 *Guidelines for Sewage Disposal from Land Developments* was deleted in September of 2013 when the OWTS Policy was incorporated into the Colorado River Basin Plan (R7-2013-0049).

In Chapter 5-Plans, Policies, and Issues, Section II-Regional Water Board Policies, Subsection B-Sewage Disposal from Land Developments, there is a reference to the Colorado River Basin Regional Board's 1979 *Guidelines for Sewage Disposal from Land Developments* which had been deleted from Chapter 4-Implementation as noted above. This section does not include a discussion on how this guidance is implemented within the scope and requirements of the OWTS Policy and its continued reference may be an oversight.

In Chapter 5-Plans, Policies, and Issues, Section III-Regional Board Issues, Subsection A-Septic System Impacts to Ground Water Basins, the Colorado River Basin Plan identifies seven areas where unsewered communities with high densities of OWTS have the potential to negatively impact groundwater. Of the seven communities identified, only one, Borrego Springs, lies within San Diego County. This section provides, as staffing and finances permit, for the Regional Water Board to conduct investigations to determine the relative priority for sewerage these identified communities.

2.4.2 Water Quality Control Plan for the San Diego Region

The current version (including amendments effective on or before September 1, 2021) of the Water Quality Control Plan for the San Diego Region (San Diego Basin Plan) addresses OWTS in several areas.

In Chapter 4-Implementation, OWTS is identified as a point source category (page 4-4, which is controlled by the Regional Board under the Waste Discharge Requirements (WDRs) permitting program (page 4-2). OWTS is also listed as an example of waste discharges subject to WDRs (page 4-11). In the section *Nitrogen in Interconnected Ground Waters and Surface Waters* (page 4-16), OWTS are listed as discharges with significant total nitrogen loads that may require total nitrogen effluents limits for projects or facilities that discharge to land near surface water bodies. The control mechanism included in this section is that the Regional Board may and most likely will adopt WDRs that require a reduced concentration in the proposed discharge effluents, reduction in total nitrogen loads, and or compliance with more stringent water quality objectives in receiving waters for the protection of beneficial uses of water resources.

This section identifies the OWTS Policy conditional waiver of WDRs for smaller OWTS that meet minimum design and siting conditions, including setbacks to surface water bodies, which allow for diffusion, dilution, and dispersion of an effluent plume before the affected ground water discharges to a surface stream. This section provides for OWTS that do not qualify for the waiver must be regulated with WDRs and includes as examples OWTS located at rural parks, schools, campgrounds, mobile home parks, roadside rest stops, small commercial or residential subdivisions, restaurants, resort hotels/lodges, small correctional facilities, temporary fire-fighting camps, and RV dump locations, including RV parks. For OWTS that pose a threat to surface water quality due to their size or proximity to a surface water body, the Regional Board will most likely require a Report of Waste Discharge (ROWD) to include a nitrate study.

OWTS are specifically addressed in the San Diego Basin Plan in the section *Onsite Wastewater Treatment Systems* (page 4-46). This section provides guidelines for new or replacement OWTS to proponents of projects involving new discharges of wastes to individual OWTS and also provides certain actions by local agencies to minimize water quality problems resulting from new OWTS. These include prohibiting OWTS where existing community sewerage collection systems are reasonably available, prohibiting use of new OWTS for any subdivision of land unless the governing body having jurisdiction determines that the use of individual OWTS will be in the best public interest, assuring individual OWTS are maintained to the satisfaction of the responsible health officer through local ordinances, and considering the cumulative impacts of individual OWTS discharges as part of the approval process for development.

This section also provides information on the OWTS Policy and its conditional waiver (page 4-49). It provides that the Regional Board will review specific proposals for OWTS that do not meet waiver conditions specified in the OWTS Policy or conditions specified in the applicable LAMP at the request of the local agency. For such OWTS proposals, a ROWD must be filed with the Regional Board and WDRs must be obtained or waived by the Regional Board prior to recordation of the final map and/or issuance of a building permit (page 4-51).

In the section *Control of Nonpoint Source Pollution* (page 4-131), OWTS are included in a list of examples of categories of nonpoint source pollution. This section provides that the Regional Board will generally

refrain from imposing effluent requirements on dischargers who are implementing Best Management Practices (BMP) in accordance with a waiver of waste discharge requirements. The BMPs become the primary mechanism for meeting water quality standards.

The Total Maximum Daily Loads (TMDL) section (page 4-185) of the San Diego Basin Plan provides information on the TMDL program regulating the amount of a pollutant that can be discharged into a water body and still maintain its water quality standards. Pollutant loadings in excess of the TMDL are expected to have an adverse effect on water quality by causing exceedances of the applicable water quality standards. Allowable pollutant loadings are calculated and assigned to all point source (as waste loading allocations) and nonpoint source (as load allocations) discharges to ensure that the applicable water quality standards are not exceeded in the receiving water. The TMDL for a pollutant in the receiving water, and the waste loading and loading allocations for a pollutant discharged from different sources into a water body are calculated at levels that, when each are met, are expected to result in the attainment of the associated water quality objectives for the pollutant and protection of the applicable beneficial uses in the receiving water. TMDLs are programs for the implementation of existing water quality standards and are incorporated into the Basin Plan. TMDLs are not self-implementing or directly enforceable for sources in the watershed but are implemented through the programs and authorities of the San Diego Water Board, including incorporating discharge prohibitions in the Basin Plan, issuing individual or general WDRs, or conditional waivers of WDRs.

In Chapter 5-Plans and Policies, the OWTS Policy is referenced but provides no additional details (page 5-13). In the Regional Board Resolutions section describing San Diego Regional Board resolutions important to the implementation of the Basin Plan, the following resolutions are referenced that have a nexus to OWTS:

- Resolution 79-44: *A Resolution Concerning 'Guidelines for New Community and Individual Sewerage Facilities.'*
- Resolution No. R9-2005-0036: *A Resolution Adopting an Amendment to the Water Quality Control Plan for the San Diego Region (9) to Incorporate Total Maximum Daily Loads (TMDLs) for Total Nitrogen and Total Phosphorus in the Rainbow Creek Watershed, San Diego County.*
- Resolution No. R9-2010-0001: *A Resolution Amending the Water Quality Control Plan for the San Diego Basin (9) to Incorporate Revised Total Maximum Daily Loads for Indicator Bacteria, Project I – Twenty Beaches and Creeks in the San Diego Region (Including Tecolote Creek).*

Chapter 7 of the San Diego Basin Plan provides information on TMDLs adopted into the Basin Plan. OWTS have been identified as a source or a potential source in the following TMDLs.

- TMDLs for Total Nitrogen and Total Phosphorus in the Rainbow Creek Watershed (2005)
 - Identifies OWTS, along with agricultural operations, as a source of total nitrogen to Rainbow Creek through the groundwater pathway (groundwater/surface water interface at Rainbow Creek).
 - Establishes numeric targets of 10 mg NO₃-N/L for Nitrate (as Nitrogen) and 1.0 mg/L for Total Nitrogen.
 - Establishes a Total Nitrogen Mass Load of 3,834 kg N/yr to Rainbow Creek, including 200

kg N/yr assigned to OWTS. It should be noted that although agricultural operations were also identified as contributing to this total nitrogen mass load through the groundwater pathway, they were omitted as a source for the total nitrogen mass load allocation.

- Establishes an Annual Nutrient Loading Capacity for Total Nitrogen of 1,658 kg/yr for Rainbow Creek with a compliance date for attainment set at December 31, 2021.
 - Provides for OWTS owners, as nonpoint source dischargers, to implement pollution prevention methods and increase the use of applicable management measures and practices where needed to control and reduce nutrient discharges to Rainbow Creek.
 - Provides for the Regional Board to incorporate the OWTS Policy into the Basin Plan to address OWTS.
 - Provides for NPS dischargers to be subject to Regional Board enforcement action for failing to comply with applicable waiver conditions, WDRs, discharge prohibitions.
 - **As the OWTS Policy was adopted into the Basin Plan in 2015 with no other conditions relating to OWTS, OWTS discharges are currently regulated by DEHQ under the LAMP and conditional waiver contained in the OWTS Policy, or by the Regional Board under SWRCB Order WQ 2014-0153-DWQ or individual waste discharge requirements.**
- Revised TMDLs for Indicator Bacteria, Project 1-Twenty Beaches and Creeks in the San Diego Region (including Tecolote Creek) (2010)
 - Identifies persons as responsible for controllable nonpoint sources bacteria discharges causing or contributing to the impairments at beaches and creeks to include the owners and operators of individual OWTS.
 - Does not assign a Load Allocation to individual OWTS, which is equivalent to being assigned a load allocation of zero (not expected to or allowed to discharge a pollutant load as part of the TMDL).
 - Provides that nonpoint sources are or can be regulated under WDRs or conditional waivers of WDRs and for the San Diego Regional Board to utilize their regulatory tools and work with the nonpoint source dischargers and/or stakeholders when developing the WDRs.
 - **Currently OWTS discharges within the geographical area of this TMDL regulated by DEHQ under the LAMP and conditional waiver contained in the OWTS Policy, or by the Regional Board under SWRCB Order WQ 2014-0153-DWQ or individual waste discharge requirements.**

~~It is important to note that the San Diego Basin Plan water quality objectives for nitrate in groundwater for the municipal beneficial use is typically 45 mg/L. However, the water quality objective for municipal beneficial use for nitrate in groundwater in the Rainbow Creek watershed is 10 mg/L and in the Warner Valley Hydrologic Area is 5 mg/L. These water quality objectives may impact OWTS usage in these areas.~~

2.5 LAMP Water Quality Assessment Program

The OWTS Policy Section 9.3.2 requires the local agency with an approved LAMP to maintain a Water Quality Assessment Program (WQAP). The WQAP's purpose is to determine the general operation status of OWTS, to evaluate the impact of OWTS discharges, and to assess the extent to which groundwater

and local surface water quality may be adversely impacted.

2.5.1 The focus of the WQAP are the areas with characteristics or conditions listed below:

2.5.1.1 Degree of vulnerability to pollution from OWTS due to hydrogeological conditions.

DEHQ has identified two conditions that fall within this focus category and that are not address under a specific focus category, shallow groundwater and steep slopes. The areas identified with periods of shallow groundwater include the Rainbow Valley area in Fallbrook, the Citrus Avenue area in Escondido, the Valley Center and Woods Valley Roads area in Valley Center, the Granite Hills area in El Cajon, and in the Ramona Basin. Slopes, including steep slopes, are found in most areas of San Diego County.

The current site evaluation process in the LAMP ensures new and replacement OWTS meet the current standards relating to shallow groundwater and steep slopes, providing protection to ground and surface waters equivalent to the OWTS Policy for these focus conditions.

2.5.1.2 High Quality waters or other environmental conditions requiring enhanced protection from the effects of OWTS.

For this focus category, DEHQ has identified the following 24 reservoirs that store local and imported water supplies and that require protection from the effects of OWTS.

Table 2.5-1: San Diego County Reservoirs	
Barrett - City of San Diego	Cuyamaca - Helix Water District
Dixon - City of Escondido	El Capitan - City of San Diego
Henshaw - Vista Irrigation District	Hodges - City of San Diego
Jennings- Helix Water District	Loveland - Sweetwater Authority
Lower Otay - City of San Diego	Maerkle - Carlsbad Municipal Water District
Miramar - City of San Diego	Morena - City of San Diego
Morro Hill - Rainbow Municipal Water District	Murray - City of San Diego
Olivenhain - San Diego County Water Authority	Poway - City of Poway
Ramona - Ramona Municipal Water District	Red Mountain- Fallbrook Public Utility District
San Vicente - City of San Diego	Sutherland - City of San Diego
San Dieguito - Santa Fe Irrigation District	Sweetwater - Sweetwater Authority
Turner - Valley Center Municipal Water District	Wohlford - City of Escondido

The current LAMP includes protective siting and design standards for new and replacement OWTS in areas near streams and reservoirs to provide protection to these reservoirs equivalent to the OWTS Policy for this focus condition.

2.5.1.3 Shallow soils requiring a dispersal system installation that is closer to ground surface than is standard.

The current site evaluation process in the LAMP ensures new and replacement OWTS meet the current standards relating to shallow soils, providing protection to ground and surface waters equivalent to the OWTS Policy for this focus condition.

2.5.1.4 OWTS is located in area with high domestic well usage.

San Diego County imports about 80% of its water. According to the State Water Board, Division of Drinking Water's *Drinking Water Watch* database, most of the population within the county (3,413,684) is within the service boundaries of a public water system, with 98% of this population being provided drinking water from a surface water source. Those areas not served by a surface water source have drinking water supplied by domestic and public water wells.

The current site evaluation process and setback standards in the LAMP ensure new and replacement OWTS are located to protect water quality in areas where groundwater is utilized to provide drinking water through water wells, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.5 Dispersal system is located in an area with fractured bedrock.

Although fractured rock generally comprises about 73% of the unincorporated areas of the county, varying characteristics and depths of soil overlay much of the fractured rock. The current site evaluation process and setback standards in the LAMP ensure new and replacement OWTS are located to prevent surfacing effluent and to protect water quality, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.6 Dispersal system is located in an area with poorly drained soils.

Most soils maps show generalized areas of soil categories, but they do show that soils in San Diego County vary greatly. This demonstrates the purpose of the site evaluation process, including the requirement for percolation testing. The current site evaluation process and percolation test and soil evaluation requirements in the LAMP ensure new and replacement OWTS are located to prevent surfacing effluent and to protect water quality, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.7 Surface water is vulnerable to pollution from OWTS.

DEHQ works closely with other county departments to implement local ordinance aimed at reducing pollution to vulnerable surface water bodies. These include ordinance related to the permitting of new and replacement OWTS, ordinance addressing the response to reports of surfacing sewage and effluent, ordinance implementing watershed protection requirements, including those to minimize or eliminate polluted surface water runoff discharges into a county-maintained stormwater conveyance system. The County of San Diego has developed several programs to protect vulnerable surface waters, including a successful OWTS training and pumping rebate program.

These ordinances and programs, along with the current standards contained in the LAMP, minimize or eliminate surface water vulnerability to pollution from OWTS, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.8 Surface water within the watershed is listed as impaired for nitrogen or pathogens.

This condition focuses on surface water within a watershed that is listed as impaired for nitrogen or pathogens. Data from the *California 2010 Clean Water Act Section 303(d) and 305(b) Integrated Report* for the San Diego and Colorado River Basin Regions show three listed water bodies with OWTS as a potential source to the impairment. These three water bodies are Rainbow Creek (impairment due to nutrients), the Lower San Diego River (impairment due to pathogens), and the Tijuana River (impairment due to pathogens). The data also show the Santa Margarita Lagoon and River as impaired for nitrogen and pathogens, but with unknown sources.

Of these water bodies, Rainbow Creek and the Lower San Diego River have adopted TMDLs to address the impairments. TMDLs or alternative projects are under development by the RWQCB to address the impairments for the Santa Margarita Lagoon and River and the Tijuana River.

~~2.5.1.8.1~~ The OWTS Policy addresses impaired water bodies in Tier 3 of the policy as follows.

~~2.5.1.8.1.1~~ The OWTS Policy provides discretion for local agencies to incorporate special provisions for OWTS near impaired water bodies as part of a LAMP. The County of San Diego LAMP does not contain special provisions for OWTS near impaired water bodies and the other provisions in Tier 3 of the policy are applicable for OWTS near impaired water bodies.

~~2.5.1.8.1.2~~ For areas where there is an adopted TMDL, the TMDL implementation plan must be used to address the impairment and this implementation plan supersedes all other requirements in Tier 3. Currently, except for a specified load reduction for OWTS in the Rainbow Creek TMDL, there are no special requirements in the San Diego Basin Plan for the siting, design, or permitting of OWTS in areas of impaired water bodies, beyond those approved in this LAMP.

~~2.5.1.8.1.3~~ For areas where there is no TMDL, new and replacement OWTS within 600 feet of an impaired water body listed in Attachment 2 of the policy must meet specified supplemental treatment requirements. Currently there are no impaired water bodies listed in Attachment 2 of the OWTS Policy in San Diego County. However, these Tier 3 requirements would be

implemented by DEHQ for new and replacement OWTS that are within the scope and coverage of the LAMP should a water body become listed in Attachment 2 in the future.

2.5.1.8-4.4 OWTS near impaired water bodies that are not listed in Attachment 2 of the policy, and do not have a TMDL, and are not covered by special provisions in a LAMP are not addressed by Tier 3.

As the State and Regional Boards are responsible for adopting water quality policies and regulations in California, any local agency OWTS program must be consistent with these State policies and regulations. Currently, the LAMP contains provisions that are consistent with existing State policies and regulations. Should any future water quality policy or regulation be adopted by the RWQCB to address impaired water bodies in San Diego County, DEHQ will implement those requirements for new and replacement OWTS that fall ~~under~~ within the scope and coverage of the LAMP.

2.5.1.9 OWTS is located within an area of high OWTS density.

DEHQ looked at this condition by reviewing the density of OWTS in locations of parcels with areas equal to or less than 0.5 acres. Many of these areas are also located within public water system boundaries and are served by public water. The current subdivision of land, site evaluation process, and minimum siting and design requirements in the LAMP ensure new and replacement OWTS are located to protect water quality, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.10 A parcel's size and its susceptibility to hydraulic mounding, organic or nitrogen loading, and whether there is sufficient area for OWTS expansion in case of failure.

The current subdivision of land, site evaluation process, and minimum siting and design requirements in the LAMP ensure new and replacement OWTS are located to protect water quality, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.11 Geographic areas that are known to have multiple, existing OWTS predating any adopted standards of design and construction including cesspools.

Although there may be some individual OWTS that meet this condition, there are no known geographical areas meeting this description. DEHQ has no information of any existing cesspools at this time. The County of San Diego has had adopted standards in ordinance for the design and construction of OWTS since at least the early 1960s. These standards have been updated at least twice since that time.

The current site evaluation process and minimum siting and design requirements in the LAMP ensure new and replacement OWTS are located to protect water quality, providing protection equivalent to the OWTS Policy for this focus condition.

2.5.1.12 Geographic areas that are known to have multiple, existing OWTS located within current setback standards or those setbacks DEHQ finds appropriate for that area.

Although there may be individual OWTS that meet this condition, there are no known geographical areas that meet this description. Existing OWTS requiring repair that cannot meet a current standard must be issued a variance. Typically, a reduction in the size of the required dispersal field will be proposed rather than a reduction to a setback adopted to protect water quality. Variances for repairs to OWTS are only issued when necessary and only in substantial conformance, to the greatest extent practicable, with the provisions of the LAMP, consistent with the OWTS Policy. All new OWTS and OWTS for new development projects must meet the minimum requirements of the LAMP.

2.5.2 The WQAP also includes the monitoring and analysis of the following:

2.5.2.1 Water Quality Data to include data for nitrates and pathogens from OWTS. As water quality data is not collected as part of the local OWTS permitting program, DEHQ may review the following data sources to meet this requirement.

2.5.2.1.1 Any domestic well or public water system sampling reports available to DEHQ.

2.5.2.1.2 Any water quality testing reports performed as part of a National Pollutant Discharge Elimination System (NPDES) permit.

2.5.2.1.3 Data contained in the California Water Quality Assessment database.

2.5.2.1.4 Groundwater sampling performed as part of Waste Discharge Requirements.

2.5.2.1.5 Groundwater data collected as part of the Groundwater Ambient Monitoring and Assessment Program and available in the GeoTracker database.

2.5.2.2 A review of complaints, variances, failures and other inspection data compiled in accordance with Section 2.6.1.

2.6 LAMP Reporting

Consistent with the OWTS Policy, DEHQ will perform the following data collection, reporting, and notifications.

2.6.1 Annual Report

2.6.1.1 Pursuant to the OWTS Policy Section 3.3, as a local agency permitting OWTS, DEHQ will report annually to the San Diego Regional Water Board with a copy to the Colorado River Basin Regional Water Board. The annual report shall include the following information.

2.6.1.1.1 The number and location of complaints pertaining to OWTS operation and maintenance, and identification of those which were investigated and how they were resolved.

2.6.1.1.2 The applications and registrations issued as part of the local septic tank cleaning registration program pursuant to Section 117400 et seq. of the California Health and Safety Code.

2.6.1.1.3 The number, location and description of permits issued for new and replacement OWTS and which Tier the permit was issued. Tier designations can be found in the State Water Board's OWTS Policy.

2.6.1.1.4 All groundwater and surface water data generated by DEHQ, submitted in the format prescribed in the OWTS Policy.

2.6.1.2 In addition to the reporting requirements in Section 2.6.1.1, OWTS Policy Section 9.3.3 requires a local agency with an approved LAMP to also annually report a summary of the status of the following items.

2.6.1.2.1 The number, location and description of permits issued for OWTS where a variance from the approved LAMP was granted.

2.6.1.2.2 A summary of the status of the Water Quality Assessment Program (WQAP).

2.6.2 Five-Year Evaluation Report

OWTS Policy Section 9.3.3 requires a local agency with an approved LAMP to submit, every fifth year after approval, an evaluation of the monitoring program and an assessment of whether water quality is being impacted by OWTS. The evaluation report shall identify any changes to the LAMP to address impacts from OWTS for those OWTS within the scope and coverage of the LAMP.

2.7 Notifications

Prior to issuance of a permit to install a new or replacement OWTS that is within 1,200 feet of an intake point for a surface water treatment plant for drinking water, in the drainage catchment in which the intake point is located, and located such that it may impact water quality at the intake point, DEHQ will notify the owner of the public water system electronically or in writing of the proposed OWTS permit installation and request any recommendations or comments to be provided to DEHQ within 15 days of receipt of the notification. If the owner of the public water system cannot be identified, this notice will be provided to the State Water Resources Control Board, Division of Drinking Water program. The notification will include a copy of the permit application that includes the following information.

2.7.1 A topographical plot plan for the parcel showing the OWTS components, property boundaries, proposed structures, physical address, and name of property owner.

2.7.2 The estimated wastewater flows, intended use of proposed structure generating the wastewater,

soil data, and estimated depth to seasonally saturated soils.

CHAPTER 3.0 GENERAL REQUIREMENTS

3.1 Registration Requirements

No Qualified Professional, Licensed Installer, and Qualified Service Provider shall perform a site evaluation, percolation test, OWTS design, OWTS installation, or OWTS monitoring, servicing, or reporting without a valid registration with DEHQ.

3.1.1 Individual Qualified Professionals, Licensed Installers, and Qualified Service Providers shall complete and submit a registration form **and associated fees** to DEHQ and schedule an initial consultation. The registration shall include, at a minimum, the individual's name, current license number and status, and the mailing address where any correspondence or notifications shall be mailed.

3.1.2 The ~~Qualified Professional~~ **registrant** shall demonstrate ~~the following information and a sufficient scope of knowledge~~ **and experience related to the specific service or services the individual performs during an initial consultation. This information includes, but is not limited to, the following:**

3.1.2.1 Laws, rules, regulations, and ordinance relating to OWTS.

3.1.2.2 Minimum requirements for OWTS siting, design, and construction.

3.1.2.3 Demonstration of a minimum of one (1) year experience designing, installing, monitoring or servicing OWTS to the extent of their licensure or registration. For individuals designing, installing, monitoring, or servicing OWTS with supplemental treatment for nitrogen or pathogens, a demonstration of a minimum of two (2) years of experience is required.

3.1.3 Once registered, the individual shall submit an annual renewal application providing updated information, where applicable.

3.1.4 A registration may be revoked or suspended upon a violation of the requirements of this Chapter. DEHQ shall provide a written 30 calendar day notice of the intent to revoke or suspend a registration. The written notice shall describe the specific violation or violations and shall schedule an office consultation to provide the individual an opportunity to provide any information related to the matter.

3.1.4 An Owner-Builder **who intends to apply for a permit to install, repair, replace, or add to an OWTS** shall complete an initial consultation with DEHQ to demonstrate sufficient knowledge on the siting, design, and construction of an OWTS to effectively construct **and install** an OWTS that meets the minimum requirements of this LAMP.

3.2 Qualified Professionals

3.2.1 General Description

A Qualified Professional is an individual licensed or certified by the State of California to design OWTS

and practice as professionals for other associated reports, to the extent allowed under their license or registration and their education and experience. A Qualified Professional shall be a Professional Engineer licensed under Business and Professions Code, Division 3, Chapter 7, a Professional Geologist licensed under Business and Professions Code, Division 3, Chapter 12.5, or a Registered Environmental Health Specialist licensed under Health and Safety Code, Division 104, Part 1, Chapter 4, or other persons as authorized in the Business and Professions Code.

Qualified Professionals shall prepare plans and reports in accordance with accepted industry standards, including the provision of complete and accurate minimum information as required and specified in this LAMP or as requested by DEHQ. All plans, specifications, reports, and other documents shall be prepared by, or under the responsible charge of, a licensed Qualified Professional and shall include his or her name and license number. All plans, specifications, and reports shall bear the signature and seal or stamp of the licensee and the date of signing and sealing or stamping, if appropriate.

3.2.1.1 Professional Engineer Scope of Service

Business and Professions Code Section 6731 defines civil engineering to include activities in connection with fixed works for sewerage, including the preparation or submittal of designs, plans and specifications and engineering reports and the coordination of the work of professional, technical, or special consultants.

According to the California Department of Consumer Affairs, Board for Professional Engineers, Land Surveyors, and Geologists guidance, civil engineers prepare design and repair recommendations for drainage systems, septic systems, foundations, and retaining walls. They also prepare grading plans and topographic maps of the elevations and contours of the land. Geotechnical engineers are civil engineers who have obtained additional experience and passed a specialized geotechnical engineering examination which authorizes them to use the titles “Geotechnical Engineer,” “Soil Engineer,” or “Soils Engineer.” Geotechnical engineering includes the investigation and engineering evaluation of earth materials including soil, rock, groundwater, and man-made materials and their interaction with earth retention systems, foundations, and other civil engineering works. Geotechnical engineers apply the principles of soil mechanics and the earth sciences and are knowledgeable about engineering laws, formulas, construction techniques, and performance evaluation of civil engineering works influenced by earth materials.

3.2.1.2 Certified Engineering Geologist and Certified Hydrogeologist Scope of Practice

The Business and Professions Code, Division 3, Section 12.5 provides for licensing for persons engaged in the practice of geology. Section 7802 defines geology as that science which treats of the earth in general; investigation of the earth’s crust and the rocks and other materials which compose it; and the applied science of utilizing knowledge of the earth and its constituent rocks, minerals, liquids, gases and other materials for the benefit of mankind.

According to the California Department of Consumer Affairs, Board for Professional Engineers, Land Surveyors, and Geologists guidance, geologists conduct surface and underground investigations of the

earth materials, history and structure of sites and provide interpretations of what they see. Certified Engineering Geologist and Certified Hydrogeologists are the two current licensed specialties. Certified Engineering Geologists apply geologic principles to the safe development and grading of land, building of structures, search for groundwater resources, cleanup of underground contamination and repairing of geologic hazards. They investigate geologic constraints such as landslides, ground subsidence, earthquake faults and erosion and have special training in geology for working on civil engineering problems. Certified Engineering Geologists evaluate the underground conditions of properties in a variety of ways to aid in finding out the engineering and environmental aspects of a project or site. They are also familiar with regulations pertaining to land use and repair that require permits from various governmental agencies. Certified Hydrogeologists apply geologic principles to the search for and cleanup of subsurface contamination and the discovery and development of groundwater resources. They complete Phase I, II and III environmental investigations. Certified Hydrogeologists evaluate the underground conditions at sites in a variety of ways to find out the environmental aspects of a project.

Business and Professions Code, Section 7839 specifically prohibits Certified Engineering Geologists or Certified Hydrogeologists from offering or practicing civil engineering in any of its various recognized branches.

3.2.1.3 Registered Environmental Health Specialist

Health and Safety Code Section 106615(e) provides the scope of practice in environmental health to mean the practice of environmental health by Registered Environmental Health Specialists in the public and private sector and includes, but is not limited to, organization, management, education, enforcement, consultation, and emergency response for the purpose of prevention of environmental health hazards. The promotion and protection of the public health and the environment in several areas, including housing, land use, solid, liquid, and hazardous materials management, and onsite septic systems. Section 106620 provides an exemption to Section 6731 of the Business and Professions Code and authorizes a Registered Environmental Health Specialist to design OWTS. Except for the design of OWTS, this exemption does authorize a Registered Environmental Health Specialist to design any of the other fixed works defined in Section 6731.

3.2.2 Professional Standards and Practice within Area of Competence

Qualified Professionals shall act in accordance with the provisions of their respective licensing laws and regulations, including Health and Safety Code Section 106715, and California Code of Regulations, Title 16, Sections 475 and 3065, including the following.

3.2.2.1 Qualified Professionals shall practice and perform work only in the field or fields in which they are by education and/or experience fully competent and proficient.

3.2.2.2 Qualified Professional shall provide professional services for a project in a manner that is consistent with the laws, codes, ordinances, rules, and regulations applicable to that project, and may obtain and rely upon the advice of other professionals (e.g., architects, attorneys, professional engineers, professional land surveyors, and other qualified persons) as to the intent

and meaning of such laws, codes, and regulations.

3.2.2.3 Qualified Professionals (together with those whom the professional may engage as consultants) shall perform, or offer to perform, only those professional services for which they are qualified by education, training, experience, and licensure as required by law, in the specific technical and scientific areas involved.

3.2.2.4 Qualified Professionals shall act with competence and reasonable care and shall apply the technical knowledge and skill which is ordinarily practiced by professionals in good standing, practicing in this state under similar circumstances and conditions.

3.2.2.5 Qualified Professionals shall only express professional opinions that have a basis in fact, are within the scope of the professional's own experience or knowledge and are generally accepted principles.

3.2.2.6 Qualified Professionals shall not misrepresent data and/or its relative significance in any technical report.

3.2.2.7 Qualified Professionals shall not misrepresent the completeness of the professional documents submitted to a government agency.

3.2.3. A Qualified Professional who fails to provide plans, specifications, reports, and other documents that meet the minimum required information as specified in this LAMP or as requested by DEHQ, or who provide incomplete, false, or misrepresented information, including actions as described in Business and Professions Code Section 6775 and 7860, Health and Safety Code Section 106715, and California Code of Regulations, Title 16, Sections 475 and 3065 may be subject to referral to the appropriate licensing Board or Committee for investigation.

3.3 Licensed Installers

3.3.1 General Description

Licensed Installers shall be a General Engineering Contractor (Class A), General Building Contractor (Class B), Sanitation System Contractor (Specialty Class C-42), or Plumbing Contractor (Class C-36). Such licensed installer shall install all new and replacement OWTS in accordance with California Business and Professional Code Sections 7056, 7057, and 7058 and Article 3, Division 8, Title 16 of the California Code of Regulations.

3.3.2 Minimum Standards

OWTS shall be installed, constructed, reconstructed, repaired, added to, modified, or abandoned as follows.

3.3.2.1 Under a valid permit issued by DEHQ, the standards provided by state and local laws and

regulations, and this LAMP.

3.3.2.2 Using appropriate materials and accepted trade standards for good and workmanlike construction.

3.3.2.3 In accordance with plans or specifications as approved by DEHQ. All changes to approved plans or specifications must be submitted to DEHQ for review and receive approval prior to the commencement of any work.

3.3.3 Any person who commences work to install, construct, and/or repair an OWTS without first obtaining a permit, or without first ensuring a valid permit was obtained by another qualified person, is in violation of San Diego County Regulatory Code Section 68.325. Any person who fails to meet the minimum standards in this LAMP, local ordinance or State law, including but not limited to, the Business and Professions Code Division 3, Chapter 9, may be subject to enforcement action and/or referral to the California Contractor's State Licensing Board for investigation.

3.3.4 Unlicensed individuals who commence work to install, construct, and/or repair an OWTS in violation of the requirements of local ordinance, this LAMP, and Business and Professions Code Division 3, Chapter 9 shall be subject to referral to the California Contractor's State Licensing Board for investigation.

3.4 Qualified Service Providers

3.4.1 A Qualified Service Provider must be certified and/or competently trained by the manufacturer of an OWTS with supplemental treatment to install, maintain, service, and repair the specific model/type of OWTS.

3.4.2 The qualified service provider shall be responsible for and capable of providing the activities listed in Section 9.6.3.

3.5 Owner-Builders

A property owner who intends to obtain a permit to install all or part of an OWTS must first review, initial and sign an Owner-Builder Declaration and Notice to Property Owner form provided by DEHQ and complete a consultation with DEHQ demonstrating sufficient knowledge and skill to effectively install an OWTS that meets the minimum requirements of this LAMP. A property owner who demonstrates adequate knowledge and skill may obtain a permit to install an OWTS in accordance with an OWTS Layout Report completed by a Qualified Professional and approved by DEHQ.

3.6 Mandatory Public Sewer Connection Requirements

3.6.1 Installation of new or replacement OWTS where public sewer is available. Public sewer availability is defined when all the following applies:

3.6.1.1 The property on which the structure is located abuts a public sewer, or a public sewer is located within 200 feet of the building.

3.6.1.2 The property is within the boundaries of the sewer district or annexation to the sewer district has been completed.

3.6.1.3 No easements must be obtained to access the sewer line.

3.6.2 Section 3.3.1 does not apply to replacement OWTS where the connection fees and construction costs are greater than twice the total cost of the OWTS and an OWTS can be installed that will meet the minimum requirements of this LAMP and not affect groundwater or surface water to a degree that makes it unfit for drinking or other uses.

3.6.3 When a public sewer is not available, because one or more of the conditions above have not been satisfied, the drainage system of a building shall be connected to an approved and permitted OWTS.

3.7 Land Use and Development Requirements

3.7.1 County DEHQ and Local Land Use Agency Coordination

Most development and building standards require the approval of an OWTS by the agency having jurisdiction either as part of the permitting process or before certain permits can be issued or are considered finalized. Although separate processes, DEHQ coordinates the review and permitting of OWTS and other sanitation facilities with other local land use agencies to the extent practical. This coordination is important as the siting and design of an OWTS is dependent on many factors, including the location, scope and extent of a specific project. A DEHQ OWTS approval or permit is never a substitute for a required local grading, land use, or building permit. Similarly, no local land use approval or permit is a substitute for an OWTS approval or permit, or a guarantee that such a permit can be issued. This section discusses requirements in the San Diego Regulatory Code for projects where OWTS usage is proposed.

Whenever an application for a land use project requires DEHQ's review (i.e., proposes OWTS as a means of sewage disposal), the applicant shall submit an application requesting such a review on a form provided by DEHQ and shall submit any additional documents requested, including an OWTS Layout Report. DEHQ shall notify the applicant in writing that the OWTS Layout Report is approved, or, if it is disapproved, the notice shall state the reasons for the disapproval.

3.7.1.1 No property shall be developed more than its capacity to absorb sewage effluent properly by the means provided in this LAMP.

3.7.2 OWTS on Existing Lots

The process for obtaining an OWTS Installation Permit for development on a legal lot in the County of San Diego is described in this section. This process must be completed even if a lot has previously been

found suitable for OWTS usage by the County at that time. Typically, any such prior determination will be noted in land use records, e.g., through a map or plan notation that the lot is “approved” or “certified” for a septic system, or in a separate County-issued “certificate of compliance”. These notes and certificates may also state conditions for an acceptable OWTS, such as a minimum required leach line length. However, these map and plan notations and certificates of compliance were issued based on conditions and/or standards **in effect** at that time, may not reflect current minimum requirements, and may exceed the limits of DEHQ’s current permitting authority. Proposed OWTS must meet the requirements at the time of application for an OWTS Installation Permit.

There are several reasons that prior County certifications as part of the land use process do not ensure that an OWTS permit will be issued. First, DEHQ is only authorized to issue OWTS Installation Permits pursuant to the OWTS Policy, this approved LAMP, and local ordinance. Second, site characterization work and analysis performed to support prior County certifications may have been the best practice at the time (e.g., in a period of below normal rainfall), but may be inadequate to support a current OWTS Installation Permit. Third, new information may have come to light since a certification was issued, due to measurements taken on or near the site under different rainfall conditions. This is more likely to be the case for older certifications. Fourth, these certifications are not based on detailed project and OWTS designs and layout plans but were based on a general determination that the lot was suitable for OWTS usage. Finally, these certifications provide no legal entitlement. Even if a certification was construed as a permit to construct an OWTS, that permit would expire after one year unless the system was actually constructed, inspected, and given final approval.

The information used to determine a lot was suitable for OWTS usage may still be relevant at many sites. This is more likely when the information relied on for the certification is recent, meets current requirements, and was collected during a normal average rainfall year.

3.7.3 Subdivisions of Land, ~~and~~ Lot Line Adjustments, **Additional Dwellings**

3.7.3.1 Basin Plan Requirements.

The Implementation Chapter of the Basin Plan for the San Diego Region provides guidelines for OWTS usage that includes assumptions for local agencies noted below.

3.7.3.1.1 Prohibit the use of new community and individual OWTS where existing community sewerage collection systems are reasonably available. The determination of whether or not existing systems are reasonably available should be the responsibility of the local agency or agencies having jurisdiction over the project.

3.7.3.1.2 Prohibit the use of new individual OWTS for any subdivision of land unless the governing body having jurisdiction determines that the use of individual disposal systems will be in the best public interest.

3.7.3.1.3 Assure that individual OWTS are maintained to the satisfaction of the responsible health officer. This could be accomplished through establishment of special maintenance

districts, by the amendment of existing ordinances to assure adequate maintenance documented through periodic inspections, or other alternatives as deemed appropriate by the local health officer.

3.7.3.1.4 Consider the cumulative impacts of individual OWTS discharges as a part of the approval process for development.

3.7.3.2 Local Requirements.

3.7.3.2.1 ~~OWTS proposed as the means of sewage disposal for subdivisions of land of five parcels or greater shall be evaluated by the appropriate RWQCB. Subdivisions of land of four or fewer parcels shall meet the following requirements.~~ Purpose: The review of applications for subdivisions of land, lot line adjustments, major and minor use permits, building permits, grading permits, and other land use development projects is to determine that the site is suitable for OWTS usage or other sanitation facilities, and that an OWTS can be installed that meets the minimum requirements of the San Diego Regulatory Code and this LAMP. Where applicable, the review is also to determine that there is adequate potable well water supply to serve the project.

3.7.3.2.2 Local requirements for **OWTS on each lot of a proposed** subdivisions of land **application** must be consistent with the OWTS Policy and any requirements provided in applicable Basin Plans. All proposed individual lots must be documented as suitable for OWTS usage and that **each lot has been approved for** ~~can support~~ **the installation of** an OWTS that meets all current requirements in this LAMP, as required pursuant to the requirements of San Diego Regulatory Code, Title 8, Zoning and Land Use Regulations.

3.7.3.2.3 A person applying to subdivide property or for a lot line adjustment shall submit an OWTS Layout Report to demonstrate that there is an adequate area for an OWTS and adequate area reserved for OWTS equal to 100% of the area required for the system on each proposed lot that complies with the minimum requirements for OWTS in local ordinance and this LAMP. This demonstration shall be made as part of the application process.

3.7.3.2.4 Where more than one structure ~~is~~ being served by an **existing** OWTS **is located on a parcel** ~~are~~ proposed for a subdivision of land or lot line adjustment and where the structures will be on separate lots or where the OWTS will cross proposed property lines, these OWTS must be repaired ~~and~~ **or** replaced such that each OWTS is contained in whole on the lot and meets all setback requirements for the structure being served.

3.7.3.3 Lot Size/Density Requirements

3.7.3.3.1 The average density for any subdivision of property made pursuant to the Subdivision Map Act proposing to use OWTS as a means of sewage disposal shall not exceed the allowable density values ~~of OWTS~~ in Table 3.7-1 for a single-family dwelling (**SFD**), or its equivalent.

Table 3.7-1: Average Allowable Densities for Subdivision Lots¹ Utilizing OWTS

Average Annual Rainfall (in/yr)	Allowable Density (acres/ SFD single-family dwelling unit)
0 – 15	2.5
>15 – 20	2.0
>20 – 25	1.5
>25 – 35	1.0
>35	0.75

¹–The allowable densities specified in this table do not apply in the Warner Valley groundwater basin because the *Water Quality Control Plan for the San Diego Basin* (Basin Plan) establishes a groundwater quality objective of 5 milligrams per liter as nitrate for the Warner Valley Hydrologic Area. Larger lot densities may be required in the Warner Valley basin, use of OWTS with supplemental treatment, and the submittal of a nitrate study may be required for projects proposing use of OWTS to ensure compliance with this water quality objective.

3.7.3.3.2 ~~Adjustments to these densities may be allowed when additional studies, including but not limited to a nitrate loading study, completed by a qualified professional demonstrate no adverse impacts to groundwater or surface water above water quality objectives will occur. Lots created for commercial developments with the potential for flows that exceed 1,000 gallons per day will also require such studies. Where those studies show there will be impacts to groundwater or surface water quality that exceed applicable RWQCB Basin Plan water quality objectives, an applicant may propose the use of OWTS with supplemental treatment to its design certification as per of the requirements of this LAMP to mitigate those impacts, if appropriate, or may increase lot sizes to eliminate any adverse groundwater or surface water impacts. Where zoning regulations require greater lot sizes, those regulations shall take precedent.~~ The average density for any subdivision of property made pursuant to the Subdivision Map Act proposing to use an OWTS with supplemental treatment for nitrogen reduction with drip dispersal utilizing vegetative uptake design as a means of sewage disposal shall not exceed the allowable density values in Table 3.7-2 for a single-family dwelling, or its equivalent. Lot reductions for OWTS located within a geographical area of a TMDL, an Advanced Protection Management Program (APMP), or areas with Basin Plan concerns, conditions or restrictions related to OWTS discharges will be considered on a case-by-case basis in consultation with the appropriate Regional Board.

Table 3.7-2: Average Allowable Densities for Lots Utilizing OWTS with Supplemental Treatment for Nitrogen Reduction

Total Average Rainfall	2-Bedrooms (300 gpd)	3-Bedrooms (450 gpd)	4-Bedrooms (600 gpd)	5-Bedrooms (750 gpd)	6-Bedrooms (900 gpd)
0-6	1.75	2.50	2.50	2.50	2.50
>6-9	1.50	1.75	2.50	2.50	2.50
>9-12	1.00	1.50	1.75	2.00	2.50
>12-15	0.75	1.00	1.50	1.75	2.00
>15-20	0.50	0.75	1.00	1.50	1.50
>20-25	0.50	0.75	0.75	1.00	1.50
>25-30	0.50	0.50	0.75	1.00	1.00
>30-35	0.50	0.50	0.75	0.75	1.00

Notes:

1-For use with OWTS with supplemental treatment for nitrogen reduction with drip dispersal and vegetative uptake of wastewater.

2-Table based on Hantzsche and Finnemore nitrate loading mass balance with Nitrate-Nitrogen result of under 10 mg/L. Table defaults to OWTS Policy allowable density or highest calculated allowable density where nitrate > 10 mg/L.

3-Mass Balance Inputs: 17.5 mg/L Total Nitrogen in Effluent; 30% Denitrification with Drip Dispersal and Plant Uptake; 0 mg/L Background Nitrate-Nitrogen in Rainwater; Wastewater Volume of 150 gpd per bedroom; 10% of average annual rainfall available for deep percolation (San Diego County Rainfall 30-Year Average for Period of Record July 1971-June 2001).

https://www.sandiegocounty.gov/content/dam/sdc/dpw/FLOOD_CONTROL/floodcontroldocuments/Average%20Annual%20Rainfall.pdf

Hantzsche, Norman N. and Finnemore, E. John (1992) Predicting Ground-Water Nitrate Nitrogen Impacts, Groundwater, July-August 1992, Vol. 30, No. 4

3.7.3.3.3 Additional or Multiple Dwellings on Parcels. ~~3.7.3.3.3.1~~ A proposal for additional dwelling unit or units ~~that are not considered an accessory use to the primary dwelling~~ (single detached, duplex, semi-detached, triplex, stacked, attached, multi-dwelling) utilizing OWTS on existing parcels with a proposed or existing primary dwelling, or other proposed or existing dwellings, shall conform to the average allowable densities in Table 3.7-1 ~~or the requirements in Section 3.7.3.3.2 for OWTS with supplemental treatment for nitrogen reduction and drip dispersal with vegetative uptake design, unless a different density was addressed and approved as part of the subdivision of land, other development permit or process, or building permit process.~~ These additional dwelling units may include, but are not limited to, ~~accessory dwelling units, granny flats, casitas~~ **second dwelling units**, farm labor camps, farm labor housing, and employee housing.

3.7.3.3.4 Accessory Dwelling Units. The following are requirements for OWTS serving Accessory dwelling units. These are dwelling units that are considered an accessory use to a primary dwelling allowed on a parcel.

3.7.3.3.4.1 Proposed Accessory Dwelling Units utilizing OWTS located within a geographical area of a TMDL, APMP or areas with Basin Plan conditions or restrictions related to OWTS discharges will be considered on a case-by-case basis in consultation with the appropriate Regional Board, if needed.

~~3.7.3.3.3.2 Where additional dwelling units were not evaluated as part of the subdivision of land, other development process, or building permit process, additional evaluation shall be required to ensure the OWTS can meet all LAMP requirements and the density of the OWTS usage will not impact water quality over established water quality objectives. The additional evaluation shall be submitted to the DEHQ in an OWTS Layout Report and shall include, but is not limited to, a site investigation to determine the site is suitable for the proposed OWTS usage and a Nitrate Loading Study to evaluate potential nitrogen loading impacts from the proposal. An approved OWTS Layout Report is required before a building permit is issued. OWTS for proposed additional dwelling units on a parcel that cannot meet LAMP requirements or show a potential to impact water quality are outside the scope of this LAMP and cannot be approved by DEHQ.~~

3.7.3.3.4.2 The primary dwelling unit utilizing an OWTS must meet the average allowable densities in Table 3.7-1 for a standard OWTS or the requirements of Section 3.7.3.3.2 for OWTS

with supplemental treatment for nitrogen reduction.

3.7.3.3.4.3 A total of 900 gallons per day low strength domestic wastewater volume (total 6-bedroom equivalent for parcel) is allowed using a standard OWTS and dispersal field as the means of sewage disposal. A designated reserve area shall be adequate for a full replacement (100%) of the primary dwelling and accessory unit dispersal field required absorption areas.

3.7.3.3.4.4 A total of 1,500 gallons per day of low strength domestic wastewater volume (total 10-bedroom equivalent for parcel) is allowed using an OWTS with supplemental treatment for nitrogen reduction and drip dispersal with vegetative uptake design as the means of sewage disposal. A designated reserve area shall be adequate for a full replacement (100%) of the primary dwelling and accessory unit dispersal field required absorption areas.

3.7.3.3.4.5 The additional required septic tank capacity and dispersal field absorption area must either be added to the exiting OWTS or provided in a new OWTS to meet the minimum requirements of the LAMP.

3.7.3.3.4.6 Soil profile, percolation testing and/or groundwater testing may be required where insufficient data exists to determine compliance with absorption area and depth to groundwater requirements.

3.7.3.3.4.7 All OWTS must meet all other standards (setbacks, slopes, etc.) in LAMP.

3.7.3.3.5 Temporary Occupancies

3.7.3.3.5.1 Where there is no connection to a public sewer available, temporary occupancy authorized under Section 6100 of the San Diego Zoning Ordinance must have sanitation facilities approved by DEHQ.

3.7.3.3.5.2 A trailer (Health Care Trailer) authorized by San Diego County Zoning Ordinance Section 6118(b)(3) and San Diego Regulatory Code Section 52.204 exclusively for temporary occupancy for health care purposes may be connected to an OWTS serving a permanent single-family dwelling if the OWTS/ parcel meets the following requirements.

3.7.3.3.5.2.1 The parcel is not located within a geographical area of a TMDL, an APMP, or areas with Basin Plan or Regional Board conditions or restrictions related to OWTS discharges within an area of concern for OWTS discharges.

3.7.3.3.5.2.2 Records for the parcel and existing OWTS does not show a history of OWTS failures, repairs, complaints, or variances.

3.7.3.3.5.2.3 The parcel is of sufficient size to accommodate the existing OWTS and the required designated reserve area and meet minimum setback requirements.

3.7.3.3.5.3 An applicant for a permit to install a Health Care Trailer shall complete an application for the connection of the health care trailer to an existing OWTS on a form provided by DEHQ.

3.7.3.3.5.4 Parcels and/or existing OWTS that cannot meet the above requirements will be evaluated on a case-by-case basis and may need further evaluation, expansion of the existing OWTS, or the installation of a new OWTS.

3.7.3.3.5.5 Applications where the OWTS cannot meet minimum requirements or may pose a threat to public health or water quality shall not be approved.

3.7.4 Land Use Development Permit Review

Any person making an application for a land development use project, such as a minor or major use permit or land development project, or to modify or expand an existing land use permit, shall submit an OWTS Layout Report and any other required information or studies as requested to DEHQ for review and approval as part of the application process.

3.7.5 Building Permit and Change of Occupancy Review

3.7.5.1 No permit to install an OWTS shall be issued unless the applicant provides proof of the submittal of an application for a building permit. A property owner with an OWTS who is required to obtain a building permit for a building addition or other remodeling of an existing building or to add another stand-alone building, or for other construction which requires a building permit on the property shall obtain an OWTS Layout Report approval before a building permit or other approval is issued.

3.7.5.2 The site evaluation and other studies shall address the minimum density of dwelling units allowed for the parcel as shown in Tables 3.7-1 and 3.7-2. ~~Proposed dwelling units above the minimum in Table 3.7-1 shall be addressed in a Nitrate Loading Study prior to the issuance of a building permit.~~

3.7.5.3 A property owner with an OWTS who is required to obtain the building official's approval for a change of use or occupancy of an existing building, shall also obtain DEHQ's approval before a building permit or other approval is issued. This approval may require an OWTS Layout Report.

3.7.5.4 Because of the many factors that determine if a specific parcel is suitable for OWTS usage, it is recommended property owners complete the OWTS Layout Report and confirm the site can support the use of an OWTS before a significant investment is made towards building or architectural plans.

3.7.6 Grading Permit Review

3.7.6.1 An applicant for a grading permit to grade property where there is an existing OWTS or approved OWTS Layout Report but not installed OWTS shall obtain approval from DEHQ as part of the grading plan and permit approval process and shall demonstrate that the proposed grading will not interfere with the area where the OWTS has been installed or has been approved to be installed, including the areas designated for reserve. The proposed grading is reviewed to determine OWTS areas are not impacted

and still meet minimum requirements of the LAMP, including but not limited to the standards for slopes, setbacks, primary and designated reserve dispersal areas, soil cover, soil depth, and depth to groundwater.

3.7.6.2 The OWTS Layout Report approval will indicate if a field check of completed grading is required prior to issuance of the OWTS permit. This field check is not the same as local land use agency grading approval. It is the responsibility of the owner/applicant to ensure all required permits are obtained from the local land use agency.

3.7.6.3 Major and minor grading plans will be reviewed by DEHQ prior to grading to determine impacts to nearby OWTS. After completion of the grading, notice must be provided to DEHQ to schedule a field check, unless the field check of completed grading is waived as part of the OWTS Layout Report approval.

3.7.6.4 Grading on a property that has the potential to impact an existing or proposed OWTS or water well that does not require a local grading permit shall be subject to review by DEHQ.

3.7.7 Potable Water Supply

An applicant for an OWTS Installation Permit for a new primary dwelling unit must provide proof of potable water from a public water purveyor or from an approved well.

3.7.7.1 Proof of potable water from a public water purveyor is met with the submittal of a service availability letter from the water purveyor.

3.7.7.2 Proof of potable water from a domestic water well is met with the submittal of a laboratory reports that meets the following minimum water quality monitoring requirements:

~~3.7.7.2.1 1) a~~ At least one water sample obtained from the well within one year of submittal to DEHQ for review shall be negative for the presence of total coliform bacteria and fecal coliforms or *Escherichia coli* (E. coli)

~~3.7.7.2.2; 2) a~~ At least one water sample obtained from the well and analyzed for Nitrate (as nitrogen) shall be less than the maximum contaminant level as specified in the California Code of Regulations, Section 64431 (10 mg/L)

~~3.7.7.2.3; 3) o~~ Other sampling that may be required by the Director of Environmental Health based on known or suspected sources of pollution in the area that may affect the water quality of the well.

3.7.7.2.4 The samples shall be analyzed by a laboratory certified by the State Water Resources Control Board for that analysis pursuant to California Health and Safety Code, Division 101, Part 1, Chapter 4, Article 3, commencing with Section 100825.

3.7.7.3 An application for an OWTS will not be approved for a water supply that does not meet the

provisions of this section.

3.7.8 Temporary Onsite Wastewater Treatment Systems

The Director may issue a permit for a specific time period to a property owner authorizing construction of a temporary OWTS when the Director determines that it is highly probable that the property will have access to a public sewer within 24 months from the date of a permit application and the owner is able to demonstrate the ability to install a permanent OWTS that meets the minimum requirements of this LAMP. In approving a permit for a temporary onsite wastewater treatment system, the Director may prescribe conditions necessary to protect public health. The permit may be issued if the property owner agrees to all of the following:

3.7.8.1 The property owner agrees to connect the property to the sanitary sewer within 30 days after it becomes available to the property.

3.7.8.2 If the public sewer does not become available to the property within the period of time specified in the permit the property owner shall apply for a permit to install an OWTS within 30 days after receiving notice from the Director and shall install an approved OWTS within one year from the date the Director issues the permit.

3.7.8.2 The property owner will pump and remove all sewage and other wastes from the temporary OWTS and destroy the system following the procedures in section 3.8.6.

3.7.8.3 The property owner will not occupy the building to which the temporary OWTS will be installed or allow any other person to occupy the building until the property owner installs the system and the Director approves it.

3.7.8.4 The permit for a temporary OWTS is valid only for the time period specified and is not transferable. If the property owner sells or transfers the property, the permit for a temporary OWTS will terminate and the new property owner or transferee shall either obtain a new permit for the temporary OWTS, connect to public sewer, or install an approved OWTS that meets the minimum requirements of this LAMP.

3.7.9 Alternative Toilets or Other Sanitation Facilities

Alternative toilets or other sanitation facilities may be authorized in lieu of connection to a public sewer or to an OWTS, including during a disaster, special event or as described below. All alternative toilets and other sanitation facilities shall meet the minimum requirements in Appendix IV.

3.7.9.1 A holding tank, vault or composting dry toilet for a campsite, park or trail, when no public sewer is available, and it is impracticable to connect water to the area where the toilet is to be located.

3.7.9.2 A chemical toilet for a temporary structure for a commercial use when no public sewer is available and site conditions make it impracticable or impossible to install an OWTS.

3.7.9.3 A chemical toilet for a commercial use when no public sewer is available, the property has an existing OWTS to serve normal daily wastewater flows, and the project only needs occasional supplemental sewer capacity. The chemical toilets may be authorized only if it is determined the expansion of the existing OWTS or the installation of a new OWTS would be prohibitively expensive based on the amount of occasional use expected.

3.7.9.4 A chemical toilet for an extractive use or other industrial use in an isolated area such as a facility that receives solid waste, when no public sewer is available and site conditions make it impracticable or impossible to install an on-site wastewater treatment system.

3.7.9.5 A holding tank where public sewer is not available, the installation of an OWTS is not practicable, and the property will be able to connect to public sewer within 18 months of the issuance of permit to install and use a holding tank. The permit to use a holding tank is valid for 18 months and may be eligible for an additional 18-month extension if it is demonstrated property owner has been unable to connect through no fault of their own and the holding tank is being properly maintained. Holding tanks shall meet the minimum standards in Appendix IV.

3.8 General OWTS Permitting Process

The general process for the permitting of OWTS includes the completion and submittal of an OWTS Layout Report, which provides information relating to the project description, wastewater flow and characteristics, site evaluation elements, and OWTS design and, once approved, the issuance of a Permit to Install an OWTS is presented in this section. This process is consistent with the requirements of the San Diego Regulatory Code, Title 6, Division 8, Chapter 3 (County Code sections 68.301 et seq.), the OWTS Policy, and this LAMP.

Persons seeking a Permit to Install an OWTS from the County should review these documents as well as applicable grading, building, and land use rules from the appropriate municipal jurisdiction.

For repairs to an existing OWTS that meet the minimum requirements in Section 6.7, a licensed installer or Owner-Builder may apply for a repair permit with an *Application for Authorization for Repair Permit*. An OWTS Layout Report may be required for OWTS repairs where site conditions or other conditions warrant, as determined by DEHQ, to ensure the installation of a properly sited and designed OWTS that is protective of water quality and public health.

The process typically starts with the site evaluation and percolation testing performed by a Qualified Professional. Depending on the depth of the borehole and groundwater, a permit may be required for soil borings installed for the soil profile investigation. A Percolation Test permit is required for percolation testing. This permit provides for an initial inspection to review the application and map information and verify the proposed OWTS location is accurate and meets all setbacks or other requirements, to verify that the appropriate testing procedure was used, to inspect the test hole construction and test reference point, to review the raw field data recorded prior to arrival, and to observe and document the readings during the final 30 minutes of the test on the percolation test form.

This initial inspection allows DEHQ staff to identify any site conditions or issues that may have bearing on the approval of proposed OWTS. The information gathered from the site evaluation and percolation testing, and any other information needed, such as a slope stability study or nitrate loading study, is compiled and submitted as a complete OWTS Layout Report.

An OWTS Layout Report is completed as a prerequisite to obtaining an OWTS Installation Permit. In addition, in accordance with San Diego Regulatory Code Section 68.326, proof of potable water and proof of the submittal of an application for a building permit are also prerequisite to obtaining an OWTS Installation Permit. Additionally, the OWTS Layout Report may be submitted as a requirement of a land use project where OWTS usage for sewage disposal is proposed, including subdivisions of land, use permit applications, lot line adjustments, and certificates of compliance.

3.8.1 Submittal of Building, Grading, other Information

3.8.1.1 ~~Proof that building plans, bearing the appropriate stamp which documents plan have been submitted to the local land use agency, must be submitted with the OWTS Layout Report. The building plans need not be approved by the local land use agency before being submitted. Final building plans, bearing the appropriate stamp which documents plans have been submitted and approved by the local land use agency, must be submitted to DEHQ for review prior to an OWTS installation permit is issued to verify the OWTS Layout Report is consistent with the approved building plans.~~ Any significant building plan amendments must be timely provided to DEHQ to complete the OWTS Layout Report review. Any OWTS Layout Report approved without plan amendments first being submitted to DEHQ that may have a material effect on the OWTS compliance with local ordinance and/or this LAMP shall be null and void. A new OWTS Layout Report shall be submitted to include all required information for review.

3.8.1.2 Grading plans, ~~for grading that may impact the OWTS or water well, and those~~ if required by the local land use agency, must also be submitted to DEHQ for review. The grading plan must be submitted for review and approval before any grading work is initiated. If the approved OWTS Layout Report requires a field inspection of the complete grading, that field inspection must be completed before an OWTS Installation Permit is issued.

3.8.1.3 Proof that the site is served by potable water from a public water purveyor or an approved well must be provided prior to the issuance of an OWTS Installation Permit. A copy of a “will serve” letter from the public water purveyor or copies of laboratory reports for the approved water well pursuant to Section 3.7.7 may provide this proof of potable water.

DEHQ will review all information to determine consistency with the project description applicable requirements. Depending on the scope of the proposed project, DEHQ may request other information and documentation, as needed to determine the site is suitable for OWTS usage.

3.8.2 Submittal of the OWTS Layout Report

The OWTS Layout Report provides information to ensure the area of the lot or parcel proposed for the siting and installation of an OWTS suitable for the proposed system and meets all minimum

requirements, including setback distances. The OWTS Layout report must provide all the minimum required information to be considered complete and must be signed, dated, and stamped (if applicable) by the appropriate Qualified Professional. In accordance with San Diego Regulatory Code, Section 68.382, Qualified Professionals may be required to demonstrate their knowledge of County of San Diego ordinances and policies related to the design of OWTS.

The OWTS Layout Report includes the following minimum information:

- Project description.
- Wastewater sources, flows and characteristics description.
- Site evaluation and percolation testing results, and other information as required by the LAMP or Director of Environmental Health.
- Proposed OWTS type (conventional, supplemental) and design specifications.
- Verification that all setback distances are met.

3.8.2.1 Project Description

At a minimum, the project description must include the type and scope of the land use project/permit or building permit being applied for, the location and extent of any **existing or proposed** grading ~~is proposed~~ at the site, public sewer provider information (name of the sewer district the parcel is within and/or the distance to the nearest public sewer to the site), the water source approved for the site (proof of water well potable water, public water provider name), the location of all public water lines on or within 20 feet of the property, and the sign-off of the OWTS Layout by the local water district or company, if required (Vista Irrigation District, Rincon del Diablo, Yuima, County Service Areas).

The OWTS Layout Report must include accurate, to-scale maps and plot plans showing required information, as determined by the Director of Environmental Health.

3.8.2.2 Wastewater Sources, Flows and Characteristics Description

The sources of wastewater generated at the site are provided, including a description of the number and types of buildings (residential/commercial) and the uses of these buildings. The projected and peak wastewater flows are calculated and provided, with the source of the flow information included. The characteristics of the wastewater generated at the site is described, to include at a minimum biological oxygen demand (BOD), total suspended solids (TSS), fats, oils and grease (FOG), and total nitrogen (TN), with the source of the information included. **Additional information on wastewater flows and characteristics are provided in Chapter 6.0, Siting and Design Criteria.**

3.8.2.3 Site Evaluation and Percolation Testing

Each element of the site evaluation and percolation testing described below must be performed by an appropriate Qualified Professional. In addition to this information, a determination as to the nature and extent of nitrogen loading from the proposed OWTS may be required.

3.8.2.3.1 Soil Depth and Characteristics and Percolation Testing

These elements provide information as to the degree the soil in the proposed OWTS dispersal area can accept wastewater discharge over a period of time.

Soil structure and texture are determined by evaluation of site soils from borings or excavations. **Soil profile borings and percolation test holes are installed under a Percolation Test/Soil Profile Permit.** Borings over 20 feet in depth or that extend into groundwater require a Well Permit. It is the applicant's responsibility to ensure any required permits are obtained prior to the start of work.

Percolation testing, with results in minutes per inch (MPI), is conducted ~~as a means~~ to estimate water absorption capacity of the soil. The test is conducted with clean water (wastewater will typically percolate slower than clean water) and test results can be used to establish the dispersal system design. The minimum number of percolations tests must be performed in accordance with the specifications and methods described in this LAMP. A Percolation Test Permit is required for all percolation testing **and shall include the minimum information as required by DEHQ.** It is the applicant's responsibility to ensure any required permits are obtained prior to the start of work.

Percolation testing **performed under a permit issued by DEHQ** is required for all OWTS design and installation areas. Additional percolation testing may be required as conditions warrant, including for areas where grading or other soil disturbance has occurred in the proposed OWTS location, the OWTS dispersal system is being shifted out of the previously tested area, or an OWTS other than the system previously considered is being proposed. ~~DEHQ may require the submittal of a percolation design plan for approval prior to percolation testing. The approval of a percolation test design plan expires after one year. However,~~ **Unless site conditions change, percolation test results conducted under a permit and observed by DEHQ staff** ~~unless site conditions change, percolation test results~~ typically remain valid for the specific area tested.

Note: Grading or clearing of brush for the purposes of completing a percolation test may ~~require~~ **be subject to permitting or other requirements by other agencies, such as** the implementation of storm water best management **practices,** and/or **local land use agency** ~~Planning and Development Services (PDS)~~ approval. It is the applicant's responsibility to meet all other local, state or federal requirements.

3.8.2.3.2 Unsaturated Soil Interval

This element determines the amount of unsaturated soil available to treat wastewater in the proposed dispersal area. This is the distance between the bottom of the infiltrative **surface area** ~~layer~~ of the dispersal system and the highest anticipated groundwater level or the shallowest impervious subsurface layer at a site.

Details of the unsaturated soil interval and minimum depth to groundwater from the bottom of the **infiltrative surface area** ~~dispersal field~~ can be found in Chapter ~~4.06.0~~, Section **4.16.3.**

3.8.2.3.3 Limiting Geological Features

This element determines any geological features that may limit the suitability of the site for OWTS usage, including, but not limited to, **impervious subsurface layers**, grading areas, fill areas, degree of slope, slope stability, unstable land masses, and rock outcropping areas.

3.8.2.3.4 OWTS Design Specifications

The scope and extent of the proposed project, the wastewater amounts and characteristics, and the site evaluation information are used to determine the appropriate siting and design criteria for the OWTS in accordance with the provisions of this LAMP. The information on the design criteria for the OWTS are provided in greater detail in Chapter ~~4.0~~**6.0**.

3.8.2.3.5 Siting of OWTS on Lot/Net Usable Land Area

This element ensures there is sufficient area available for the original OWTS installation and for the required 100% replacement area that meets all setback requirements to water wells, structures, easements, watercourses, or geologic limiting factors. Site plans must be to-scale, accurate and provide sufficient information to enable DEHQ to complete a review of the proposal to determine the OWTS meets all required provisions of this LAMP.

3.8.2.3.6 Other Information as Required

Additional information or studies may be required based on the proposed project, the potential for cumulative effects on ground and surface waters, and site-specific conditions. OWTS proposed for areas with steep slopes, **and** shallow groundwater, ~~subdivision developments, commercial establishments, or OWTS that receive greater than 1,500 gallons per day of sewage~~ are examples of conditions where additional studies may be required. The following are examples of additional information that may be requested as part of an OWTS Layout Report review.

3.8.2.3.6.1 Groundwater Level Study. This study will assess the probable rise in the water table underlying the site, including effects of rainfall, OWTS recharge, landscape irrigation and groundwater pumpage. The study must follow the procedures as outlined in this LAMP.

~~3.8.2.3.6.2 Nitrate Loading Study. A nitrate loading study, with the minimum required information pursuant to Section 4.4 and Appendix II, looks at the cumulative nitrate load, using an acceptable mass balance method, to determine whether the nitrate load from the project has the potential to cause the concentration of nitrate in ground water to exceed applicable groundwater quality objectives. This study is required for development projects where the average allowable density of dwelling units specified in Table 3.7-1 cannot be met. Additionally, a study may be required where the use and occupancy of a project proposed development of a property has the potential to impact water quality over established water quality objectives.~~

~~3.8.2.3.6.3~~ Slope Stability Study. A slope stability study with the minimum required information pursuant to Section 4.5 is required for proposed OWTS and dispersal fields in slopes

exceeding 2530% where the initial site evaluation has identified evidence of instability, such as unconsolidated fill, significant erosion rills, tension cracks, evidence of prior earth movement or slides, and leaning trees. The slope stability study shall ~~to~~ determine the potential for land movement to impact the dispersal system as well as the potential impact of the dispersal field to affect slope stability. The study shall ~~and to~~ identify any mitigating actions, if applicable, to effectively maintain slope stability with dispersal field usage.

~~3.8.2.3.6.4~~ 3.8.2.3.6.3 Microbiological Travel Time Study. This study is required for dispersal systems located within 600' feet of public well and where the dispersal field depth exceeds 20' feet (Policy 9.4.10.3).

3.8.2.3.7 Federal Class V Injection Wells. OWTS that receive wastewater, such as from floor drains or sinks that receive non-domestic, industrial or commercial wastes, or that receive domestic wastewater from multifamily or non-residential establishments and have a capacity to serve 20 or more persons per day are considered Class V Injection Wells pursuant to Code of Federal Regulations, Title 40, Part 144 - Underground Injection Control Program.

Owners of OWTS that are considered a Class V Injection Well covered under the scope of this LAMP shall comply with Environmental Protection Agency registration requirements and provide proof of such registration to DEHQ prior to the issuance of a Permit to Install an OWTS or an OWTS Replacement/Repair Permit. OWTS that receive non-domestic wastewater are not covered under the scope of coverage of the LAMP and will be referred to the RWQCB for oversight.

3.8.3 OWTS Layout Report Approval

3.8.3.1 Completeness Review

DEHQ will review the OWTS Layout Report for completeness, along with any required plans or other requested information. Submitters of OWTS Layout Reports that do not provide the minimum required information in sufficient detail to enable the review of the proposed OWTS will be notified of the missing or insufficient items. The review of the OWTS Layout Report will be suspended until such time all required information is submitted. If **all** the required information is not submitted within one year from the date of the original submittal, ~~60 days from date of the letter requesting such information~~, then the OWTS Layout Report submittal will be **expired** ~~considered withdrawn~~ **and a new OWTS Layout Report must be submitted for review for the project.** ~~The applicant may submit a request in writing proposing an alternative date to submit the required information but should be no more than 120 days from the date of the letter.~~

3.8.3.2 Standards Compliance Review

Once a complete report is submitted, DEHQ will review the information for compliance with the requirements of the San Diego Regulatory Code, the OWTS Policy, and this LAMP. DEHQ may require additional testing or information, if needed to be able to complete the standards review. **Any additionally requested testing or information shall be submitted to DEHQ within 60 days from the date of the letter**

requesting such information. The applicant may submit a request in writing proposing an alternative date to submit the required information. Except in the case of ongoing groundwater monitoring, if the additional testing or information is not submitted within one year of the date the OWTS Layout Report was received, the OWTS Layout Report will be expired. For cases where ongoing groundwater monitoring is required to provide ~~In some cases, this additional testing may include~~ depth to groundwater measurements during a normal average rainfall year, ~~test borings or groundwater monitoring wells shall be installed under permit from DEHQ within one year of the date the OWTS Layout Report was submitted or the OWTS Layout Report will be expired. As this additional testing may delay an OWTS Layout Report approval is dependent on sufficient rainfall and may take for a year or more,~~ the OWTS Layout Report shall not expire after one year as long as a test boring or groundwater monitoring well(s) has been installed under permit and monitoring is in progress.

3.8.3.3 OWTS Layout Report Approval

3.8.3.3.1 OWTS Layout Reports that meet the prescribed minimum requirements will be approved. The approval may include a requirement for a field inspection by DEHQ after the completion of any grading at the site to ensure consistency with the approved OWTS Layout Report.

3.8.3.3.2 OWTS that fail to meet the prescribed minimum requirements in this LAMP will not be approved as proposed. For those proposals which are outside the **scope of** coverage of this LAMP, applicants may submit the proposal to the appropriate Regional Board for consideration. Because of the potential for delays or disapproval, it is recommended that applicants submit an OWTS Layout Report and obtain approval before incurring costs for detailed building plans and architectural fees.

3.8.3.3.3 Approval of the OWTS Layout Report remains valid for one year. DEHQ may approve the use of elements or data included in a previously approved but expired OWTS Layout Report where appropriate. This approval may be withdrawn if it is determined the approval was issued in error or on the basis of incorrect, inaccurate, or incomplete information, or in violation of the San Diego Regulatory Code, the OWTS Policy, and this LAMP, or when there is a change in regulatory requirements, or other circumstances, or a change to the conditions of the property since the date of approval, which would cause the proposed OWTS to fail to meet minimum requirements.

3.8.4 Approved OWTS Layout Report Related to Other Permits

Where OWTS is proposed as the means of sewage disposal, local land use agencies may require the submission of proof of a parcel's suitability for OWTS usage before issuing related permits, such as grading or building permits. The approval of the OWTS Layout Report is typically used to provide this documentation.

3.8.4.1 In the unincorporated parts of the County where OWTS is proposed as the means of sewage disposal, County land use agencies may require an approved OWTS Layout Report and a valid OWTS Installation Permit before building plans are approved or a building permit is issued. Other local land use agencies also typically require **an approved OWTS Layout Report, or** that an OWTS Installation Permit be issued before building plans will be approved or a building permit issued.

3.8.4.2 Local land use agencies typically require that all OWTS installation inspections be completed and the OWTS Installation Permit is finalized by DEHQ before occupancy permits are issued.

3.8.5 OWTS Installation and Repair Permits

3.8.5.1 Application for Permit to Install OWTS. An application for installation of the OWTS must be submitted on a form provided by DEHQ and may be submitted at the time of the OWTS Layout Report or at any time prior to the permit issuance. The permit to install an OWTS shall be issued to a Licensed Installer or an Owner-Builder after an OWTS Layout Report has been approved, and any other information or grading requirements, including any field inspections, have been submitted and DEHQ review has been completed. A separate application is required for each OWTS installation.

3.8.5.2 Application for Permit to Repair OWTS. ~~For repairs~~ An application for a repair of an existing OWTS that meets the minimum requirements in Section 6.7, shall be made on a form provided by DEHQ. ~~the application shall be made on an Authorization to Repair.~~ The repair permit shall be issued to a Licensed Installer or an Owner-Builder.

3.8.5.3 The minimum information for an application for a permit ~~Repairs to existing OWTS that meet the minimum requirements in Section 6.7~~ The application shall be made on a form or forms provided by the DEHQ and shall include the following minimum information, and other information as requested by DEHQ:

3.8-1: Minimum Application and OWTS Layout Site Plan Elements			
Professional's name, mailing address, email address, and phone number			
Type of proposed construction (Ex: Residential, Commercial, Industrial)			
Scope of work: Residential:	Type of Construction	# Bedrooms	
Scope of work: Commercial:	Business Type	Volume of Wastewater	Character and Strength of Wastewater
Commercial Food Service-location, design, and size of oil/grease interceptor			
Legal Basis of parcel (map and lot number, plat number, etc.)			
Vicinity Map; Scale (engineer scale not to exceed 1" = 60'); North arrow; Layout does not exceed 11" x 17" paper			
Property Lines and lot dimensions (provide an over sheet (larger scale allowed) and detail sheet(s) for large parcels)			
Topographic lines and elevation points (include pad grade, finished floor, septic tank, leach lines, slope arrows, slope range, etc.).			
Existing and proposed primary and reserve Onsite Wastewater Treatment System (OWTS) tank and dispersal design detail			
All setback distances are shown on layout map			
All proposed and existing grading; Rock outcroppings/significant features; Slopes in excess of 20%; Evidence of unstable land mass or slope instability, such as unconsolidated fill, significant erosion rills, tension cracks, evidence of prior earth movement or slides, and leaning trees.			

All known, recorded easements on or within 20 feet of lot boundaries (open-space, utility, road, waterline, etc.)
Identify source of potable water; Location of all public waterlines on or within 20 feet of property and signed water line statement
Location of all wells on or within 150 of feet of property line; Location of all Public wells within 600 feet of property line
Location of drinking water reservoir within 2,500' of property line ¹
Location of drainage ways; location of streams, springs, ponds, flood plains, lakes within 200 feet of property line ¹
All soils testing information (deep borings, test holes, and/or percolation tests) plotted on the design (matched to flagged locations in field)
Name of groundwater basin ² ; Depth to groundwater data and specific method used to determine depth to groundwater
Location of all stormwater treatment and retention features located on site and within 25 feet of property line
Sign-off of layout by local water district or company, if required (Vista Irrigation District, Rincon del Diablo, Yuima, County Service Areas)

1-To include surface water bodies information based on the United States Geological Service's *The National Map* (<https://apps.nationalmap.gov/viewer/>) as well as site specific observations of local drainage patterns.

2-To include groundwater basin information based on California Natural Resources Agency's Groundwater Basin Boundary Descriptions-San Diego Region 9-XXX Basins (<https://data.cnra.ca.gov/dataset/bbd9>)

3.8.5.4 It is recommended the applicant/permittee maintain all OWTS related records and paperwork, including project control numbers, obtained from each Department or agency so information that may be requested is readily available.

3.8.5.2 Issuance of Permit. An OWTS Installation Permit will be issued after all required information has been submitted and DEHQ finds the proposal meet all minimum requirements. The Director of Environmental Health may deny an OWTS Installation Permit for an OWTS that meets the requirements if it is determined that the OWTS **may contaminate** ~~will have any adverse effects on~~ an underground source of water or ~~on the~~ **impact** public health and safety. Applications for OWTS installation that do not meet the minimum requirements will be denied. **No person shall initiate any work to install, construct, reconstruct, repair, add to, modify or abandon an OWTS without first obtaining a permit issued by DEHQ.**

3.8.5.3 Installation Work. Work may commence to install the OWTS in accordance with the approved permit only after an OWTS Installation Permit has been issued. AAAs soon as the installation work is completed, the permittee shall provide a minimum **2448**-hours advance notice to DEHQ that the system is ready for an inspection. **Inspection times are based on staff availability.** No person shall backfill, or cause another person to backfill, an OWTS installation before DEHQ inspects and approves the work.

3.8.5.4 Finalization of OWTS Installation Permit. The OWTS Installation Permit will be noted as competed or "finalized" only after the inspection verifies the OWTS was installed as approved and permitted and that the OWTS meets all minimum requirements. **An OWTS Installation Permit is not completed or "finalized" until all required information is submitted, such as telemetry and maintenance contract**

documentation, engineer's letter or supplemental treatment system installation statement of certification, as-built drawings, or an annual operating permit.

3.8.5.4.1 Notice will be provided to the permittee of any components of the installation that do not conform to the approved permit or to applicable minimum standards. Corrections shall be made as specified and a request for a reinspection must be made within ten (10) business days. If the permittee fails to correct the deficiency as specified or fails to request a reinspection, the OWTS Installation Permit may be revoked and/or other appropriate enforcement actions may be taken.

3.8.5.4.2 Submittal of revised drawings or "As Built" drawings. A revised OWTS Layout drawing shall be submitted if the actual installation of the OWTS differs from that shown in the approved OWTS Layout Report drawing at the completion of the installation and prior to the finalization of the permit.

The "As Built" drawing shall reflect changes made in specifications and show exact dimensions, geometry, and location of all elements of the work completed under the permit. In addition to the requirements listed in Table 3.8-1, the items noted in Table 3.8-2 are required for the revised drawings.

3.8-2: Revised OWTS Layout Drawing "As Built" Elements
Actual location and dimensions of all OWTS components and dispersal system drawn to scale.
Any changes to the scale of the drawing.
Distances from OWTS components and dispersal system to buildings, driveways, water wells, property lines, other dispersal systems.
All changes in materials used, including material type, locations used, changes in sizes, etc.
Actual location of all utilities, stormwater features, drainage courses at the site.
All unexpected obstructions or issues encountered, and the solutions used.
Sufficient information to enable the OWTS to be located in the future.

3.8.6 OWTS Destruction Permit

3.8.6.1 A permit is required to destroy a cesspool, septic or other tank, or seepage pit. A person may apply for an OWTS Destruction Permit by submitting an application on a form provided by the DEHQ along with the associated fee.

3.8.6.2 Any component of an OWTS shall not be considered properly destroyed until a permit has been obtained and the destruction **certified by the applicant, or** inspected and approved by DEHQ, **if required.**

3.8.6.3 A cesspool, septic or other tank, and seepage pit that has been abandoned or has been discontinued otherwise from further use, or to which no waste or soil pipe from a plumbing fixture is connected, shall have the sewage removed therefrom and be completely filled with earth, sand, gravel, concrete, or other approved material **within 30 calendar days from the date of abandonment or disuse.**

3.8.6.4 The top cover or arch over the cesspool, septic or other tank, or seepage pit shall be removed before filling, and the filling shall not extend above the top of the vertical portions of the sidewalls or

above the level of the outlet pipe, ~~if an inspection has been called and~~ **is required for** the cesspool, septic or other tank, or seepage pit ~~has been inspected~~. After such inspection, **or where an inspection is not required**, the cesspool, septic tank, or seepage pit shall be filled to the level of the top of the ground.

3.8.6.5 No person owning or controlling a cesspool, septic tank, or seepage pit on the premises of such person or in that portion of a public street, alley, or other public property abutting such premises, shall fail, refuse, or neglect to comply with the provisions of this section or upon receipt of notice so to comply from the DEHQ.

3.8.6.6 Where disposal facilities are abandoned consequent to connecting premises with the public sewer, the ~~permittee~~ **property owner** making the connection shall fill abandoned facilities under permit, and inspection if required, by DEHQ within 30 days from the ~~time~~ **date** of connecting to the public sewer.

3.8.6.7 A property owner or a person in control of a cesspool, septic tank, seepage pit, or other unsafe component used for sewage disposal shall immediately secure any unsafe or hazardous conditions as soon practicable.

3.9 General Operation and Maintenance Requirements

3.9.1 All OWTS shall be operated and maintained pursuant to the minimum requirements of this section, or pursuant to an Operations and Maintenance Plan compiled by the qualified professional.

3.9.2 All standard OWTS shall be operated and maintained pursuant to the following:

3.9.2.1 **As an option to meet the requirements of OWTS Policy Section 8.2.4 and to preserve the integrity of the dispersal system's absorptive capacity**, ~~septic tanks~~ shall be pumped on a regular basis, but at least once every three years. The frequency of pumping may be modified based on **documented** actual measured scum (**floating layer**) and sludge (**settled bottom layer**) accumulation rates. **At no time shall the scum layer be within three (3) inches of the outlet pipe opening and the sludge layer be within eight (8) inches of the outlet pipe opening.**

3.9.2.2 All septic tank pumping records shall be maintained by the property owner for a minimum of six years and shall be provided to the DEHQ upon request.

3.9.2.3 Effluent filters shall be cleaned or replaced in accordance with the manufacturer's recommendations.

3.9.2.4 All at grade risers shall be maintained safe and secure at all times.

3.9.2.5 All OWTS with supplemental treatment ~~and OWTS with lift stations and alarm systems~~ shall have a written OWTS Operations and Maintenance Plan developed by a qualified professional and approved by DEHQ at the time of installation. The property owner shall ensure that the OWTS Operations and Maintenance Plan is implemented as written. ~~Changes to the plan must be submitted in writing to DEHQ for approval.~~

3.9.2.6 A standard OWTS with lift stations and alarm system shall have a written OWTS Operations and Maintenance Plan developed by a qualified professional and approved by DEHQ at the time of installation.

CHAPTER 4.0 SITE EVALUATION REQUIREMENTS

This chapter provides the minimum requirements for the evaluation of a site or parcel to determine if the site or parcel is suitable for the usage of an OWTS for sewage disposal for the described proposed activity, including a proposed building permit or development project. As the hydrology, geology, topography, and climate vary in the San Diego County region, this site evaluation is integral to determine if a parcel is suitable for OWTS usage.

4.1 Groundwater Information and Testing Requirements

This section provides the method to be used for determining groundwater levels when siting and designing OWTS with the purpose to:

- Protect the groundwater quality by maximizing treatment of the sewage effluent prior to its entering into the groundwater.
- Protect the public health from surfacing sewage or effluent caused by high groundwater.
- Provide a methodology for the evaluation of groundwater depths to determine minimum groundwater separation requirements at sites using or proposing to use an OWTS.

A minimum five-foot separation is required to be maintained between the bottom of a standard OWTS dispersal system and the highest anticipated groundwater level. Additional separation may be required depending on the percolation test results for the site in accordance with Table 6.3-1. For OWTS with supplemental treatment, the required separation may be reduced to no less than two feet. This reduction may be allowed based on the level of additional treatment provided by the supplemental treatment system.

Groundwater typically fluctuates seasonally depending on local geology and rainfall amounts. Rising groundwater levels have been documented in certain areas that use imported water. Groundwater levels fall in response to lack of rain and well extractions and rise in response to rainfall and in some cases, increased irrigation return water from agriculture operations. Fluctuations in groundwater elevations from a few inches to greater than twenty feet have been documented. Major fluctuations have been documented in areas such as the Ramona and Valley Center basins.

OWTS in areas with high groundwater may result in sewage effluent backing up into homes and surfacing on the ground creating public health hazards and may contribute to the impairment of surface and groundwater resources.

During the above normal rainfall periods in the late 1970's and early 1980's, previously approved lots were observed to have high groundwater impacting the proposed or existing OWTS. In 1980, a requirement to determine the actual or potential groundwater levels be verified prior to issuing OWTS permits was implemented. The requirement specified depths of test holes be used based on the season and also specified that the presence or absence of groundwater in these holes would be adequate to determine if an OWTS permit could be issued.

Since 1980, several wide fluctuations in the quantity of rainfall have been documented. Over periods of time, there have been drought cycles followed by cycles of normal to above normal rainfall. During periods of normal or above normal rainfall, the 1980 groundwater requirement was generally sufficient to determine if high groundwater was a concern prior to issuing an OWTS permit. Experience has shown that there are instances where the absence of groundwater in a ten, fifteen or even twenty-foot-deep observation boring on a lot does not guarantee that groundwater will not rise to within five feet from the bottom of the proposed OWTS during periods of normal or above normal rainfall. In some cases, the only certain way to determine depth to high groundwater on a site is to observe the groundwater depth during or immediately after an above average rainfall season. An OWTS Installation Permit cannot be issued in areas where groundwater has been documented to rise to a level that would violate the dispersal field separation requirements of this LAMP or in low lying areas with a potential for flooding.

4.1.1 Development Projects

Subdivisions, parcel maps, boundary adjustments, certificates of compliance, and other development projects within the scope of coverage of this LAMP that propose to utilize OWTS as the means of sewage disposal are required to submit documentation that the project site or each lot can support an OWTS that meets the minimum requirements of this LAMP. ~~For subdivisions of five or more lots, RWQCB approval is required when OWTS is proposed.~~ Documentation of soil testing results to show groundwater separation requirements have been met is a part of the site evaluation process and is included in the OWTS Layout Report.

To meet this requirement for soil evaluation and groundwater separation, test borings and/or monitoring wells for monitoring groundwater shall be installed under permit from DEHQ, ~~if required.~~ Maps showing the location of the borings and their logs shall be included in the OWTS Layout Report. The qualified professional must determine the actual and potential high groundwater levels in the area of the proposed OWTS at the time of the OWTS Layout Report submittal to DEHQ for review.

The qualified professional must include sufficient documentation and sources to support any conclusions, including the likelihood that seeps or springs would develop as a result of the OWTS discharge, and that the historic high groundwater elevation will not encroach upon the minimum separation required between the bottom of the proposed OWTS and the highest anticipated groundwater level. The supporting data may include, but not be limited to, data on the site topography, soils, geology, basin studies, hydro-geologic studies, and groundwater monitoring data from the on-site ~~observation~~ test borings and/or monitoring wells through an average or above normal average rainfall year and/or where full groundwater recharge has occurred.

Transient high groundwater conditions (spikes) must be documented thoroughly if encountered. A written discussion by the qualified professional must be included in the OWTS Layout Report along with groundwater monitoring log(s) for review and concurrence.

~~DEHQ and/or the RWQCB may require a comprehensive hydro-geologic study. This study may include but not be limited to; data such as rainfall, total imported water use, projected water use, surface drainage, geologic formations, depth of water table and other relevant data.~~

4.1.2 Existing Lots

4.1.2.1 If a groundwater investigation has been completed for an existing lot, additional test borings may be required if a site review reveals any evidence of groundwater changes, including but not limited to; plant growth, ponding water, or OWTS failures in the area. DEHQ staff will specify the depth and the locations of the additional test borings in consultation with the qualified professional in charge of the project.

4.1.2.2 When groundwater is observed in the borings and DEHQ has reason to believe that groundwater could rise to an unacceptable level during the course of a normal rainfall season, monitoring may be required to determine that groundwater will not rise to an elevation that will not provide the minimum separation required from the bottom of the proposed OWTS. Monitoring, if required, must be conducted during the course of an above average annual rainfall year and/or when full groundwater recharge has occurred.

4.1.2.3 When groundwater is not observed in the boring but there is evidence of past high groundwater levels, such as soil mottling or documentation of groundwater rise on adjacent properties, groundwater monitoring may be required.

4.1.2.4 The groundwater separation requirements will be met when the test results, including no groundwater observed in the test borings, indicate the minimum soil depth and groundwater separation requirements can be met, and there is no known history of rising groundwater, no evidence of groundwater changes, as noted in section ~~4.1.3.1~~ 4.1.2.1.

4.1.2.5 The qualified professional must include sufficient documentation and sources to support any conclusions, including the likelihood that seeps or springs would develop as a result of the OWTS discharge, and that the historic high groundwater elevation will not encroach upon the minimum separation required between the bottom of the proposed OWTS and the highest anticipated groundwater level.

4.1.2.6 The supporting data shall include, but not be limited to, data on the site topography, soils, geology, basin studies, hydro-geologic studies, and groundwater monitoring data from the on-site ~~observation~~ test borings and/or monitoring wells through an average or above normal average rainfall year and/or where full groundwater recharge has occurred.

4.1.3 Procedure for Groundwater Determination

4.1.3.1 Number of Test Borings. Test borings and/or monitoring wells must be installed in the area of the proposed OWTS dispersal field to demonstrate area can meet groundwater separation requirements. The minimum number of test borings shall be one boring. In areas of high groundwater, a minimum of one test boring and/or monitoring well shall be installed and groundwater monitored until sufficient data is obtained to document any observed groundwater levels during normal to above normal annual rainfall an average or above average rainfall year and/or where full groundwater recharge has occurred.

Additional test borings may be required depending on the areal extent of the dispersal area and site-specific conditions. **A permit is required to install a test boring or monitoring well. Construction standards for a test boring is provided in Appendix V – Test Boring Construction Standards.** ~~A Well Permit boring or monitoring well permit is required for borings over 20 feet in depth or when groundwater is present, or when installing a well or piezometer.~~

4.1.3.2 Minimum Depth of Test Borings. Test borings in the area of an OWTS **dispersal system** shall extend to a minimum of 15 feet or to 10 feet below the depth of the required soil separation based on the percolation rate, as shown in Table 6.3-1, whichever is greater, unless refusal is reached. In no case shall there be less than 5 feet of unsaturated, permeable soil below the bottom of the infiltrative surface depth, unless an OWTS with supplemental treatment is proposed. An OWTS with supplemental treatment for nitrogen reduction shall have no less than two feet of unsaturated, permeable soil below the bottom of the infiltrative surface depth. An OWTS with supplemental treatment for pathogen reduction shall have no less than three feet of unsaturated, permeable soil beneath the dispersal infiltrative surface depth. Deeper depths may be required depending on site-specific conditions as determined by DEHQ or the project qualified professional. Site-specific conditions may include, but not be limited to: the proposed depth of the system, local geology, soil types encountered, elevation and terrain, features on site, evidence and/or knowledge of historic ground water levels in the area, and the anticipated fluctuation of the groundwater table in times of normal to above normal annual rainfall.

4.1.3.3 To allow time for groundwater to stabilize in the test boring, the groundwater measurement must be taken after a minimum of 72 hours from boring completion. ~~A forty-eight (48-) hours' notice must be provided to DEHQ for staff to observe the boring after the 72-hour time period. The qualified professional and/or the property owner shall be responsible for securing open borehole or other excavations to protect the public from any hazards related to the test borings. For locations that will need ongoing groundwater monitoring, the test borings that encounter groundwater shall be converted to monitoring wells.~~

~~4.1.3.4~~ **4.1.3.4** During periods of below normal average rainfall, or after periods of drought where there has not yet been sufficient ground water recharge, the absence of groundwater in test borings or monitoring wells in areas where groundwater is suspected may not satisfy the groundwater separation testing requirements. In these areas, it shall be necessary for ongoing monitoring of **test borings or monitoring wells** for a sufficient period of time to determine groundwater levels during **an average or above average rainfall year and/or where full groundwater recharge has occurred** ~~normal to above normal rainfall~~. **In addition to the groundwater measurements obtained by the qualified professional, DEHQ shall obtain groundwater measurements from the test borings on a periodic basis during wet weather and dry weather, if site conditions warrant (irrigation practices), to observe site conditions, monitoring well conditions, and to validate the groundwater measurement data.**

4.2 Soil Information and Testing

In addition to the groundwater testing requirements, ~~t~~The following soil testing requirements are designed to characterize the soil, including structure and texture, underlying the proposed dispersal field to determine the suitability for OWTS usage. The evaluation shall determine that adequate soil depth is

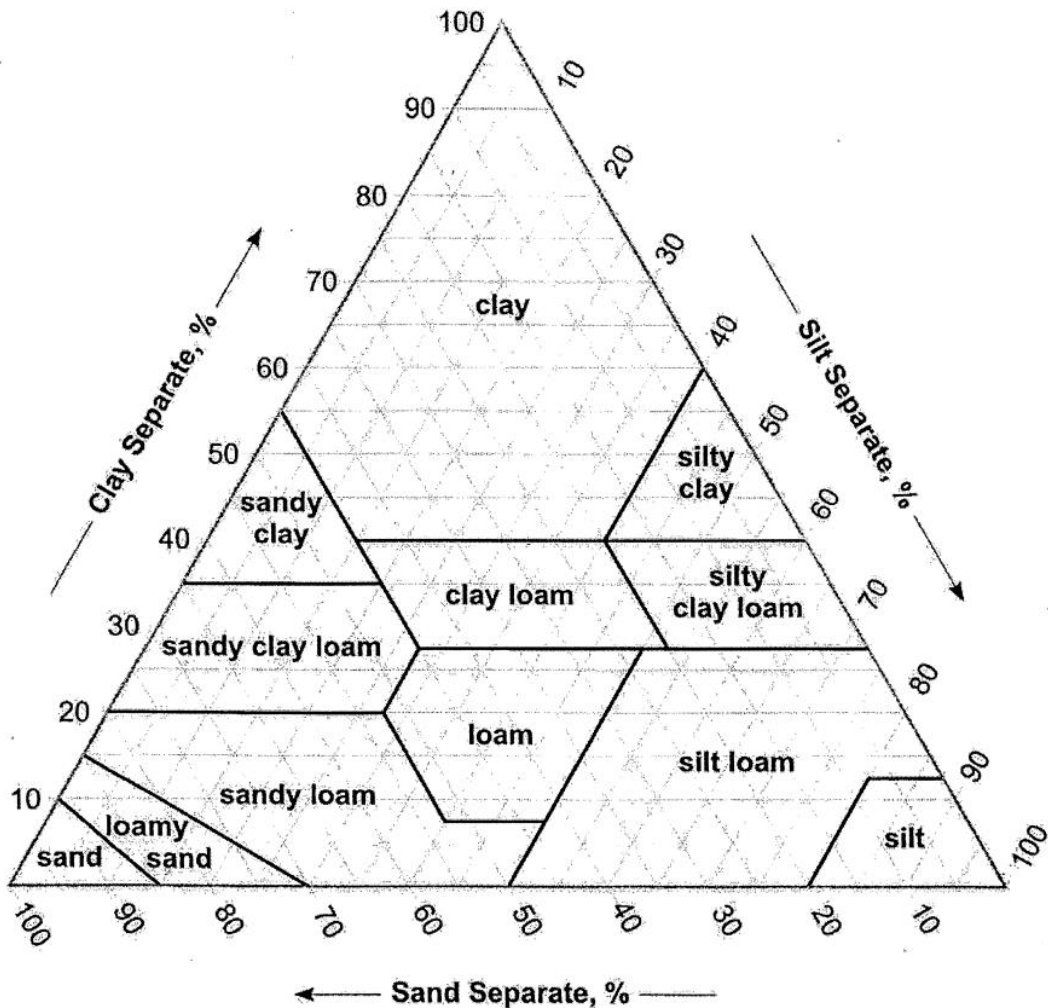
present in the dispersal area. Soil depth is measured vertically to the point where bedrock, hardpan, impermeable soils, or saturated soils are encountered or are anticipated to be encountered or an adequate depth has been determined. Soil depth shall be determined through the use of soil profile(s) in the dispersal area and the designated dispersal system reserve area, as viewed in soil borings or excavations in representative areas.

~~A boring or monitoring well permit is required for borings over 20 feet in depth, when groundwater is present, or when installing a monitoring well.~~ A permit is required to install a test boring or monitoring well. Construction standards for a test boring is provided in Appendix V – Test Boring Construction Standards. All boring results are to be reported, including any results indicating the area is not suitable for OWTS usage and test holes which encountered groundwater or refusal.

4.2.1 All test holes, deep borings and monitoring wells shall have the soil class that describes the relative amount of sand, clay, silt and combinations thereof as defined by the soil textural triangle developed by the United States Department of Agriculture (USDA) as the Soil Survey Manual, Handbook 18).

Figure 4.2-1: Soil Textural Triangle

Soil Textural Triangle



4.2.2 Rock fragment content of native soil surrounding the dispersal system shall not exceed 50% by volume for rock fragments sized as cobbles or larger and shall be estimated using either the point-count or line-intercept methods.

4.2.3 Leach Line System and Deep Bed Dispersal System Minimum Soil Testing Requirements

4.2.3.1 At least one deep boring **in the primary and one boring in the designated reserve area** should extend to a depth of at least 15 feet below the bottom of the infiltrative surface depth, or a minimum of 10' **feet** below the required vertical separation to groundwater as found in Table 6.3-1, or to

impermeable material. Additional borings may be required to document uniformity of soil conditions in the dispersal and reserve areas.

4.2.3.2 Backhoe excavations may be required to demonstrate uniformity of soil throughout the leach field area(s).

4.2.3.3 ~~Leach lines~~ **Dispersal systems** in steep slopes: A minimum of ~~two~~ **one** soil profile borings **in the primary and one boring in the designated reserve area** shall be installed to a depth of 10 feet below the proposed infiltrative surface depth, or a minimum of 10 feet below the required vertical separation as found in Table 6.3-1, to demonstrate uniform conditions throughout the disposal area. **Additional borings may be required, as needed to make this demonstration.**

~~4.2.4~~ **Deep Bed Systems Minimum Soil Testing Requirements**

~~4.2.4.1 At least one deep boring should extend to a depth of at least 10 feet below the bottom of the deep bed infiltrative surface depth, or a minimum of 10 feet below the required vertical separation as found in Table 6.3-1, or to impermeable material.~~

~~4.2.4.2 Backhoe excavations may be required to demonstrate uniformity of soil throughout the seepage pit area when the pit is proposed in an area of variable soil conditions.~~

~~4.2.5~~ **4.2.4 Vertical Seepage Pit System Minimum Soil Testing Requirements**

~~4.2.5.1~~ **4.2.4.1** Vertical seepage pits are restricted to coastal, sedimentary basins where the groundwater has been excepted from the water quality objectives as provided in the Water Quality Control Plans.

~~4.2.5.2~~ **4.2.4.2** At least one deep boring should extend to a depth of at least 10 feet below the bottom of the seepage pit ~~or to the minimum vertical separation from groundwater, whichever is greater.~~

~~4.2.6~~ **4.2.5 Drip Dispersal System Minimum Soil Requirements**

~~4.2.6.1~~ **4.2.5.1** At least one boring **in the primary and one boring in the designated reserve area** to a minimum of **five (5) feet** below infiltrative **surface** depth or to refusal to determine adequate soil interval. **Additional borings may be required to document uniformity of soil conditions in the dispersal and reserve areas.**

4.3 Percolation Testing Requirements

This section provides a consistent method for percolation testing in San Diego County. The objective is to determine the absorption capacity of the soils in the dispersal system and reserve areas necessary to properly size the OWTS with adequate infiltration surface area based on an expected hydraulic conductivity of the soil and the rate of loading and to provide for a system intended to allow for a long-term expectation of satisfactory performance.

The length of time needed for percolation tests will vary in different types of soil. Percolation rates must be determined on the basis of the test data obtained after the soil has had opportunity to become saturated and to swell for at least 24-hours. Saturation means that the void spaces between the soil particles are full of water. This can be accomplished in a relatively short period of time. Swelling is caused by intrusion of water into the individual soil particles. This is a slow process, especially in clay-type soil, and is the reason for requiring a prolonged soaking period. After the 24-hour presoak, the percolation testing proceeds based on the conditions resulting from the presoak, in accordance with the procedures in this LAMP.

Percolation testing performed under permit and inspection from DEHQ is required for all site evaluations. However, previous percolation test data using test methods approved since 1978 may be valid and may be used at a later time to design and size an OWTS for a project. The percolation test record (percolation test form or other record with parcel or subdivision maps) shall be included in the OWTS Layout Report.

Sufficient tests must be made in separate holes to assure that the results are valid and represent the extent of the proposed dispersal areas.

4.3.1 All percolation testing for dispersal systems shall be conducted under permit and inspection from DEHQ ~~and through the use of~~ using the procedures in Appendix I-Percolation Testing Procedures. Qualified professionals shall provide DEHQ with ~~24~~48-hour notice prior to conducting a percolation test.

4.3.2 The test shall be performed by ~~or under the direct supervision of~~ a qualified professional ~~or subordinate~~ who ~~is registered~~ ~~has completed a consultation~~ with the DEHQ and ~~has~~ demonstrated knowledge of San Diego County ordinances relating to OWTS and the requirements of this LAMP.

4.3.3 Areas that are within the minimum setback distances established to protect public health and/or water quality shall not be used for waste disposal or percolation testing. The following areas are considered unsuitable for the location of the dispersal system or designated reserve area:

4.3.3.1 Areas within any easement which is dedicated for surface or subsurface improvement.

4.3.3.2 ~~Paved or unpaved driveways, P~~paved areas or areas proposed for paving or areas occupied by or proposed to be occupied by structures or other surface features.

4.3.3.3 Areas not owned or under the control of the property owner unless the area is dedicated as a recorded easement for waste disposal purposes.

4.3.3.4 Low-lying areas subject to flooding or with a potential for flooding.

~~4.3.3.5~~4.3.4 Grading or clearing of brush for the purposes of completing a percolation test may require approval from other agencies, including County of San Diego Planning and Development Services and Department of Public Works, and may require the implementation of stormwater best management practices.

~~4.3.3.6~~ **4.3.5** Percolation rates in excess of 120 minutes per inch demonstrate impermeable soil that is not considered suitable for an OWTS.

~~4.3.4~~ **4.3.6** Number of Percolation Test Holes

~~4.3.4.1~~ **4.3.6.1** A minimum of six (6) percolation test holes located within the primary and designated reserve areas is required at sites where the average percolation rates is 60 minutes per inch (MPI) or less.

~~4.3.4.2~~ **4.3.6.2** An additional two (2) (for a minimum of eight percolation test holes) is required when the average percolation rate is more than 60 MPI.

4.3.6.3 At least four (4) percolation test holes at each deep bed location should be provided to represent soil types within the infiltrative surface area of the deep bed. This testing should represent the entire sidewall depth of the pit.

4.3.6.4 Vertical seepage pits are required to perform capacity testing in accordance with the requirements of Appendix II- Vertical Seepage Pit Dispersal Systems Percolation Test Procedures.

~~4.3.4.3~~ **4.3.6.5** Percolation test holes shall be representative of the site conditions throughout the entire dispersal systems with equal consideration of the primary and designated reserve dispersal areas.

~~4.3.4.4~~ **4.3.6.6** Additional test holes may be necessary on a site-specific basis for reasons that include, but are not limited to the following:

~~4.3.4.4.1~~ **4.3.6.6.1** Unacceptable percolation test results, including percolation tests not performed in accordance with the procedures set forth in Appendix I-Percolation Test Procedures of this LAMP.

~~4.3.4.4.2~~ **4.3.6.6.2** Failed percolation test results or rates that do not meet the minimum rates for the dispersal system proposed for use.

~~4.3.4.4.3~~ **4.3.6.6.3** Areas of the dispersal field require additional testing to define limits for exclusion of unsuitable areas.

~~4.3.4.4.4~~ **4.3.6.6.4** The dispersal system is located out of a concentrated area or is of larger areal extent than typical single residential unit installation.

~~4.3.4.4.5~~ **4.3.6.6.5** Soil conditions are variable or inconsistent.

4.3.5 **4.3.7** Depth of Percolation Test Holes

~~4.3.5.1~~ **4.3.7.1** Test holes shall be representative of the dispersal system installation depth.

~~4.3.5.2~~ **4.3.7.2** Conditions which may require testing deeper than the dispersal system depth:

~~4.3.5.2.1~~ **4.3.7.2.1** Areas of potential impervious soil layers to a minimum of five feet below bottom of infiltrative surface ~~or~~ if dispersal field.

~~4.3.5.2.2~~ **4.3.7.2.2** Slopes exceeding ~~25~~ **30**%.

~~4.3.5.2.3~~ **4.3.7.2.3** Other factors as might be determined by DEHQ ~~or accepted engineering practices.~~

~~4.3.6~~ **4.3.8** Soil Classification

~~4.3.6.1~~ **4.3.8.1** All test holes and deep borings shall have the soil class that describes the relative amount of sand, clay, silt and combinations thereof as defined by the soil textural triangle developed by the United States Department of Agriculture (USDA) as the Soil Survey Manual, Handbook 18}.

~~4.3.6.2~~ All percolation testing results are to be reported, including any results not meeting minimum rates suitable for OWTs usage and test holes which encountered groundwater or refusal.

~~4.3.7~~ **4.3.9** Location of and Identification of Percolation Test Holes

~~4.3.7.1~~ **4.3.9.1** Test holes shall be staked and flagged so the test holes can be easily located.

~~4.3.7.2~~ **4.3.9.2** The test holes must be identified with all of the following:

~~4.3.7.2.1~~ **4.3.9.2.1** A test hole number or letter

~~4.3.7.2.2~~ **4.3.9.2.2** The depth of the test boring

~~4.3.7.2.3~~ **4.3.9.2.3** Lot/parcel number or letter if associated with a subdivision or lot line adjustment.

~~4.3.8~~ **4.3.10** Drilling of Borings for Percolation Test Holes

~~4.3.8.1~~ **4.3.10.1** Diameter of each test hole shall be a minimum of 6 inches and a maximum of 12 inches.

~~4.3.8.2~~ **4.3.10.2** If a backhoe excavation is used, a test hole at 12–14 inches in depth shall be excavated into the bottom of the trench.

~~4.3.9~~ **4.3.11** Preparation of Percolation Test Holes

~~4.3.9.1~~ **4.3.11.1** The sides and bottom of the holes shall be scarified so as to remove the areas that

became smeared by the auger or other tool used to develop the hole.

~~4.3.9~~**4.3.11.2** All loose material shall be removed from the hole.

~~4.3.10~~**4.3.12** Percolation Test Procedures

Percolation testing shall be performed in accordance with the procedures provided in Appendix I - Percolation Test Procedures of this LAMP.

~~4.3.11~~**4.3.13** ~~Leach Line Dispersal Systems~~**Requirements**

~~4.3.11.1~~**4.3.13.1** Percolation testing shall be performed in accordance with the requirements of this Chapter and the procedures in Appendix I-Percolation Test Procedures of this LAMP. Percolation tests, along with deep borings, and backhoe excavations are used to demonstrate that the primary dispersal area and reserve locations are located in areas of uniform soil, and that no site conditions exist which could adversely affect the performance of the system or result in surfacing sewage or groundwater ~~degradation~~**contamination**.

~~4.3.11~~**4.3.13.2** Leach line **dispersal** systems are limited to soils with percolation rates of 120 minutes per inch or less. ~~Percolation rates in excess of 120 minutes per inch are unsuitable for the installation of an OWTS dispersal system.~~

~~4.3.12~~ ~~Deep Bed Dispersal Systems~~

~~4.3.12.1~~ ~~Percolation testing shall be performed in accordance with the requirements of this Chapter and the procedures in Appendix I-Percolation Test Procedures of this LAMP. Percolation tests, along with deep borings, and backhoe excavations are used to demonstrate that the primary dispersal and designated reserve locations are located in areas of uniform soil, and that no site conditions exist which could adversely affect the performance of the system or result in surfacing sewage or groundwater degradation.~~

~~4.3.12.2~~**4.3.13.3** **Deep bed dispersal systems are limited to soils with** ~~The~~ percolation rates ~~shall not exceed~~ **of 30 minutes per inch or less** in any portion of the deep bed dispersal areas. Individual rates exceeding 30 minutes per inch may be considered with additional soil testing.

~~4.3.12.3~~**4.3.13.4** Depth of percolation test **for deep bed dispersal systems** must be representative of the bottom infiltrative depth of the pit- **and all soil types of the sidewall infiltrative surface area of the deep bed. The testing should represent the entire sidewall depth of the pit.**

~~4.3.12.4~~ ~~Any percolation tests for deep bed dispersal systems, which were approved based on testing prior to November 1978, will require additional percolation testing unless the previous testing meets current requirements.~~

~~4.3.13~~ ~~Vertical Seepage Pit Dispersal Systems~~

~~4.3.13.1 Percolation testing, shall be performed in accordance with the requirements of this Chapter and the procedures in Appendix I Percolation Test Procedures of this LAMP. Percolation tests, along with deep borings, and backhoe excavations are used to demonstrate that the primary dispersal and designated reserve locations are located in areas of uniform soil, and that no site conditions exist which could adversely affect the performance of the system or result in surfacing sewage or groundwater degradation.~~

~~4.3.14 Drip Dispersal Systems~~

~~4.3.14.1 Percolation testing shall be performed in accordance with the requirements of this Chapter and the procedures in Appendix I Percolation Test Procedures of this LAMP. Percolation tests, along with deep borings, and backhoe excavations are used to demonstrate that the primary dispersal and designated reserve locations are located in areas of uniform soil, and that no site conditions exist which could adversely affect the performance of the system or result in surfacing sewage or groundwater degradation.~~

~~4.3.15~~**4.3.13.5 Where D**dispersal systems **are proposed in S**steep **S**slopes **of—over 25**~~30~~**% to a maximum of 40%, percolation testing must provide data representative of the entire primary and designated reserve dispersal areas to demonstrate that conditions are uniform below the entire disposal areas. The minimum testing required is the same as above for regular dispersal. Additional testing may be required to determine extent of uniformity of soils, if needed.**

~~4.3.15.1 Testing must provide data representative of the entire primary and designated reserve dispersal areas to demonstrate that conditions are uniform below the entire disposal areas. The minimum testing required is the same as above for regular dispersal.~~

~~4.3.16~~**4.3.14** Percolation Test Results Reporting

~~4.3.16.1~~**4.3.14.1** Percolation testing is only one critical factor in the site evaluation process. All test data and required information shall be submitted as part of a comprehensive OWTS Layout Report on approved DEHQ forms with appended data or information as needed.

~~4.3.16.2~~**4.3.14.2** All percolation testing results are required to be reported, including any results not meeting minimum rates suitable for OWTS usage and test holes which encountered groundwater or refusal.

4.3.14.3 Reports shall be signed and stamped, if appropriate, with an original signature by the qualified professional who either performed or supervised the testing.

~~4.4~~ **Nitrate Loading Study**

~~A nitrate loading study using an acceptable mass balance method, must demonstrate that the proposed project will not cause the concentrations of wastewater constituents in ground water to exceed~~

~~applicable ground water quality objectives for nitrate. The study must look at the existing impacts to groundwater and surface water along with the potential impacts from the proposed project to address the cumulative impacts to water resources.~~

~~4.4.1 General Requirements~~

~~4.4.1.1 A Nitrate Loading Study may be required, as determined by DEHQ, where conditions exist such that a proposed OWTS usage or replacement/repair OWTS may have the potential to impact groundwater to levels above the water quality objectives provided in the Water Quality Control Plan (Basin Plan) for that area.~~

~~4.4.1.2 A Nitrate Loading Study shall be required for all subdivisions of land proposed for OWTS usage that do not meet the minimum density requirements in Table 3.7-1.~~

~~4.4.1.3 A Nitrate Loading Study shall be required for building permits for an additional dwelling unit on a lot that will not meet the density requirements in Table 3.7-1.~~

~~4.4.2 The nitrate loading study shall provide the minimum information provided in Appendix II.~~

4.5.4.4 SoilSlope Stability Initial Screening and Study

4.4.1 A slope stability study is required for nNatural slopes and cut slopes **of from 2530% to 40%** in areas proposed for OWTS usage, **where the initial site evaluation has identified evidence of slope instability, such as unconsolidated fill, significant erosion rills, tension cracks, evidence of prior earth movement or slides, and leaning trees.** ~~require additional study as to the stability and suitability for OWTS usage.~~ Cut slopes are excavated along natural hillsides, through ridges and mesas, and into existing embankment. The stability of slopes can be assessed through appropriate geotechnical investigation, analysis, and design thereby preventing landslides, slip outs, slumps, severe erosion, and safety issues.

~~4.5.14.4.2 A SoilSlope Stability study shall be completed by a qualified professional shall be required for OWTS usage proposed on slopes with grades of 25% or greater.~~

~~4.5.24.4.3 A Soil Stability study shall be required for OWTS usage proposed where site conditions exist that may indicate unstable landforms including, but not limited to, unconsolidated fill, significant erosion rills, tension cracks, evidence of prior earth movement or slides, and leaning trees. Areas with evidence of unstable land forms~~**masses** ~~are not appropriate for OWTS usage.~~

~~The study shall also address any potential risk to the slope stability from the use of an OWTS at that location.~~

~~4.5.34.4.4~~ The **SoilsSlope** Stability study must assess the risk of slope failure, creep, or other movement which could damage the OWTS or endanger life or property. **The study shall also address any potential risk to the slope stability from the use of an OWTS at that location.**

The Report shall include data, calculations, and assumptions used in formulating the opinion and any recommendations. The OWTS design shall incorporate the recommendations of the assessment report.

~~4.5.4~~~~4.4.5~~ 4.4.5 The qualified professional shall propose additional site investigation, borings, and/or testing, as needed, to provide sufficient data to complete the slope stability evaluation.

~~4.5.5~~~~4.4.6~~ 4.4.6 Minimum report information. The Slope Stability Report must contain the following minimum information:

~~4.5.5.1~~~~4.4.6.1~~ 4.4.6.1 Description of project and site including proposed location of OWTS dispersal system.

~~4.5.5.2~~~~4.4.6.2~~ 4.4.6.2 Description of any proposed or completed grading for the project at the site.

~~4.5.5.3~~~~4.4.6.3~~ 4.4.6.3 Description of the evaluation and investigation completed at the site.

~~4.5.5.4~~~~4.4.6.4~~ 4.4.6.4 Site specific topography map scaled to two (2) foot contours, cross section(s) of hillside soil profiles.

~~4.5.5.5~~~~4.4.6.5~~ 4.4.6.5 Results of site evaluation, including observed slope conditions and geotechnical investigation results.

~~4.5.5.6~~~~4.4.6.6~~ 4.4.6.6 Specific recommendations for grading actions, if warranted.

~~4.5.5.7~~~~4.4.6.7~~ 4.4.6.7 OWTS dispersal design recommendations to include scaled dispersal system layouts and profiled designs based on accurate topography.

CHAPTER 5.0 SITE EVALUATION AND OWTS LAYOUT REPORT REQUIREMENTS

5.1 General

5.1.1 All test data and required information shall be submitted on approved DEHQ forms with appended data or information.

5.1.2 Reports shall be signed with an original signature and stamp, if applicable, by each qualified professional who either performed or supervised the testing or otherwise contributed to the report.

5.1.3 San Diego County Code, Section 68.328 requires all percolation testing to be done by a civil engineer, geologist, or environmental health specialist, registered in the State of California. These qualified professionals are required to be registered with DEHQ.

5.1.4 The percolation test is only one critical factor in siting an OWTS. Site considerations may require special evaluation by a qualified professional to technically address issues such as high groundwater, steep slope, nitrate impacts, cumulative impacts, mounding, and horizontal transmissibility.

5.1.5 Qualified professionals who employ technicians are responsible for the work performed by the technician. It is incumbent upon the qualified professional to properly train, equip, and supervise anyone performing work under his or her direction and license. **It is expected that technicians or subordinate personnel will receive adequate direct supervision while performing field work to ensure compliance with the minimum requirements of this LAMP.**

5.2 OWTS Layout Report Information

An OWTS Layout Report of the proposed project, building construction, and OWTS is required. All maps and schematics should be prepared using standard engineer's scale **on no larger than 11" x 17" size paper. The appropriately sized paper should be based on the scope and scale of the project.**

5.2.1 The OWTS Layout Report shall contain the minimum information listed in Table 5.2-1 or as required by the department.

Table 5.2-1: Minimum Information Requirements for OWTS Layout Reports
Site address
Assessor's Parcel Number
Owner's Name, mailing address, and phone number
Qualified Professional's name, mailing address, and phone number
Type and scope of project (e.g., new dwelling, new structure, guesthouse, an addition, use permit, subdivision of land)
Number of existing bedrooms, number of proposed bedrooms
Sources of wastewater to proposed OWTS

Volume, character, and strength of wastewater with supporting calculations, data and sources of information
Commercial Food Service-location, design, and sizing of oil/grease interceptor
Number of existing or proposed bedrooms (as determined by Section 6.5.1)
Legal Basis of parcel (e.g., map and lot number, plat number
Vicinity Map, Scale (Engineer scale not to exceed 1" = 60'), North arrow, Map does not exceed 11" x 17" paper
Property Lines and lot dimensions (provide an over sheet (larger scale allowed) and detail sheet(s) for large parcels
Topographical lines and elevation points (include pad grade, finished floor, septic tank, leach lines, slope arrows, percent slope and direction of fall, slope range, etc.)
Locations of existing and proposed primary and designated reserve dispersal areas
Septic tank and dispersal areas sizing calculations and design details
All setback distances shown on layout
All existing and proposed grading, rock outcroppings, slopes in excess of 20%
All known, recorded easements on or within 20 feet of lot boundaries (open-space, utility, road, waterline, etc.)
Identify source of potable water, location of all public waterlines on or within 20 feet of property and signed water line statement
Location of all public waterlines on or within 20 feet of property and signed water line statement;
Location of all wells on or within 150 feet of property, location of all public wells within 600 feet of property line
Location of drinking water reservoirs within 2,500 feet of property line ¹
Location of drainage ways within 50 feet of property line ¹
Location of streams, springs, ponds, flood plains, lakes within 200 feet of property line ¹
Location of all stormwater treatment and retention facilities
Location and discussion of any impaired water bodies, Basin Plan special provisions or exception areas, or other special conditions in proximity to site
Methods and results of all soils testing, including soil profile test holes and percolation tests, plotted on the design (matches flagged locations in the field)
Name and description of groundwater basin underlying or near property, if applicable.² Methods and results of depth to groundwater analysis and testing, if required
Methods and results of nitrate loading analysis, if required
Methods and results of soil slope stability study, if required
Methods and results of other required studies/information
Sign off of proposed layout by local water district or company, if required

1-To include surface water bodies information based on the United States Geological Service's *The National Map* (<https://apps.nationalmap.gov/viewer/>), as well as site specific observations of local drainage patterns.

2-To include groundwater basin information based on California Natural Resources Agency's Groundwater Basin Boundary Descriptions-San Diego Region 9-XXX Basins (<https://data.cnra.ca.gov/dataset/bbd9>), as well as site specific groundwater levels.

5.2.2 The additional information is required for OWTS with Supplemental Treatment

Table 5.2-2: Minimum Information Requirements for OWTS Layout Reports with Supplemental Treatment
All items listed in Section 5.2.1
A list of all supplemental treatment components
A map showing all supplemental treatment components on the parcel
The OWTS design specifications and configuration for the primary dispersal area
The OWTS design specifications and configuration for the designated reserve area
Sizing calculations from the qualified professional
GeoFlow Design specification worksheet for drip dispersal line (if used) (i.e., GeoFlow worksheet)
Pump(s) specifications and pump curve(s), friction and head loss calculations
Control or alarm box with telemetric reporting specifications
Documentation of the 24-hour emergency storage above the alarm on float(s)

5.3 — ~~Minimum Nitrate Loading Study Information~~

~~Applicant shall provide the minimum information required pursuant to Section 4.4 and Appendix II for sites where a nitrate loading study is required.~~

5.45.3** Minimum Steep Slope/Slope Stability Study Information**

Applicant shall provide the minimum information required pursuant to Section **4.4 4.5** for sites where a slope stability study is required.

CHAPTER 6.0 SITING AND DESIGN CRITERIA

6.1 General

The most common type of OWTS found in San Diego County consists of a septic tank connected to a dispersal field consisting of rock or chamber filled leach lines. Variations of this system may include a septic tank connected to either a deep bed or vertical seepage pit dispersal system.

In some areas, the dispersal field may be located at a higher elevation than the building site. In this instance, a pump-system is used to deliver the wastewater from the location of the septic tank or building site to the dispersal field location where it is then distributed by gravity flow. These OWTS are considered standard OWTS because no further sewage treatment is performed between the septic tank and the dispersal field. In all cases, the septic tank provides the initial treatment of the sewage with the resultant effluent wastewater discharged below the ground surface, where it undergoes further treatment processes, including digestion by bacteria, in the unsaturated soil zones underlying the dispersal field. When sited and designed properly, these OWTS are designed to operate in all normal weather conditions with minimal maintenance, including cleaning of effluent filters and periodic septic tank pumping to remove accumulated scum and sludge from the septic tank. Areas subject to flooding or low-lying areas that have a potential of flooding or are not suitable for OWTS usage.

In addition to standard OWTS, OWTS with supplemental treatment that meet the minimum requirements of this LAMP are allowed for use where additional treatment of wastewater is needed. OWTS with supplemental treatment must be certified by the National Sanitation Foundation and are designed to reduce nitrogen or pathogen concentrations in wastewater prior to discharge to the dispersal field. This additional treatment may be required to mitigate for shallow groundwater or inadequate soil interval depth, or to meet any other requirement to prevent or minimize impacts to groundwater or surface waters. OWTS with supplemental treatment typically use drip dispersal as the method for effluent disposal. Due to the complexity of these systems, ongoing maintenance contracts and annual operating permits are also required.

This chapter provides general siting and design criteria. Specific septic tank and subsurface dispersal field requirements are provided in Chapters 7.0 and 8.0.

6.1.1 The siting, design, operation and maintenance of all new OWTS must meet the provisions of the site evaluation and other required studies for that specific parcel and these standards.

6.1.2 No person shall connect any dwelling, structure or other source of sewage or wastewater to an existing OWTS without first obtaining approval from DEHQ.

6.2 Primary and Designated Reserve Dispersal Area Requirements

6.2.1 In addition to primary system design criteria, all OWTS design proposals, for both new construction and additions to an existing structure, must show a designated reserve area sufficient to accommodate 100% of the design of the active **primary** OWTS. Parcels previously approved with a

reserve area smaller than the current standards must meet current design standards.

6.2.2 The designated reserve area shall not be built on or used for any other purposes that conflicts with or prevents the use of the area for OWTS.

6.2.3 To ensure the designated reserve area is sufficiently sized for potential repairs to the primary OWTS, and because costs to install OWTS with supplemental treatment may be prohibitive to some property owners, the design of the dispersal system that determines the extent of the 100% designated reserve area shall be consistent with the dispersal system design of the primary OWTS. A proposed 100% designated reserve area based on a dispersal system for OWTS with Supplemental Treatment shall only be allowed if the primary system is an OWTS with supplemental treatment.

6.3 Groundwater Separation Requirements

6.3.1 The minimum depths from the highest known groundwater to the bottom of the dispersal system are noted in Table 6.3-1. The minimum separation of the bottom of dispersal system to groundwater for vertical seepage pits is 10 feet.

6.3.2 The groundwater separation depth (minimum soil depth) for sites where an OWTS with Supplemental Treatment for nitrogen reduction are used may be reduced to a minimum of two (2) feet, where appropriate.

6.3.3 The groundwater separation depth (minimum soil depth) for sites where an OWTS with Supplemental Treatment for pathogen reduction are used may be reduced to a minimum of three (3) feet, where appropriate.

6.3.4 A reduction of the minimum separation may be approved where percolation rates are less than five (5) minutes per inch if the site does not overlie groundwater protected for drinking water supplies or is located more than 2,500 feet from an impaired water body or drinking water reservoir or tributary but shall be no less than eight (8) feet for a leach line and ten (10) feet for a seepage pit.

Table 6.3-1: Minimum Separation Depths to Groundwater and Minimum Soil Depth from the Bottom of the Dispersal System	
Percolation Rate	Minimum Separation and Depth
Percolation Rate ≤ 1 MPI	Twenty (20) feet
1 MPI < Percolation Rate ≤ 5 MPI	Twenty (20) feet
5 MPI < Percolation Rate ≤ 30 MPI	Eight (8) feet
30 MPI < Percolation Rate ≤ 120 MPI	Five (5) feet
Percolation Rate > 120 MPI	Not suitable for OWTS Usage
MPI = minutes per inch Notes: 1) Minimum depth groundwater separation for seepage pits is ten (10) feet. 2) Minimum depth groundwater separation for OWTS with Supplemental Treatment designed for nitrogen reduction is two (2) feet. 3) Minimum depth	

groundwater separation for OWTS with Supplemental Treatment designed to reduce pathogens is three (3) feet. 4) Percolation rates for leach lines shall be 120 MPI or faster. 5) Percolation rates for deep bed dispersal systems shall be 30 MPI or faster.

6.3.5 Testing procedures to determine the groundwater separation are found in Chapter 4.0, Section 4.1.

6.4 Wastewater Characterization

Accurate characterization of the pollutant load in wastewater is necessary to determine coverage under this LAMP. The scope of coverage and prescribed standards in this LAMP are limited to OWTS treating low strength domestic wastewater. Low strength wastewater is wastewater having a 30-day average concentration of biochemical oxygen demand (BOD) of 300 mg/L or less, total suspended solids (TSS) of 330 mg/L or less, or fats, oil, and grease (FOG) concentration of 100 mg/L or less prior to the septic tank or other OWTS treatment unit.

Wastewater with pollutant loads above the concentrations listed above are considered high strength and are not permitting through the local permitting program, with the exception of high strength wastewater from a commercial food service building up to 900 mg/L BOD and has a properly sized and functioning oil/grease interceptor. Proposed projects generating high strength wastewater must make application with the appropriate Regional Board.

The main constituents of concern to groundwater are nitrogen and pathogens. Protection of groundwater quality is achieved by implementing the minimum vertical separation distance between the dispersal system and the highest expected rise of the water table. This provides an unsaturated zone wherein high degrees of physical, biological, and chemical treatment occur. Surface waters are similarly protected by the implementation of lateral setback requirements. The buildup of nitrogen in groundwater is potentially the most significant long-term consequence of OWTS usage.

6.4.1 Residential Wastewater Characterization

OWTS serving single family residential uses can be assumed to have low strength wastewater characteristics, as discussed above. ~~Mass loadings and e~~Concentrations of constituents in typical domestic wastewater are available in various publication, including the EPA Design Manual, 1980 and 2002 versions. Total nitrogen concentrations in effluent have been reported to vary from 25 mg/L to as much as 100 mg/L, with the average generally being in the range of 35 to 45 mg/L (US EPA 1980).

The OWTS Layout Report shall include a description of any activity within a residential structure that may affect the characteristics of the wastewater generated. These activities shall include home-based food preparation for non-residents such as cottage food operations or micro enterprise home kitchens.

6.4.2 Nonresidential Wastewater Characterization

Nonresidential establishments can have significant variation in wastewater characteristics. The

wastewater stream from existing facilities should be sampled to determine the pollutant loads. New proposed projects can be estimated based on available data, including charts in the EPA Design Manuals relating to per fixture mass loading or contributions. Characterization data from existing facilities with similar use and occupancies can also provide this information.

6.5 Wastewater Flow Determinations

The required hydraulic capacity for an OWTS is determined initially from the estimated wastewater flow. Accurate characterization of raw wastewater, including daily volumes and peak flows are critical for effective system design.

6.5.1 Residential Wastewater Flows

For the purposes of designing a dispersal system, residential dwelling projected flows are based on the number of bedrooms in the dwelling unit, as determined below, and a minimum volume of 75 gallons per day per person water usage and a minimum occupancy of two persons per bedroom. The number of bedrooms is calculated in accordance with the following guidelines.

6.5.1.1 The living room, dining room, family room, kitchen, bathrooms, and utility rooms are not considered bedrooms. All other rooms shall be considered as potential sleeping rooms. All other habitable rooms totaling at least seventy (70) square feet in size are to be considered bedrooms suitable for sleeping purposes, regardless of whether or not they contain closets or have access to a bathroom. Dens, libraries, studies, weight rooms, sewing rooms, workshops, etc., shall be considered bedrooms, unless they conform to the criteria listed below.

6.5.1.1.1 Rooms that open to a living room, dining room, family room, kitchen, or entry way, and have a single, un-obstructive opening (no doors) with a minimum 50% opening of the total wall space (minimum 6' wide) with archways or other acceptable means shall not be considered as bedrooms, due to the lack of personal privacy presented by the opening.

6.5.1.1.2 Rooms that can only be accessed through another bedroom are to be considered part of that bedroom, such as master suite, and not an additional bedroom.

6.5.1.2 Plans for proposals where it is not clear if a specific room is to be considered a bedroom may be rereviewed on a case-by-case basis by DEHQ.

6.5.1.3~~6~~ Projects proposing the relocation or modification of doorways are to be reviewed and approved by the building department to address any structural considerations such as load bearing walls prior to approval or sign-off by DEHQ. Projects proposing

6.5.2 Commercial Wastewater Flow

6.5.2.1 If there is no actual data available to determine the wastewater flow from the commercial facility, data from the following water use computation table may be used. Other data sources may be used as

approved by the DEHQ (i.e., EPA Design Manual, CA Plumbing Code App. H).

Table 6.5-1: Commercial Minimum Wastewater Flow Guidelines	
Type of Establishment	Gallons Per Person Per Day (Unless otherwise indicated)
Rooming Houses	50 gal
Boarding Houses	60 gal
Motels/Hotels	50 gal
Restaurant and cocktails lounges	100 gal/seat or 35 per
Bars or cocktail lounges	20 gal
Campgrounds with bathhouse	35 gal
Recreational Vehicle Camps	100 gal/per space
Tourist Camps with individual bath units	75 gal
Retail Markets with public toilets	150 gal/per fixture
Retail Markets without public toilets	0.1 gal/sq ft
Day Camps (no meals served)	15 gal
Day Schools and Day Care facilities with no cafeteria or showers	15 gal
Boarding Schools	25 gal
Day Workers at schools/offices (per shift)	100 gal
Institutions other than hospitals (involuntary)	175 gal
Industrial Buildings (gallons/person/shift, exclusive of industrial waste) with food cafeteria	25 gal
Industrial Building no food cafeteria	15 gal
Picnic Parks (toilet waste only gallon/picnicker)	5 gal
Swimming Pools and Bathhouses	10 gal
Country Clubs (per resident member)	100 gal
Drive-In Theaters (per car space) with snack bar	10 gal
Movie Theaters (per seat) with snack bar	10 gal
Airports (per passenger)	5 gal
Self-service Laundries	1000 gal/machine
Stores (per toilet fixture per employees/public use	150 gal /fixture
Service Stations (per vehicle served)	10 gal
Public Gatherings (auctions, ball games, fairs, etc.)	10 gal
Food preparation wholesale	250 gal/employee/shift
Churches-no kitchen	5 gal/seat
Churches-with kitchen	10 gal/seat
Kennels	10 gal/dog
Note: Structure occupancies not classified above shall base their sewage flows on one-year actual water use of a similar occupancy supplied by the applicant. Other flows may be proposed based on approved criteria, such as Manual of Septic Tank Practice or the EPA Design Manual-Onsite Wastewater and Disposal Systems or other acceptable publication.	

6.6 Minimum Setbacks

Setbacks in layout designs refer to the required spacing in distance from components of the OWTS and to structures, property lines, easements, watercourses, wells, or grading. Specific setback requirements are specified in the Table 6.6-1.

Table 6.6-1: OWTS Minimum Setback Requirements			
Setback Descriptions	Septic Tank	Leach lines and deep beds (depth < 10')	Vertical Pits and deep beds (depth > 10')
Water Well – Private ¹	100'	100'	150'
Water Well – Public ¹	150'	150'	200'
Property Line	5'	5'	10'
Structures, driveways, swimming pools, trees	5'	8'	10'
Water Mains (public)	--	25'	25'
Private Utility Trenches ⁵	--	10'	10'
Road Easements ^{3,2}	--	10' from edge of ultimate easement width	10' from edge of ultimate easement width
Septic Tank	--	5'	10'
Drip Lines	5'	6'	6'
Leach Lines ³	5'	10' center to center	15 10' from edge of excavation
Seepage Pits and deep beds	10'	15' from edge of excavation	20' from edge of excavation
Cut Slope ^{4,5}	10'	5' horizontal distance for every 1' in rise setback from top of cut slope to maximum of 100'	5' horizontal distance for every 1' in rise setback from top of cut slope
Unstable Land Mass ⁶	--	100'	100'
Graywater Dispersal Areas	5'	5'	5'
Ephemeral Stream or Drainage Course ^{5,7}	--	100 50' from centerline or top of bank	100 50' from centerline or top of bank
Stormwater retention features, man-made lined ponds and ditches ⁸	25'	25'	25'
Springs, Flowing Surface Water Bodies, Streams, Creeks, Rivers, Intermittent Water Bodies	--	100' from edge of flow line or top of bank	100' from edge of flow line or top of bank
Pond, Lake, Reservoir, Vernal Pools, Wetlands, Other Surface Water Bodies	--	200' from spillway elevation or from where the edge of that water body is the high-water mark, whichever is greater	200' from spillway elevation or from where the edge of that water body is the high-water mark, whichever is greater

Aqueduct ²⁹	--	100' to pipeline	100' to pipeline
Lake, Reservoir, Flowing Water Body for OWTS located >1,200 feet of Surface Water Intake of Public Water System	--	400'	400'
Lake, Reservoir, Flowing Water Body for OWTS located 1,200'-2,500' of Surface Water Intake of Public Water System	--	200'	200'

~~1-The minimum setback required to a public well is 150 feet and increases to 200 feet where the depth of the dispersal system exceeds 10 feet in depth. The minimum setback may be increased if site conditions show the minimum setback is insufficient to protect groundwater supplies. Setback includes hand dug wells.~~
~~2-A reduction to 50' may be considered with engineering to demonstrate no risk of sewage moving laterally to pipeline trench.~~
~~3-The setback may increase if the 5:1 (5' horizontal distance for every 1' vertical distance to a road cut is greater than the minimum setback.~~
~~4-The maximum 100' setback would also be applied to the top of an eroded bank or natural slope in excess of 60%. A reduction in setback to 50' may be considered with engineering to demonstrate no risk of sewage surfacing on the face of the bank or slope.~~
~~5-Setback increases to a 5:1 (5' horizontal distance for every 1' vertical distances) setback if drainage is greater than 10' in depth.~~
1-The minimum setback may be increased if site conditions show the minimum setback is insufficient to protect groundwater supplies. Setback includes hand dug wells. All wells must meet well standards as required pursuant to San Diego Regulatory Code Title 6, Division 7, Chapter 4.
2-The setback may increase if a cut slope is present and the 5:1 (5' horizontal distance for every 1' vertical distance) setback to the road cut is greater than the minimum road easement setback.
3-The distance between leach lines can be reduced to 6' from sidewall to sidewall for sites where the design percolation rate is 30 MPI or less, the effluent is distributed by equal distribution (distribution box), and the slope is 10% or less.
4-For drip dispersal: a 3:1 setback (3' horizontal distance for every 1' in vertical rise) from top of cut slope to maximum of 25' is permitted.
5-The maximum 100' setback would also be applied to the top of an eroded bank or natural slope more than 60%. A reduction in setback to 50' may be considered when the site evaluation demonstrates adequate site characteristics to prevent sewage surfacing on the face of the bank or slope.
6-Other setback distance allowed, if recommended by a geotechnical report prepared by a qualified professional (see slope stability requirements).
7-Setback may be reduced to 50' where a hydrological report prepared by a qualified professional demonstrates that site conditions prevent migration of wastewater to the water body and where the OWTS is not located within a geographical area of a TMDL or APMP or within an area of concern as identified by the Regional Board.
8-This setback may be reduced to 10' for man-made features that are fully lined, contained in a watertight tank or pipe, or where the site evaluation shows no potential for comingling with wastewater.
9-A reduction to 50' may be considered when the site evaluation demonstrates adequate site characteristics to prevent sewage moving laterally to pipeline trench.

6.7 Minimum Repairs

This section provides the minimum requirements for the permitting of the repair of defective OWTS

components and/or defective dispersal field. All repairs or replacements shall be performed under a permit issued by DEHQ.

6.7.1 All metal, fiberglass, or cement septic tanks or treatment units that are in a state of disrepair, are showing signs of deterioration, or are no longer watertight must be replaced with a septic tank that meets the requirements of this LAMP. All wood septic tanks shall be replaced at the time of an OWTS repair.

6.7.2 All existing brick-lined or open seepage pits shall be properly destroyed by backfilling with approved material or completely rock filled if intended for future use.

6.7.3 All repairs to OWTS or replacements shall meet existing standards of construction and design, including setbacks ~~or meet standards required by Tier~~. Where an OWTS repair or replacement cannot meet an existing standard of construction or design, repairs that are in substantial conformance, to the greatest extent practicable, with the requirements of this LAMP may be authorized under a variance in accordance with Chapter 10, Section 10.3.

6.7.4 All OWTS experiencing reduced or slower dispersal field absorption rates under normal design parameters and require pumping to keep sewage from seeping or surfacing is considered a defective system and shall be repaired.

6.7.5 All sewage or wastewater must be prevented from surfacing from an OWTS by pumping as needed by a business licensed pursuant to the California Health and Safety Code Section 117415.

6.7.6 A minimum OWTS repair shall be as previously approved by DEHQ for the parcel.

6.7.7 For OWTS serving a residential structure(s) where no reserve design has been previously approved, the equivalent of leach line length for one bedroom as provided in Table 8.2-1 shall be added to an existing OWTS dispersal system to provide opportunity for the existing infiltrative surface area to recover some functionality. Where the original design of the OWTS makes it impractical to add equivalent leach line length, the dispersal system shall be replaced.

6.7.8 For OWTS serving a commercial structure where no repair design has been previously approved, a minimum infiltrative surface of 50% of the leach line requirement for the current peak use or equivalent shall be added to the exiting OWTS dispersal system to provide opportunity for the existing infiltrative surface to recover some functionality. Where the original design of the OWTS makes it impractical to add equivalent leach line length, the dispersal system shall be replaced.

6.7.9 Where the entire dispersal field is not functional and no recovery of adsorption infiltrative surface is expected, as in extensive root intrusion, the dispersal field shall be replaced. A site evaluation may be required.

6.7.10 Any OWTS which has had more than one repair due to dispersal field failures, shall install a full replacement dispersal field. A site evaluation may be required to determine the siting and design

requirements for the OWTS to prevent future defective conditions in the replacement installation.

6.7.11 Removal of non-habitable accessory structures, root invasive trees, paved areas (sidewalks, secondary driveway, patios, etc.), or other structures/items occupying the remaining usable land area for OWTS dispersal may be required prior to the approval of the repair.

6.7.12 Dispersal system repairs or replacement must be located in an area of native soils not previously used for OWTS dispersal.

6.7.13 A site evaluation and OWTS Layout Report shall be required where changes are proposed to the use or occupancy of the residence, facility, or site, or where there will be changes to the approved wastewater characteristics or volumes, or where modifications to the OWTS is required.

CHAPTER 7.0 SEPTIC TANK REQUIREMENTS

All new and replacement septic tanks must **be installed under a permit issued from DEHQ and** meet the minimum sizing and design specifications and requirements for septic tanks provided in this chapter.

7.1 Construction

7.1.1 New and replacement septic tanks must be approved by the International Association of Plumbing and Mechanical Officials (IAPMO) or stamped by a California registered civil engineer as meeting the requirements of the 2019 California Code of Regulations (CCR), Title 24, Part 5, Appendix H. ~~All materials used in constructing a concrete septic tank shall be in accordance with CCR, Title 24, Part 5, Chapter 14, Table 14-1.~~ Metal and wooden septic tanks are prohibited. The building sewer from house cleanout to septic tank must meet local jurisdiction plumbing code requirements.

7.1.2 Septic tanks shall be designed **and sized** to produce adequate clarified effluent and space for sludge and scum accumulations.

7.1.3 Septic tanks must be constructed of solid and durable materials and not subject to excessive corrosion or decay and shall be watertight and possess two compartments. The inlet compartment shall be no less than two thirds the total capacity, and the second compartment at least one third of the total capacity. **Septic tanks, pump chambers or other related components of an OWTS including risers may be required to undergo a water tightness test at the site of the installation, if needed.**

7.1.4 Septic tanks must be anchored to counter any potential buoyant forces. Septic tanks shall be certified by the manufacturer to allow for burial without being water filled to allow for routine maintenance or to be used as a holding tank as needed.

7.1.5 Septic tanks shall be installed per the manufacturer's instructions.

7.1.6 The bottom of the excavation for the tank shall extend into native or compacted soils to eliminate potential settling issues, **unless otherwise provided for in manufacturer's installation instructions.**

7.1.7 Septic tank location must take into account maintenance and pumping requirements including vehicle access and distance and elevation lift to pumper truck.

7.1.8 All tanks must have an uncapped tee ~~or a 90-degree elbow~~ fitting on the inlet. Inlet tees must extend at least 14 inches below the liquid level.

7.1.9 Outlet tees must be uncapped and must extend at least 12 inches below the liquid level.

7.1.10 The outlet elevation shall be between **two (2)** and **six (6)** inches lower than the inlet elevation to ensure proper fall without a significant loss of volume.

7.1.11 Fall between the outlet of the septic tank and the dispersal field shall be continuous with a

minimum fall that ensures the outlet pipe is **four (4)** inches higher than the top of the first siphon in a serial system or **four (4)** inches above the top of the ~~leach rock or~~ **distribution box** or other components used in the dispersal system on a uniform or level distribution system.

7.1.12 Septic tanks shall have access to the tank by at least two access openings or manholes. For inlet compartments over 12 feet in length, an additional manhole is required over the baffle wall. Septic tank access openings with greater than **six (6)** inches of cover must have risers to within **six (6)** inches of finished grade. Risers and lids that are at or above grade must be watertight and lockable or require tools to be opened.

7.1.13 Septic tank risers must have a current IAPMO certification or must be reviewed and approved by DEHQ prior to use. **Septic tank risers must be installed in accordance with the manufacturer's instructions or where no instructions are provided as approved by DEHQ.** Concrete risers and lids must be constructed of Type V concrete or be protected from corrosion from sewer gases. The interior diameter of the riser shall be a minimum of ~~eighteen (18)~~ inches.

7.1.14 New and replacement OWTS septic tanks shall be designed to prevent solids in excess of three-sixteenths (3/16) of an inch in diameter from passing to the dispersal system. **Any one of the following actions shall be deemed in compliance with this requirement.**

7.1.14.1 The continual use and maintenance of ~~Septic tanks that use a~~ National Sanitation Foundation/American National Standard Institute (NSF/ANSI) Standard 46 certified septic tank filter at the final point of effluent discharge from the OWTS and prior to the dispersal system ~~shall be deemed in compliance with this requirement.~~

7.1.14.2 The installation and maintenance of a septic tank with an additional 500 gallons of capacity than that required pursuant to Tables 7.2-1 and 7.2-2 to increase retention and solids settling time.

7.1.14.3 The regular pumping of the septic tank. The records of septic tank pumping shall be retained by the property owner and provided to DEHQ when requested to demonstrate compliance with this section. Regular pumping of the septic tank means at least every three years or when any one of the following conditions exist:

7.1.14.3.1 The combined thickness of sludge and scum layers exceeds one-third of the tank depth of the first compartment.

7.1.14.3.2 The scum layer is within three (3) inches of the outlet tee or device.

7.1.14.3.3 The sludge layer is within eight (8) inches of the outlet tee or device.

7.1.15 Septic tanks installed **within 10 feet of or within** areas of vehicular traffic, **including driveways and parking lots**, must be certified to withstand the proposed loads **by installation of a traffic-rated septic tank** or ~~have by~~ an engineered traffic slab installed to accommodate the proposed loads.

7.1.16 Materials for drainage piping shall comply with the requirements of the County Plumbing Code. Approved piping is shown in Table 7.1-1 and must be in accordance with the referenced standards.

Table 7.1-1: Approved Pipe Material		
Pipe Material	Standard Reference: Pipe	Standard Reference: Fittings
ABS (Schedule 40)	ASTM D2661 ASTM D2680*	ASME A112.4.4 ASTM D2661 ASTM D2680*
PVC (Schedule 40)	ASTM D1785 ASTM D2665 ASTM F794*	ASME A112.4.4 ASTM D2665 ASTM F794* ASTM F1866
NOTES: Approved for underground and aboveground drain, waste, vent pipe, and fittings *For building Sewer applications Source: California Plumbing Code Table 701.2		

7.2 Minimum Septic Tank Sizing

7.2.1 Minimum septic tank size is ~~1200~~ 1000 gallons.

7.2.2 Septic tanks shall be sized according to anticipated wastewater flows from the structure(s). The following standard sizes shall apply:

Table 7.2-1: Minimum Septic Tank Volume Requirements-Primary Dwelling	
1-3 bedroom single family dwelling (0-450 GPD ¹)	1200 1000 gallons
4 bedroom single family dwelling (451-600 GPD)	1500 1200 gallons
5-6 bedroom single family dwelling (601-900 GPD)	2000 1500 gallons
Duplex – 2-4 Bedrooms	2000
Triplex – 5-6 Bedrooms	2400 2000
Fourplex – 7-8 Bedrooms	3000
For commercial applications and commercial residential units with combined total bedrooms greater than six (minimum 2000 gallon tank).	1125 gallons + (.75)(Peak Flow in GPD)

1-GPD means gallons per day

7.2.3 Second Dwelling Unit Tank Sizing: Since each dwelling unit can have a kitchen, dishwasher, garbage disposer, and laundry facilities, the septic tank sizing shall be calculated as separate flows, even if a common tank is used, as noted in the Table 7.2-2.

Table 7.2-2: Minimum Septic Tank Volume Requirements-Multiple Dwellings		
Primary Dwelling	Second Dwelling	Minimum Tank Size
1 Bedroom	1 Bedroom	1000 Gallons
2 Bedrooms	1 Bedroom	1200 Gallons

2 Bedrooms	2 Bedrooms	1500 Gallons
3 Bedrooms	1 Bedrooms	1500 Gallons
3 Bedrooms	2 Bedrooms	1500 Gallons
4 Bedrooms	1 Bedrooms	1500 Gallons
4 Bedrooms	2 Bedrooms	2000 Gallons

Dwellings with more than a combined total of six bedrooms or larger shall be calculated using the formula for commercial applications in Table 7.2-1. Separate tanks shall be used for each dwelling even if connected to a common disposal field.

7.3 Oil/Water Separator Requirements

7.3.1 OWTS receiving FOG concentrations greater than 100 mg/L prior to the septic tank or other OWTS treatment component are not covered within the scope of coverage of this LAMP and local authority. Proposals for these OWTS must be submitted to the appropriate Regional Board for approval. Waste streams from a commercial food service building with a maximum BOD 900 mg/L and a properly sized and functioning oil/greased interceptor may be permitted under the LAMP and local authority.

7.3.2 Commercial food service buildings with a maximum BOD concentration of 900 mg/L utilizing OWTS must have a properly sized and functioning oil/grease interceptor (grease trap). Those commercial food service buildings that do not have a properly sized and functioning oil/grease interceptor are not covered under the conditional waiver contained in the OWTS Policy. Properly sized oil/grease interceptor means an interceptor designed and sized in accordance with the applicable provisions of the California Plumbing Code and in compliance with any local permitting requirements.

7.3.2 All oil/grease interceptors installed with an OWTS shall be maintained in good working order and shall be pumped at a frequency that prevents fats and grease from surfacing or entering into the OWTS and dispersal field.

7.4 Sewage Effluent Pump Systems

Gravity flow of effluent from the septic tank to the dispersal field is required. However, when gravity flow is not an option, the use of a sewage effluent pump system is allowed. Sewage ejector pumps are used for moving raw sewage from a plumbing fixture to an existing septic tank for treatment while a sewage effluent pump is used for moving wastewater effluent after the primary treatment has been completed in the septic tank or treatment component.

The following requirements apply to all OWTS where a sewage effluent pump is proposed to move effluent from the septic tank to the dispersal field.

7.4.1 Only clarified septic tank effluent shall be allowed to be pumped to the dispersal field of the OWTS. The use and maintenance of a National Sanitation Foundation/American National Standard Institute (NSF/ANSI) Standard 46 certified septic tank filter at the final point of effluent discharge from the OWTS satisfies this condition.

7.4.2 The effluent pump system shall be designed by a qualified professional. The qualified professional shall provide an operating manual to the property owner. Instructions should be provided to allow periodic testing of the alarm system along with contact information for maintenance or repairs.

7.4.3 The general design shall include the following information:

7.4.3.1 Percolation or capacity data for the dispersal system ~~as needed~~.

7.4.3.2 A detailed layout drawn to scale which includes elevations.

7.4.4 Pump tank design to include the following:

7.4.4.1 A cross-section complete with elevations of control switches, measured in inches, from the bottom of the chamber.

7.4.4.2 Emergency storage volume to allow for a 24-hour holding capacity after the alarm sounds. This is the volume between the invert bottom of the inlet “tee” and the high-water alarm float in the on position.

7.4.4.3 The pump “off switch” is to be set per the manufacturer’s specifications. It is recommended that the pump remain submerged to allow for cooling and to prevent contact with sewer gases.

7.4.4.4 Float control switches are to be set to allow for a pump cycle ~~batch size of approximately 100 to 200 gallons to allow the pumps to cycle two or~~ **of three to four** times per day.

7.4.4.4 Maintenance ports (manholes) with a minimum diameter of 22 inches are to be provided and shall extend at least two inches above finished grade. **The maintenance ports shall be maintained secured and watertight at all times.** The final grade is to allow for drainage away from the maintenance ports.

7.4.4.5 Pump and surge tanks shall meet the same requirements as a septic tank. **Tanks that extend above grade shall be approved by the manufacturer for above ground use.**

7.4.5 All **effluent** pump system data and alarm system data, including make, model, and description of **effluent** pump, **control panel**, alarms, switches, and switch box are to include the following:

7.4.5.1 Pump type shall be for sewage effluent and data is to include the pump curve, U.L. approval and other test certifications.

7.4.5.2 **Mechanical and electrical equipment shall be of such durable hardware, workmanship and installation as to insure against operational failure with normal maintenance. All mechanical**

and electrical components, installations and operations shall comply with all applicable Electrical and Mechanical Codes and any local permitting requirements.

Any potential air space connections through conduit between the pump tank and the electrical/control panel shall be sealed to prevent sewer gases from corroding exposed electrical connections.

7.4.5.3 The alarm system is to contain an ~~audio~~-audible and visual alarm that will remain on until turned off by the owner or maintenance person. The alarm shall be installed on a separate circuit from the pump.

7.4.5.4 The pump system for commercial installations shall have at a minimum two ~~be based on~~ alternating ~~duplex~~ pumps.

7.4.5.5 A single family dwelling may use ~~the duplex~~ two alternating pumps or a single pump. When using a single pump, the following criteria shall be met:

7.4.5.5.1 The single pump is to be a ~~commercially~~ pre-engineered, commercially available pump suitable for continuous duty.

7.4.5.5.2 The pump must be capable of handling total flow, with minimum flow of 15 gallons per minute as measured at the discharge point of the forced main.

7.4.6 The design of the force main and venting system is to include the following:

7.4.6.1 Provide head-loss calculation addressing all fittings and elevations from pump to surge chamber.

7.4.6.2 The force main ~~is to~~ shall be two (2) inches to three ~~ou~~ inches in diameter, rated for the head-loss calculated pressure, velocity, and be of approved material.

7.4.6.3 The connection between the force main and pumps shall allow for ease of pump removal and maintenance.

7.4.6.4 A check valve is required unless the qualified professional determines that the amount of backflow will be insignificant to the system design. The qualified professional should account for the volume of the effluent within the force main, when sizing the pump chamber and pump cycle.

7.4.6.5 The pump tank and surge tank shall be cross vented to each other to avoid venting to the atmosphere.

7.4.7 The design of the surge tank that allows for the simulation of gravity flow to the dispersal field shall include the following:

7.4.7.1 The surge tank shall have a capacity equal to or greater than the volume of effluent being pumped during one batch cycle to prevent overflow with a minimum volume of 200 gallons.

7.4.7.2 The inlet pipe shall have an air gap separation between the inlet and highest effluent level (outlet) that is equal to or greater than two times the inlet pipe diameter, to prevent a siphon effect.

7.4.7.3 The ~~vertical~~ outlet drainpipe ~~in of~~ the surge tank shall be constructed of approved ~~ABS or PVC Schedule 35 or 40 PVC~~ four (4) inch drainpipe with one-half (1/2) inch holes drilled every four (4) inches along the pipe length ~~and. The outlet drainpipe shall protrude from the surge tank drain opening vertically into the surge chamber at an elevation that~~ extends beyond the typical fill level of the surge tank based on the batch cycle volume.

7.4.8 Pumps, venting, and electrical components shall be installed by licensed installers in accordance with all applicable plumbing, electrical, and mechanical codes and as per installation specifications/instructions.

7.4.9 All required building permits shall be obtained prior to the initiation of work.

7.4.10 An OWTS Installation Permit is required for the installation of a sewage effluent pump system. The pump system shall be inspected and hydraulically tested for proper operation by the system installer in the presence of DEHQ staff.

CHAPTER 8.0 SUBSURFACE DISPERSAL ~~FIELD~~ **SYSTEM** REQUIREMENTS

8.1 General

Many different designs and configurations are used for the subsurface dispersal of wastewater effluent but all incorporate soil infiltrative surfaces that are located in buried excavations. The primary infiltrative surface is the bottom of the excavation, but the sidewalls also may be used for infiltration. Perforated pipe is installed to distribute the wastewater over the infiltrative surface. A porous medium, typically gravel or crushed rock, is placed in the excavation below and around the distribution pipe to support the pipe and spread the localized flow from the distribution pipes across the excavation cavity. Other gravelless or chamber-type system components may be substituted for the rock and pipe. The porous medium maintains the structure of the excavation, exposes the applied wastewater to more infiltrative surface, and provides storage space for the wastewater within its void fractions (interstitial space, typically 30-40% of the volume) during peak flows with gravity systems. Untreated building paper or geotextile fabric is placed over the porous medium before backfilling with native soil to prevent the introduction of backfill material into the porous medium.

Most standard OWTS disperse wastewater either through a uniform distribution or serial distribution system. Uniform distribution is mostly used in flat, level areas and distributes wastewater flow equally among all trenches using a distribution box. Uniform distribution aids in maintaining unsaturated flow below the infiltrative surface, which results in wastewater retention times in the soil that are sufficiently long to effect treatment and promote subsoil reaeration. Uniform distribution design also results in more complete utilization of the infiltrative surface. A serial distribution system is commonly used on a sloping site. Rather than dividing the flow equally among all trenches, serial distribution provides for the first trench in the dispersal field to receive all the effluent from septic tank. When this first trench fills, ~~an overflow box sends~~ the effluent overflows **through the crossover pipe** into the next trench. In this manner, each trench in the system is used successively to its full capacity. This method of distribution makes full hydraulic use of all bottom and sidewall infiltration surfaces, creates the maximum hydrostatic head over the infiltration surfaces to force the water into the surrounding soil, and eliminates the problem of dividing flows evenly among independent trenches. However, because continuous ponding of the infiltrative surfaces is necessary for the system to function, the trenches suffer hydraulic failure more rapidly and progressively because the infiltrative surfaces cannot regenerate their infiltrative capacity. Thus, only the portion of the system required to absorb the wastewater is used. During periods of high flow or low absorptive capacity of the soil, more trenches will be used. When flows are low or during the hot dry summer months, the lower trenches may not be needed, so they may drain and dry out, automatically resting more trenches, which rejuvenates their infiltrative surfaces.

8.1.1 No new or replacement dispersal systems shall be covered by an impermeable surface, such as paving, building foundation slabs, plastic sheeting, or other material that may damage integrity of OWTS and/or prevents oxygen transfer to the soil limiting treatment.

8.1.2 The primary and designated reserve dispersal areas shall be installed within areas of undisturbed soil that is not used or proposed for use for other purposes, unless that use is consistent with the intent of these standards, such as use of vegetation in a dispersal area. Drip dispersal proposed for areas used

for an existing dispersal is prohibited.

8.1.3 A dispersal field design shall meet the minimum distance requirements to its dispersal field components or to other dispersal field components and shall not be placed or proposed for placement over an existing or previously used dispersal area. Deep bed dispersal systems may be approved in an area where a previous shallow dispersal system was used if all dispersal system material is removed to native soils.

8.1.4 Areas that are within the minimum setback distances established to protect public health and/or water quality shall not be used for waste disposal or percolation testing. The following areas are considered unsuitable for the location of the dispersal system or designated reserve area:

8.1.4.1 Areas within any easement which is dedicated for surface or subsurface improvement.

8.1.4.2 Paved areas or areas proposed for paving or areas occupied by or proposed to be occupied by structures or other surface features.

8.1.4.3 Areas not owned or under the control of the property owner unless the area is dedicated as a recorded easement for waste disposal purposes.

8.1.4.4 Low-lying areas subject to flooding or with a potential for flooding.

8.1.5 Metal tape or other approved material may be installed within the dispersal field for the purpose of providing a means for locating the dispersal field components at a later date using a metal detector. This item shall be proposed as part of the permit and approved by DEHQ prior to installation.

8.1.6 Materials for drainage piping shall comply with the requirements of the County Plumbing Code. Approved piping is shown in Table 7.1-1 and must be in accordance with the referenced standards. Piping for drip dispersal shall be of approved material.

8.1.7 Dispersal systems for OWTS located within the Borrego Springs area where the Regional Board has identified OWTS discharges to have the potential to negatively impact groundwater shall be of shallow leach line trench or shallow bed construction.

8.2 Minimum Residential Wastewater Infiltrative Areas

8.2.1 Residential leach line systems shall be sized based on the leach line requirement provided in Table 8.2-1. Table 8.2-1 is based on the maximum four-square feet per linear foot of infiltrative surface allowed pursuant to the OWTS Policy and as a function of percolation rate and the number of bedrooms for a single-family dwelling.

Table 8.2-1: Minimum Residential Leach Line Length Based on Percolation Test Rate

Perc Rate	Number of Bedrooms							Perc Rate	Number of Bedrooms					
MPI	1	2	3	4	5	6		MPI	1	2	3	4	5	6
1	200	200	240	270	280	300		61	370	460	571	761	952	1142
2	200	200	240	270	280	300		62	380	470	580	773	966	1160
3	200	200	240	270	280	300		63	390	480	592	789	987	1184
4	200	220	260	290	300	310		64	400	490	602	802	1003	1203
5	200	240	290	320	320	340		65	420	500	611	815	1019	1223
6	200	250	300	340	350	360		66	420	510	625	833	1042	1250
7	210	260	310	350	370	380		67	430	520	636	847	1059	1271
8	210	265	320	360	390	400		68	440	530	647	862	1078	1293
9	220	270	320	360	400	410		69	450	540	662	882	1103	1324
10	220	275	330	370	410	420		70	460	550	674	898	1123	1347
11	220	280	340	380	420	430		71	470	560	686	915	1143	1372
12	230	285	340	380	430	440		72	480	570	703	938	1172	1406
13	230	290	350	390	430	450		73	490	580	717	955	1194	1433
14	235	295	350	400	440	460		74	500	590	731	974	1218	1461
15	240	300	360	400	450	470		75	510	600	750	1000	1250	1500
16	240	300	360	410	450	490		76	520	610	765	1020	1276	1531
17	240	305	370	410	460	500		77	530	620	781	1042	1302	1563
18	250	310	370	420	460	510		78	540	630	804	1071	1339	1607
19	250	310	380	420	470	520		79	550	640	821	1095	1369	1642
20	250	315	380	430	470	520		80	560	650	846	1128	1410	1692
21	260	320	380	430	480	530		81	570	660	865	1154	1442	1731
22	260	320	390	440	480	530		82	580	670	886	1181	1476	1772
23	260	325	390	440	490	550		83	590	680	915	1220	1524	1829
24	260	330	400	450	500	560		84	600	690	938	1250	1563	1875
25	260	330	400	450	500	560		85	610	700	962	1282	1603	1923
26	270	335	400	450	510	570		86	620	710	996	1327	1659	1991
27	270	340	410	460	515	575		87	630	720	1023	1364	1705	2045
28	270	340	410	460	515	575		88	640	730	1051	1402	1752	2103
29	270	345	420	470	525	585		89	650	740	1092	1456	1820	2184
30	280	350	420	470	525	585		90	665	755	1125	1500	1875	2250
31	280	350	420	480	535	595		91	680	770	1125	1500	1875	2250
32	280	355	430	480	535	595		92	695	785	1125	1500	1875	2250
33	290	360	430	490	545	605		93	710	800	1125	1500	1875	2250
34	290	360	440	490	545	605		94	725	815	1125	1500	1875	2250

Table 8.2-1: Minimum Residential Leach Line Length Based on Percolation Test Rate														
Perc Rate	Number of Bedrooms							Perc Rate	Number of Bedrooms					
MPI	1	2	3	4	5	6		MPI	1	2	3	4	5	6
35	290	365	440	500	555	615		95	740	830	1125	1500	1875	2250
36	300	370	440	500	555	615		96	755	845	1125	1500	1875	2250
37	300	370	450	500	555	615		97	770	860	1125	1500	1875	2250
38	300	375	450	510	565	625		98	785	875	1125	1500	1875	2250
39	300	380	460	510	565	625		99	800	890	1125	1500	1875	2250
40	300	380	460	520	575	635		100	815	905	1125	1500	1875	2250
41	310	385	460	520	575	635		101	830	920	1125	1500	1875	2250
42	310	390	470	530	585	645		102	845	935	1125	1500	1875	2250
43	310	390	470	530	585	645		103	860	950	1125	1500	1875	2250
44	310	395	480	540	595	655		104	875	965	1125	1500	1875	2250
45	320	400	480	540	595	655		105	890	980	1125	1500	1875	2250
46	320	400	480	540	595	655		106	905	995	1125	1500	1875	2250
47	320	405	490	550	605	665		107	920	1010	1125	1500	1875	2250
48	330	410	490	550	605	674		108	935	1025	1125	1500	1875	2250
49	330	410	500	560	615	697		109	950	1040	1125	1500	1875	2250
50	330	415	500	560	615	723		110	965	1055	1125	1500	1875	2250
51	340	420	500	560	635	750		111	980	1070	1125	1500	1875	2250
52	340	420	510	570	649	779		112	995	1085	1125	1500	1875	2250
53	340	425	510	580	674	809		113	1010	1100	1125	1500	1875	2250
54	340	430	520	580	702	843		114	1025	1115	1125	1500	1875	2250
55	340	430	520	586	732	879		115	1040	1130	1125	1500	1875	2250
56	350	435	520	612	765	918		116	1055	1145	1125	1500	1875	2250
57	350	440	530	641	801	962		117	1070	1160	1125	1500	1875	2250
58	350	440	530	673	841	1009		118	1085	1175	1125	1500	1875	2250
59	350	445	540	708	884	1061		119	1100	1190	1125	1500	1875	2250
60	360	450	563	750	938	1125		120	1120	1210	1125	1500	1875	2250

8.2.2 Second Dwelling Units. Dispersal fields for second dwelling units can be designed as a separate system serving only the second dwelling unit or can be a common system serving both the existing dwelling and the second dwelling unit. Dispersal fields shall be sized based on independent dwelling units, even if a common dispersal field is proposed. The dispersal system size shall be equal to the sum of the two individual system sizes. It is recommended that second dwelling units that may be subject to a subdivision of land in the future be served by its own OWTS.

EXAMPLE - For a 3-bedroom main dwelling and a 2-bedroom second dwelling unit and assuming a 16 MPI percolation rate:

Separate Systems:

3-bedroom main dwelling requires 360 feet of leach line and 100% reserve area

2-bedroom second dwelling requires 300 feet of leach line and 100% reserve area

Common System:

3-bedroom main dwelling plus 2-bedroom second dwelling requires 660 feet of leach line and 100% reserve area.

8.2.3 The minimum total leach line requirement for accessory structures with plumbing fixtures shall be 200 feet, regardless of the projected wastewater flows.

8.3 Minimum Commercial Wastewater Infiltrative Areas

8.3.1 Non-residential infiltrative surface area shall be calculated by a qualified professional using ~~projected average~~ **peak** daily wastewater flows and the maximum application rates as determined from stabilized percolation rate provided in Table 8.3-1. The minimum total infiltrative surface area shall be equal to 200 feet of leach line (800 square feet of infiltrative surface), regardless of flow.

TABLE 8.3-1: APPLICATION RATES AS DETERMINED FROM STABILIZED PERCOLATION RATES

Percolation Rate (min/inch)	Application Rate (gal/day/ft ²)		Percolation Rate (min/inch)	Application Rate (gal/day/ft ²)		Percolation Rate (min/inch)	Application Rate (gal/day/ft ²)
1	1.2		31	0.522		61	0.197
2	1.2		32	0.511		62	0.194
3	1.2		33	0.5		63	0.19
4	1.2		34	0.489		64	0.187
5	1.2		35	0.478		65	0.184
6	0.8		36	0.467		66	0.18
7	0.8		37	0.456		67	0.177
8	0.8		38	0.445		68	0.174
9	0.8		39	0.434		69	0.17
10	0.8		40	0.422		70	0.167
11	0.786		41	0.411		71	0.164
12	0.771		42	0.4		72	0.16
13	0.757		43	0.389		73	0.157
14	0.743		44	0.378		74	0.154
15	0.729		45	0.367		75	0.15
16	0.714		46	0.356		76	0.147
17	0.7		47	0.345		77	0.144
18	0.686		48	0.334		78	0.14

19	0.671		49	0.323		79	0.137
20	0.657		50	0.311		80	0.133
21	0.643		51	0.3		81	0.13
22	0.629		52	0.289		82	0.127
23	0.614		53	0.278		83	0.123
24	0.6		54	0.267		84	0.12
25	0.589		55	0.256		85	0.117
26	0.578		56	0.245		86	0.113
27	0.567		57	0.234		87	0.11
28	0.556		58	0.223		88	0.107
29	0.545		59	0.212		89	0.103
30	0.533		60	0.2		90-120	0.1

Example – The project is an industrial office with 50 employees that work one daily shift with no daily visitors.

According to Table 6.5-1, the minimum daily wastewater flow is derived from 15 gallons per person per day. The resultant daily flow is 750 gallons per day. This is also the peak daily flow as this business does not have other sources of domestic wastewater.

The percolation rate for the site was determined to be 20 minutes per inch. According to Table 8.3-1, the application rate is 0.657 gallons per day per square foot. 750 gallons per day peak daily flow divided by 0.657 gallons per day per square foot application rate = 1,142 square feet of infiltrative area required.

A leach line is based on an infiltrative surface of four-square feet of infiltrative area per linear foot of trench. The leach line length is determined by dividing 1,142 square feet by four = 286 linear feet of leach line. Note: the minimum length of leach line required regardless of flow is 200 feet.

8.4 — Wastewater Infiltrative Area Equivalency Charts

~~8.4.1 Deep bed dispersal system infiltrative areas shall have an equivalent infiltrative surface area as required for residential leach lines provided in Table 8.2-1. Table 8.6-2 provides infiltrative surface areas for deep bed dispersal systems.~~

~~8.4.2 Vertical seepage pit infiltrative surface areas have an equivalent infiltrative surface area as required for residential leach lines provided in Table 8.2-1. Appendix III provides equivalent infiltrative surface areas for residential leach lines and Appendix IV provides the infiltrative surface areas for vertical seepage pits~~

~~8.4.2.2 The sizing of commercial dispersal fields utilizing leach lines or deep beds shall have the design and length equivalent to the infiltrative surface area as calculated per 8.3.1.~~

8.5.8.4 Minimum Leach Line Dispersal System Requirements

8.4.1 General Requirements.

8.4.1.1 Leach lines systems are the primary means of effluent dispersal for the majority of OWTS within San Diego County for sites with a percolation test rate of up to 120 minutes per inch.

8.4.1.2 A leach line consists of a trench, rock or other approved filter material, such as rock-less chambers, a perforated pipe, filter protecting fabric or paper, and soil cover.

~~8.5.18.4.1.3 Dispersal fields proposing to use~~ Leaching chambers or binded expanded polystyrene synthetic aggregate units ~~must comply with IMAPO~~ may be used instead of the rock construction. All installations must comply with IMAPO PS 63 and IGC 276, respectively, and must be installed in accordance with the manufacturer's specifications. No reduction in sizing is permitted with the use of these rock alternative construction components. To stabilize the installation before inspection, the soil or required fill material shall be added between and around the leaching chambers or binded expanded polystyrene synthetic aggregate units, leaving the uppermost surface of the units exposed to allow for system inspection.

~~8.5.28.4.2~~ Leach Line Dimensions and Specifications. Leach lines in the dispersal field shall meet the following:

Table 8.5-18.4-1 : Leach Line Dispersal System Dimensions and Specifications	
Minimum/Maximum length of individual leach line ¹	See Note Based on Design
Maximum length of individual leach line	100'
Minimum width of trench	18"
Maximum width of trench	36"
Maximum Grade of clean, washed rock	1.0'" to 1.5"
Minimum depth of rock below perforated pipe	12"
Maximum depth of rock below perforated pipe	36"
Minimum depth of rock over perforated pipe	42"
Minimum total rock in trench (12" under, 4" around perforated pipe , and 2" above pipe)	20 18"
Minimum soil cover over rock ¹²	12"
Maximum soil cover over rock ¹²	48" or 4' 24"
Minimum depth of trench (20 18" rock + 12" soil cover)	32" 30"
Maximum depth of trench (20 42" rock + 48 18" soil cover)	68" (5.7') 60" or 5.0'
Minimum grade of trench bottom (end to end)	level
Maximum grade of trench bottom (end to end)	2" per 100'
Distance between leach lines ³	10', center to center
Maximum slope	25 30%
Maximum slope with Slope Stability Study	40%

~~1- Soil cover requirements must also conform to the requirements of the manufacturer of any gravel-less/chamber design.~~
~~2- Minimum length of individual leach line dependent on design.~~ 1- A minimum of one observation port every hundred feet is required. 2- Soil cover requirements must also conform to the requirements of the manufacturer of any gravel-less/chamber design. Maximum soil cover for 5' trench is 18". 3-The distance between leach lines can be reduced to 6' from sidewall to sidewall for sites where the design percolation rate is 30 MPI or less, the effluent is distributed by equal distribution (distribution box), and the slope is 10% or less.

8.4.3 Leach Line Dispersal System Sizing. The minimum infiltrative surface area for leach lines shall be determined pursuant to Section 8.2 for OWTS serving residential uses and Section 8.3 for OWTS serving commercial uses.

~~8.5.3~~**8.4.4** Materials and Construction

~~8.5.3.1~~**8.4.4.1** All piping and materials used in leach line systems shall be of approved material including gravel-less/chamber systems must have IAPMO approval and must be approved by DEHQ prior to installation. No reduction in sizing is allowed for the use of chambers.

~~8.5.3.2~~**8.4.4.2** Leach lines that utilize gravel shall be filled with clean, washed rock graded at 1.0 to 1.5 inches in size and shall be covered with straw, untreated building paper or a geotextile fabric prior to backfill to prevent the infiltration of soil into the rock.

~~8.5.3.3~~**8.4.4.3** Where leach lines are installed in an equal distribution design, an approved watertight distribution box, of sufficient size to accommodate the expected wastewater volume and necessary field lateral lines, shall be constructed at the head of each dispersal field with watertight inlet and outlets.

~~8.5.3.3.1~~**8.4.4.3.1** Each field lateral shall be connected separately to a distribution box, shall not be subdivided, and shall provide equal distribution.

~~8.5.3.3.2~~**8.4.4.3.2** Equal distribution is not required for repairs when infeasible to install or where site conditions warrant a different distribution method design, such as serial distribution system (sloping sites or when trench length variability is necessary), or pressure dosed systems.

~~8.5.3.4~~**8.4.4.3.3** Where multiple leach lines are proposed on sloping ground, or where variable trench lengths are necessary, a serial system, with dams and siphon crossover/overflow pipes may be used to connect the leach lines.

~~8.5.3.5~~**8.4.4.3.4** No leach line shall be placed under concrete, blacktop, roadway or structure. If necessary to cross under such construction, watertight lines of ASB ABS Schedule 40 or other approved materials shall be used. Leach lines must be maintained in an open area and may not be compacted or driven over. Barricades may be required to maintain this area when requested by DEHQ.

~~8.5.6~~**8.4.5** Leach Lines on Steep Slopes

~~8.5.6.1~~**8.4.5.1** Leach line dispersal systems are limited to slopes of ~~25~~**30** percent or less unless the site evaluation and **slope stability screening, or a Soil Slope Stability Study, if required,** indicate the area is suitable for leach line dispersal system. The maximum slope allowed for leach line trenches is 40 percent.

~~8.5.6.2~~**8.4.5.2** Trenches shall follow the surface contours to minimize variations in trench depth and shall only be installed perpendicular to the slope.

~~8.5.6.3~~**8.4.5.3** The following requirements must be met for the installation of leach line trenches on slopes exceeding ~~25~~**30** percent. The design parameters are applicable only to slopes exceeding ~~25~~**30** percent and are not intended to be used in any other situation.

~~8.5.6.3.1~~**8.4.5.3.1** All leach lines on steep slopes shall be installed in five (5) foot deep trenches with 12 inches of leach rock below the leach pipe or with approved chambers or other gravel-less system. **To accommodate the trench depth, the maximum soil cover shall be 18 inches.**

~~8.5.6.3.2~~**8.4.5.3.2** The ~~design~~**installation** of disposal systems on steep slopes requires ~~the an~~ experienced and expertise **qualified profession** to address conditions relative to soil, slope stability, and subsurface conditions which require **competent** professional judgment and technical knowledge. Designs for steep slope systems will only be approved when submitted by a qualified professional registered in the State of California.

~~8.5.6.3.3~~**8.4.5.3.3** Any grading proposed to create a stable work area for leach line installation may ~~be required to obtain~~ a grading permit. It is the responsibility of the applicant to ensure all required permits have been obtained prior to initiating any work.

~~8.6~~8.5 Minimum Deep Bed Dispersal System Requirements

~~8.6.1~~**8.5.1 General Requirements.** Deep bed dispersal systems may be used for **new and replacement OWTS** throughout San Diego County if specific soil and site conditions exist. ~~The use of the horizontal seepage pit is generally only considered for sites where adequate area does not exist for a leach line system. These dispersal systems are also used in certain Borrego Springs areas where coarse sand is uniformly present and overlies a deep groundwater basin.~~ Deep beds are approved for use only in soils with percolation rates of 30 minutes per inch or ~~less~~**faster**.

~~8.6.2~~**8.5.1.1** The minimum depth to groundwater from the bottom of the deep bed dispersal shall meet the requirements of Table 3.7-1 or 10 feet, whichever is greater.

~~8.6.3~~**8.5.2 Deep Bed Dispersal Dimensions and Specifications.** Deep bed dispersal systems shall meet the requirements as shown in Table 8.6-1.

Table 8.6-18.5-1: Deep Bed Dispersal System Dimensions and Specifications	
Minimum/ Maximum length of individual deep beds ¹	Per Section 8.6.4 Based on Design

Maximum length of individual deep beds	100'
Minimum width of deep bed	4'
Maximum width of deep bed	6'
Range of grade of clean, washed rock	1.0" to 3.0"
Minimum depth of rock over perforated pipe manifold	42"
Minimum total rock in deep bed	6'
Maximum total rock in deep bed	7'
Minimum soil cover over rock ¹ (Soil Cap)	2'
Maximum soil cover over rock ¹ (Soil Cap)	5'
Minimum total depth of deep bed (6' rock + 2' soil cover)	8'
Maximum total depth of deep bed (7' rock + 5' soil cover)	12'
Minimum grade of deep bed	level
Maximum grade of deep bed	2" per 100'
Distance between deep beds < 10' depth	15', edge to edge
Distance between deep beds > 10' depth	20', edge to edge
Maximum slope	25-30%

1-A minimum of one observation port every hundred feet is required.

8.6.4.1.5.3 Deep Bed Dispersal System Sizing. Infiltrative Surface Areas

8.6.4.1.5.3.1 Residential Dispersal System Sizing. The sizing of residential dispersal fields utilizing deep bed dispersal systems shall have the equivalent infiltrative surface area as required for residential leach lines. Table 8.5-2 provides the minimum required deep bed dispersal system infiltrative surface areas that are equivalent to the leach line requirements based on number of bedrooms and percolation rate (derived from Table 8.2-1). provided in Table 8.2-. Deep bed dimensions to achieve the square footage requirements are provided in Table 8.5-3. Appendix III provides tables showing equivalent infiltrative surface areas for residential leach lines.

Table 8.5-2: Minimum Residential Deep Bed Dispersal Infiltrative Square Foot Surface Area Requirement Based on Percolation Rate

Perc Rate MPI	1 Bedroom	2 Bedrooms	3 Bedrooms	4 Bedrooms	5 Bedrooms	6 Bedrooms
1	800	800	960	1080	1120	1200
2	800	800	960	1080	1120	1200
3	800	800	960	1080	1120	1200
4	800	880	1040	1160	1200	1240
5	800	960	1160	1280	1280	1360
6	800	1000	1200	1360	1400	1440
7	840	1040	1240	1400	1480	1520
8	840	1060	1280	1440	1560	1600
9	880	1080	1280	1440	1600	1640
10	880	1100	1320	1480	1640	1680

11	880	1120	1360	1520	1680	1720
12	920	1140	1360	1520	1720	1760
13	920	1160	1400	1560	1720	1800
14	940	1180	1400	1600	1760	1840
15	960	1200	1440	1600	1800	1880
16	960	1200	1440	1640	1800	1960
17	960	1220	1480	1640	1840	2000
18	1000	1240	1480	1680	1840	2040
19	1000	1240	1520	1680	1880	2080
20	1000	1260	1520	1720	1880	2080
21	1040	1280	1520	1720	1920	2120
22	1040	1280	1560	1760	1920	2120
23	1040	1300	1560	1760	1960	2200
24	1040	1320	1600	1800	2000	2240
25	1040	1320	1600	1800	2000	2240
26	1080	1340	1600	1800	2040	2280
27	1080	1360	1640	1840	2060	2300
28	1080	1360	1640	1840	2060	2300
29	1080	1380	1680	1880	2100	2340
30	1120	1400	1680	1880	2100	2340

Note: Square foot equivalent is equal to leach line length in Table 8.2-1 times four (four square feet per linear foot).

The minimum infiltrative surface area for residential deep beds in the certain Borrego Springs areas with uniform coarse sands overlying a deep groundwater basin shall be calculated from Table 8.3-1 using percolation rates of 1-5 minutes per inch.

Table 8.5-3: Minimum Residential Deep Bed Dispersal Systems Dimensions and Infiltrative Surface Areas						
Length Linear Feet	4' Wide		5' Wide		6' Wide	
	6' Rock Depth	7' Rock Depth	6' Rock Depth	7' Rock Depth	6' Rock Depth	7' Rock Depth
	Infiltrative Surface Area - Square Feet					
1	16	18	17	19	18	20
10	160	180	170	190	180	200
15	240	270	255	285	270	300
20	320	360	340	380	360	400
25	400	450	425	475	450	500
30	480	540	510	570	540	600
35	560	630	595	665	630	700
40	640	720	680	760	720	800
45	720	810	765	855	810	900
50	800	900	850	950	900	1000
55	880	990	935	1045	990	1100
60	960	1080	1020	1140	1080	1200
65	1040	1170	1105	1235	1170	1300

70	1120	1260	1190	1330	1260	1400
75	1200	1350	1275	1425	1350	1500
80	1280	1440	1360	1520	1440	1600
85	1360	1530	1445	1615	1530	1700
90	1440	1620	1530	1710	1620	1800
100	1600	1800	1700	1900	1800	2000

Example – The project is a deep bed dispersal system for a 3-bedroom residence with a percolation rate of 20 minutes per inch.

According to Table 8.5-2, a 3-bedroom residence with a percolation test result of 20 minutes per inch requires 1,520 square feet of infiltrative area. Using Table 8.5-3, this infiltrative area is satisfied with either: a) 1-deep bed with dimensions of 80 feet long x 5 feet wide x 7 feet of rock; or b) 2-deep beds with dimensions of 40 feet long x 5 feet wide x 7 feet of rock; or c) 3-deep beds with dimensions of 30 feet long x 5 feet wide x 6 feet of rock.

8.6.4.2.8.5.3.2 Commercial Dispersal System Sizing. The sizing of commercial dispersal fields utilizing deep bed dispersal systems shall have the equivalent infiltrative surface area as calculated pursuant to Section 8.3.

Table 8.6-2: Deep Bed Dispersal Systems Infiltrative Surface Areas

Horizontal Seepage Pit Dimensions	4' Wide			5' Wide			6' Wide		
	6' Rock Depth	7' Rock Depth	8' Rock Depth	6' Rock Depth	7' Rock Depth	8' Rock Depth	6' Rock Depth	7' Rock Depth	8' Rock Depth
Length Linear Feet	Infiltrative Surface Area Square Feet								
1	16	17	20	17	19	21	18	20	22
10	160	170	200	170	190	210	180	200	220
15	240	255	300	255	285	315	270	300	330
20	320	340	400	340	380	420	360	400	440
25	400	425	500	425	475	525	450	500	550
30	480	510	600	510	570	630	540	600	660
35	560	595	700	595	665	735	630	700	770
40	640	680	800	680	760	840	720	800	880
45	720	765	900	765	855	945	810	900	990
50	800	850	1000	850	950	1050	900	1000	1100
55	880	935	1100	935	1045	1155	990	1100	1210
60	960	1020	1200	1020	1140	1260	1080	1200	1320
65	1040	1105	1300	1105	1235	1365	1170	1300	1430
70	1120	1190	1400	1190	1330	1470	1260	1400	1540
75	1200	1275	1500	1275	1425	1575	1350	1500	1650
80	1280	1360	1600	1360	1520	1680	1440	1600	1760
85	1360	1445	1700	1445	1615	1785	1530	1700	1870
90	1440	1530	1800	1530	1710	1890	1620	1800	1980
100	1600	1700	2000	1700	1900	2100	1800	2000	2200

~~8.6.4.3~~ **8.5.3.3** Primary and reserve deep bed dispersal systems cannot be combined in one common pit.

~~8.6.4.4~~ **8.5.3.4** Deep bed dispersal systems shall follow the surface contours to minimize variations in bed depth and shall only be installed perpendicular to the slope. The ~~pit excavations~~ deep bed design configurations may arc or bend under the following conditions:

~~8.6.4.4.1~~ **8.5.3.4.1** The maximum ~~deflection~~ **bend or arc** cannot exceed a total of 45 degrees in any direction ~~without increasing the pit length to compensate for loss of sidewall area.~~

~~8.6.4.4.2~~ ~~Bends or arcs totaling greater than 45 degrees may be accepted on a case-by-case basis. A correction factor will be required, increasing the total length, due to sidewall loss.~~

~~8.6.4.4.3~~ **8.5.3.4.2** U-shaped and H-shaped ~~bends~~ **configurations of trench** are not allowed.

8.5.3.5 Deep Bed Dispersal on slopes over 30% are not permitted.

~~8.6.5~~ **8.5.4** Materials and Construction

~~8.6.5.1~~ **8.5.4.1** The use of concrete pit liners is allowed **in sandy areas where the integrity of the sidewall cannot be maintained during construction.** If used, the concrete pit liners shall meet the testing standards established by the International Association of Plumbing and Mechanical Officials (IAPMO) or otherwise approved by DEHQ.

~~8.6.5.2~~ **8.5.4.2** All **deep beds** ~~pits~~ must be filled with clean leach line rock to the cap depth. If pit liners are used, the interior of the liners must also be filled with rock. The rock should be graded at 1.0 to 3.0 inches in size and shall be covered with straw, untreated building paper or a geotextile fabric prior to backfill to prevent the infiltration of soil into the rock.

~~8.6.5.3~~ **8.5.4.3** A manifold system constructed of a four (4) inch loop of Schedule 40 perforated pipe shall be installed in the **deep bed** ~~pit~~ to allow for distribution of the effluent throughout the entire **trench** ~~pit~~. The manifold shall be placed one (1) foot from the sidewall of the deep bed and shall run the length and width of the deep bed in a rectangular pattern. A minimum of ~~four~~ **two (42)** inches of rock shall be placed over the manifold piping.

~~8.6.5.4~~ **8.5.4.4** Where more than one deep bed is installed, a watertight distribution box of sufficient size to accommodate the necessary field lateral lines shall be constructed at the head of each dispersal field with watertight inlet and outlets.

~~8.6.5.4.1~~ **8.5.4.4.1** Each field lateral shall be connected separately to a distribution box, shall not be subdivided, and shall provide equal distribution.

~~8.6.5.4.2~~ **8.5.4.4.2** Equal distribution is not required for repairs when infeasible to install or where site conditions warrant a different distribution ~~method~~ **design**, such as **a** serial distribution

system (sloping sites or when trench length variability is necessary), or pressure dosed systems.

~~8.6.5.5~~**8.5.4.5** A hybrid system combining a deep bed dispersal for the primary or reserve design, and leach lines for the other system is allowed. However, a combination of the two types of systems, used for a single primary or reserve design, is not allowed. The determination of the design to be installed as the primary dispersal and the reserve dispersal systems shall be based on the feasibility of installation, taking into consideration access, grading, or other obstacles to install the system once the house and any other structures or appurtenances are constructed. **When necessary,** ~~t~~The design with the most installation restrictions shall be installed as the primary dispersal system.

~~8.6.5.6~~**8.5.4.6** It is the responsibility of the applicant to ensure any work involving excavations or open borings meets all applicable state and local laws and regulations, including the requirements of the California Labor Code and associated regulations.

~~8.7~~8.6 Minimum Vertical Seepage Pit Dispersal System Requirements

~~8.7.1~~**8.6.1 General Requirements.** Vertical seepage pits are a type of dispersal system allowed in limited areas of San Diego County with specific requirements on their use. Vertical seepage pits are only allowed in areas where **the Regional Board has excepted the water body from the Municipal and Domestic Supply Beneficial Use** ~~beneficial uses have been excepted from the sources of drinking water policy~~ in the San Diego Basin Plan. Vertical seepage pits are not permitted in areas with interior granitic formations or fractured rock. An interactive map of beneficial uses is found at the following link: <https://gispublic.waterboards.ca.gov/portal/apps/webappviewer/index.html?id=1f58bd97fdcd45329a5e16e373ede24d>

~~8.7.2~~**8.6.2 Vertical Seepage Pit Dimensions and Specifications.** Vertical seepage pits shall meet the dimensions and specifications as shown in Table ~~8.7-1~~**8.6-1**.

Table 8.7-18.6-1: Vertical Seepage Pit Dispersal System Dimensions and Specifications	
Minimum diameter of seepage pit	3'
Maximum diameter of seepage pit	4'
Range of grade of clean, washed rock	1.0" to 3.0"
Minimum depth of rock below soil cover (soil cap)	10'
Maximum depth of rock below soil cover (soil cap)	See Table 6.3-1 Per Appendix II
Minimum soil cover over rock [‡] (Soil Cap)	2'
Maximum soil cover over rock [‡] (Soil Cap)	5'
Minimum total depth of seepage pit	12'
Maximum total depth of seepage pit	See Table 6.3-1 Per Appendix II
Distance between pits	20', edge to edge
Maximum slope	25 30%

[‡] **1**-Soil cover (cap depth) over 5' may be permitted with documentation of justification.

~~8.7.3~~8.6.3 Depth and Sizing of Vertical Seepage Pits

~~8.7.3.1~~8.6.3.1 The total depth of the seepage pit shall be based on the location, type of soil, and groundwater level as determined by the site investigation, soil profile, and ~~percolation~~ testing procedures in Appendix III.

8.6.3.2 Vertical seepage pits on slopes over 30% are not permitted.

~~8.7.3.2~~ The minimum infiltrative surface shall be an equivalent to the leach line infiltrative surface area requirement shown in Appendix III.

~~8.7.4~~8.6.4 Materials and Construction

~~8.7.4.1~~8.6.4.1 All pits must be filled with clean washed leach line rock to the cap depth. The rock shall be graded at 1.0 to 3.0 inches in size and shall be covered with straw, untreated building paper, ~~or a~~ geotextile fabric, **or concrete** prior to backfill to prevent the infiltration of soil into the rock.

~~8.7.4.2~~8.6.4.2 A 4-inch ~~ABS or PVC Schedule 40~~ **approved** pipe shall be installed from the ground surface to the bottom of each seepage pit for clean-out, pumping and verification of the total pit depth. The pipe shall have perforations from the cap depth to the bottom of the pit and be of solid construction from the cap depth to the ground surface. A screw fit cap must be placed on top of the riser to allow access.

~~8.7.4.3~~8.6.4.3 Where more than one vertical seepage pit is proposed for the primary or reserve system, a watertight distribution box of sufficient size to accommodate the necessary field laterals ~~lines~~ shall be constructed at the head of each dispersal field with watertight inlet and outlets.

~~8.7.4.3.1~~8.6.4.3.1 Each seepage pit shall be connected separately to a distribution box, shall not be subdivided, and shall provide equal distribution.

~~8.7.4.3.2~~8.6.4.3.2 Equal distribution is not required for repairs when infeasible to install or where site conditions warrant a different distribution ~~method~~ **design**, such as serial **distribution** ~~dam and siphon~~ or pressure dosed systems.

~~8.8~~8.7 Minimum Drip Dispersal **System** Requirements

~~The construction requirements for pressurized drip dispersal systems shall be as follows.~~

~~8.8.1~~8.7.1 General Requirements

~~8.8.1.1~~8.7.1.1 The area of the drip dispersal system shall be planted with appropriate vegetation, **such as grass**, to allow for uptake of nutrients from the wastewater.

~~8.8.1.2~~8.7.1.2 The drip dispersal system shall be designed and maintained as necessary to reduce orifice

clogging and root intrusion.

~~8.8.1.3~~ **8.7.1.3** The drip dispersal system shall be designed, located and maintained to prevent vehicular traffic over it. **The use of barriers incorporated into the landscape for this purpose are recommended. Barriers or barricades may be required if warranted.**

~~8.8.1.4~~ **8.7.1.4** The setbacks required between drip dispersal systems and other components of the OWTS as well as structures, property lines, easements, watercourses, wells, or grading shall be the same as required for leach **lines, unless otherwise noted.** See the setback table found in Chapter 6.0 of this LAMP for the complete list of setbacks.

~~8.8.1.5~~ **8.7.1.5** Drip dispersal systems are pressure distribution systems and head loss calculations shall be provided to ensure proper hydraulic pressure at the emitter.

~~8.8.1.6~~ **8.7.1.6** ~~All components of the STS supplemental treatment system shall be certified in writing by the qualified professional who designed the STS supplemental treatment system~~ **shall certify in writing that the installation of all components** was completed per the approved design.

~~8.8.2~~ **8.7.2 Drip Dispersal System Dimensions and Specifications.** Drip dispersal systems shall meet the dimensions and specifications as shown in Table ~~8.8-18.7-1~~.

Table 8.8-18.7-1: Drip Dispersal System Dimensions and Specifications	
Minimum area	Per Table 8.3-1
Minimum depth of soil cover ¹ ±	6"
Maximum depth of soil cover	12"
Minimum emitter longitudinal spacing on emitter line	2'
Minimum Distance between emitter lines	2'
Maximum slope	25 30 %
Maximum slope with Slope Stability Study	40%

*1-Minimum soil cover for OWTS designed to reduce pathogens shall be 12 inches.

~~8.8.3~~ **8.7.3 Design and Sizing**

~~8.8.3.1~~ **8.7.3.1** Pressurized drip dispersal systems shall be designed by a qualified professional and installed per the manufacturer's recommendations. Calculations to size the drip dispersal infiltrative area shall use the peak daily flow and the application rates provided Table 8.3-1 **based on the highest percolation rate.**

~~8.8.4~~ **8.7.4 Materials and Construction**

~~8.8.4.1~~ **8.7.4.1** Drip dispersal system emitter lines shall be designed as a continuous loop circuit with no dead-ends.

~~8.8.4.2~~ **8.7.4.2** Vacuum release valves shall be installed at the highpoint of the emitter lines.

~~8.8.4.3~~ **8.7.4.3** Drip dispersal systems shall be time dosed over a 24-hour period. Demand control dosing shall override timed dosing in periods of flow where timed dosing cannot accommodate the excessive flow.

~~8.8.4.4~~ **8.7.4.4** Drip dispersal systems shall be designed to have a minimum operating pressure at the emitter head of 10 pounds per square inch (psi), a maximum operating pressure of 45 psi, a maximum system operation pressure of 60 psi, and a maximum discharge rate per emitter of 1.5 gallons per hour.

~~8.8.4.5~~ **8.7.4.5** All drip dispersal systems shall incorporate an automatic mechanism for backwashing or flushing the drip lines and filters.

8.7.4.6 Areas where drip dispersal is installed or to be installed shall be protected from any activity or disturbance impacting the system design and construction during and after construction. Barricades or construction fencing may be required.

8.8 Minimum Shallow Bed Dispersal System Requirements

8.8.1 General Requirements

8.8.1.1 A shallow bed dispersal system may be used in specific areas of San Diego County, including the Borrego Springs area, on level ground with uniform coarse sand, sand, loamy coarse sand, or loamy sand soils with percolation rates of three (3) minutes per inch or faster.

8.8.1.2 Leaching chambers or binded expanded polystyrene synthetic aggregate units may be used instead of the rock construction. All installations must comply with IMAPO PS 63 and IGC 276, respectively, and must be installed in accordance with the manufacturer's specifications. No reduction in sizing is permitted with the use of these rock alternative construction components. To stabilize the installation before inspection, the soil or required fill material shall be added between and around the leaching chambers or binded expanded polystyrene synthetic aggregate units, leaving the uppermost surface of the units exposed to allow for system inspection.

8.8.2 Shallow Bed Dimensions and Specifications

Table 8.8-1: Shallow Bed Dispersal System Dimensions and Specifications	
Minimum Infiltrative Area	Per Section 8.8.3
Grade of clean, washed rock	1.0" to 1.5"
Minimum depth of rock below perforated pipe	12"
Maximum depth of rock below perforated pipe	18"
Minimum depth of rock over perforated pipe	2"
Minimum soil cover over rock ¹	12"
Maximum soil cover over rock ¹	18"
Minimum/Maximum depth of bed	36"

Minimum grade of trench bottom (end to end)	level
Maximum grade of trench bottom (end to end)	2" per 100'
Distance between beds	10', edge to edge

8.8.3 Minimum Shallow Bed Sizing Requirements

8.8.3.1 Residential Dispersal System Sizing. The sizing of residential dispersal fields utilizing shallow bed dispersal systems shall have the equivalent infiltrative surface area as required for residential leach lines for percolation rates associated with sandy soils. Table 8.8-2 provides the minimum required shallow bed dispersal system infiltrative surface areas that are equivalent to the leach line requirements based on percolation rate (derived from Table 8.2-1). Shallow bed infiltrative surface areas shall have dimensions that provide the minimum square footage required in Table 8.8-2 based on the bottom of the bed area only.

Table 8.8-2: Minimum Residential Shallow Bed Infiltrative Surface Area Square Foot Requirement (Based on Percolation Rate of 1-3 MPI)					
1 Bedroom	2 Bedrooms	3 Bedrooms	4 Bedrooms	5 Bedrooms	6 Bedrooms
800	800	960	1080	1120	1200

Example – The project is a shallow bed dispersal system for a 3-bedroom residence with a percolation rate of less than 3 minutes per inch.

According to Table 8.5-2, a 3-bedroom residence with a percolation test result of 20 minutes per inch requires 960 square feet of infiltrative area. This infiltrative area may be satisfied with any dimension with a bottom infiltrative surface area equal to 960 square feet. For example, the following dimensions all provide a bottom infiltrative surface area of 960 square feet: 96 feet x 10 feet; 48 feet x 20 feet; 32 feet x 30 feet; 40 feet x 24 feet.

8.8.4 Materials and Construction

8.8.4.1 All piping and materials shall be of approved material.

8.8.4.2 Shallow beds that utilize gravel shall be filled with clean, washed rock graded at 1.0 to 1.5 inches in size and shall be covered with straw, untreated building paper or a geotextile fabric prior to backfill to prevent the infiltration of soil into the rock.

8.8.4.3 The wastewater shall be distributed equally across the shallow bed using a watertight distribution box of sufficient size to accommodate the necessary bed distribution lines. Distribution lines of approved (4) inch perforated pipe shall be installed within the bed and spaced between three (3) and six (6) feet with apart, center to center. No part of the perimeter sidewall of the bed shall be more than three (3) feet from a distribution line. For shallow beds up to eight (8) feet in width, the distribution lines may be configured with each lateral placed one (1) foot from the perimeter sidewall of the shallow bed and shall run the length of the bed. The terminal ends of the distribution lines may be manifolded over the width of the bed in a rectangular pattern. Two (2) inches of rock shall be placed over the distribution lines.

8.8.4.4 Where more than one shallow bed is installed, a watertight distribution box of sufficient size to accommodate the necessary beds shall be constructed at the head of each dispersal field with watertight inlet and outlets. Each bed shall be connected separately to a distribution box, shall not be subdivided, and shall provide equal distribution.

8.8.4. No shallow bed shall be placed under concrete, blacktop, roadway or structure. If necessary to cross under such construction, watertight lines of approved materials shall be used. Shallow beds must be maintained in an open area and may not be compacted or driven over. Barricades may be required to maintain this area when requested by DEHQ.

8.8.5 Slopes. Shallow bed dispersal systems are not permitted in areas with sloping conditions.

8.9 Other Dispersal ~~Systems~~ Fields

Other dispersal field designs may be proposed for use and will be evaluated on a case-by-case basis.

9.0 ONSITE WASTEWATER TREATMENT SYSTEMS WITH SUPPLEMENTAL TREATMENT

9.1 General

OWTS with supplemental treatment are OWTS that includes some type of advanced or supplemental treatment **for nitrogen and/or pathogens** in addition to the primary treatment that occurs in a septic tank used with a standard OWTS. ~~OWTS with supplemental treatment are designed to reduce nitrogen or pathogens and may be used to overcome specific site constraints generally having to do with high groundwater or shallow soils and provide the additional treatment necessary that will not be provided in the soil.~~

9.2 Design Criteria

9.2.1 Supplemental Treatment Requirements for Nitrogen

9.2.1.1 Supplemental treatment components for residential use designed to reduce nitrogen shall be NSF/ANSI 245 certified, ~~or certified by a third party tester approved by DEHQ,~~ to meet a 50 percent (50%) reduction in total nitrogen when comparing the 30-day average influent to the 30-day average effluent. As the NSF/ANSI 245 certification is limited to residential use up to 1500 gallons per day, any OWTS with supplemental treatment proposed for residential uses with higher flows or for commercial uses must be designed by a California licensed Civil Engineer to address the specific wastewater characteristics and flows from the proposed use.

9.2.1.2 The effluent concentration of nitrogen from an OWTS with supplemental treatment can be assumed to be 50% of that typical for a standard OWTS serving a single-family residence.

9.2.2 Supplemental Treatment Requirements for Pathogens

9.2.2.1 Supplemental treatment components designed to perform disinfection shall provide sufficient pretreatment of the wastewater so that effluent from the supplemental treatment components does not exceed a 30-day average total suspended solids of 30 milligrams per liter and shall further achieve an effluent fecal coliform bacteria concentration less than or equal to 200 Most Probable Number (MPN) per 100 milliliters.

9.2.2.2 OWTS with supplemental treatment designed for the reduction of pathogens shall have a minimum soil depth and minimum depth to the anticipated highest level of groundwater below the bottom of the dispersal system of three (3) feet.

9.2.2.3 All dispersal systems for OWTS with supplemental treatment designed for the reduction of pathogens shall have a minimum soil cover of ~~twelve (12)~~ inches.

~~9.2.2.4 For OWTS with supplemental treatment designed for pathogen reduction, the minimum soil depth to high groundwater shall not be less than three (3) feet.~~

~~9.2.2.5 For OWTS with supplemental treatment designed for pathogen reduction, all dispersal systems shall have at least 12" of soil cover. Where a drip line dispersal system is used to enhance vegetative nitrogen uptake, the dispersal system shall have at least six (6) inches of soil cover.~~

9.2.3 A **supplemental treatment system STS** must be installed by a licensed installer certified to install the specific **supplemental treatment system STS** proposed and the system must be installed according to the design approved in the OWTS Layout Report. **The design engineer must be contacted prior to the installation to provide the required installation supervision and certification.**

9.2.4 Treated effluent from all **supplemental treatment system STS** shall be discharged to a subsurface dispersal system consisting of leach lines, vertical seepage pits, ~~horizontal seepage pits~~ **deep bed trenches**, or pressurized drip dispersal systems.

9.2.5 System sizing for dispersal systems that utilize leach lines, vertical seepage pits, and ~~horizontal seepage pits~~ **deep bed trenches** shall be the same as those used for ~~conventional~~ **standard** OWTS.

9.2.6 Pressurized drip dispersal systems shall be designed by a qualified professional and installed per the manufacturer's recommendations. Calculations to size the drip dispersal system shall use the peak daily flow, **the slowest percolation rate documented from the site evaluation**, and the application rates provided Table 8.3-1.

9.2.7 A minimum two (2) foot separation between the bottom of the dispersal system to the highest anticipated level to which groundwater could be expected to rise is required for OWTS with supplemental treatment. For **supplemental treatment system STS** designed for pathogen reduction, a minimum three (3) foot separation is required.

9.2.8 A minimum of two (2) foot of permeable soil, **as required for the type of dispersal system in Chapter 8.0**, must exist below the bottom of the **supplemental treatment system STS** dispersal system.

9.2.9 The **supplemental treatment system STS** shall be equipped with a visual and audible alarm as well as a ~~two-way~~ telemetric alarm that alerts the owner, **the qualified service provider, and DEHQ** of system malfunctions. The telemetric alarm requirement shall not apply to cases where it is demonstrated that a telemetric alarm is impracticable.

9.2.10 The **supplemental treatment system STS** design shall include a ~~petcock~~ **sampling port** for sample collection on the dosing pump discharge line or other suitable location representative of effluent quality.

9.2.11 Septic tanks, pump chambers or other related components of an **supplemental treatment system STS** including risers ~~shall~~ **may be required to** undergo a water tightness test at the site of the installation **if needed**. Anti-floatation devices shall be utilized as needed.

9.3 Operation and Maintenance Requirements, Annual Operating Permit

9.3.1 All **supplemental treatment systems STS** require an annual operating permit issued by DEHQ. The

annual operating permit will define the monitoring and maintenance requirements as specified by the manufacturer and/or qualified professional who designed the system.

9.3.2 An operation and maintenance manual shall be provided by the manufacturer or qualified professional that includes the qualified professionals name, address, telephone number, and business and professional license number. A copy shall be maintained at the site and shall be available to the qualified service provider and to DEHQ upon request. **The OWTS owner shall maintain compliance with the requirements of the operations and maintenance manual at all times.**

9.3.3 **The owner is responsible to ensure that the** ~~All supplemental treatment system STS must be~~ **is** maintained by a qualified service provider and a maintenance contract must be ~~maintained~~ **in effect** at all times.

9.3.4 All **supplemental treatment systems** ~~STS~~ require, at a minimum, biannual inspections by the qualified service provider to ensure proper operation and maintenance of the system, **or more often if required by the manufacturer or qualified professional**. The property owner is responsible for ensuring copies of the inspection results are provided to the DEHQ within 30 days of the inspection being completed.

9.3.5 The drip dispersal system shall be flushed once every three months for the first year or until vegetation is established, whichever occurs first. Flushing shall occur every six months thereafter.

9.3.6 The qualified service provider shall be responsible for the following:

9.3.6.1 Assessing the **supplemental treatment system** ~~STS~~ to determine operational status.

9.3.6.2 Performing routine activities required to keep the system operational.

9.3.6.3 Responding to emergencies in a timely manner.

9.3.6.4 Collecting and recording information regarding operational status of treatment components and recommending timely maintenance, replacement, or pumping of various components as required.

9.3.6.5 Monitoring system performance through collection and analysis of effluent samples when appropriate.

9.3.6.6 Reporting system operational status/or system performance to the property owner and DEHQ.

9.3.6.7 Serving as an informational resource for the property owner.

9.3.7 The property owner is responsible for reporting all failures, malfunctions, service requests, alarms, or other instances where a **supplemental treatment system** ~~STS~~ requires the attention of a

qualified service provider by providing a copy of the inspection results to DEHQ within 30 days of the inspection being completed~~72 hours of the incident occurring~~.

9.3.8 Failure of a property owner to monitor, maintain, or operate the supplemental treatment system as designed and in accordance with the terms of the permit, the manufacturer's specifications, and the operations and maintenance manual approved for the system may result in revocation of the annual permit pursuant to section 68.335 of the Regulatory Code or other enforcement action.

9.3.9 Failure of a property owner to maintain an annual operating permit, to provide the biannual inspection results, or to provide notifications of supplemental treatment system STS issues to DEHQ may result in revocation of the permit pursuant to section 68.335 of the Regulatory Code or other enforcement action.

CHAPTER 10.0 ONSITE WASTEWATER TREATMENT SYSTEMS REQUIRING CORRECTIVE ACTION

All OWTS have the potential to result in surfacing ~~sewage or effluent or sewage~~ to ground surface or backing up into plumbing fixtures ~~for various reasons, including the due to age of the system, root intrusion, improper use or maintenance, misuse or improper design.~~ These OWTS will require corrective action to mitigate any risk to public health or contamination of the environment.

10.1 Defective and Existing Nonconforming OWTS

10.1.1 An OWTS that allows sewage, human excrement or other liquid wastes to be disposed of so that the waste is not confined underground is a defective system. The property owner shall promptly repair a defective OWTS by connection to a public sewer system or by repair or replacement of the OWTS under permit and inspection by DEHQ. The OWTS shall be pumped by a ~~permitted hauler~~ **registered sewage pumper** as necessary to keep the sewage confined to underground until the corrective action has been completed.

10.1.2 Any existing OWTS that do not meet the minimum siting and design requirements of this LAMP are considered existing, nonconforming OWTS. Sites with existing, nonconforming OWTS that propose changes in construction, use, ~~or~~ occupancy, ~~or~~ land use or development **and** which **propose** ~~may cause~~ expansion of the nonconforming conditions of the OWTS shall not be approved.

10.1.3 Existing OWTS where the RWQCB is requiring corrective actions for nitrate reduction or pathogen reduction with ongoing monitoring and reporting, including as part of a TMDL implementation plan, a TMDL restoration plan or other actions, are not covered by the local permitting program or LAMP and are under the purview of the RWQCB. Permit applications received for OWTS not covered under the LAMP scope of coverage will be referred to the appropriate RWQCB.

10.2 Corrective Action Requirements

10.2.1 **Complaints:** DEHQ will record all complaints received alleging violations of the San Diego County Code or LAMP relating to OWTS into the department's database system. These alleged violations are assigned to staff for investigation and follow up. Complaint response is prioritized based on the potential threat to health and safety. For example, priority complaints include surfacing sewage and open or unsafe septic tanks, pump tanks, or seepage pit excavations. Complaints alleging work being performed without a permit or by unlicensed persons are also recorded in the Department's database system and are assigned to staff for investigation and follow-up.

For priority complaints, DEHQ will ~~initiate complete~~ an investigation within 24 hours ~~of notification to determine the validity of the complaint or other notification of a defective OWTS.~~ **Non-priority complaint investigations will be initiated as soon as feasible.** Investigative activities are based on the nature of the complaint received and may include, but are not limited to, a site inspection to observe conditions relating to the installation, construction, operation, and maintenance of an OWTS, the completion of an

OWTS operation and maintenance survey, the provision of OWTS operation and maintenance guidance/information, the performance of dye testing as an indicator of surfacing sewage or effluent, the review of repair and pumping records, the requirement of an evaluation of the OWTS and/or site conditions by a qualified professional, and the requirement for the submittal of a technical report documenting the site conditions and/or operational status of the OWTS. See guidelines for OWTS performance evaluation in Appendix III – OWTS Performance Evaluation Guidelines.

As a site visit conducted in response to an alleged complaint also provides an opportunity to conduct outreach and education to OWTS owners, staff will provide a copy and a summary of the completed survey results to the property owners or tenants that participate in the operations and maintenance survey. Staff may also provide guidance information on OWTS maintenance and operation to property owners or tenants to further enhance education of OWTS usage.

10.2.2 Any OWTS that is found to be defective shall have a notice of violation issued to the property owner requiring action to eliminate the immediate health hazard through pumping of the septic tank by a licensed sewage hauler or elimination of wastewater flows to the failing OWTS. The notice of violation will also require a repair **or replacement** to be completed to the OWTS as needed within a reasonable time frame.

~~10.2.3 The proposed repair shall be evaluated by DEHQ to ensure it meets the minimum design requirements of this LAMP or is in substantial conformance to the greatest extent practicable.~~

~~10.2.4 Groundwater separation requirements to the bottom of the dispersal system and the highest anticipated groundwater level for repairs shall be as provided in Table 6.3 1:~~

10.2.35 The repair shall be completed under permit and inspection by DEHQ. **An OWTS Layout Report may be required to determine the siting, design, and construction of the repair or replacement prior to the issuance of a permit.**

10.2.4 Compliance with all local ordinance codes and requirements, as well as all corrective action requirements, are conditions of coverage under the conditional waiver of waste discharge requirements contained in the OWTS Policy (OWTS Policy 12.0, 12.0.5, 12.0.7). Failure to comply with corrective action requirements or enforcement orders constitute a failure to meet the conditions of the waiver of waste discharge requirements contained in the OWTS Policy (OWTS Policy 11.7) and the OWTS may be referred to the Regional Water Quality Control Board for oversight (OWTS Policy 2.6).

10.2.5 Additional abatement or enforcement actions may be implemented as provided for in section 10.4 and, where DEHQ determines the surfacing sewage is an immediate threat to public health and safety, the actions authorized under the San Diego County Code Section 69.101 et Seq.

10.3 Variances

10.3.1 All new and replacement OWTS shall meet the minimum requirements of this LAMP and local ordinance.

10.3.2 In cases where an existing OWTS cannot meet the minimum requirements, often where area is limited for the proposed repair or replacement, A property owner may submit a written request for a variance or waiver of a specific standard or requirement. ~~where area is limited for the proposed repair or a replacement of an existing OWTS and shall propose~~ An alternative OWTS design that maintains substantial conformance to the minimum requirements in this LAMP shall be prepared by a qualified professional and shall be submitted with the variance request.

10.3.3 DEHQ shall consider the ~~waiver~~variance request to determine if the alternative repair or replacement siting and design proposal is in substantial conformance, to the greatest extent practicable, with the OWTS Policy and this LAMP, is adequate to accommodate the sewage flows from the buildings being served by the OWTS, is able to achieve the same practical protection to public health and groundwater effect as afforded by the LAMP requirements by modification of ~~leach trench dimensions~~the system design, and is able to provide an adequate level of protection to adjacent properties, and is able to minimize any impacts to groundwater.

10.3.4 DEHQ may approve a ~~waiver~~variance request upon determination that the proposed alternative will not result in ~~any adverse effects on~~ contamination of an underground source of water or on public health or safety.

10.3.5 Property owners with ~~waiver~~variance requests that are not approved by DEHQ and the new or replacement OWTS does not meet the minimum conditions and requirements in the ordinance code or LAMP shall notify the RWQCB that the OWTS does not meet the requirements of the approved LAMP and is not covered by the local permitting program. (OWTS Policy 2.6)

10.3.6 Repairs may be approved between individual leach lines where sufficient area exists and when if it is demonstrated that the parcel has no other applicable area to repair the dispersal system.

10.4 Abatement and Enforcement

10.4.1 A violation of any standard or requirement contained in this LAMP is considered a public nuisance and is subject to nuisance abatement under the provisions of Title 1, Division 6, Chapter 2 of the San Diego Regulatory Code. In addition, DEHQ may also institute any other legal remedy available to the County, including the provisions of Title 1, Division 8 of the San Diego Regulatory Code.

10.4.2 All time, services, and materials costs associated with the voluntary and involuntary abatement of violations of this chapter shall be recovered in accordance with fees established in Section 68.107 of this Code. (Involuntary abatement is that action that is performed by, under the direction of, or at the initial expense of the County of San Diego. Voluntary abatement is the abatement of a violation by the owner or designee after notification by the Director that such violation exists and must be abated.)

10.5 Denial of Coverage under the OWTS Policy/Conditional Waiver

~~10.4.1~~ **10.5.1** ~~An owner of a new or replacement OWTS that does not meet the conditions and requirements set forth in this approved LAMP shall notify the appropriate RWQCB by submitting any required documentation, such as a report of waste discharge (OWTS Policy, Section 2.6). DEHQ shall make a referral to the appropriate RWQCB when an OWTS is identified under this subsection. DEHQ may deny coverage under the OWTS Policy to any OWTS that is not in compliance with Section 6.1 of the OWTS Policy (conditions required to maintain coverage under the conditional waiver), or not able to adequately protect the water quality of the waters of the State as determined by the Regional Board. DEHQ shall make a referral to the appropriate RWQCB when an OWTS is identified under this subsection (OWTS Policy 6.2).~~

~~10.4.2~~ **10.5.2** ~~DEHQ may deny coverage under the OWTS Policy to any OWTS that is not in compliance with Section 6.1 of the OWTS Policy, or not able to adequately protect the water quality of the waters of the State. DEHQ shall make a referral to the appropriate RWQCB when an OWTS is identified under this subsection.~~ An owner of a new or replacement OWTS that does not meet the conditions and requirements set forth in this approved LAMP shall notify the appropriate RWQCB **of lack of coverage under the LAMP and** by submitting any required documentation, such as a report of waste discharge (OWTS Policy, Section 2.6). DEHQ shall make a referral to the appropriate RWQCB when an OWTS is identified under this subsection.

~~10.5~~ **10.6** Appeals

A property owner may submit a written request for an appeal on a form provided by DEHQ within fourteen (14) days of the issuance of a Notice of Intent to Record a Notice of Violation issued for a violation of San Diego County Code, Title 6, Division 8, Chapter 3, in accordance with the procedures in Title 1, Division 8, or of notice of an administrative decision by the Department, in accordance with the procedures in Title 1, Division 6.

CHAPTER 11.0 ONSITE WASTEWATER TREATMENT SYSTEMS NEAR IMPAIRED WATER BODIES

11.1 OWTS Policy Requirements

The OWTS Policy Tier 3 provides regulation of OWTS near impaired water bodies. Although local agencies are authorized under the OWTS Policy to implement provisions of Tier 3 to the extent of their local authority, implementation of Tier 3, including notifications and enforcement requirements for OWTS determined to be in Tier 3, is the responsibility of the RWQCBs (OWTS Policy SED, OWTS Policy 4.7).

OWTS Policy Tier 3 is not designed to supersede other state policy ~~created~~ **established** to address impaired water bodies but is designed to provide interim measures until state policy or actions can be adopted or approved to address the impairment.

The OWTS Policy Tier 3 requires an ~~Advanced Protection Management Program (APMP)~~ to address impaired water bodies. The APMP may be implemented through three specific methods (1) an adopted TMDL, (2) special provisions incorporated into an approved LAMP, **if any** or (3) for those water bodies listed in Attachment 2 of the OWTS Policy, in accordance with OWTS Policy Sections 10.8, 10.89, and 10.0.

OWTS near impaired water bodies that are not listed in Attachment 2, and do not have a TMDL, and are not covered by special provisions in an approved LAMP, are not addressed by Tier 3 of the OWTS Policy.

11.2 Adopted TMDL

If there is an adopted TMDL, the geographical area is defined by the TMDL and the APMP is in accordance with the TMDL Implementation Plan. The TMDL implementation plan supersedes all other requirements in Tier 3.

If an impaired water body has an adopted TMDL, property owners of existing, new and replacement OWTS must comply with the TMDL implementation plan requirements. A local agency permitting program may include permitting requirements for these OWTS, if **these requirements fall** within the scope of coverage of the LAMP and local ordinance **authority**. If these OWTS fall outside of a local permitting program, the RWQCB is responsible for implementation **using existing regulatory tools, such as** ~~through the~~ prohibitions, WDRs, or waivers of WDRs.

Some OWTS within a TMDL geographical may not be covered under the scope of coverage of ~~this~~ **the** LAMP and the extent of the local permitting program authority. These include existing OWTS and OWTS with supplemental treatment where ongoing effluent sampling and reporting are required. Owners of OWTS that are not covered under the LAMP or local permitting program or who cannot meet the minimum requirements provided in Section 10.8 will be referred to the RWQCB.

11.2.1 TMDL for the Rainbow Creek Watershed

This TMDL was adopted in 2005 (Resolution R9-2005-0036) and identified the Rainbow Creek watershed as the geographical area to address excessive nutrients in Rainbow Creek contributing to excessive algal growth.

While the TMDL mainly addresses surface runoff sources, the TMDL **also** identified owners of OWTS and agricultural operations as controllable sources via the groundwater pathway. However, since the contribution from the agricultural operations could not be calculated at that time, the entire **calculated** total nitrogen load of 200 kg/year via the groundwater pathway was assigned to the OWTS source.

The TMDL implementation plan included using waste discharge requirements, waivers, or prohibitions as necessary to control the discharges. To address OWTS, the TMDL implementation plan provided to incorporate the regulations developed by the SWRCB, which is **now** the OWTS Policy. The OWTS Policy was adopted into the San Diego Basin Plan by the RWQCB in 2015. As no other provisions for OWTS were ~~adopted~~ **established** as part of the adoption of the OWTS Policy or were including with the adoption of this TMDL, **OWTS discharges are currently regulated by DEHQ under the LAMP and conditional waiver contained in the OWTS Policy, or by the Regional Board under SWRCB Order WQ 2014-0153-DWQ, or individual waste discharge requirements.** ~~the conditional waiver contained in the OWTS Policy remains the effective process for OWTS covered under this policy.~~

11.2.2 TMDL for Indicator Bacteria, Project 1-Twenty Beaches and Creeks in the San Diego Region (including Tecolote Creek)

This TMDL was adopted in 2010 (Resolution R9-2010-0001) to address bacteria with the geographical areas identified as the Pacific Ocean shoreline, Tecolote Creek, Forester Creek, Lower San Diego River, and Chollas Creek.

The TMDL identified the owners of individual OWTS as responsible for controllable nonpoint source bacteria discharges causing or contributing to the impairment at the beaches and creeks but were not assigned a load allocation. Any controllable nonpoint source that has not been assigned a load allocation or has a load allocation of zero is not expected or allowed to discharge a pollutant load as part of the TMDL. The TMDL implementation plan included the use of waste discharge requirements, waivers, or prohibitions to control discharges. As no other provisions for OWTS were adopted as part of this TMDL, the conditional waiver contained in the OWTS Policy remains the effective process for OWTS covered under this policy.

In 2019, the RWQCB issued Investigative Order R9-2019-0014 to existing MS4 dischargers to investigate to identify the sources, pathways and amounts of human fecal material discharges in the Lower San Diego River watershed, including from OWTS. A final report is due in **June** 2024. Based on the results of the Investigative Order, the RWQCB may elect to revise the TMDL and/or existing waste discharge requirements or waivers or adopt new prohibitions or waste discharge requirements or waivers.

OWTS discharges are currently regulated by DEHQ under the LAMP and conditional waiver contained in the OWTS Policy, or by the Regional Board under SWRCB Order WQ 2014-0153-DWQ or individual waste

discharge requirements.

11.2.3 Other Impaired Water Bodies with OWTS Implications

11.2.3.1 Santa Margarita River TMDL Restoration Plan Project

The Santa Margarita River was added to the Clean Water Act section 303(d) Impaired Waters list for nutrients (nitrogen and phosphorus) in 2012. Excessive nutrient loading to the Santa Margarita River and its tributaries causes and/or contributes to exceedances of Water Quality Objectives and adversely impacts the Cold Freshwater Habitat (COLD), Warm Freshwater Habitat (WARM), and Rare, Threatened, or Endangered Species (RARE) beneficial uses designated to the river. Excessive discharge of nutrients also has the potential to adversely impact the Municipal and Domestic Supply (MUN) beneficial use through impact to large groundwater basins in the Santa Margarita watershed (Watershed). Furthermore, nutrients discharged to the surface waters and groundwater in the Watershed have been shown to contribute to the eutrophication impairment of the Santa Margarita River Estuary (Estuary). Major sources of nutrients to the river include Municipal Separate Storm Sewer Systems (MS4s) and agricultural land uses in San Diego and Riverside counties.

To address the impairment, the objective of the Santa Margarita River Water Quality Restoration Plan (River Restoration Plan) is to reduce nutrient loads entering the river and achieve numeric targets in order to restore and maintain the chemical, physical, and biological integrity of the river as well as the downstream Estuary.

To achieve this, the RWQCB is developing the River Restoration Plan to address impairments for nutrients and eutrophication in the river consistent with the Impaired Waters Policy. The River Restoration Plan relies on implementing existing permits, policies, and plans and tracking the effectiveness of the permits, policies, and plans in achieving nutrient load reductions, numeric targets, and beneficial uses through monitoring.

The River Restoration Plan would address the impairment and restore beneficial uses consistent with a 2015 memorandum from the United States Environmental Protection Agency on alternative responses to impaired waters that retain more flexibility and efficiency than the traditional approach to setting total maximum daily loads (TMDLs). Should the San Diego Water Board approve the River Restoration Plan, the County of San Diego and County of Riverside, as stormwater permit Copermittees, and enrollees to agricultural ~~waste discharge requirements (WDRs)~~ in the watershed will be required to track the progress of the River Restoration Plan through monitoring.

Although OWTS discharges to the subsurface have not been directly addressed in the technical reports associated with this project, the RWQCB have indicated that implementation may include additional requirements for owners of OWTS in the area through the local permitting program or LAMP. However, no details have been provided and no formal request for changes to the approved LAMP have been received ~~from~~ by the RWQCB in accordance with the OWTS Policy Section 4.4. The River Restoration Plan process will be monitored to identify additional information as it becomes available.

Currently, as there is no TMDL for this water body, no special provisions in the LAMP, and it is not listed in the OWTS Policy Attachment 2, pursuant to the OWTS Policy, this water body is not addressed by Tier 3 of the OWTS Policy. **OWTS discharges are currently regulated by DEHQ under the LAMP and conditional waiver contained in the OWTS Policy, or by the Regional Board under SWRCB Order WQ 2014-0153-DWQ or individual waste discharge requirements.**

11.2.3.2 Tijuana River TMDL Development Project

The lower six miles of the Tijuana River and the Tijuana River Estuary (the Tijuana River Valley) have been listed as Clean Water Act 303(d) impaired water body segment due to excessive bacteria and trash resulting in the impairment of numerous beneficial uses, primarily those associated with aquatic life (e.g., warm freshwater, estuarine, and marine habitat, rare and endangered species, etc.), and human health (e.g., contact and noncontact water recreation, fishing, shellfishing, etc.). OWTS have been listed as a potential source as part of this listing.

Currently, the San Diego RWQCB is developing a TMDL to address these impaired waters with a draft TMDL staff report undergoing internal review. Following internal review, the staff report will be submitted for external scientific review and will then be available for public review. A Basin Plan amendment will also be prepared for adoption by the San Diego RWQCB and for approval by the State Water Resources Control Board, Office of Administrative Law, and the U.S. Environmental Protection Agency.

No information is available at this time related to any proposed load allocations or requirements for owners of OWTS above that already required in the LAMP. This process will be monitored to identify any additional information as it becomes available.

Currently, as there is no TMDL for this water body, no special provisions in the LAMP, and it is not listed in the OWTS Policy Attachment 2, pursuant to the OWTS Policy, this water body is not addressed by Tier 3 of the OWTS Policy. **OWTS discharges are currently regulated by DEHQ under the LAMP and conditional waiver contained in the OWTS Policy, or by the Regional Board under SWRCB Order WQ 2014-0153-DWQ or individual waste discharge requirements.**

11.3 Special APMP Provisions if Contained in the Approved LAMP

The OWTS Policy provides that if there is no TMDL adopted for the impaired water body, but the local permitting program has elected to include special APMP provisions to address specific OWTS near impaired water bodies in the approved LAMP, then the impairment is addressed through this APMP to the scope and extent provided by these special provisions. The special provisions may be substantive and/or procedural. **Once a TMDL is adopted for the water body, its implementation plan supersedes all Tier 3 requirements, including the special APMP provisions.**

This LAMP does not contain any special provisions specifically related to the siting and design of OWTS near impaired water bodies but implements ~~the~~ **OWTS Policy Tier 2** minimum standards ~~in the LAMP~~ as applied to all new and replacement OWTS **within the extent of authority of local ordinance and the scope**

of coverage of the LAMP. Some OWTS near impaired water bodies may fall outside the scope of coverage of this LAMP or the extent of the local permitting program authority. New and replacement permit applications received by DEHQ for OWTS that fall outside the scope of coverage of this LAMP are referred to the appropriate RWQCB.

11.4 Local Implementation of OWTS Policy Attachment 2 APMP for New and Replacement OWTS

The OWTS Policy provides interim APMP requirements for impaired water bodies listed in Attachment 2 of the policy. As the TMDL program and process is the Federal and State required approach to address impaired water bodies, the Attachment 2 APMP requirements are intended to be interim measures until such time the TMDL is completed.

Currently, there are no water bodies listed in Attachment 2 within the jurisdiction of the local permitting program. Should a water body be listed in a future OWTS Policy update to Attachment 2, DEHQ will implement the Section 10.8 requirements for new and replacement permit applications for OWTS within the scope of coverage of the LAMP and to the extent of local permitting program authority. Owners of OWTS ~~owners~~ that are not covered under the scope of the LAMP or local permitting program or who cannot meet the minimum requirements provided in Section 10.8 will be referred to the appropriate RWQCB.

APPENDIX I

PERCOLATION TEST PROCEDURES

This ~~Chapter~~ Appendix provides the procedures ~~is~~ to be used to establish clear direction and methodology for percolation testing in San Diego County. The objective is to determine the area necessary to properly treat and maintain sewage underground; to size the OWTS with adequate infiltrative ~~on~~ surface area based on an expected hydraulic conductivity of the soil and the rate of loading; and to provide for a system intended to allow for a long-term expectation of satisfactory performance.

All percolation testing for dispersal systems except vertical seepage pits in San Diego County shall be conducted ~~through the use of~~ using the following procedures. The testing shall be performed by or under the direct supervision of a ~~California registered professional engineer, geologist or environmental health specialist (qualified professional)~~ registered with ~~who has attended an orientation session conducted by the Department of Environmental Health (DEHQ) and demonstrated knowledge of San Diego County laws and policies relating to OWTS.~~ Any deviation from these procedures shall be authorized only after receiving written approval ~~by~~ from DEHQ. ~~For testing requirements for horizontal and vertical seepage pits and subsurface drip systems, see the Chapters in this LAMP covering those types of dispersal systems~~ are found in Appendix II.

Note: It is the responsibility of the applicant and property owner to meet any additional requirements and obtain any approvals or permits from other agencies relating to the ~~g~~ Grading or clearing of brush for the purposes of completing a percolation test, including ~~may require approval from Planning and Development Services and requires~~ the implementation of stormwater best management practices.

1.0 Test Holes ~~TEST HOLES~~

1.1 Number of Test Holes

1.1.1 A minimum of ~~four~~ six test holes is required when percolation rates are less than 60 minutes per inch (MPI).

1.1.2 An additional two (2) ~~minimum of six~~ test holes is required when the average percolation rate is more than 60 MPI.

1.1.3 At least four (4) percolation test holes at each deep bed location should be provided to represent soil types within the infiltrative surface area of the deep bed. This testing should represent the entire sidewall depth of the pit.

1.1.4 Additional test holes may be necessary on a site-specific basis for reasons that include, but are not limited to the following:

1.1.4.1 Unacceptable percolations test results, including percolation tests not performed in

accordance with these procedures, or failed tests results or rates that do not meet the minimum rates for the dispersal system proposed for use.

1.1.4.2 Areas of the disposal field requiring defined limits for exclusion.

1.1.4.3 The disposal system is located out of a concentrated area or is larger in areal extent than typical single residential unit installation.

1.1.4.4 Soil conditions are variable or inconsistent.

1.2 Depth of Testing

1.2.1 Test holes shall be representative of the dispersal system installation depth.

1.2.2 Conditions which may require testing deeper than leach line depth:

1.2.2.1 Areas of potential impervious soil layers to a minimum of five feet below bottom of infiltrative surface of dispersal field. ~~Shallow consolidated rock or impervious soil layers.~~

1.2.2.2 Slope exceeds ~~25~~30%.

1.2.2.3 Other factors as might be determined by ~~DEHQ~~ sound geotechnical engineering practices.

1.3 Soil Classification

1.3.1 All test holes and deep borings shall have soil types described according to the American Society for Testing and Materials (ASTM) Soil Classification System (Unified) that describes the relative amount of sand, clay, silt and combinations thereof as defined by the soil textural triangle developed by the United States Department of Agriculture (USDA) as the Soil Survey Manual, Handbook 18.

1.3.2 All borings percolation testing results are to be reported, including any results not meeting minimum rates suitable for OWTS usage and test holes, which encountered groundwater or refusal. ~~Comments about consolidation and friable characteristics are encouraged.~~

1.4 Location and Identification of Percolation Test Holes

1.4.1 Test holes shall be representative of the dispersal area demonstrating site conditions throughout the entire sewage disposal system with equal consideration of primary and reserve leach fields.

1.4.2 ~~Identification of Test Holes~~ Test holes shall be Staked and flagged so the test holes can be located.

1.4.3 Test holes must be identified with all of the following:

1.4.3.1 A test hole number or letter.

1.4.3.2 The depth of the test boring.

1.4.3.3 Lot/parcel number or letter if associated with a subdivision or lot line adjustment.

1.5 Drilling of Borings for Test Holes

1.5.1 Diameter of each test hole shall be a minimum of six (6) inches and a maximum of 12 inches.

1.5.2 If a backhoe excavation is used, a test hole at 12–14 inches in depth shall be excavated into the bottom of the trench so that the testing depth is representative of the type of dispersal system proposed.

1.6 Preparation of Test Holes

1.6.1 The sides and bottom of the holes shall be scarified so as to remove the areas that became smeared by the auger or other tool used to develop the hole.

1.6.2 All loose material should be removed from the hole.

1.6.3 Add two (2) inches of pea gravel to bottom of hole. A secure/stable perforated pipe may be used in the test hole.

1.6.4 Identify and mark a fixed reference point for a consistent measuring point.

2.0 Presoaking the Test Holes~~PRESOAKING THE TEST HOLES~~

2.1 Procedure

2.1.1 Carefully fill the test hole with 12-14 inches of clear water.

2.1.2 **Four-Hour Presoak:** Maintain 12-14 inches of clear water for a minimum of four (4) hours. After four hours, allow the water column to drop overnight. (Testing must be done within 15-30 hours after the initial four-hour presoak).

2.1.3 **Overnight Option Pre-Soak:** If clay soils are present, it is recommended to maintain the 12–14-inches of water shall be maintained overnight. A siphon can be used to maintain the supply at a constant level.

2.1.4 **Sandy Soils Modified Presoak:** In highly permeable sandy soils with no clay and/or silt, the presoak procedure may be modified. If, after filling the hole twice with 12-14 inches of clear water,

the water seeps completely away in less than 30 minutes, proceed immediately to Case 2, **Section 3.3.3 Item 3** and refill to **six (6)** inches above the pea gravel. If the test is done the following day, a presoak will be necessary for at least an hour in order to reestablish a wetted boundary.

2.2 Saturation and Swelling

2.2.1 Saturation means that the void spaces between soil particles are full of water. This can be accomplished in a short period of time.

2.2.2 Swelling is caused by the intrusion of water into the individual soil particles **that** are full of water. This is a slow process, especially in clay-type soil and is the reason for requiring a prolonged soaking.

Use of Inserts

- ~~1. If sidewalls are not stable or sloughing results in changing depth, the test hole may be abandoned or retested after means are taken to shore up the sides. The holes shall be re-cleaned prior to resuming the test.~~
- ~~2. Options for shoring or maintaining test hole stability:
 - a. Hardware cloth (1/8 inch grid)
 - b. Perforated pipe or containers
 - c. Gravel pack (NOTE: A correction factor is necessary if a gravel pack is used. Show all calculations on the test report.)~~

3.0 **Determination of Percolation Rates**~~DETERMINATION OF PERCOLATION RATES~~

3.1 Determine Case Procedure. ~~Depending on t~~The soil type and permeability, and the results of the presoak, ~~variations in~~**will determine** the procedures **to be** used for determining percolation rates ~~can be allowed~~. Testing shall proceed **following the presoak** based on the conditions outlined in the following cases.

Case 1 – Water remains overnight in the test hole ~~following the four hour (Unless an overnight siphon is used.)~~

Case 2 – No water remains in test hole: Soil with a fast percolation rate is ~~encountered~~ **expected based on a test** where two columns of 12-14 inches of water **each completely** percolates **into the soil** in less than 30 minutes ~~for each column during the presoak~~.

Case 3 – No water remains in the test hole: 15-30 hours after the four-hour presoak **and soil is not expected to show a fast percolation rate as determined using the two columns of 12-14 inches of water test, as indicated in Case 2.**

3.2 Case 1 Procedure **-Water Remains in Test Hole**

3.2.1 Begin test 15-30 hours after presoak. Clean out the silt and mud and add 2 inches of pea gravel to the test hole.

3.2.2 Adjust depth of water to six (6) inches in the hole.

3.2.3 From a fixed reference point, take two (2) readings at thirty (30) minute intervals and report the percolation rate as the slower of the two readings.

NOTE: When a minimum amount of water remains due to a damaged hole or silting, the hole may be cleaned out and tested under Case 3, starting with the presoak.

3.3 Case 2 Procedure - No Water Remains in Test Hole-Fast Percolating Soils

3.3.1 Begin test 15-30 hours after presoak if proceeding the next day. Clean out the silt and mud and add two (2) inches of pea gravel.

3.3.2 Fill the hole twice with 12-14 inches of water. Observe to see if each column of water seeps away in less than 30 minutes. If so, proceed with the percolation test. If not, go to Case 3. If the test is done the following day, a presoak will be necessary for at least an hour in order to reestablish a wetted boundary.

3.3.3.1 Refill hole to 6 inches above the pea gravel bottom.

3.3.4.2 Measure from a fixed reference point at ten (10)-minute intervals over a period of one (1) hour to the nearest 1/16th inch. Add water to maintain six (6) inches of water at each 10-minute time interval.

3.3.5.3 Continue 10-minute readings as long as necessary to obtain a "stabilized" rate with the last two water drop rate readings not varying more than 1/16th inch or for a duration of four (4) hours.

3.3.6 The last water level drop is used to calculate the percolation rate.

~~5. The last water level drop or the water level drop that represents the slowest rate will be used to considered in the percolation rate.~~

3.4 Case 3 Procedure - No Water Remains in Test Hole

3.4.1 Begin test 15-30 hours after presoak.

3.4.2 Clean out the silt and mud and add two (2) inches of 3/8 inch pea gravel.

3.4.3 Maintain water level at six (6) inches above pea gravel for one (1) hour to reestablish a wetted boundary. It is not necessary to record data points for the first hour as this is an adjustment period. Then proceed with the four (4) hour test.

3.4.4 Adjust the water depth to six (6) inches above the pea gravel buffer and measure from a fixed reference point at 30-minute intervals to the nearest 1/16th inch. ~~NOTE: It is not necessary to record data points for the first hour as this is an adjustment period and a reestablishment of a wetted boundary.~~

3.4.5 Refill the hole as necessary between readings to maintain a six (6) inch column of water over the pea gravel. If a fall of one (1) inch or less is recorded, the test can continue without refilling until the next 30-minute reading interval.

3.4.6 Continue recording readings at 30-minute intervals for a minimum of four (4) hours.

3.4.7 The last water level drop is used to calculate the percolation rate.

4.0 Calculations and Measurements ~~CALCULATIONS AND MEASUREMENTS~~

4.1 The percolation rate is reported in minutes per inch (MPI).

4.2 Calculation Example:

For example, a test result of a ¾ th inch fall over a 30-minute time interval with a ¾ th inch fall would be calculated as follows:
30 minutes divided by ¾ inch = 40 minutes per inch (mpi) MPI (30 minutes times 4 = 120, then 120 divided by 3 = 40)
Or Convert fraction to decimal equivalent: 3 divided by 4 = 0.75 inch, then 30 minutes divided by 0.75 inch = 40 MPI

4.3 Measurement Principles

4.3.1 The documented time interval for readings are to reflect must be the actual times and are to be maintained as near as possible to the intervals outlined for the test: (either 10- or 30-minutes intervals).

4.3.2 The accuracy and precision of the water level drop measurement and reading is important to ensure the dispersal field sizing is appropriate for the site, as the leach line requirement may differ greatly with only a small change in percolation rate.

4.3.3 The water drop should be measured to the nearest 1/16th of an inch. Measurements that fall within a 1/16th of an inch interval shall be recorded to the 1/16th of an inch that represents the slower

rate. For example, a reading over a 30-minute interval observed between 3/8th inch (80 MPI) and 7/16th inch (69 MPI) is recorded as 3/8th inch, the slower of the two rates.

~~Measurements to the nearest 1/16th inch should be adjusted to the slowest rate, e.g., a reading observed between 3/8 inch and 5/16 inch (80 mpi and 96 mpi) would be reported as 96 mpi. Round to the Lower 1/8th inch.~~

~~4.2.34.3.4 Measurements on an engineering scale using a decimal inch rule (tenths of an inch) should be to the nearest tenth of an inch increment as can be accurately read. follow the same principle, e.g., a reading observed between 0.4 inch and 0.3 inch (75 mpi and 100 mpi) would be reported as 100 mpi. Round to the lower .1 inch.~~

~~4.2.44.3.5 Measurements, Special Considerations~~ Measurement from a fixed reference point shall be from a marked section on the perforated pipe or on the platform that is stable and represents the center of the test hole.

~~4.2.54.3.6 A percometer or other approved water measuring device is encouraged and required~~ when the depth of a test hole is greater than 60 inches in depth. Accurate measurement is vital as the percolation rate calculated from these measurements will determine the required absorption area. ~~and in cases For test holes of testing~~ deeper than 60 inches, the report shall include a description of the measurement method and how the borings were cleaned out and prepared for testing.

4.3.7 Adjustment Factor for Gravel Packed Percolation Test Holes. The following shall be used to correct for the void space difference when a test hole is used completed with a full gravel pack.

4.3.7.1 Equation Inputs

Cross-Section Area of Test Hole	$A_H = .25 \pi D_H^2$
Cross-Section Area of Pipe	$A_P = .25 \pi D_P^2$
Cross-Section Area of Gravel Pack	$A_G = A_H - A_P$
Drainable Voids in Gravel Pack	$= n (A_G) *$
Total Voids	$A_P + n (A_G) = A_P + n (A_H - A_P)$
* A test should be run on the actual rock used to establish the Void Ratio (n).	

4.3.7.2 Adjustment Factor (AF) Calculation:

$$AF = \frac{A_H}{A_P + n (A_H - A_P)}$$

$$AF = \frac{.25 \pi D_H^2}{.25 \pi D_P^2 + n (.25 \pi D_H^2 - .25 \pi D_P^2)}$$

$$AF = \frac{D_H^2}{D_P^2 + n (D_H^2 - D_P^2)}$$

4.3.7.3 Application: Adjusted Percolation Rate = MPI x AF

4.3.7.4 Typical Values:

4.3.7.4.1 For n = 0.35

4.3.7.4.2 Adjustment Factors:

Pipe Diameter	Hole Diameter	Adjustment Factor
4"	6"	1.57
4"	8"	1.95
4"	10"	2.20

~~Correction Factors~~

~~Void factor for gravel pack:~~

5.0 ~~Reports~~ **REPORTS**

5.1 Report Requirements

5.1.1 All test data, **field notes**, and required information shall be submitted on approved DEHQ forms with appended data or information as needed. ~~A minimum of three copies is required.~~

5.1.2 Reports shall be signed with an original signature by the consultant who either performed or supervised the testing.

5.1.3 San Diego County Code, Section 68.328 requires all percolation testing to be done by a civil engineer, geologist, or environmental health specialist, registered in the State of California. These qualified professionals ~~s~~ are required to be ~~on an approved list on file~~ **registered** with DEHQ.

5.1.4 The percolation test is only one critical factor in siting an OWTS. Site considerations may require special evaluation by a qualified professional to technically address issues such as high groundwater, steep slope, nitrate impacts, cumulative impacts, (mounding, and horizontal transmissibility).

5.1.5 Qualified professionals who employ technicians are responsible for the work performed by the technician. It is incumbent upon the qualified professional to properly train, equip, and supervise

anyone performing work under his or her direction and license.

6.0 ~~Adjustment Factor for Gravel Packed Percolation Test Holes~~

6.1 ~~Calculations~~

X Section Area of Test Hole	$A_H = .25 \pi D_H^2$
X Section Area of Pipe	$A_P = .25 \pi D_P^2$
X Section Area of Gravel Pack	$A_G = A_H - A_P$
Drainable Voids in Gravel Pack	$= n (A_G) *$
Total Voids	$A_P + n (A_G) = A_P + n (A_H - A_P)$
* A test should be run on the actual rock used to establish the Void Ratio (n).	

6.2 ~~Adjustment Factor (AF) Calculation:~~

$$AF = \frac{A_H}{A_P + n (A_H - A_P)}$$

$$AF = \frac{.25 \pi D_H^2}{.25 \pi D_P^2 + n (.25 \pi D_H^2 - .25 \pi D_P^2)}$$

$$AF = \frac{D_H^2}{D_P^2 + n (D_H^2 - D_P^2)}$$

6.3 ~~Application~~

6.3.1 ~~Adjusted Percolation Rate = MPI x AF~~

6.4 ~~Typical Values~~

Pipe Diameter	Hole Diameter	Adjustment Factor
4"	6"	1.57
4"	8"	1.95
4"	10"	2.20
4"	12"	2.37

Standard and Requirements for Design and Installation of OWTS in Soils with Poor Percolations Rates

In soils having percolation rates in excess of 60 mpi, the following criteria shall apply:

- ~~1. Percolation tests shall be conducted at a minimum of six (6) different locations on the site within the proposed area of the subsurface sewage disposal field.~~
- ~~2. There shall be a minimum of ten (10) feet of soil above any impervious formation such as rock, clay, adobe and/or water table. Fractured rock and consolidated granites will not be considered as soil. Deep testing can be required to ensure uniform conditions exist below the disposal field.~~

~~Vertical Seepage Pit Dispersal Systems Percolation Test Procedures~~

~~All vertical seepage pits for new construction will require percolation testing by a qualified professional certified to perform percolation tests in San Diego County. A waiver of testing can be considered where adequate information exists as to soil types, depth and permeability. Percolation testing for vertical seepage pits shall be completed per the following guidelines.~~

- ~~1. A 12 to 48 inch diameter test hole shall be excavated to a depth of at least 10 feet deeper than the proposed installation depth.~~
- ~~2. A minimum 10 foot separation between the bottom of the vertical seepage pit and the anticipated high groundwater level is required.~~
- ~~3. Boring logs shall be recorded and included with all test reports indicating soil strata depths and types and visual classification according to the unified soil classification system along with any groundwater encountered.~~
- ~~4. The overdrill must be checked for the presence of groundwater a minimum of 24 hours after the completion of the test boring to allow time for groundwater to stabilize in the hole. After the groundwater reading is recorded, the test hole shall be backfilled to a depth 10 feet above the bottom of the test hole or the groundwater level whichever is shallower.~~
- ~~5. The pit shall be filled with water to the cap depth and a continuous pre-soak shall be maintained at the proposed cap level for a minimum 8-hour period. In highly permeable soils when cap levels cannot be maintained during pre-soak, the test shall be conducted at a depth no higher than the pre-soak level which was attained. Document the pre-soak attempt with gallons of water used. In no case shall less than 5,000 gallons of water be used within a 1-hour period in the attempted pre-soak when the cap level cannot be maintained. The depth of the test shall be noted on the boring log and in no case shall the sidewall of permeable soil below the cap level be less than ten feet.~~
- ~~6. Upon completion of the pre-soak period, fill the pit to cap level and determine uniformity of soil by measuring the falling head. Distance to the water level shall be measured at 15 minute intervals, or more frequently if needed, until the drop stops or the pit empties. A graph of the drop in water level shall be attached to all proposals submitted by the qualified professional. If non-uniform rates persist, the soil will not be considered uniform and the tests discontinued as they will not be approved by DEH.~~
- ~~7. If the procedure in Item no. 4 demonstrates uniform soil, proceed with a two-hour static head or falling head capacity test.~~

- a. ~~Static Head~~ — The pit shall be filled with water to the cap depth and the water column shall be maintained at that level for two hours. The amount of water added to maintain this level must be documented. The 24-hour capacity is determined by multiplying by 12. Adjustment to a four foot diameter pit is made if a lesser size test hole is used.
 - b. ~~Falling Head~~ — The pit shall be filled with water to the cap depth and the column of water shall be allowed to drop for a two hour period. The distance dropped shall be measured and the amount of water absorbed determined. This amount is multiplied by 12 to determine the 24-hour capacity. Adjustment to a four foot diameter pit is made if a lesser size test hole is used.
8. ~~The minimum capacity for a new OWTS using vertical seepage pits as the dispersal system shall be 5 times the volume of the required septic tank or 5000 gallons per day whichever is greater. All individual vertical seepage pit shall have a minimum capacity of 1,667 gallons per day.~~
9. ~~Each pit must meet these minimum criteria to be acceptable. The qualified professional may include safety factors as he feels the situation warrants.~~
10. ~~It shall be the responsibility of the qualified professional to maintain all test holes or pits in a safe manner prior to backfill or capping to prevent a hazard or accident.~~

APPENDIX II

VERTICAL SEEPAGE PIT DISPERSAL SYSTEMS

PERCOLATION TEST PROCEDURES

1.0 All vertical seepage pits for new construction will require percolation testing by a qualified professional certified to perform percolation tests in San Diego County. **Testing requirements may be amended** ~~A waiver of testing can be considered~~ where adequate information exists as to soil types, depth and permeability. Percolation testing for vertical seepage pits shall be completed per the following ~~guidelines~~ **procedures**.

2.0 A 12 to 48-inch diameter test hole shall be excavated to a depth of at least 10 feet deeper than the proposed installation depth.

3.0 A minimum 10-foot separation between the bottom of the vertical seepage pit and the anticipated high groundwater level is required.

4.0 Boring logs shall be recorded and included with all test reports indicating soil strata depths and types and visual classification according to the USDA *Soil Survey Manual, Handbook 18* ~~unified soil classification system~~ along with any groundwater encountered.

5.0 **Groundwater Check:** The ~~overdrill~~ **test hole** must be checked for the presence of groundwater a minimum of 24-hours after the completion of the test boring to allow time for groundwater to stabilize in the hole, **if present. A 24-hour notice shall be provided to DEHQ to schedule an inspection of the groundwater reading.** After the groundwater reading is recorded, the test hole shall be backfilled to a depth 10 feet above the bottom of the test hole or to the groundwater level whichever is shallower.

6.0 **Pre-Soak:** The pit shall be filled with water to the **proposed** cap depth and a continuous pre-soak shall be maintained at the proposed cap **depth** level for a minimum 8-hour period. In highly permeable soils where ~~cap~~ **cap** levels cannot be maintained during pre-soak, the test shall be conducted at a depth no higher than the **actual** pre-soak level ~~which that was attained~~ **observed and documented during the pre-soak**. Document the pre-soak attempt with gallons of water used. In no case shall less than 5,000 gallons of water be used within a 1-hour period in the attempted pre-soak when the cap level cannot be maintained. The depth of the test **hole** shall be noted on the boring log and in no case shall the sidewall of permeable soil below the cap level be less than ten ~~feet~~ **feet**.

7.0 **Soils Uniformity Test:** Upon completion of the pre-soak period, fill the pit to cap level and determine uniformity of soil by measuring the falling head. Distance to the water level shall be measured at 15-minute intervals, or more frequently if needed, until the drop stops or the pit empties. A graph of the drop in water level shall be attached to all proposals submitted by the qualified professional. **If the graph shows non-uniform or varying rates in water level drop over time persist,** the soil will ~~not~~ be considered **non-uniform and not suitable for vertical seepage pit dispersal**

system usage, and the tests discontinued as they will not be approved by DEH.

8.0 Absorption Capacity Test: If the procedure in ~~Item No. 4~~ **Section 7.0** demonstrates uniform soil, proceed with a two-hour static head or falling head capacity test.

8.1 Static Head – The pit shall be filled with water to the cap depth and the water column shall be maintained at that level for two hours. The amount of water added to maintain this level must be documented. The 24-hour capacity is determined by multiplying by 12. Adjustment to a four-foot diameter pit is made if a lesser size test hole is used.

8.2 Falling Head – The pit shall be filled with water to the cap depth and the column of water shall be allowed to drop for a two-hour period. The distance dropped shall be measured and the amount of water absorbed determined. This amount is multiplied by 12 to determine the 24-hour capacity. Adjustment to a four-foot diameter pit is made if a lesser size test hole is used.

9.0 The minimum **absorptive** capacity for a new OWTS using vertical seepage pits as the dispersal system shall be five (5) times the volume of the required septic tank or 5000 gallons per day whichever is greater. All individual vertical seepage pits shall have a minimum capacity of 1,667 gallons per day.

~~9. Each pit must meet these minimum criteria to be acceptable. The qualified professional may include safety factors as he feels the situation warrants.~~

10.0 It shall be the responsibility of the **property owner and** qualified professional to maintain all test holes or pits in a safe manner prior to backfill or capping to prevent a hazard or accident.

~~NITRATE LOADING STUDY MINIMUM REQUIREMENTS~~

~~1.0 — Project Description~~

~~1.1 — Description of the site and proposed project. This description should be consistent with the project description as provided in OWTS Layout and in any planning project application.~~

~~1.2 — Description and location of special environmental conditions or features (303(d) water bodies impaired for nitrogen or nutrients within 600 feet of project location, shallow water table areas, TMDL geographical areas, Basin Plan exception areas, areas of known groundwater surface water interface, areas where groundwater is the source of drinking water, drinking water reservoirs, etc.)~~

~~2.0 — Wastewater Characteristics~~

~~2.1 — Description of the wastewater constituents to include at a minimum TSS, BOD, FOG, and nitrogen as nitrate. If any wastewater is being diverted from the OWTS, as with the use of conservation fixtures or gray water systems, a discussion of the impacts to the constituents in the wastewater must be provided (more concentrated). Discussion of reduction of constituents based on any proposed supplemental treatment in accordance with the specific certification or design.~~

~~2.2 — Description of projected daily flow and peak daily flow based on the proposed use and occupancy.~~

~~3.0 — Chemical and Physical Properties of Soil~~

~~3.1 — Description of the chemical and physical properties of the soils underlying the dispersal fields in relation to the potential for mass transport of nitrogen.~~

~~3.2 — Description and location of potential conduits for pollution or contamination to migrate through subsurface (abandoned wells, stormwater features, former stream bed locations, etc.)~~

~~4.0 — Groundwater Analysis~~

~~4.1 — Description of: Depth to groundwater, seasonal fluctuations of groundwater depth, general directional flow and gradient.~~

~~4.2 — Description of the current use of groundwater in the general location of the site, at the site, and of the future expected use of groundwater as related to the proposed project.~~

~~5.0 — Denitrification/Nitrogen Reduction~~

~~5.1 — Discussion of the potential for denitrification: denitrification in the soil column as the effluent percolates to the water table.~~

~~5.2 — Description of any nitrogen uptake by plants or other vegetation, if any, in the discharge area.~~

6.0 — Cumulative Impacts

~~6.1 — Description of OWTS density within the township range section.~~

~~6.2 — Description of all current off-site sources of nitrogen to soil and groundwater in the general up gradient area and the contribution of nitrogen load from these sources to the project site.~~

~~6.3 — Description of all current on-site sources of nitrogen to soil and groundwater and the contribution of nitrogen load from these sources.~~

~~6.4 — Description of all proposed future on-site sources of nitrogen to soil and groundwater and the contribution of nitrogen load from these sources.~~

~~6.5 — Discussion of cumulative impact from all sources.~~

7.0 — The Hantzsche and Finnemore Mass Balance Equation

$$\frac{I * Nw(1-d) + RNb}{(I + R)}$$

$$I = \frac{Q * 365 * 12}{A * 7.48 * 43560}$$

Q = Volume of wastewater generated, in gallons per day.

Nw = Total nitrogen concentration of wastewater from development, in milligrams per liter.

A = Parcel size, in acres.

d = Fraction of nitrate nitrogen loss due to denitrification in the soil.

R = Average recharge rate of rainfall, in inches per year.

Nb = Background nitrate nitrogen concentration of rainfall recharge at the water table, exclusive of waste water influences, in milligrams per liter.

I = Volume rate of wastewater entering the soil averaged over the gross developed area for dwelling unit, in inches per year.

7.1 — Standardized Equation Inputs:

~~7.1.1 Volume of wastewater generated (Q), in gallons per day. Use 150 gpd per bedroom for residential, use actual flows from similar use, or Table 6.5-1 for commercial. Maximum flows allowed under LAMP is 3,500 gallons per day.~~

~~7.1.2 Total nitrogen concentration of wastewater from project, in mg/L (Nw). Use 45 mg/L for residential use and occupancies. For residential uses up to 1,500 gallons per day flow proposing an~~

~~NSF/ANSI 245 certified residential OWTS with Supplemental Treatment, the value of 22.5 mg/L may be used (50% reduction). The specific proposed certified OWTS with Supplemental Treatment must be appropriate for the project description and flows. For commercial uses, the concentration must be derived from actual sample results from a similar commercial use or calculated from fixture contributions using data provided in the EPA Design Manual or other approved source. As the NSF/ANSI 245 certification is limited to residential uses up to 1,500 gallons per day, any OWTS with supplemental treatment proposed for commercial uses must be designed by a qualified professional to address the specific wastewater characteristics and flows from the proposed use. The calculated total nitrogen in the treated effluent identified in this design may be used for this input parameter.~~

~~7.1.3 Parcel size (A), in acres. Use actual size of project.~~

~~7.1.4 Fraction of nitrate-nitrogen loss due to denitrification (d) in the soil. Range 0% to 30%. Use 0 for coarse sand or perc rates <1 MPI. Use 0.1 for sandy soils; 0.2 for sandy loam, loam, silt loam; use 0.3 for clay loam, clay or silty clay soils.~~

~~7.1.5 Average recharge rate of rainfall (R), in inches per year. Use last 10 year average or historic from San Diego County Flood Control District source. May use another source (CIMIS, Western States) if no other data is available. Average recharge rate of rainfall is rainfall less surface runoff and loss from evapotranspiration.~~

~~7.1.6 Background nitrate-nitrogen concentration of rainfall recharge (Nb) at the water table, in mg/L. Use 1.0 mg/L.~~

~~7.1.7 Volume rate of wastewater entering the soil (I) averaged over the gross developed area, in inches per year. Must include all sources of wastewater on parcel.~~

~~7.2 Resultant average concentration (Nr) — result must be less than the applicable WQO for that area: MUN 10 mg/L (Warner Springs 5 mg/L), Rainbow Creek Watershed 1 mg/L receiving water limit??, SMR Watershed??~~

~~7.3 Other site-specific considerations:~~

~~A discussion of other site specific conditions which may have an impact on the nitrate loading from the OWTS should be provided. Conditions may include the following:~~

~~Within 1 mile of an area of localized recharge
Within 1 mile of gaining stream gw/sw interface
Nearby 303d waters, TMDL geographical areas, reservoirs
Seasonal high depth to groundwater/
Thickness (in feet) of vadose zone
Total thickness (in feet) of clay in vadose zone and % clay in vadose zone
Number of OWTS in 1 mile radius of site, location and distance of municipal supply wells
Nitrate concentration in onsite well, nearby water wells, public well
Other sources of nitrate in 1 mile radius~~

~~Number of water wells in 1-mile radius of site, or service by public water system~~
~~Date of water well installations at the site, in area~~
~~Depth of wells, depth of grout seals~~
~~Within a city Sphere of Influence~~
~~Number of sewer homes within 1-mile radius of site~~

~~8.0 — Conclusions and Recommendations~~

~~8.1 — Discussion of Nitrogen Loading Results Relative to Proposed Method of Wastewater Disposal~~

~~8.2 — Discussion of total nitrogen impact from proposed project.~~

~~8.6 — Discussion of any mitigating site conditions: See above ...Effects of dilution of the effluent along the flow path to the surface water body.~~

~~8.3 — Description of the methods proposed to mitigate any known or future impact to soil and groundwater from nitrogen at or around the project site.~~

~~8.4 — Report signed, dated and stamped by qualified professional.~~

~~8.5 — Attachments/appendices contain sufficient documentation and data, with sources, to support claims/conclusions made in report.~~

APPENDIX III

OWTS PERFORMANCE EVALUTION GUIDELINES

1.0 Introduction

1.1 The purpose of these evaluations is to determine, on an individual basis, whether existing OWTS are functional and meet minimum standards of performance established by this LAMP. These guidelines set forth procedures for conducting performance evaluations, to assure consistency and thoroughness in verifying the functioning status of existing septic systems that have been identified as a potential source of surfacing effluent or sewage.

1.2 The following performance criteria are established as minimum requirements:

1.2.1 There is no surfacing effluent at any time.

1.2.2 There is always positive flow from the leachfield and from the septic tank with no backup to the tank or house plumbing during high groundwater conditions.

1.2.3 There is an adequately sized septic tank for the structure(s) being served and it is serviceable – e.g., access riser for maintenance. The septic tank must be watertight and constructed of approved materials.

2.0 Inspection Responsibility

2.1 The inspection shall be performed by a Qualified Professional with competency in the operation and maintenance of OWTS and trained in these procedures.

3.0 Background Data

3.1 Prior to conducting the field performance inspection all background information pertaining to the property and septic system shall be compiled and reviewed. This should include permit information, site plan, “As Built” drawings, prior inspection results, PUMPING RECORDS, etc.

3.2 The site plan must show the location of the septic tank and leachfield, the location of all buildings, decks, cutbanks, creeks, wells, reserve area, direction and percentage of slope, and any other conditions that may affect the operation of the OWTS. If a reserve area is not specified or if any proposed construction encroaches upon it, a new reserve area must be identified. The evaluation must address system performance during wet weather conditions as well as dry weather conditions.

4.0 Initial Site Evaluation

4.1 The property should be walked to confirm the location of the septic tank, leach field and other pertinent features of the OWTS. In verifying leach field location, the length of each line and depth of

drainpipe (below ground surface) shall also be determined. This may require probing with a metal rod or actual excavation.

4.2 The septic tank and leach field areas should be checked for any obvious signs of existing system problems such as surfacing effluent, odors, graywater bypasses, lush vegetation in leach field area, or any other condition that may suggest an existing or impending problem. If the dispersal system has dual leach fields, the diversion valve shall be located and checked to see if it is functional and to determine which leach field is in service. All observations, including both problem conditions and no problem conditions, must be noted.

4.3 A hand-augured boring (3-inch minimum) shall also be installed alongside (but not within) the leach field area for observation of groundwater conditions. An initial reading to depth to water from ground surface shall be taken at the time of boring installation. The boring shall be left open for the remainder of the performance inspection to allow for the water level to stabilize for a minimum of 1-hour and for a final reading to be taken. The boring shall be backfilled before leaving the site.

5.0 Septic Tank Inspection

5.1 Access Risers: the septic tank shall be located and determine if permanent access risers have been installed. If the tank is equipped with risers, observe and document the general condition. Ideally, the risers should be properly grouted to the top of the septic tank to prevent groundwater and/or surface water intrusion. The lids of the risers should also be properly sealed to prevent odors and the entry of insects. Any observed defects in the access risers shall be noted. Where there are no access risers installed, the property owner shall be provided information about access risers and may be required to install them.

5.2 The septic tank lids shall be carefully removed with as little disturbance to the existing landscape as possible. Once the tank is open, the structural condition of the septic tank, including signs of cracking, flaking, and pitting, or other structural defects, shall be observed and documented. A steel rod may be used to probe the walls and bottom of the tank. The pumping of the tank may not be necessary to perform this procedure. The inlet and outlet sanitary tees must also be inspected to assure they are in satisfactory condition, properly positioned, and free of scum accumulation, rocks, root matter or other obstructions. The condition should be noted. Additional testing to verify the structural integrity of the septic tank may be required.

5.3 The liquid level in the tank should be measured with respect to the outlet pipe. In a properly functioning system, the level in the tank should be even with the invert (i.e., bottom of the outlet pipe-see Figure 1). If the liquid level is below the outlet pipe, the tank is leaking. If the liquid level is above the outlet pipe, the leach field is either flooded or obstructed. The depth of the wastewater above or below the outlet pipe must be measured and noted.

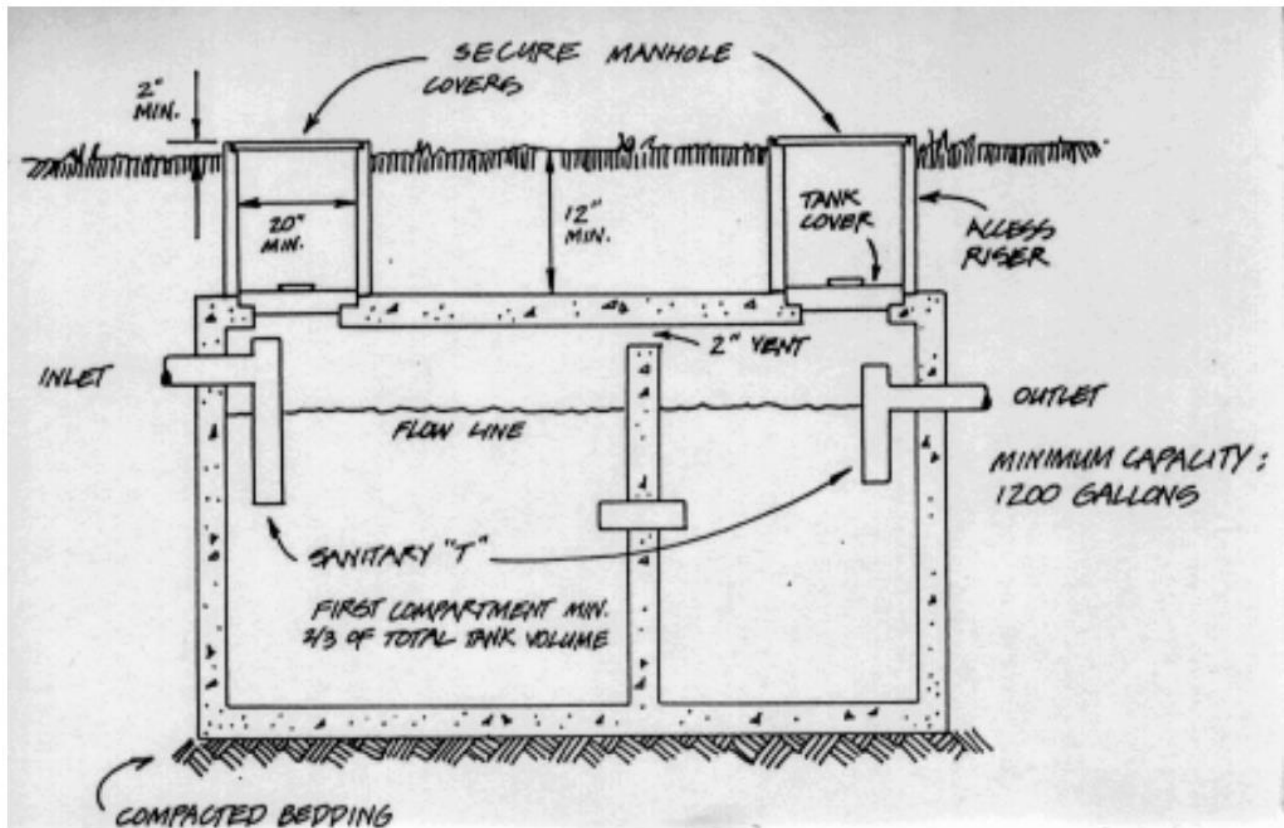


Figure 1

5.4 The capacity of the septic tank (in gallons) shall be determined from measurements of the width, length and depth (below outlet pipe) of the tank. The capacity shall then be compared with the water use/wastewater flow rates for the property. A minimum septic capacity of 3 times the maximum expected daily wastewater flow is considered adequate for an existing OWTS. Additional septic tank capacity may be required.

6.0 Hydraulic Load Test-Septic Tank

6.1 General: This test is conducted only for standard gravity-fed leach fields and does not apply if the OWTS uses a pump. A separate test is conducted for pump systems. The hydraulic load test is conducted by surcharging the septic tank with about 150 gallons of water over a 20–30-minute period, then observing the rise of water in the tank and the subsequent draining process. Tracer dye may be used to assist in observing leachfield failure. A garden hose discharging into the outlet side of the tank can be used to surcharge the tank. The hose outlet should remain well above the wastewater level in the tank to prevent cross contamination. Before starting the test, the flow rate from the hose should be determined (i.e., with a 5-gallon bucket and stopwatch) to properly gauge the amount of surcharge water added to the tank. Alternatively, a portable water meter can be installed between the house faucet and the hose to directly measure the water volume added.

6.2 Test Procedures: The step-by-step procedures for the hydraulic load test are provided below:

6.2.1 Measure the location of the static wastewater line in the septic tank at the outlet side as an initial reference point.

6.2.2 Begin surcharging the tank with water to start the hydraulic load test. Note the time of the start of the surcharge.

6.2.3 Observe and record the time and any rise in the liquid level at the outlet pipe reference point during the filling and the time and the wastewater level at the end of the filling. Typically, the liquid level will rise from 0.5 to 1-inch, at which point the liquid level should stabilize for the remainder of the filling, then return to the initial level in a matter of minutes after filling is stopped.

6.2.4 After the filling cycle is finished, the wastewater level decline in the septic tank is observed until the initial level is reached, and the time to achieve this is recorded. If the initial level is not attained within 30-minutes, the test is terminated, and the final wastewater level is noted.

7.0 System Rating

Based on the water level reading during the test, a hydraulic performance rating is assigned to the system in accordance with guidelines provided in Table 1.

Table 1: Hydraulic Load Test Rating Guidelines	
Rating	Septic Tank Response to Hydraulic Loading
Excellent	No noticeable rise in water level during filling.
Good	Maximum water level rise of about 1-inch, with rapid decline to initial level within 5-minutes after end of filling.
Satisfactory	Maximum water level rise of about 2-inches, with decline to initial level within about 15-minutes after end of filling.
Marginal	Water level rise of more than 3-inches, with decline to initial level within about 30-minutes after end of filling.
Poor	Water level rise of more than 3-inches, with decline not reaching initial level within 30-minutes after end of filling
Failed	Water level rise of more than 3-inches, with no noticeable decline within 30-minutes after end of filling.

8.0 Hydraulic Load Test-Pump Systems

8.1 General: For systems equipped with an effluent pump, the following inspection procedures are followed. Figure 2 provides a diagram of a typical pump system installation for reference. Remove the pump access cover and tank lid, taking care that no soil or other material enters the tank. Note any signs of scum or sludge buildup, indications of previous pump failure (such as scum line above the highwater

alarm switch), or evidence of soil or roots entering the tank. Also, inspect the float controls to see that they have free movement, and check the electrical junction box (if located in the tank or access riser) for any obvious signs of corrosion. If the water level in the basin is normal (i.e., between the high and low water controls) proceed with testing of the pump systems.

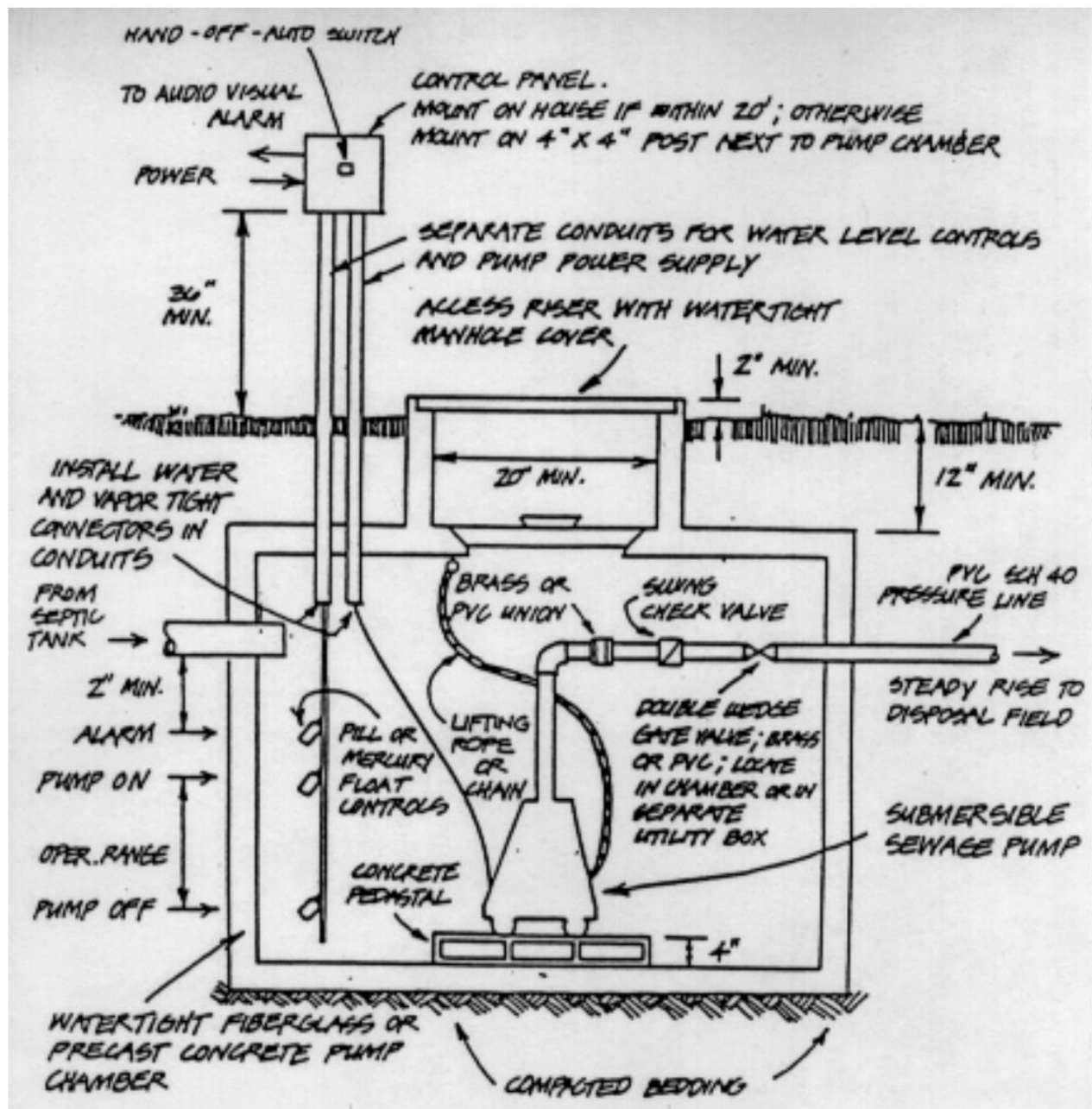


Figure 2

8.2 Test Procedures: The step-by-step procedures for the hydraulic load test are provided below:

8.2.1 The pump test is conducted by adding sufficient water to the tank to activate the pump "on"

control and observing the performance of the system over at least one pumping cycle. The total amount of water added should be about 150 gallons, to approximate the same hydraulic loading of the leachfield as for gravity systems. Using a garden hose, the water may be added to the outlet side of the septic tank, or directly to the pump tank. If filling the pump tank directly, care should be taken to minimize turbulence and disturbance of sediment or sludge that may have collected in the basin. This can be best accomplished by directing the stream of water against the interior side of the chamber, rather than directly toward the bottom of the pump chamber.

8.2.2 Observe the filling of the tank, measure and document the point at which the pump is activated. Immediately stop the filling operation and observe the pumping cycle until the pump shuts-off. While the pump is discharging, examine the piping system for any leaks. Even small leaks could be a forewarning of possible breaks in the pressure line at some point in the future; and these should be corrected as soon as possible. Measure and document the depth at which the pump shuts-off and calculate the volume of water between the “on” and “off” measurements. Compare this does with the design dose volume specified for the system. If the dose is too high or too low, float controls should be repaired/ adjusted by a licensed and properly qualified contractor. DEHQ may issue a notice for corrective action.

The pumping cycle (from “on” to “off”) levels should be timed and the results recorded on the inspection form. Typically, if the pump is sized and operating properly, pump operation lasts 1-5 minutes per does. Pump cycles lasting longer than this may indicated leachfield clogging and/or pump deficiencies. If this is observed, it must be documented and further investigation of the pump and leachfield should be conducted to determine the specific cause.

If during filling of the basin, the pump does not activate when the water reaches the high liquid level control (i.e., “on” float), discontinue the pump test. This indicates a pump failure, defective float switch or wiring problems and will require by a licensed and properly qualified contractor. The pump system failure must be documented and communicated immediately to the owner/ resident. DEHQ may issue a notice for corrective action.

9.0 Final Leachfield Inspection

At the completion of the hydraulic load test at the septic tank or the pump tank, the dispersal field area and downslope areas are checked again for indications of surfacing effluent, wetness, or odors. If any of these conditions exist at the conclusion of the hydraulic load test, this shall be considered conclusive evidence of system failure. If the field observations of wetness are not obviously the result of the hydraulic load test, further investigation may be necessary to determine if the dispersal field is failing and the cause of the failure. The cause of the seepage may be related to gopher holes, site drainage or erosion problems, excessive water use or the age of the system.

9.1 Inspection and Testing Completion

9.1.1 All inspection and testing procedures and field activities must be fully documented.

9.1.2 All access lids and risers must be replaced and secured before leaving the site.

9.1.3 Test results indicating surfacing effluent must be reported to DEHQ immediately and action must be taken to immediately mitigate the surfacing effluent (i.e., pumping of tank by registered pumper). Repairs to correct the OWTS must be initiated as soon as feasible under a permit issued by DEHQ.

Minimum Leach Line Length and Equivalent Infiltrative Surface Area

Table III-1: Minimum Leach Line Length and Infiltrative Surface Area Equivalents Based on Percolation Rate, Number of Bedrooms, and Allowance of Four Square Feet per Linear Foot Infiltrative Surface Area												
Perce Rate MPI	Number of Bedrooms											
	1		2		3		4		5		6	
	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet
1	200	800	200	800	240	960	270	1080	280	1120	300	1200
2	200	800	200	800	240	960	270	1080	280	1120	300	1200
3	200	800	200	800	240	960	270	1080	280	1120	300	1200
4	200	800	220	880	260	1040	290	1160	300	1200	310	1240
5	200	800	240	960	290	1160	320	1280	320	1280	340	1360
6	200	800	250	1000	300	1200	340	1360	350	1400	360	1440
7	210	840	260	1040	310	1240	350	1400	370	1480	380	1520
8	210	840	265	1060	320	1280	360	1440	390	1560	400	1600
9	220	880	270	1080	320	1280	360	1440	400	1600	410	1640
10	220	880	275	1100	330	1320	370	1480	410	1640	420	1680
11	220	880	280	1120	340	1360	380	1520	420	1680	430	1720
12	230	920	285	1140	340	1360	380	1520	430	1720	440	1760
13	230	920	290	1160	350	1400	390	1560	430	1720	450	1800
14	235	940	295	1180	350	1400	400	1600	440	1760	460	1840
15	240	960	300	1200	360	1440	400	1600	450	1800	470	1880
16	240	960	300	1200	360	1440	410	1640	450	1800	490	1960
17	240	960	305	1220	370	1480	410	1640	460	1840	500	2000
18	250	1000	310	1240	370	1480	420	1680	460	1840	510	2040
19	250	1000	310	1240	380	1520	420	1680	470	1880	520	2080
20	250	1000	315	1260	380	1520	430	1720	470	1880	520	2080
21	260	1040	320	1280	380	1520	430	1720	480	1920	530	2120
22	260	1040	320	1280	390	1560	440	1760	480	1920	530	2120
23	260	1040	325	1300	390	1560	440	1760	490	1960	550	2200
24	260	1040	330	1320	400	1600	450	1800	500	2000	560	2240

25	260	1040	330	1320	400	1600	450	1800	500	2000	560	2240
26	270	1080	335	1340	400	1600	450	1800	510	2040	570	2280
27	270	1080	340	1360	410	1640	460	1840	515	2060	575	2300
28	270	1080	340	1360	410	1640	460	1840	515	2060	575	2300
29	270	1080	345	1380	420	1680	470	1880	525	2100	585	2340
30	280	1120	350	1400	420	1680	470	1880	525	2100	585	2340
	Number of Bedrooms											
Perc Rate MPI	1		2		3		4		5		6	
	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet
31	280	1120	350	1400	420	1680	480	1920	535	2140	595	2380
32	280	1120	355	1420	430	1720	480	1920	535	2140	595	2380
33	290	1160	360	1440	430	1720	490	1960	545	2180	605	2420
34	290	1160	360	1440	440	1760	490	1960	545	2180	605	2420
35	290	1160	365	1460	440	1760	500	2000	555	2220	615	2460
36	300	1200	370	1480	440	1760	500	2000	555	2220	615	2460
37	300	1200	370	1480	450	1800	500	2000	555	2220	615	2460
38	300	1200	375	1500	450	1800	510	2040	565	2260	625	2500
39	300	1200	380	1520	460	1840	510	2040	565	2260	625	2500
40	300	1200	380	1520	460	1840	520	2080	575	2300	635	2540
41	310	1240	385	1540	460	1840	520	2080	575	2300	635	2540
42	310	1240	390	1560	470	1880	530	2120	585	2340	645	2580
43	310	1240	390	1560	470	1880	530	2120	585	2340	645	2580
44	310	1240	395	1580	480	1920	540	2160	595	2380	655	2620
45	320	1280	400	1600	480	1920	540	2160	595	2380	655	2620
46	320	1280	400	1600	480	1920	540	2160	595	2380	655	2620
47	320	1280	405	1620	490	1960	550	2200	605	2420	665	2660
48	330	1320	410	1640	490	1960	550	2200	605	2420	674	2696
49	330	1320	410	1640	500	2000	560	2240	615	2460	697	2788
50	330	1320	415	1660	500	2000	560	2240	615	2460	723	2892

51	340	1360	420	1680	500	2000	560	2240	635	2540	750	3000
52	340	1360	420	1680	510	2040	570	2280	649	2596	779	3116
53	340	1360	425	1700	510	2040	580	2320	674	2696	809	3236
54	340	1360	430	1720	520	2080	580	2320	702	2808	843	3372
55	340	1360	430	1720	520	2080	586	2344	732	2928	879	3516
56	350	1400	435	1740	520	2080	612	2448	765	3060	918	3672
57	350	1400	440	1760	530	2120	641	2564	801	3204	962	3848
58	350	1400	440	1760	530	2120	673	2692	841	3364	1009	4036
59	350	1400	445	1780	540	2160	708	2832	884	3536	1061	4244
60	360	1440	450	1800	563	2252	750	3000	938	3752	1125	4500
	Number of Bedrooms											
Perc Rate MPI	1		2		3		4		5		6	
	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet
61	370	1480	460	1840	571	2284	761	3044	952	3808	1142	4568
62	380	1520	470	1880	580	2320	773	3092	966	3864	1160	4640
63	390	1560	480	1920	592	2368	789	3156	987	3948	1184	4736
64	400	1600	490	1960	602	2408	802	3208	1003	4012	1203	4812
65	420	1680	500	2000	611	2444	815	3260	1019	4076	1223	4892
66	420	1680	510	2040	625	2500	833	3332	1042	4168	1250	5000
67	430	1720	520	2080	636	2544	847	3388	1059	4236	1271	5084
68	440	1760	530	2120	647	2588	862	3448	1078	4312	1293	5172
69	450	1800	540	2160	662	2648	882	3528	1103	4412	1324	5296
70	460	1840	550	2200	674	2696	898	3592	1123	4492	1347	5388
71	470	1880	560	2240	686	2744	915	3660	1143	4572	1372	5488
72	480	1920	570	2280	703	2812	938	3752	1172	4688	1406	5624
73	490	1960	580	2320	717	2868	955	3820	1194	4776	1433	5732
74	500	2000	590	2360	731	2924	974	3896	1218	4872	1461	5844
75	510	2040	600	2400	750	3000	1000	4000	1250	5000	1500	6000

76	520	2080	610	2440	765	3060	1020	4080	1276	5104	1531	6124
77	530	2120	620	2480	781	3124	1042	4168	1302	5208	1563	6252
78	540	2160	630	2520	804	3216	1071	4284	1339	5356	1607	6428
79	550	2200	640	2560	821	3284	1095	4380	1369	5476	1642	6568
80	560	2240	650	2600	846	3384	1128	4512	1410	5640	1692	6768
81	570	2280	660	2640	865	3460	1154	4616	1442	5768	1731	6924
82	580	2320	670	2680	886	3544	1181	4724	1476	5904	1772	7088
83	590	2360	680	2720	915	3660	1220	4880	1524	6096	1829	7316
84	600	2400	690	2760	938	3752	1250	5000	1563	6252	1875	7500
85	610	2440	700	2800	962	3848	1282	5128	1603	6412	1923	7692
86	620	2480	710	2840	996	3984	1327	5308	1659	6636	1991	7964
87	630	2520	720	2880	1023	4092	1364	5456	1705	6820	2045	8180
88	640	2560	730	2920	1051	4204	1402	5608	1752	7008	2103	8412
89	650	2600	740	2960	1092	4368	1456	5824	1820	7280	2184	8736
90	665	2660	755	3020	1125	4500	1500	6000	1875	7500	2250	9000
	Number of Bedrooms											
Per Rate MPI	1		2		3		4		5		6	
	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet	Leach Line Feet	Equivalent Square Feet
91	680	2720	770	3080	1125	4500	1500	6000	1875	7500	2250	9000
92	695	2780	785	3140	1125	4500	1500	6000	1875	7500	2250	9000
93	710	2840	800	3200	1125	4500	1500	6000	1875	7500	2250	9000
94	725	2900	815	3260	1125	4500	1500	6000	1875	7500	2250	9000
95	740	2960	830	3320	1125	4500	1500	6000	1875	7500	2250	9000
96	755	3020	845	3380	1125	4500	1500	6000	1875	7500	2250	9000
97	770	3080	860	3440	1125	4500	1500	6000	1875	7500	2250	9000
98	785	3140	875	3500	1125	4500	1500	6000	1875	7500	2250	9000
99	800	3200	890	3560	1125	4500	1500	6000	1875	7500	2250	9000
100	815	3260	905	3620	1125	4500	1500	6000	1875	7500	2250	9000

101	830	3320	920	3680	1125	4500	1500	6000	1875	7500	2250	9000
102	845	3380	935	3740	1125	4500	1500	6000	1875	7500	2250	9000
103	860	3440	950	3800	1125	4500	1500	6000	1875	7500	2250	9000
104	875	3500	965	3860	1125	4500	1500	6000	1875	7500	2250	9000
105	890	3560	980	3920	1125	4500	1500	6000	1875	7500	2250	9000
106	905	3620	995	3980	1125	4500	1500	6000	1875	7500	2250	9000
107	920	3680	1010	4040	1125	4500	1500	6000	1875	7500	2250	9000
108	935	3740	1025	4100	1125	4500	1500	6000	1875	7500	2250	9000
109	950	3800	1040	4160	1125	4500	1500	6000	1875	7500	2250	9000
110	965	3860	1055	4220	1125	4500	1500	6000	1875	7500	2250	9000
111	980	3920	1070	4280	1125	4500	1500	6000	1875	7500	2250	9000
112	995	3980	1085	4340	1125	4500	1500	6000	1875	7500	2250	9000
113	1010	4040	1100	4400	1125	4500	1500	6000	1875	7500	2250	9000
114	1025	4100	1115	4460	1125	4500	1500	6000	1875	7500	2250	9000
115	1040	4160	1130	4520	1125	4500	1500	6000	1875	7500	2250	9000
116	1055	4220	1145	4580	1125	4500	1500	6000	1875	7500	2250	9000
117	1070	4280	1160	4640	1125	4500	1500	6000	1875	7500	2250	9000
118	1085	4340	1175	4700	1125	4500	1500	6000	1875	7500	2250	9000
119	1100	4400	1190	4760	1125	4500	1500	6000	1875	7500	2250	9000
120	1120	4480	1210	4840	1125	4500	1500	6000	1875	7500	2250	9000

APPENDIX IV

INFILTRATIVE SURFACE AREA OF VERTICAL SEEPAGE PITS

Table IV 1: Infiltrative Surface Area of Vertical Seepage Pits			
4' Diameter Seepage Pit		5' Diameter Seepage Pit	
Rock Depth (Feet)	Infiltrative Surface Area (Square Feet)	Rock Depth (Feet)	Infiltrative Surface Area (Square Feet)
10	151	10	196
15	214	15	275
20	276	20	353
25	339	25	432
30	402	30	511
35	465	35	589
40	528	40	668
45	591	45	746
50	654	50	825
55	716	55	903
60	779	60	982
65	842	65	1060
70	905	70	1139
75	968	75	1218
80	1031	80	1296

APPENDIX IV

ALTERNATIVE TOILET MINIMUM REQUIREMENTS

1.0 Temporary Holding Tank Requirements

1.1 A holding tank may be permitted in lieu of an OWTS if public sewer is not available, an OWTS is not practicable; and the Director determines that the property will be able to connect to public sewer within 18 months of the permit application.

1.2 A permit is required to install temporary holding tanks and is valid for 18 months. The Director may grant one extension for up to an additional 18 months if it is determined the holding tank is being properly maintained and the property owner has been unable to connect with the sewer through no fault of the property owner.

1.3 A licensed installer or an Owner-Builder applying for a permit to install a sewage holding tank shall complete an application form provided by DEHQ and submit with the following information.

1.3.1 Holding tank schematic and specifications, including materials of construction, tank certification information, tank volume and dimensions, and venting provisions.

1.3.2 Plot plan or drawing of parcel showing the location of wells, structures, sanitary sewer lines, and water lines, the proposed location of the tank, distance to any setback features, and accessibility for pumping.

1.3.3 Total daily peak flow calculations from all plumbing facilities, the volume of flow for the number of days between expected pumping events.

1.3.4 The total design capacity of the holding tank to include the volume required for the number of days expected between pumping events (minimum of seven days) plus a minimum of seven days capacity required for reserve capacity (minimum total capacity of 14 days).

1.3.5 The pumping frequency required to maintain the system at a level below the alarm set points.

1.3.6 Alarm schematics and specifications.

1.3.7 Piping construction and configuration.

1.3.8 Methods of securing the tank and access points.

1.4 A back-flow prevention device must be installed on any water supply located within 50 feet of the holding tank.

1.5 The holding tank shall only be cleaned and pumped, and the waste disposed of, by a business registered pursuant to section 117420 of the California Health and Safety Code. Before a permit is issued for the installation of the holding tank system, the owner of the system must submit a copy of the service contract with a licensed sewage pumping company.

1.6 Operational records, including pumping receipts and alarm maintenance records, must be maintained by the owner and provide to DEHQ upon request.

1.7 Holding tanks shall meet the setbacks set forth in this LAMP for septic tanks.

1.8 Holding tanks shall be constructed of approved sound, durable materials not subject to excessive corrosion or decay and shall be watertight. Each tank shall be structurally designed to withstand all anticipated earth or other loads and shall be installed level and on a solid bed. Holding tank materials must comply with those for septic tanks in the LAMP.

1.9 The liquid holding design capacity shall be a minimum of seven (7) days of peak wastewater flow generation plus a minimum of seven days capacity required for reserve capacity. The tank shall have a minimum capacity of 1,500 gallons (represents a minimum capacity for 107 gallons per day peak wastewater flow).

1.10 The system shall be operated and maintained to prevent sewage overflow or leakage from the system at all times. The system shall not create any nuisance conditions. No person shall allow the contents of a holding tank to be discharged onto the surface of the ground, into storm water facilities, or threaten or impact water quality of surface water bodies.

1.11 The system shall have both audible and visual alarms.

1.11.1 The alarms shall be set to signal at the "time to pump" level (minimum seven days capacity) and at a level indicating the waste is approaching the full capacity of the holding tank (maximum three fourths capacity).

1.11.2 The audible and visual alarms shall be located in an area that can be readily seen and heard, with battery power where electrical power is not available.

1.12 The holding tank shall be watertight and there shall be no discharge from the tank.

1.13 The holding tank shall be placed in an area with easy access for pumping.

1.14 No person shall allow the contents of a holding tank to be discharged onto the surface of the ground, into storm water facilities, or threaten or impact water quality of surface water bodies. No person shall use or maintain a holding tank that creates a health hazard, a nuisance condition, or that threatens or impacts a surface water body.

2.0 Vault Privy

2.1 A vault privy is a structure for disposal of human waste without the use of water. It

consists of a shelter build above a subsurface vault into which human waste falls. The vault privy has no water connection.

2.2 Vault privies may be allowed for temporary or limited use areas, where primitive type picnic grounds, campsites, camps and recreation areas are to be maintained, and when a septic tank and disposal field is not practicable as determined by DEHQ. The vault privy shall meet the minimum setback requirements as for septic tanks. Vault privies shall not be used for seasonal dwellings, commercial facilities, or single-family dwellings. Monitoring and an annual operating permit may be required to ensure protection of water quality.

2.3 The size and construction specifications of the vaulted privy shall be designed by a professional engineer licensed by the State of California and shall incorporate these standards. The design shall include, but is not limited to, provisions for the sizing of the vault and shelter elements, excavation and soil preparation for the vault, the selection of a precast vault or for in-place vault construction, construction of floor slab, seat risers, venting, cleanouts for removal of accumulated wastes, and housing or shelter for the privy.

2.4 Plans shall be submitted to DEHQ for review and approval. An installation permit shall be obtained for the installation of a vault privy. This permit is in addition to any building or other permit required by other agencies.

2.5 Materials and Construction. A vaulted privy shall incorporate the following features. These features are in addition to any other standards required by law. Where another standard is required by law, that standard shall apply.

2.5.1 A vault of sufficient capacity to serve the daily and long term needs of users, shall be capable of withstanding all anticipated forces, and shall be of sufficient capacity to store wastes until serviced by a registered pumper. A vault shall be no less than 1000 gallons.

2.5.2 The vault must be designed to be inaccessible to rodents and insects. Interior floors, walls, ceilings, partitions, and doors shall prevent any entry of vermin.

2.5.3 A concrete floor slab, base, or vault lid on which the privy housing can be erected. Wood floors are not permitted.

2.5.4 The material around the privy base graded to divert surface water away from the vault.

2.5.5 Seat risers extending directly from the concrete base, or floor slab, and constructed of
Impervious, easily cleanable material.

2.5.6 Comfortable, impervious and easily cleanable seats with tight-fitting lids which completely cover the privy seat hole when not in use. The seat opening shall be covered with attached, open-front toilet seats with lids, both of which can be raised to all use as

a urinal. A toilet tissue holder shall be provided for each seat.

2.5.7 A privy housing structure that provides privacy and shelter and is fly-tight. The enclosing walls and roof must have no openings or cracks which are not sealed or screened. Structures shall be free of hazardous surface features, such as exposed nail points, splinters, sharp edges, and rough or broken boards, and shall provide privacy and protection from the elements. The housing shall be finished in a light color.

2.5.8 A tight-fitting door which is equipped with a self-closing device.

2.5.9 Vents, windows, or openings which are completely screened. Building ventilation of a minimum of three (3) square feet shall be equally divided between the bottom and top halves of the room. All vents shall be screened with 16 mesh screening of durable material.

2.5.10 Vaults shall be vented to the outside atmosphere by a flue or vent duct having a minimum inside diameter of four (4) inches, extend from the privy vault to a point at least one (1) foot above the roof peak, and is screened at the outlet and capped to divert precipitation. A deodorant may be added routinely to the vault chambers to control odors.

2.5.11 A properly sloped roof of impervious material, with an overhang to prevent ponding of water and leakage into the structure.

2.5.12 Adequate illumination of the privy interior from natural or artificial sources.

2.5.13 A minimum of 16 square feet of floor space provided for a single seat sanitary privy. Multiple-seat privies to provide 12 square feet of floor space for each seat or urinal to be installed. User privacy, through the installation of privacy partitions around the structure or by use of inside door latches.

2.5.14 Vaulted privies must provide an adequate number of seats to serve the number of visitors and the type of activity at the site. A ratio of one privy seat for each 15 visitors of each sex, with a minimum of one unit for each sex in common use facilities, is normally adequate. Sanitation facilities for use by employees must meet the requirements of California Code of Regulations, Title 8.

2.5.15 Urinals may be substituted for men's privy seats on the basis of one urinal for one toilet seat up to a minimum of one-third of the required toilet seats.

2.5.16 The risers to support the privy seats must extend at least 13 inches above the privy floor slab but should not have a height of greater than 18 inches. In multiple-seat privies the seats must have a center-to-center separation of not less than 2.5 feet.

2.6 Maintenance Requirements.

2.6.1 No water-carried sewage shall be placed in vault privies. Vault privies shall be maintained in a sanitary manner and in good repair at all times to prevent health hazards, nuisance conditions, and pollution of surface waters. The vault shall not be allowed to become filled with excreta to a point within two (2) feet of the ground surface. The excreta in the vault shall be removed as necessary to fulfill these requirements.

2.6.2 The vault shall only be cleaned and pumped, and the waste disposed of, by a business registered pursuant to section 117420 of the California Health and Safety Code. The property owner shall maintain pumping records and provide to DEHQ upon request or as required by an annual operating permit. The privy shall be maintained in a sanitary condition and in good repair at all times. Vault privies shall be located in an area accessible for pumping by a registered septic pumper.

2.6.3 No person shall allow the contents of vault privies to be discharged onto the surface of the ground, into storm water facilities, or threaten or impact water quality of surface water bodies.

3.0 Portable Chemical Toilets

3.1 A portable chemical toilet is any self-contained chemical toilet facility that is housed within a portable toilet shelter. The portable toilet has no direct water connection.

3.2 Portable toilets may be approved for temporary or limited use areas, such as construction sites for use by employees, and special events. Portable chemical toilets shall meet the minimum setback requirements as for septic tanks. Portable chemical toilets shall not be allowed for seasonal dwellings or single-family dwellings or commercial facilities that are not limited use or remote where the installation of an OWTS would be impracticable.

3.3 Materials and Construction. Chemical toilets shall incorporate the following features. These features are in addition to any other standards required by law. Where another standard is required by law, that standard shall apply.

3.3.1 Chemical toilet facilities shall provide sufficient space for comfortable use. A minimum area of eight (8) square feet, with a minimum width of two and one half (2½) feet, shall be provided for each toilet seat. A minimum area of ten (10) square feet, with a minimum of two and one half (2½) square feet shall be required when a urinal is included. Sufficient additional space shall be included if hand-washing facilities are within the facility.

3.3.2 Toilets shall be designed, constructed, and maintained so as to prevent the access of flies and vermin, and of flies to the excreta.

3.3.3 The inside surfaces of all toilets shall be of durable, non-absorbent material and shall be smooth and easily cleanable. Toilets shall be maintained in a clean and sanitary manner, free of odor, soilage, and stains at all times.

3.3.4 The toilets shall be ventilated and provided with self-closing doors, lockable from the inside.

3.3.5 The tanks for chemical toilets shall be constructed of durable, easily cleanable material. Tank size shall be sufficient to contain the initial chemical charge and provide capacity for at least one day's use for forty persons. Size and construction shall be such as to prevent splashing on the occupant and on the ground surface while being transported. A minimum tank capacity of 40 gallons shall be installed in the toilet.

3.3.6 Chemicals capable of controlling odors and liquefying solids shall be used in chemical toilets.

3.3.7 The removal and disposal of contents of chemical toilets shall be by a registered septic pumper to a water pollution control plant that is approved to accept the waste. Records of disposal shall be maintained by the chemical toilet owner and provided to DEHQ upon request.

3.3.8 Each chemical toilet shall be identified with the name and phone number of the business responsible for servicing them.

3.4 Maintenance Requirements.

3.4.1 Chemical toilets shall be maintained in a sanitary condition and in good repair at all times. Chemical toilets shall be maintained, and the excreta shall be removed as necessary to prevent a health hazard or nuisance conditions.

3.4.2 The chemical toilet shall only be cleaned and pumped, and the waste disposed of, by a business registered pursuant to section 117420 of the California Health and Safety Code. The chemical toilet owner shall maintain pumping records and provide to DEHQ upon request.

3.4.3 No person shall allow the contents of chemical toilets to be discharged onto the surface of the ground, into storm water facilities, or threaten or impact water quality of surface water bodies.

4.0 Composting Toilets

4.1 A composting toilet may be approved for a campsite, park or trail, when no public sewer is available, no water connection is available, and the installation of an OWTS or vaulted privy is impracticable. A composting toilet shall not be approved for any structure with plumbing fixtures served by or that can be served by a water supply. Composting toilets are not an approved means of wastewater management to address a defective OWTS.

4.2 All composting toilets shall meet NSF/ANSI Standard 41: Non-Liquid Saturated Treatment System and shall have the NSF mark. NSF-certified systems should be verified to meet six requirements: 1) The toilet system can handle the stated capacity for an extended period plus

occasional overload. 2) The toilet system has no offensive odors. 3) The composted output has been demonstrated to meet specific bacterial content levels. 4) Advertising, literature and labeling are not misleading. 5) Products undergoing testing are not accessible to the manufacturer. 6) Lab test results are confirmed by parallel testing of toilet systems operating in the field.

4.3 Composting toilets shall be installed, maintained and used in accordance with the manufacturer's specifications and any local, state or federal laws or regulations, and shall not create any health hazard, nuisance conditions, or threaten or impact the water quality of surface water bodies.

4.3 No residual waste resulting from the use of a composting toilet shall be used as a soil amendment or fertilizer or disposed where persons or animals may come into contact with such waste. Disposal shall be at a facility, such as a sanitary landfill, transfer station, or other approved facility, that is permitted to take such material.

4.4 No person shall use or maintain a composting toilet that is not in compliance with these standards or that creates a health hazard, nuisance condition, or threaten or impact water quality of surface water bodies.

APPENDIX V

TEST BORING CONSTRUCTION STANDARDS

1.0 Test borings installed under the percolation test permit shall meet the following requirements:

1.1 The boring shall be six (6) inches in diameter drilled to the appropriate depth.

1.2 A 2-inch perforated pipe shall be installed from the bottom of the borehole to the level below grade where groundwater may be expected. Non-perforated pipe shall be installed for the remainder of the borehole and shall extend two (2) feet above grade. Care should be taken to not introduce foreign substances or soil into the borehole during placement of the pipe.

1.3 Taking care to avoid bridging, pea gravel shall be added from the bottom of the borehole to a minimum of six (6) inches above the perforated pipe.

1.4 A minimum of six (6) inches of transition seal of bentonite or other approved material shall be placed in the annular space between the pea gravel and cement annular seal. Potable water shall be added to the bentonite transition seal where it exists in dry form allowing sufficient time for the bentonite to properly hydrate prior to the placement of the neat cement annular seal.

1.5 The annular space from the top of the bentonite transition seal to ground surface shall be sealed with neat cement.

1.6 Neat cement shall be mixed thoroughly at a ratio of one 94-pound sack of Portland cement to 5-10 gallons of potable water.

1.7 The annular seal shall extend above ground surface, be sloped away from the pipe and shall be of such construction to prevent entrance of surface water, pollutants, contaminants, and prevent unauthorized access.

1.8 The top of the casing shall be fitted with a secured, watertight cap.

1.9 The test boring shall be marked sufficiently to protect from damage and to be easily located. Barricades or barriers may be required, if necessary, to protect the test boring.

2.0 The test boring shall be destroyed under permit from DEHQ by complete removal of casing and filling the remaining open borehole with neat cement or by filling the casing with neat cement from bottom to one to three feet below grade. The casing shall be cut off below grade and the neat cement shall be brought up over the end of the pipe and into the excavation for a complete seal.