San Diego County Traffic Advisory Committee



Committee Secretary
5510 Overland Avenue #410, Room 470, M.S. 0-334
San Diego, California 92123-1239
kenton.jones@sdcounty.ca.gov

Represented Agencies

County of San Diego Fire Authority
California Department of
Transportation
California Highway Patrol
Independent Insurance Agents
& Brokers of San Diego
San Diego County Bicycle Coalition
San Diego County Department of
Public Works
San Diego County Office of Education
Pacific Safety Center
San Diego County Shriffs
Department

January 28, 2025

TO: Community Planning/Sponsor Group Chairpersons

FROM: Secretary, Traffic Advisory Committee

MEETING NOTICE

Attached is the preliminary agenda for the February 7, 2025 meeting of the Traffic Advisory Committee (TAC).

If your community group has not previously provided input on the proposed agenda items in your jurisdiction and your group would like to provide input, we recommend you place the relevant items on your next available community group meeting agenda for discussion. Please let us know if your group decides to review an item and TAC staff will ensure that your group has adequate time to review before the item is placed on a future TAC meeting agenda.

After reviewing the data and discussing alternatives, the TAC submits a recommendation to the Board as to what it believes to be the most appropriate action based upon sound traffic engineering principles, the California Vehicle Code, and driver expectation. The Board of Supervisors will make a final decision as to what action will be taken after reviewing TAC recommendations and community group input, when available.

If you do have any questions or need additional information regarding this procedure, please contact me at kenton.jones@sdcounty.ca.gov. TAC staff is available to provide background information on items and to answer questions you may have.

This TAC meeting on February 7, 2025, will be conducted with a virtual meeting platform option. Please join us in person or use this link below to join the meeting:

Join on your computer, mobile app or room device

Click here to join the meeting Meeting ID: 253 532 247 016

Passcode: t4kW3R7g

<u>Download Teams</u> | <u>Join on the web</u>

Or call in (audio only)

+1 619-343-2539,,850630120# United States, San Diego

Phone Conference ID: 850 630 120#

Very truly yours,

Kenton R. Jones, Secretary

San Diego County Traffic Advisory Committee

KRJ:bb Attachment

SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

February 7, 2025 ~ 9:00 AM 5510 Overland Ave, Room 271 San Diego CA, 92123

AGENDA

I.	Call to Order / Roll Call
II.	Pledge of Allegiance

III. Approval of Minutes

IV. Announcements / Public Forum

V. Items for Review

SUBJECT		LOCATION	AREA/ COMMUNITY GROUP
SUPERVIS	ORIAL DISTRICT 2		
2-A.	INTERSECTION CONTROL	SINGLE OAK DR & ROCKCREST RD	LAKESIDE/ LAKESIDE CPG
2-B.	INTERSECTION CONTROL	ORO ST & PERSIMMON AV	EL CAJON/ LAKESIDE CPG
2-C.	RADAR CERTIFICATION	ARNOLD WY HARBISON CANYON RD TO TAVERN RD	ALPINE/ ALPINE CPG
SUPERVIS	ORIAL DISTRICT 3		
3-A.	RADAR CERTIFICATION	ELFIN FOREST RD HARMONY GROVE RD TO SAN MARCOS C/L	HARMONY GROVE/ SAN DIEGUITO CPG
SUPERVIS	ORIAL DISTRICT 4		
4-A.	INTERSECTION CONTROL	CENTRAL AV & LAMAR ST	SPRING VALLEY/ SPRING VALLEY CPG
4-B.	RADAR CERTIFICATION	AVOCADO BL EL CAJON C/L TO MADRID WY	MT HELIX/CALAVO GARDENS/ VALLE DE ORO CPG
SUPERVIS	ORIAL DISTRICT 5		
5-A.	INTERSECTION CONTROL	OLD HIGHWAY 395 & CANONITA DR/STEWART CANYON RD	MONSERATE/ FALLBROOK CPG
5-B.	INTERSECTION CONTROL	MAIN AV & ELDER ST	FALLBROOK/ FALLBROK CPG
5-C.	INTERSECTION CONTROL	MAIN AV & IVY ST	FALLBROOK/ FALLBROOK CPG

For information on joining the meeting via Microsoft Teams, please look for the meeting agenda on the Traffic Advisory Committee website at:

Single Oak Drive & Rockcrest Road



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item 2-A

SUPERVISORIAL DISTRICT: 2

SUBJECT: Intersection Control

LOCATION: Single Oak Drive & Rockcrest Road, LAKESIDE

INITIATED BY: DPW Traffic Engineering

REQUEST: All-Way Stop Controls

PROBLEM AS STATED BY REQUESTER:

The intersection of Single Oak Drive and Rockcrest Road has been identified by Traffic Engineering as meeting Option C, an intersection where motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection, and Option D, at an intersection of two residential collectors of similar design and the all-way stop would enhance the traffic operations of said intersection, of the Multi-Way Stop Application optional criteria as described in the California Manual on Uniform Traffic Control Devices (CA MUTCD), Section 2B.07, therefore an all-way stop control should be considered.

Existing Traffic Devices

Single Oak Drive is a striped two-lane, 24 to 40-foot wide, undivided highway. The roadway is striped with a no passing centerline. Single Oak Drive is signed with an intersection ahead warning sign. The road is unclassified on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Rockcrest Road is a striped two-lane, 24 to 44-foot wide, undivided highway. The roadway is striped with a no passing centerline. Rockcrest Road is stop controlled at the intersection with Single Oak Drive. The road is unclassified on the County General Plan Mobility Element Network. The road has no posted speed limit.

Average Daily Traffic Volumes	<u>09/23</u>
Single Oak Drive:	
N/o Rockcrest Road	600 SB
S/o Rockcrest Road	797 NB
Rockcrest Road:	
E/o Single Oak Drive	184 WB
W/o Single Oak Drive	569 EB

Collision Data

There has been 1 reported collision along this segment of roadway, in a 3-year period (2022-01-01 to 2024-12-31). This collision is susceptible to correction by an all-way stop

installation. This collision result in an intersection accident rate of 0.43 collisions per million vehicles entering. The statewide average is 0.36 collisions per million vehicle miles for similar four-legged intersections with stop signs (excluding 4-way stops).



PUBLIC WORKS

WILLIAM MORGAN, P.E.
INTERIM DIRECTOR OF PUBLIC
WORKS

5510 OVERLAND AVENUE, SUITE 410, SAN DIEGO, CALIFORNIA 92123-1237 (858) 694-2212

COUNTY TRAFFIC ENGINEER RECOMMENDATION

Date: January 17, 2025

Item Title: All-Way Stop Control

Location: Single Oak Drive and Rockcrest Road

The County Traffic Engineer recommends installing all-way stop controls at the intersection of Single Oak Drive and Rockcrest Road, pursuant to the following conditions:

- Section 21354 "Stop Signs on Local Highways" of the California Vehicle Code (CVC) authorizes local agencies to designate any intersection under its exclusive jurisdiction as a stop intersection.
- Section 2B.07 "Multi-Way Stop Applications" of the California Manual on Uniform Traffic Control Devices (MUTCD) provides guidelines that should and/or may be considered in an engineering study when evaluating an intersection for an all-way stop control.
- Option D of Section 2B.07 An intersection of two Residential Collectors, indicates all-way stop controls may be considered at an intersection of two residential collectors of similar design and the all-way stop control would enhance the traffic operations of said intersection. Both Single Oak Drive and Rockcrest Road are considered Residential Collectors with similar traffic operation.

•	During the period of January 1, 2022, to December 31, 2024, there were 1
	collision at the intersection. This collision resulted in an intersection accident
	rate of 0.42 vs the statewide average for similar intersections of 0.36 collision
	per million vehicles entering.

Michael L. Kenney, TE 2045 & CE 56661	Date	
Michael Kenney	1/17/25	

VOLUME

Single Oak Dr N/O Rockcrest Rd

Day: Wednesday Date: 11/6/2024

Pk Hr Factor

0.870

0.672

0.750

City: Lakeside

Project #: CA24_040214_001

0.953

0.856

0.625

	D	AILY T	OTA	ALS		_	NB 797		SB 600		EB 569		WB 184								otal 150
ADA Daviad	NID		CD		ED.							ND	104	CD		ED.		VA/D			
AM Period 0:00	NB 2		SB 2		EB		WB		7	OTAL	PM Period 12:00	NB		SB 17		EB 7		WB 4		41	TAL
0:15	1		1		1		0		3		12:15	15		14		6		3		38	
0:30	1	4	0	2	0	-	0		1	12	12:30	10	40	12	Г1	7	2.4	2	12	31	1 4 5
0:45 1:00	0	4	0	3	<u>1</u> 1	5	0		1	12	12:45 13:00	10 11	48	8 14	51	14 12	34	<u>3</u> 5	12	35 42	145
1:15	1		1		0		0		2		13:15	5		12		10		1		28	
1:30	0	4	1	2	0	2	0		1	-	13:30	9	22	10	4.4	9	20	0	10	28	126
1:45 2:00	<u>0</u> 1	1	0	2	0	2	<u>0</u> 1		2	5	13:45 14:00	<u>8</u> 9	33	8 10	44	8 10	39	<u>4</u> 4	10	28 33	126
2:15	1		0		0		0		1		14:15	12		6		12		3		33	
2:30	0	2	0	4	1	2	0	4	1	6	14:30	12	42	13	4.6	10	4.6	4	40	39	452
2:45 3:00	0	2	0	11	0	2	0	1	0	6	14:45 15:00	9 14	42	17 10	46	14 15	46	2	18	47 41	152
3:15	1		0		0		0		1		15:15	13		14		11		2		40	
3:30	2	4	0		0		0	2	2	-	15:30	16	- 7	20	- 7	15	6.4	5	47	56	105
3:45 4:00	2	4	0		<u>0</u> 1		<u>3</u> 1	3	4	7	15:45 16:00	14 12	57	13 15	57	23 8	64	<u>8</u> 3	17	58 38	195
4:15	1		1		1		1		4		16:15	16		22		15		1		54	
4:30	6	4.4	2	4	0	2	0	2	8	22	16:30	15	F.2	10	62	14	F.2	2	0	41	474
4:45 5:00	5 6	14	1 1	4	0	2	3	3	7 10	23	16:45 17:00	9 16	52	15 14	62	15 16	52	3	8	41 49	174
5:15	5		0		0		1		6		17:15	7		17		14		6		44	
5:30	11	25	2	4	1	2	3	0	17	F0	17:30	16	F2	11	C 1	16	5 4	4	12	47	101
5:45 6:00	13 14	35	<u>1</u> 1	4	<u>2</u> 1	3	<u>1</u> 1	8	17 17	50	17:45 18:00	14 9	53	19 13	61	8 18	54	3	13	41 43	181
6:15	15		0		1		5		21		18:15	10		14		12		0		36	
6:30	14	63	2	0	2	C	3	11	21	07	18:30	6	20	8	4.0	7	40	1	_	22	120
6:45 7:00	19 20	62		8	<u> </u>	6	3	11	28 31	87	18:45 19:00	<u>4</u> 1	29	<u>11</u> 6	46	11 7	48	2	5	27 16	128
7:15	23		6		8		2		39		19:15	4		3		10		1		18	
7:30	20	00	9	22	16	42	9	22	54	477	19:30	7	10	3	10	3	25	1	4	14	C.E.
7:45 8:00	17 16	80	16 10	32	12 11	43	<u>8</u> 3	22	53 40	177	19:45 20:00	6 4	18	<u>6</u> 7	18	<u>5</u> 8	25	<u>0</u> 1	4	17 20	65
8:15	15		8		9		4		36		20:15	5		6		3		1		15	
8:30	13	F0	5	24	4	24	1	0	23	120	20:30	4	1.0	4	24	6	22	0	2	14	62
8:45 9:00	15 14	59	<u>8</u> 9	31	4	31	5	9	31 32	130	20:45 21:00	3 9	16	<u>4</u> 6	21	<u>5</u> 3	22	1	3	13 19	62
9:15	10		9		5		0		24		21:15	7		4		4		0		15	
9:30	15	5 4	5	27	1	12	6	12	27	100	21:30	3	22	3	4.5	3	4.5	1	2	10	5 4
9:45 10:00	15 16	54	12	27	<u>3</u> 2	13	6	12	23 36	106	21:45 22:00	3 4	22	0	15	<u>5</u> 1	15	0	2	10 5	54
10:15	13		4		6		2		25		22:15	0		3		0		0		3	
10:30	10	40	6	27	3	10	2	1.0	21	100	22:30	1	C	1	4	1	4	0		3	1.4
10:45 11:00	9 18	48	9	27	9	18	<u>6</u> 2	16	27 38	109	22:45 23:00	2	6	<u>0</u> 1	4	4	4	<u>0</u> 1		<u>3</u> 8	14
11:15	15		6		7		2		30		23:15	1		0		1		0		2	
11:30 11:45	12 9	54	9 10	34	11 8	35	0	5	32 28	128	23:30 23:45	1 0	4	1 0	2	0	6	1 0	2	3	14
TOTALS	9	417	10	173	0	160		90	20	840	TOTALS	U	380	<u> </u>	427	<u> </u>	409	U	94		1310
SPLIT %		49.6%		20.6%		19.0%		10.7%		39.1%			29.0%		32.6%		31.2%		7.2%		60.9%
							NB		SB		EB		WB							To	otal
	DAILY TOTALS						797		600		569		184								150
AM Peak Hour		6:45		11:45		7:30		7:30		7:15	PM Peak Hour		15:30		15:30		15:00		14:00		15:30
AM Pk Volume		82		53		48		24		186	PM Pk Volume		58		70		64		18		206
Pk Hr Factor		0.891		0.779		0.750		0.667		0.861	Pk Hr Factor		0.906		0.795		0.696		0.643		0.888
7 - 9 Volume		139		63		74		31		307	4 - 6 Volume		105		123		106		21		355
7 - 9 Peak Hour		7:00		7:30		7:30		7:30			4 - 6 Peak Hour		16:15		16:00		16:45		16:45		16:15
7 - 9 Pk Volume		80		43		48		24		186	4 - 6 Pk Volume		56		62		61		15		185

Pk Hr Factor

0.875

0.705

0.861

0.667

Oro Street & Persimmon Avenue



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>2-B</u>

SUPERVISORIAL DISTRICT: 2

SUBJECT: Intersection Control

LOCATION: Oro Street & Persimmon Avenue, EL CAJON

INITIATED BY: DPW Traffic Engineering

REQUEST: All-Way Stop Controls

PROBLEM AS STATED BY REQUESTER:

The intersection of Oro Street and Persimmon Avenue has been identified by Traffic Engineering as meeting Option C, an intersection where motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection, and Option D, at an intersection of two residential collectors of similar design and the all-way stop would enhance the traffic operations of said intersection, of the Multi-Way Stop Application optional criteria as described in the California Manual on Uniform Traffic Control Devices (CA MUTCD), Section 2B.07, therefore an all-way stop control should be considered.

Existing Traffic Devices

Oro Street is an unstriped two-lane, 30-foot wide, undivided highway. The road is classified as a Light Collector on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Persimmon Avenue is an unstriped two-lane, 30 to 36-foot wide, undivided highway. Persimmon Avenue is stop controlled at the intersection with Oro Street. The road is unclassified on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Average Daily Traffic Volumes	<u>10/24</u>
Oro Street:	· · · · · · · · · · · · · · · · · · ·
N/o Persimmon Avenue	663 SB
S/o Persimmon Avenue	632 NB
Persimmon Avenue:	
E/o Oro Street	550 WB
W/o Oro Street	483 EB

Collision Data

There have been 5 reported collisions along this segment of roadway, in a 3-year period (2022-01-01 to 2024-12-31). 4 of these collisions are susceptible to correction by an all-way stop installation. These collisions result in an intersection accident rate of 1.97

& Persimmon Avenue

collisions per million vehicles entering. The statewide average is 0.36 collisions per million vehicle miles for similar four-legged intersections with stop signs (excluding 4-way stops).



PUBLIC WORKS

WILLIAM MORGAN, P.E.
INTERIM DIRECTOR OF PUBLIC
WORKS

5510 OVERLAND AVENUE, SUITE 410, SAN DIEGO, CALIFORNIA 92123-1237 (858) 694-2212

COUNTY TRAFFIC ENGINEER RECOMMENDATION

Date: January 17, 2025

Item Title: All-Way Stop Control

Location: Oro Street and Persimmon Avenue

The County Traffic Engineer recommends installing all-way stop controls at the intersection of Oro Street and Persimmon Avenue, pursuant to the following conditions:

- Section 21354 "Stop Signs on Local Highways" of the California Vehicle Code (CVC) authorizes local agencies to designate any intersection under its exclusive jurisdiction as a stop intersection.
- Section 2B.07 "Multi-Way Stop Applications" of the California Manual on Uniform Traffic Control Devices (MUTCD) provides guidelines that should and/or may be considered in an engineering study when evaluating an intersection for an all-way stop control.
- Option C of Section 2B.07 Lack of sight distance, indicates all-way stop controls can be considered when motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection.
- The operational sight distance for the westbound approach of Persimmon Avenue, looking south, does not meet the minimum required operational sight distance per County Public Road Standards.
- The operational sight distance for the eastbound approach of Persimmon Avenue, looking north, does not meet the minimum required operational sight distance per County Public Road Standards.

- Option D of Section 2B.07 An intersection of two Residential Collectors, indicates all-way stop controls may be considered at an intersection of two residential collectors of similar design and the all-way stop control would enhance the traffic operations of said intersection. Both Oro Street and Persimmon Avenue are considered Residential Collectors with similar traffic operation.
- During the period of January 1, 2022, to December 31, 2024, there were 5 collisions at the intersection. These collisions resulted in an intersection accident rate of 1.96 vs the statewide average for similar intersections of 0.36 collision per million vehicles entering.

741: 1 1 V	
Michael Kenney	_1/17/25
Michael L. Kenney, TE 2045 & CE 56661	Date

VOLUME

Oro St N/O Persimmon Ave

Day: Wednesday Date: 10/2/2024

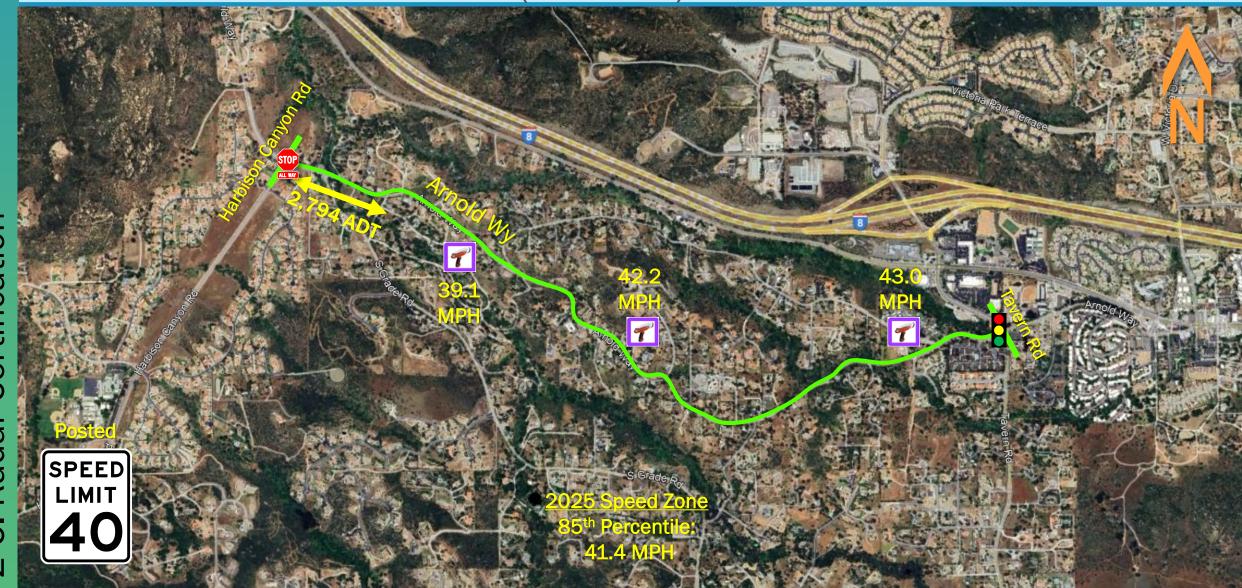
City: El Cajon

Project #: CA24_040196_012

	D	AILY T	OT/	VI C			NB		SB		EB		WB							То	otal
	U	AILTI	UIF	ALS			632		663		483		550							2,3	328
AM Period	NB		SB		EB		WB		TO	TAL	PM Period	NB		SB		EB		WB		ТО	TAL
0:00	0		3		2		0		5		12:00	7		4		5		6		22	
0:15 0:30	1		2		1 0		2 1		6		12:15 12:30	9 14		/ 14		6 6		5 10		27 44	
0:45	1	3	1	6	1	4	1	4	4	17	12:45	11	41	10	35	3	20	5	26	29	122
1:00	0		1		0		1		2		13:00	4		12		5		5		26	
1:15 1:30	0		0		0		0		2 0		13:15 13:30	8 20		13 11		10 5		6 9		37 45	
1:45	0		0	1	0		2	5	2	6	13:45	13	45	13	49	6	26	8	28	40	148
2:00	2		0		0		1		3		14:00	17		22		9		13		61	
2:15 2:30	0		1		0		1		2 0		14:15 14:30	12 15		15 10		14 8		8 9		49 42	
2:45	1	3	0	1	0		0	2	1	6	14:45	5	49	15	62	7	38	14	44	41	193
3:00	0		0		0		2		2		15:00	20		9		15		11		55	
3:15	0		2		2		0		4		15:15	7		12		9		11 15		39	
3:30 3:45	0		2	5	0	2	0	2	2	9	15:30 15:45	14 15	56	12 14	47	18 12	54	15 15	52	59 56	209
4:00	1		0		0		0	- -	1		16:00	15	-	14	-	12		11		52	
4:15	1		1		0		1		3		16:15 16:30	17 16		15		8		9		49 42	
4:30 4:45	0 1	3	0	3	0		1 1	3	3 2	9	16:30 16:45	16 9	57	11 9	49	9 5	34	6 10	36	42 33	176
5:00	1		0		0		3		4		17:00	14		14	43	8	<u> </u>	17	30	53	170
5:15	1		1		3		4		9		17:15	13		14		11		5		43	
5:30 5:45	2 4	8	2	5	1	6	1 6	14	6 14	33	17:30 17:45	12 9	48	15 14	57	8 9	36	6 7	35	41 39	176
6:00	1	0	3		1	0	1	14	6	33	18:00	10	40	8	37	8	30	13	33	39	170
6:15	3		2		2		8		15		18:15	8		13		10		6		37	
6:30 6:45	9 8	21	8 6	19	5	11	6 6	21	28 23	72	18:30 18:45	12 6	36	18 5	44	4 8	30	4 8	31	38 27	141
7:00	9		9	19	2	11	5	21	25	12	19:00	11	30	3	44	3	30	<u> </u>	21	22	141
7:15	12		8		2		7		29		19:15	5		9		4		8		26	
7:30	13	40	13	4.4	15	20	14	25	55	157	19:30	9	22	7	20	8	21	9	27	33	110
7:45 8:00	15 7	49	14 10	44	10 13	29	<u>9</u> 8	35	48 38	157	19:45 20:00	8 11	33	<u>10</u> 3	29	<u>6</u> 5	21	<u>5</u> 8	27	29 27	110
8:15	15		12		16		12		55		20:15	4		3		5		6		18	
8:30	14	47	11	4.4	7	50	13	42	45	400	20:30	6	26	6	40	6	20	4	2.4	22	00
8:45 9:00	11 4	47	11 10	44	14 7	50	9 7	42	45 28	183	20:45 21:00	5 6	26	<u>6</u> 5	18	<u>4</u> 2	20	<u>6</u> 5	24	21 18	88
9:15	10		9		6		3		28		21:15	4		6		6		3		19	
9:30	5	2.0	10		7	0-	9	2.0	31	400	21:30	0		2	4-	4	10	2	1.5	8	60
9:45 10:00	10 7	29	12 7	41	<u>5</u> 2	25	<u>9</u> 8	28	36 24	123	21:45 22:00	4	11	<u>4</u> 4	17	6	18	6	16	17 12	62
10:15	7		, 15		7		5		34		22:15	5		2		4		4		15	
10:30	10		7		3		4	•	24		22:30	2		1		3		5		11	
10:45 11:00	<u>3</u> 8	27	<u>11</u> 8	40		19	<u>5</u> 5	22	26 28	108	22:45 23:00	3	14	<u>4</u> 2	11	<u>2</u> 1	11	<u>4</u>	15	13 10	51
11:15	5		6		5		8		24		23:15	2		0		1		2		5	
11:30	4		9	•	4	. -	10		27		23:30	1	_	3	_	2	_	2		8	
11:45	1	18	6	29	8	24	5	28	20	99	23:45	2	8	2	7	1	5	2	10	7	30
TOTALS		208		238		170		206		822	TOTALS		424		425		313		344		1506
SPLIT %		25.3%		29.0%		20.7%		25.1%		35.3%	SPLIT %		28.2%		28.2%		20.8%		22.8%		64.7%
	D	AII Y I	OTA	ALS			NB		SB		EB		WB							To	otal
	DAILY TOTALS -								663		483		550							2,3	328
AM Peak Hour		7:45		7:30		7:30		7:30		7:30	PM Peak Hour		15:45		14:00		15:00		15:00		15:30
AM Pk Volume		51		49		54		43		196	PM Pk Volume		63		62		54		52		216
Pk Hr Factor 7 - 9 Volume		0.850 96		0.875 88		0.844 79		0.768 77		0.891 340	Pk Hr Factor 4 - 6 Volume		0.926		0.705 106		0.750 70		0.867		0.915 352
7 - 9 Volume 7 - 9 Peak Hour		96 7:45		88 7:30		79 7:30		77 7:30		340 7:30	4 - 6 Volume 4 - 6 Peak Hour		105 16:00		17:00		70 17:00		71 16:15		352 16:15
7 - 9 Pk Volume		51		49		54		43			4 - 6 Pk Volume		57		57		36		42		177
Pk Hr Factor		0.850		0.875		0.844		0.768		0.891	Pk Hr Factor		0.838		0.950		0.818		0.618		0.835

Arnold Way

Harbison Canyon Road to Tavern Road (2.32 miles)



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>2-C</u>

SUPERVISORIAL DISTRICT: 2

SUBJECT: Radar Certification

LOCATION: Arnold Way from Harbison Canyon Road to Tavern

Road (a distance of 2.32 miles) ALPINE

INITIATED BY: DPW Traffic Engineering

REQUEST: Radar Certification

PROBLEM AS STATED BY REQUESTER:

Arnold Way from Harbison Canyon Road to Tavern Road is posted 40 MPH Radar Enforced. Preliminary review of prevailing speeds and roadway conditions could support radar recertification of the existing 40 MPH speed limit.

Existing Traffic Devices

Arnold Way is a striped two-lane, 30 to 52 foot wide, highway. The roadway is striped with a no passing centerline and white edgeline. The road is posted with 30 MPH reverse turn advisory, 20 MPH reverse turn advisory, 35 MPH turn advisory, 30 MPH turn advisory, 25 MPH turn advisory, intersection ahead warning, stop ahead warning, signal ahead warning, school bus stop ahead warning, and narrow bridge warning signs. Arnold Way is classified as a Light Collector on the County General Plan Mobility Element Network. The roadway is posted 40 MPH/Radar Enforced.

Average Daily Traffic Volumes	03/24	<u>09/17</u>
Arnold Way:		
1,000' W/o Midway Drive	2,551	2,250
150' E/o Kyrsten Terrace	3,036	

Speed Data Arnold Way:		85th <u>Percentile</u>	10 MPH <u>Pace</u>	% in <u>Pace</u>
1,000' W/o Midway Drive	(2024) (2017)	39.1 MPH 42.0 MPH	30-39 33-42	74% 88%
760' W/o Blue Lilac Lane	(2024)	42.2 MPH	34-43	73%
150' E/o Kyrsten Terrace	(2024) (2017)	43.0 MPH 46.0 MPH	35-44 35-44	81% 77%
Speed Zone	(2024)	41.4 MPH	33-42	76%

(2017) 44.0 MPH 34-43 83%

Collision Data

There have been 17 reported collisions along this segment of roadway, 10 of which involved injury, of which 2 included a serious injury, in a 3-year period (2021-10-01 to 2024-09-30). These collisions result in a segment accident rate of 2.40 collisions per million vehicle miles. The statewide average is 1.68 collisions per million vehicle miles for similar suburban conventional 2 lanes or less with speeds less than 45 MPH.

Prepared by NDS/ATD

Prepared by National Data & Surveying Services

VOLUME

Arnold Way 1000' W/O Midway Dr

Day: Thursday Date: 3/21/2024

City: Alpine **Project #:** CA24_040040_009

	DΔII	LY TOTALS			NB		SB		EB		WB						otal
	<i>D,</i> (()	21 10 1/L23			0		0		1,390		1,161					2,5	551
AM Period	NB	SB	EB		WB		TC	TAL	PM Period	NB	SB		В	WB			TAL
0:00 0:15	0 0	0 0	0 1		1 1		1 2		12:00 12:15	0 0	0 0	1		11 16		30 35	
0:30	0	Ö	2		1		3		12:30	0	Ő	1		11		25	
0:45	0	0	0	3	1	4	1	7	12:45	0	0	1		12	50	23	113
1:00 1:15	0	0 0	1 2		3 0		4 2		13:00 13:15	0 0	0 0	1		15 18		25 36	
1:30	Ō	Ö	2		1		3		13:30	0	0	1		10		20	
1:45	0	0	0	5	0	4	1	9	13:45 14:00	0	0	1		8	51	24	105
2:00 2:15	0	0 0	1 0		0		1		14:00 14:15	0 0	0 0	2 1		15 15		37 28	
2:30	0	0	Ö		1		1		14:30	0	0	2	5	56		81	
2:45	0	0	0	1	1	2	1	3	14:45 15:00	0	0	3		34	120	66	212
3:00 3:15	0 0	0 0	0 0		1 3		1 3		15:15	0 0	0 0	7 4		20 27		94 67	
3:30	0	0	0		0		-		15:30	0	0	2	5	22		47	
3:45	0	0	1	1	0	4	1	5	15:45 16:00	0	0	2		25	94	49	257
4:00 4:15	0 0	0 0	0 1		0 3		4		16:15	0 0	0	3 4		29 25		61 66	
4:30	Ö	Ö	Ō		2		2		16:30	Ö	Ö	2	0	27		47	
4:45	0	0	1	2	3	8	4	10	16:45	0	0	2		39	120	64	238
5:00 5:15	0 0	0	2 2		1 2		3 4		17:00 17:15	0 0	0 0	2 4		29 34		54 74	
5:30	Ö	Ö	1		6		7		17:30	Ö	Ö	3	5	27		62	
5:45	0	0	4	9	3	12	7	21	17:45	0	0	3		16	106	53	243
6:00 6:15	0 0	0 0	6 2		6 8		12 10		18:00 18:15	0 0	0 0	5 5		14 9		70 67	
6:30	Ö	0	5		15		20		18:30	0	0	4	6	14		60	
6:45	0	0	8	21	20	49	28 27	70	18:45 19:00	0	0	3		13	50	49	246
7:00 7:15	0 0	0 0	10 13		17 19		32		19:00	0 0	0 0	2 1		14 11		37 26	
7:30	0	0	11		30		41		19:30	0	0	1	3	10		23	
7:45 8:00	0	0	28 13	62	20 35	86	48 48	148	19:45 20:00	0	0	<u>1</u>		<u>4</u> 7	39	18 19	104
8:15	0	0	23		35		40 58		20:15	0	0	1		10		20	
8:30	0	0	29		67		96		20:30	0	0	ţ	5	8		13	
8:45 9:00	0	0	41 54	106	34 13	171	75 67	277	20:45 21:00	0	0	1		<u>5</u>	30	13 16	65
9:15	0	0	24		13		37		21:15	0	0	1		3		10	
9:30	0	0	16		7		23		21:30	0	0	3	3	2		5	
9:45 10:00	0	0	14 9	108	15 15	48	29 24	156	21:45 22:00	0	0		2 23 2	0	11	2	34
10:00	0	0	9 11		9		20		22:15	0	0	3		0		3	
10:30	0	0	9		12		21		22:30	0	0	2	2	3		5	
10:45 11:00	0	0	13 11	42	8 17	44	21 28	86	22:45 23:00	0	0			0	3	4	14
11:00	0	0	17		6		28		23:15	0	0			0		5	
11:30	0	0	15		8		23		23:30	0	0	ļ		3	_	8	
11:45	0	0	17	60	19	50	36	110	23:45	0	0			2	5	4	18
TOTALS				420		482		902	TOTALS				970		679		1649
SPLIT %				46.6%		53.4%		35.4%	SPLIT %				58.8%		41.2%		64.6%
	DAH	IV TOTALS			NB		SB		EB		WB					To	otal
	DAII	LY TOTALS			0		0		1,390		1,161						551
AM Peak Hour				8:30		8:00		8:15	PM Peak Hour				17:45		14:30		14:30
AM Pk Volume				148		171		296	PM Pk Volume				197		137		308
Pk Hr Factor				0.685		0.638		0.771	Pk Hr Factor				0.849		0.612		0.819
7 - 9 Volume		0 0		168		257		425	4 - 6 Volume		0	0	255		226		481
7 - 9 Peak Hour 7 - 9 Pk Volume				8:00		8:00		8:00	4 - 6 Peak Hour 4 - 6 Pk Volume				17:00		16:30		16:45
Pk Hr Factor				106 0.646		171 0.638		277 0.721	Pk Hr Factor				137 0.856		129 0.827		254 0.858
TRIII FACIOI	U.	.000		0.040		0.036		0.721	7 K III Factor		0.000	0.000	0.636		0.827		0.030

Prepared by NDS/ATD

Prepared by National Data & Surveying Services

VOLUME

Arnold Way 150' E/O Kyrsten Terrace

Day: Thursday Date: 3/21/2024 City: Fallbrook
Project #: CA24_040040_010

	DΔ	ILY TOTALS			NB		SB		EB		WB							otal
					0		0		1,412		1,624						3,0	036
AM Period	NB	SB	EB		WB		TC	TAL	PM Period	NB		SB	EB		WB		TO	TAL
0:00 0:15	0	0 0	0 0		0 2		2		12:00 12:15	0		0 0	23 19		29 27		52 46	
0:30	0	0	2		0		2		12:30	0		0	28		22		50	
0:45	0	0	0	2	2	4	2	6	12:45	0		0	20	90	21	99	41	189
1:00 1:15	0	0 0	3 2		0 1		3 3		13:00 13:15	0 0		0 0	23 20		23 18		46 38	
1:30	0	0	0		1		1		13:30	0		0	10		19		29	
1:45 2:00	0	0	0	7	0	2	2	9	13:45 14:00	0		0	21 24	74	20 21	80	41 45	154
2:15	0	0	0		0				14:15	0		0	51		30		81	
2:30	0	0	1		0		1		14:30	0		0	44		33		77	
2:45 3:00	0	0	0	3	0 1		2	3	14:45 15:00	0		0	26 36	145	50 76	134	76 112	279
3:15	0	Ö	0		Ō		-		15:15	0		0	35		40		75	
3:30	0	0	0		1	2	1	2	15:30	0		0	27	422	37	177	64	200
3:45 4:00	0	0	0		<u>0</u>	2	1	2	15:45 16:00	0		0	24 33	122	24 36	177	48 69	299
4:15	0	0	1		1		2		16:15	0		0	37		36		73	
4:30 4:45	0	0 0	1 1	3	7 4	13	8 5	16	16:30 16:45	0		0 0	32 31	133	37 31	140	69 62	273
5:00	0	0	2	3	2	15	4	10	17:00	0		0	50	133	53	140	103	2/3
5:15	0	0	1		9		10		17:15	0		0	45		51		96	
5:30 5:45	0	0 0	0 3	6	6 7	24	6 10	30	17:30 17:45	0 0		0 0	38 36	169	30 30	164	68 66	333
6:00	0	0	2		9		11	30	18:00	0		0	41	103	43	101	84	333
6:15	0	0	7		9		16		18:15 18:30	0		0	32		32		64	
6:30 6:45	0	0 0	4 13	26	9 15	42	13 28	68	18:30 18:45	0		0 0	27 19	119	23 16	114	50 35	233
7:00	0	0	11		16		27		19:00	0		0	10		10		20	
7:15 7:30	0	0 0	19 19		27 24		46 43		19:15 19:30	0		0 0	8 8		12 7		20 15	
7:45	0	0	29	78	33	100	62	178	19:45	0		0	8	34	11	40	19	74
8:00	0	0	37		26		63		20:00	0		0	8		6		14	
8:15 8:30	0	0 0	45 50		24 50		69 100		20:15 20:30	0		0 0	2 8		4 6		6 14	
8:45	0	0	19	151	47	147	66	298	20:45	0		0	2	20	6	22	8	42
9:00	0	0 0	10 18		55		65 47		21:00 21:15	0		0	2 5		0		2	
9:15 9:30	0	0	15		29 25		47		21:30	0		0 0	1		3 0		8 1	
9:45	0	0	14	57	21	130	35	187	21:45	0		0	6	14	11	4	7	18
10:00 10:15	0	0 0	17 17		23 17		40 34		22:00 22:15	0		0 0	4 1		3 5		7 6	
10:30	0	0	27		21		48		22:30	0		0	4		3		7	
10:45	0	0	25	86	16	77	41	163	22:45	0		0	0	9	1	12	1	21
11:00 11:15	0	0 0	12 13		28 17		40 30		23:00 23:15	0		0 0	5 1		1 1		6 2	
11:30	0	0	18		22		40		23:30	0		0	0		2		2	
11:45	0	0	14	57	25	92	39	149	23:45	0		0	1	7	1	5	2	12
TOTALS				476		633		1109	TOTALS					936		991		1927
SPLIT %				42.9%		57.1%		36.5%	SPLIT %					48.6%		51.4%		63.5%
	DA	ILY TOTALS			NB		SB		EB		WB						To	otal
	DA	ILI TOTALS			0		0		1,412		1,624						3,0	036
AM Peak Hour				7:45		8:30		8:15	PM Peak Hour					17:00		14:45		14:15
AM Pk Volume				161		181		300	PM Pk Volume					169		203		346
Pk Hr Factor				0.805		0.823		0.750	Pk Hr Factor					0.845		0.668		0.772
7 - 9 Volume 7 - 9 Peak Hour				229 7:45		247 8:00		476 8:00	4 - 6 Volume 4 - 6 Peak Hour					302 17:00		304 16:30		606 17:00
7 - 9 Peak Hour 7 - 9 Pk Volume				161		8:00 147		298	4 - 6 Pk Volume					169		172		333
Pk Hr Factor		0.000 0.000		0.805		0.735		0.745	Pk Hr Factor		0.000	0.000		0.845		0.811		0.808
T K TH T actor		0.000		0.003		0.733		0.743	/ K III Tactor		0:000	0.000		0.043		0.011		0.000



Road Na	ame:	Arnold V	Vy		From:	Harbiso	n Canyor	Rd		To:		Tavern F	Rd	
Position):	1000' W,	/o Mi	dway D	r					Direc	tion:	EB/WB		
				•								-		
Date:		7/17/202	24		Weathe	r:	Cle	ar		Proje	ct Num	nber:	N/A	
Time Sta	art:	1:00 PM			Road Co	ndition	: Dry			Obse	rver:		County	
Time En	d:	2:25 PM			Posted S	Speed:	40	MPH		Calib	ration ⁻	Γest:	Υ	,
Speed (mph)	Num. Veh.	Cum. Pct.						Num	ber of \	/ehicles	<u> </u>			
15	ven.	PCI.			0	2	4		6	8		10	12	14
16 17				15	 									
18 19				20	3									
20				25										
21				30					_					
22 23		+	h)	35										
24	1	1.0%	Speed (mph)	40										
25			pa											
26 27	1	1.9%	be	45										
28	5	6.7%	S	50	3									
29	4	10.5%		55	3									
30 31	6 3	16.2% 19.0%		60	1									
32	10	28.6%			3									
33	8	36.2%		65	3									
34 35	10 12	45.7% 57.1%		70	∃									1
36	10	66.7%		1000/										
37	9	75.2%		100%										
38 39	5 5	80.0% 84.8%		90%										
40	5	89.5%		80%	-									
41 42	3	92.4% 96.2%	nt	70%	+									
43	4 1	97.1%	Cumulative Percent	60%	-									
44	2	99.0%	Pe	50%					_/_					
45 46	1	100.0%	ive	40%										
47		100.0%	ılat	30%					/					
48			ı					/						
49 50			J	20%										
51				10%	1									
52				0%	+	-								$\overline{}$
53 54					0	10	20	30	4	0	50	60	70	80
55									Speed (mph)				1
56					■Data Plot —— 50				th Perce	entile			85th Perc	entile
57 58														
59					-90th Percentile —			——95th Percentile						
60								DATA	ANALY	SIS				
61 62									Γ			1		
63			А۱	erage :	e Speed 35.0					Range			24 - 4	łΩ
64 65			50	th Perc	rcentile 34.4				10	mph Pa	ace		30 - 3	39
66		<u>L</u>										 		
67			85	ııı rero	rcentile 39.1				Number in Pace			78		
68 69		+	90	th Perc	rcentile 40.2				Percent in Pace 74%					
70			۵۶	th Para	rcentile 41.7							1		
Total	105		33	ui reit	.cmile	<u> </u>	41./		<u> </u>			<u> </u>		



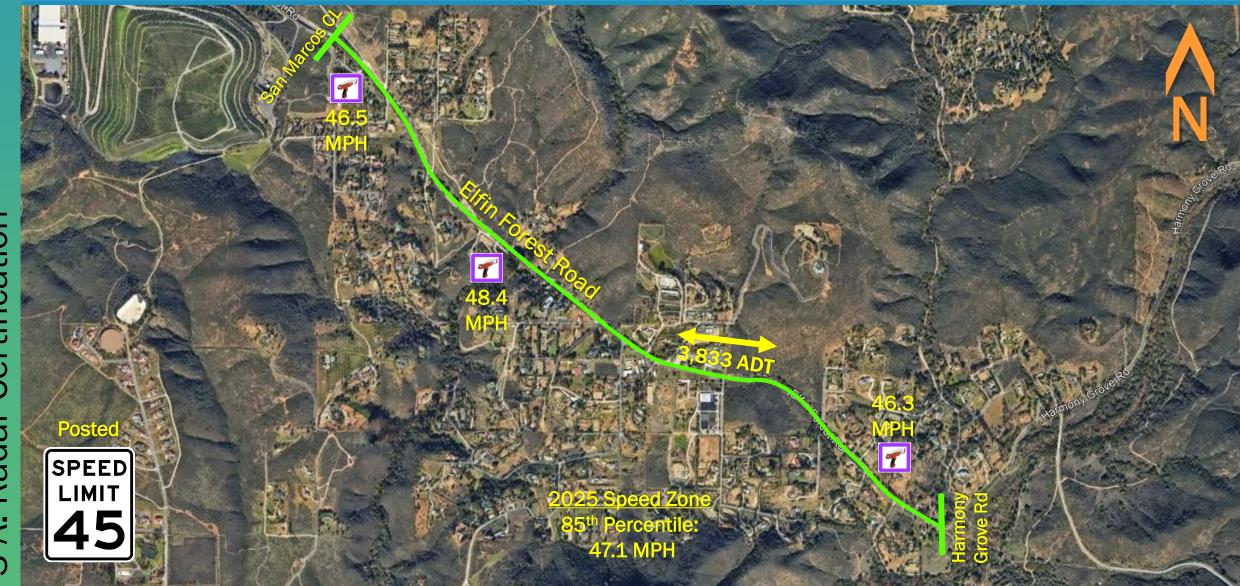
Road Na	ame:	Arnold V	Ny		From:	Harbis	on Cany	on Rd		To: Tavern Rd					
Position	1:	760' W/	o Blue	e Lilac L	n					Dire	ection:	EB/WB			
										,					
Date:		7/17/20	24		Weathe	r:	(lear		Pro	ject Nun	nber:	N/A		
Time Sta	art:	2:50 PM	l		Road Co	nditior	n: [ry		Obs	erver:		Count	ty	
Time En	d:	4:15 PM	l		Posted S	peed:	۷	0 МРН		Cali	bration	Test:	Υ		
Speed (mph)	Num. Veh.	Cum. Pct.							Numbe	r of Vehic	es				
15 16					0	2		4	6		8	10	1	.2	14
17			ł	15	+										
18			1	20]										
19 20			ł	25]										
21			1	30			_								
22			_ €												
23 24			ᇣ	35											
25			Speed (mph)	40										-	
26			ee	45				_							
27	1	0.00/	&	50											
28 29	1	0.9%													
30	3	3.8%		55	1										
31	4	7.5%		60	1										
32 33	5 5	12.3% 17.0%	ł	65	1										
34	6	22.6%	1	70	∄										
35	6	28.3%		,,											
36 37	7	34.9%		100%	_										
38	10 8	44.3% 51.9%		90%											
39	7	58.5%													
40	12	69.8%		80%											
41 42	10 5	79.2% 84.0%	Ħ	70%	+										
43	6	89.6%	Cumulative Percent	60%	-										
44	4	93.4%	Pe	50%						_/					
45 46	3	96.2% 97.2%	<u>≤</u>	40%											
47	2	99.1%	lat	30%						/					
48] [/						
49			3	20%											
50 51	1	100.0%		10%	+										
52		100.070		0%	+		$\overline{}$		4						
53					0	10	20		30	40	50	60		70	80
54 55									Spe	eed (mph)					-
56		+											05.1.5		
57				<u> </u>						Percentile			-85th F	Percenti	ie
58 59				9	90th Perc	entile			– 95th	Percentile					
60															
61					DATA A				ATA AI	VALYSIS					
62			A	verage	e Speed 38.1					Range 28 - 51					
63 64	 	+													
65			50	oth Pero	ercentile 37.8				10 mph	Pace	<u>L</u>	34	- 43		
66			ŖΓ	oth Pero	ercentile 42.2				Number in Pace 77						
67 68															
69	 		90	th Pero	ercentile 43.1				Percent in Pace 73%						
70			۵r	5th Para	rcentile 44.6										
Total	106			CII FEIC	citile		+4.					<u> </u>			



Road Na	ame:	Arnold V	Vy		From:	Harbiso	on Canyo	n Rd		To: Tavern Rd					
Position	1:	150' E/o	Kyrst	en Tr						Dire	ction:	EB/WB			
Date:		3/13/202	24		Weathe	r:	Cle	ear		Proje	ect Num	ber:	24-04	0041-00)8
Time Sta	art:	9:00 AM			Road Co	ndition	: Dr	У		Obse	erver:		Contr	actor	
Time En	d:	11:00 AN	V		Posted S	Speed:	40	MPH		Calib	ration 1	Γest:	Υ		
Speed (mph)	Num. Veh.	Cum. Pct.			•			Nu	mber of	Vehicle	es				
15 16	Veni	1 00.			0	2	4		6	8		10	1	.2	14
17				15	<u> </u>										
18 19				20											
20				25	3										
21				30											
22			Speed (mph)	35			_			_					
24			lw)	40										_	
25			pa										_		
26 27			ədo	45											
28	1	0.9%	0,	50											
29 30	1	1.9%		55	=										
31	1	1.9%		60	3										
32	1	2.8%		65	3										
33 34	3 2	5.6% 7.4%													
35	7	13.9%		70	_										
36	6	19.4%		1000/											
37 38	13 12	31.5% 42.6%		100%											
39	11	52.8%		90%											
40	9	61.1%		80%	+										
41 42	8 11	68.5% 78.7%	ij	70%	+										
43	7	85.2%	Cumulative Percent	60%	-					/					
44	4	88.9%	Pe	50%						7					
45 46	5 2	93.5% 95.4%	ï×e	40%					/						
47	2	97.2%	ם	30%											
48	2	00.40/	Ē	20%					/						
49 50	2 1	99.1% 100.0%	ರ						//						
51				10%											
52 53		-		0%	+	10		20		40				70	
54					0	10	20	30		40	50	60)	70	80
55									Speed	(mph)					
56 57					Data Plot			5	Oth Perd	centile			-85th F	ercentil	e
58					90th Perc	antila			5th Pero	antila					
59						Circiic			Juli i Ci c	critic					
60 61								DAT	A ANAL	YSIS					
62			۸۰	verage (e Speed 39.5					Range	1		20	- 50	
63							39.3						20	- 30	
64 65		+ -	50	th Perc	entile		38.7		10) mph P	ace		35	- 44	
66 67			85	th Perc	entile		43.0		Nur	mber in	Pace		8	38	
68			90	Oth Perc	entile		44.2		Per	cent in	Pace		8	1%	
69 70	400			oth Perc			45.8								
Total	108		7.				,5.0								

Elfin Forest Road

Harmony Grove Road to San Marcos city limit (north of Camino Cielo Azul) (2.00 miles)



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item 3-A

SUPERVISORIAL DISTRICT: 3

SUBJECT: Radar Certification

LOCATION: Elfin Forest Road from Harmony Grove Road to San

Marcos city limit (north of Camino Cielo Azul) (a

distance of 2.00 miles) HARMONY GROVE

INITIATED BY: DPW Traffic Engineering

REQUEST: Radar Certification

PROBLEM AS STATED BY REQUESTER:

Elfin Forest Road from Harmony Grove Road to San Marcos city limit (north of Camino Cielo Azul) is posted 45 MPH Radar Enforced. Preliminary review of prevailing speeds and roadway conditions could support radar recertification of the existing 45 MPH speed limit.

Existing Traffic Devices

Elfin Forest Road is a striped two-lane, 32 to 40 foot wide, highway. The roadway is striped with a no passing centerline and white edgeline. The road is posted with 30 MPH reverse curve advisory, 30 MPH turn advisory, 20 MPH turn advisory, 30 MPH reverse turn advisory, 25 MPH reverse turn advisory, and intersection ahead warning signs. Harmony Grove Road is classified as a Light Collector on the County General Plan Mobility Element Network. The roadway is posted 45 MPH/Radar Enforced.

Average Daily Traffic Volumes	<u>2024</u>	<u>2014</u>
Elfin Forest Road		
600' E/o Colina Encantada Way	3,785	
1,350' W/o Elfin Oaks Road	3,880	
150' W/o Elfin Forest Ln		3,657

Speed Data Elfin Forest Road:		85th <u>Percentile</u>	10 MPH <u>Pace</u>	% in <u>Pace</u>
270' N/o Elfin Glen	(2024)	46.3 MPH	38-47	80%
1,750' N/o Elfin Oaks Road At Fortuna Del Este	(2024) (2019)	48.4 MPH 49.5 MPH	39-48 41-50	73% 78%
300' N/o Camino Ciel Azul	(2024)	46.5 MPH	37-46	77%
Speed Zone	(2024)	47.1 MPH	38-47	77%

<u>Collision Data</u>
There have been 9 reported collisions along this segment of roadway, 5 of which involved injury, of which 1 included a serious injury, in a 3-year period (2021-10-01 to 2024-09-30). These collisions result in a segment accident rate of 1.07 collisions per million vehicle miles. The statewide average is 1.33 collisions per million vehicle miles for similar rolling rural conventional 2 lanes or less roads with speeds less than or equal to 55 MPH.

Prepared by NDS/ATD

Prepared by National Data & Surveying Services

VOLUME

Elfin Forest Rd 600' E/O Colina Encantada Way

Day: Thursday Date: 7/11/2024 City: Escondido
Project #: CA24_040124_011

	DΔI	LY TOTALS			NB		SB		ЕВ		WB						otal
	DAI	ETTOTALS			0		0		2,031		1,754					3,7	785
AM Period	NB	SB	EB		WB		TC	TAL	PM Period	NB	SB	Е	В	WB		ТО	TAL
0:00	0	0	0		2		2		12:00	0	0	10		23		39	
0:15 0:30	0 0	0 0	3 1		4 3		7 4		12:15 12:30	0	0	21 28		19 14		46 42	
0:45	0	0	4	8	0	9	4	17	12:45	0	0	1		14	70	31	158
1:00	0	0	2		1		3		13:00	0	0	20		17		43	
1:15 1:30	0	0 0	1 0		1 1		2 1		13:15 13:30	0	0	20 1		19 12		39 29	
1:45	0	0	0	3	0	3	_	6	13:45	0	0	3(16	64	46	157
2:00	0	0	0		2		2		14:00	0	0	33		16		49	
2:15 2:30	0 0	0 0	0 1		1		1 1		14:15 14:30	0	0	33 50		27 16		60 66	
2:45	0	0	0	1	0	3	_	4	14:45	0	0	3(33	92	69	244
3:00	0	0	0		0				15:00	0	0	63	3	28		91	
3:15	0 0	0 0	0		4		4		15:15 15:30	0	0	5! 79		42		97	
3:30 3:45	0	0	1 0	1	1 2	7	2	8	15:45	0	0	84		38 46	154	117 130	435
4:00	0	0	0		1		1		16:00	0	0	89		51		140	
4:15	0	0	1		2		3		16:15	0	0	10		43		144	
4:30 4:45	0 0	0 0	2 0	3	1 0	4	3	7	16:30 16:45	0	0	59 81		38 53	185	97 140	521
5:00	0	0	0	<u> </u>	5	-	5		17:00	0	0	68		32	100	100	321
5:15	0	0	3		7		10		17:15	0	0	88		46		134	
5:30 5:45	0 0	0 0	5 9	17	11 6	20	16 15	46	17:30 17:45	0	0	6! 49		58 22	150	123 71	428
6:00	0	0	8	17	10	29	18	40	18:00	0	0	33		26	158	59	420
6:15	0	0	7		10		17		18:15	0	0	29		30		59	
6:30	0	0	16		29	0.4	45	426	18:30	0	0	1!		21		36	100
6:45 7:00	0	0	21 19	52	35 35	84	56 54	136	18:45 19:00	0	0	29 13		16 9	93	45 22	199
7:15	0	Ö	27		50		77		19:15	0	0	2!		16		41	
7:30	0	0	31		71		102		19:30	0	0	13		14		27	
7:45 8:00	0	0	30 37	107	53 51	209	83 88	316	19:45 20:00	0	0	1! 20		18 19	57	33 39	123
8:15	0	0	22		47		69		20:15	0	0	1!		10		25	
8:30	0	0	29		32		61		20:30	0	0	13	3	7		20	
8:45 9:00	0	0	24 17	112	46 40	176	70 57	288	20:45 21:00	0	0 0	<u> </u>		10 7	46	20 14	104
9:15	0	0	17 27		40 34		61		21:15	0	0	2		3		5	
9:30	0	0	21		24		45		21:30	0	0	6		6		12	
9:45	0	0	23	88	24	122	47	210	21:45	0	0	8		6	22	14	45
10:00 10:15	0 0	0 0	16 15		16 22		32 37		22:00 22:15	0	0	4 1		3 2		7	
10:30	0	Ö	15		20		35		22:30	0	0	1		1		2	
10:45	0	0	19	65	19	77	38	142	22:45	0	0	6	12	2	8	8	20
11:00 11:15	0 0	0 0	24 19		19 20		43 39		23:00 23:15	0	0	2		2 0		4	
11:15	0	0	16		20 14		39		23:30	0	0	1		0		1	
11:45	0	0	22	81	27	80	49	161	23:45	Ö	0	1		Ö	2	1	10
TOTALS				538		803		1341	TOTALS				1493		951		2444
SPLIT %				40.1%		59.9%		35.4%	SPLIT %				61.1%		38.9%		64.6%
					NB		SB		EB		WB					Te	otal
	DAI	LY TOTALS			O IND		<u>эв</u> 0		2,031		1,754						785
									2,031		1)134					3,	
AM Peak Hour				7:15		7:15		7:15	PM Peak Hour				15:30		16:45		15:30
AM Pk Volume				125		225		350	PM Pk Volume				353		189		531
Pk Hr Factor 7 - 9 Volume		0		0.845 219		0.792 385		0.858 604	Pk Hr Factor 4 - 6 Volume		0	0	0.874 606		0.815 343		0.922 949
7 - 9 Volume 7 - 9 Peak Hour				7:15		385 7:15		7:15	4 - 6 Volume 4 - 6 Peak Hour				16:00		343 16:45		16:00
7 - 9 Pk Volume				125		225		350	4 - 6 Pk Volume				336		189		521
Pk Hr Factor	0	.000 0.00	00	0.845		0.792		0.858	Pk Hr Factor		0.000	0.000	0.832		0.815		0.905

Prepared by NDS/ATD

Prepared by National Data & Surveying Services

VOLUME

Elfin Forest Rd 1350' W/O Elfin Oaks Rd

 Day: Thursday
 City: Escondido

 Date: 7/11/2024
 Project #: CA24_040124_012

	DAIL	Y TOTALS			NB 0		SB 0		EB 2,024		WB_ 1,856						ital 880
AM Period	NB	SB	EB		WB		TC	TAL	PM Period	NB	SB	ЕВ		WB		TO	TAL
0:00	0	0	0		2		2		12:00	0	0	15		23		38	
0:15 0:30	0 0	0 0	2 2		1 3		3 5		12:15 12:30	0 0	0 0	27 28		22 25		49 53	
0:45	0	0	4	8	0	6	4	14	12:45	0	0	21	91	18	88	39	179
1:00 1:15	0 0	0 0	1 1		1 1		2		13:00 13:15	0 0	0 0	20 21		19 19		39 40	
1:30	0	0	0		1		1		13:30	0	0	14		19		33	
1:45 2:00	0	0	0	2	<u>0</u>	3	1	5	13:45 14:00	0	0	31 34	86	17 17	74	48 51	160
2:15	0	0	0		0		1		14:15	0	0	32		27		59	
2:30 2:45	0 0	0 0	1 0	1	1 0	2	2	3	14:30 14:45	0 0	0 0	47 40	153	26 32	102	73 72	255
3:00	0	0	0		0				15:00	0	0	60	133	33	102	93	255
3:15	0	0	0		3		3		15:15	0	0	52		47		99	
3:30 3:45	0 0	0 0	1 0	1	2	7	3 2	8	15:30 15:45	0 0	0 0	80 73	265	42 51	173	122 124	438
4:00	0	0	0		1		1		16:00	0	0	84		56		140	
4:15 4:30	0 0	0 0	1 1		2 1		3 2		16:15 16:30	0 0	0 0	97 62		43 37		140 99	
4:45	0	0	0	2	2	6	2	8	16:45	0	0	87	330	51	187	138	517
5:00 5:15	0 0	0 0	1 2		4 8		5 10		17:00 17:15	0 0	0 0	79 81		40 40		119 121	
5:30	0	0	5		8		13		17:30	0	0	58		54		112	
5:45 6:00	0	0	7 8	15	6 11	26	13 19	41	17:45 18:00	0	0	49 40	267	30 31	164	79 71	431
6:15	0	0	8		9		17		18:15	0	0	34		30		64	
6:30	0	0	18	60	31	00	49	1.10	18:30	0	0	15	115	30	100	45	224
6:45 7:00	0	0	26 18	60	29 43	80	55 61	140	18:45 19:00	0	0	26 12	115	18 15	109	44 27	224
7:15	0	0	26		46		72		19:15	0	0	21		16		37	
7:30 7:45	0 0	0 0	22 24	90	70 54	213	92 78	303	19:30 19:45	0 0	0 0	14 15	62	13 16	60	27 31	122
8:00	0	0	37		49		86		20:00	0	0	24		12		36	
8:15 8:30	0 0	0 0	26 29		48 33		74 62		20:15 20:30	0 0	0 0	16 12		13 9		29 21	
8:45	0	0	31	123	48	178	79	301	20:45	0	0	12	64	10	44	22	108
9:00 9:15	0 0	0 0	19 25		46 29		65 54		21:00 21:15	0 0	0 0	8 8		4 2		12 10	
9:30	0	0	17		26		43		21:30	0	0	9		7		16	
9:45 10:00	0	0	24 12	85	29 21	130	53 33	215	21:45 22:00	0	0	<u>8</u> 4	33	<u>6</u> 4	19	14 8	52
10:15	0	0	20		23		43		22:15	0	0	2		2		4	
10:30	0	0	15	71	19	0.4	34	155	22:30 22:45	0	0 0	1	12	1	10	2	22
10:45 11:00	0	0	24 28	71	21 20	84	45 48	155	23:00	0	0	5 0	12	2	10	2	22
11:15	0	0	17 16		26		43		23:15 23:30	0	0	3		0		3	
11:30 11:45	0 0	0 0	16 20	81	16 27	89	32 47	170	23:45	0 0	0 0	2 2	7	0 0	2	2	9
TOTALS				539		824		1363	TOTALS				1485		1032		2517
SPLIT %				39.5%		60.5%		35.1%	SPLIT %				59.0%		41.0%		64.9%
	DAIL	Y TOTALS			NB		SB		ЕВ		WB						tal
	- DAIL	II-TOTALS			0		0		2,024		1,856					3,8	380
AM Peak Hour				8:00		7:30		7:30	PM Peak Hour				15:30		15:15		15:30
AM Pk Volume Pk Hr Factor				123 0.831		221		330 0.897	PM Pk Volume Pk Hr Factor				334 0.861		196 0.875		526 0.939
7 - 9 Volume	C) 0		213		0.789 391		604	4 - 6 Volume		0 0		0.861 597		0.875 351		948
7 - 9 Peak Hour				8:00		7:30		7:30	4 - 6 Peak Hour				16:00		16:00		16:00
7 - 9 Pk Volume				123		221		330	4 - 6 Pk Volume				330		187		517
Pk Hr Factor	0.0	0.000		0.831		0.789		0.897	Pk Hr Factor		0.000 0.000		0.851		0.835		0.923



Road Na	ame:	Elfin For	est Ro		From:	Harmo	ny Grove	Rd		То:	San Ma	arcos CL	
Position	1:	270' N/o	Elfin	Glen						Direction:	NB/SB		
Date:		10/31/20	024		Weathe	er:	Cle	ar		Project Nui	mber:	24-04021	.6-007
Time Sta	art:	11:00 AN	√l		Road Co	ondition	: Dry	,		Observer:		Contracto	or .
Time En	d:	1:00 PM			Posted	Speed:	45	MPH		Calibration	Test:	Υ	
Speed	Num. Veh.	Cum. Pct.						Nur	mber of Ve	ehicles			
(mph) 15	ven.	PCI.			0	5		10	15		20	25	30
16 17				15	+								
18 19				20	3								
20				25	3								
21				30									
22 23			Speed (mph)	35		_	_						
24			w)	40									
25 26			pə	45								<u>. </u>	
27			Spe										
28 29				50									
30				55	3								
31				60	3								
32 33	1	0.5%		65	3								
34	2	1.5%		70	3								
35 36	3 6	3.0% 6.1%											
37	5	8.6%		100%	7								
38	11	14.2%		90%									
39 40	10 20	19.3% 29.4%		80%									
41	13	36.0%	Ħ	70%						_/			
42 43	23 18	47.7% 56.9%	Cumulative Percent	60%	_								
44	24	69.0%	Pel	50%						<i>]</i>			
45 46	14 14	76.1%	<u>.</u>	40%									
46	11	83.2% 88.8%	ılat	30%									
48	6	91.9%	Ē										
49 50	6 3	94.9% 96.4%	3	20%	1				/				
51	4	98.5%		10%									
52 53	2	99.5% 100.0%		0%	+	10			+				
54	1	100.0%			0	10	20	30	40		60	70	80
55									Speed (m	nph)			1
56 57				<u> </u>	Data Plot	:		 5	Oth Percer	ntile		-85th Perc	entile
58				9	90th Per	centile		 9	5th Percer	ntile			
59 60													
61					DATA ANAL				A ANALYS	IS			
62 63			A۱	verage :	e Speed 42.8				F	Range		33 - 5	53
64			50	th Perc					10 n	nph Pace		38 - 4	
65 66									Number in Pace 158				
67 68									Percent in Pace 80%				
69 70									Perce	reiteiit iii rate 80%			
Total	197		95	th Perc	entile		49.1						



Road Name: Elfin Forest Rd						
Date: 10/9/2024 Weather: Clear Project Number: 24-040216	To: San Marcos CL					
Time Start: 11:40 AM Road Condition: Dry Observer: Contractor Time End: 1:40 PM Posted Speed: 45 MPH Calibration Test: Y Speed Num. (mph) Veh. Pct.						
Time Start: 11:40 AM Road Condition: Dry Observer: Contractor Time End: 1:40 PM Posted Speed: 45 MPH Calibration Test: Y Speed Num. (mph) Veh. Pct.						
Time End: 1:40 PM	-002					
Speed Num. Cum. Pct.						
(mph) Veh. Pct.						
15						
16	35					
18						
19						
21						
22						
23 24 25 25 26 27 27 28 29 30 30 31 31 32 33 34 34 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 38 11 10.5% 39 11 14.6% 40 19 21.7% 40 19 21.7% 41 18 28.5% 42 25 37.8% 42 25 37.8% 43 24 46.8% 44 16 52.8% 45 32 64.8% 46 18 71.5% 47 18 78.3% 48 14 88.35% 49 9 86.9% 50 11 91.0% 51 9 94.4% 51 9 94.4%						
28 29 30 31 31 32 33 33 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 40 19 21.7% 41 18 28.5% 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 44 16 52.8% 45 32 64.8% 44 16 52.8% 45 32 64.8% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 50 11 91.0%						
28 29 30 30 31 31 32 33 33 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 38 11 10.5% 40 19 21.7% 41 18 28.5% 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 44 16 52.8% 45 32 64.8% 46 18 77.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 50 11 91.0%						
28 29 30 30 31 31 32 33 33 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 38 11 10.5% 40 19 21.7% 41 18 28.5% 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 44 16 52.8% 45 32 64.8% 46 18 77.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 50 11 91.0%						
28 29 30 30 31 31 32 33 33 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 38 11 10.5% 40 19 21.7% 41 18 28.5% 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 44 16 52.8% 45 32 64.8% 46 18 77.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 50 11 91.0%						
29						
30 31 31 32 33 33 34 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 39 11 14.6% 40 19 21.7% 41 18 28.5% 40 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 45 32 64.8% 44 16 55.8% 45 32 64.8% 47 18 78.3% 48 14 83.5% 49 9 80% 50% 60% 50% 40% 30% 50% 50 11 91.0% 50 11 91.0% 50 10%						
32 33 34 35 36 5 1.9% 37 12 6.4% 38 11 10.5% 39 11 14.6% 40 19 21.7% 41 18 28.5% 42 25 37.8% 42 25 37.8% 42 25 37.8% 44 40 50 40 40 40 40 40 40 40 40 40 4						
33						
33						
35						
36						
38 11 10.5% 39 11 14.6% 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 45 32 64.8% 46 18 71.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
39 11 14.6% 40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 45 32 64.8% 46 18 71.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
40 19 21.7% 41 18 28.5% 42 25 37.8% 43 24 46.8% 44 16 52.8% 45 32 64.8% 46 18 71.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
Town						
42 25 37.8% 43 24 46.8% 44 16 52.8% 45 32 64.8% 46 18 71.5% 47 18 78.3% 48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
48 14 83.5% 49 9 86.9% 50 11 91.0% 51 9 94.4%						
50 11 91.0% 51 9 94.4%						
50 11 91.0% 51 9 94.4%						
52 4 95.9% 0%						
53 6 98.1% 0 10 20 30 40 50 60 70	80					
54 5 100.0%	00					
Speed (mph)						
Data Plot —— 50th Percentile —— 85th Percen	ıtile					
——90th Percentile ——95th Percentile						
59 South Fercentile South Fercentile						
60 DATA ANALYSIS						
61						
62 Average Speed 44.1 Range 36 - 54						
65 Suth Percentile 43.5 10 mph Pace 39 - 48						
66 85th Percentile 48.4 Number in Pace 195	·					
0/						
90th Percentile 49.8 Percent in Pace 73%						
70						
Total 267 95th Percentile 51.4						



Maria	Poccell														
Road Na	ame:	Elfin For	est Ro	ł	From: Harmony Grove Rd				Т	o:	San Ma	rcos Cl	-		
Position	ı :	300' N/d	o Cami	ino Cie	lo Azul					C	irection:	NB/SB			
Date:		10/9/20	124		Weath	ner:		Clear		 P	roject Nur	nber:	24-04	0216-0	01
Time Sta	art:	9:40 AV			-	Conditio		Dry			bserver:		Contr		
Time En		11:40 AI				Speed		45 MPH			alibration	Tost	Y	4001	
Speed	Num.	Cum.	I		Poster	3peeu	•					Test.	T		
(mph)	Veh.	Pct.				_	_		Number			•			
15 16			1		0		5	10		15		.0	25		30
17			1	15											
18 19			1	20	3										
20			1	25	3										
21				30	1										
22 23		+	년 등	35	1										
24			<u>ב</u>	40					•						
25			Speed (mph)												
26 27		+	, p	45						-				_	
28			j	50											
29 30			1	55	-										
30			1	60	3										
32			1	65	3										
33 34			1		3										
35	9	3.5%	1	70	_										
36	9	7.0%		1000/											
37 38	11 9	11.3% 14.8%	4	100%											
39	14	20.2%	-	90%											
40	25	30.0%		80%	+										
41 42	24 28	39.3% 50.2%	Ħ	70%	+										
43	25	59.9%	Cumulative Percent	60%	+										
44	21	68.1%	Pe	50%						$-\!\!\!-\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!$					
45 46	26 14	78.2% 83.7%	- ×e	40%											
47	7	86.4%	iat	30%											
48	8	89.5%	ן בַּ												
49 50	7 6	92.2% 94.6%	5	20%											
51	1	94.9%	1	10%											
52	2	95.7%		0%	+										
53 54	7 4	98.4% 100.0%	1		0	10	20)	30	40	50	60		70	80
55		255.070	1						Spee	ed (mp	h)				+
56 57			4		Data Plo	ot			– 50th Pe	ercent	ile		-85th F	Percenti	ile
57 58		+	1												
59					90th Pe	rcentile			–95th Pe	ercent	iie				
60 61			-					D	ATA ANA	ALYSIS					
61 62	 	 	_	10 K = = =	Cncs-l	1	42					I	25	Γ 4	
63			A۱	/erage	speea		42	.ŏ		ка	nge	1	35	- 54	
64 65		+	50	th Pero	centile		42	.0		10 mp	oh Pace	1	37	- 46	
66			0.	th Dor	contilo	1	A.E.	5		Jumba	r in Paca	1	1	۵7	
67			85	ui Per	ercentile 46.5			Number in Pace 197							
68 69		1	90	th Pero	rcentile 48.2			Percent in Pace 77%							
70			05	th Par	rcentile 51.1						İ				
Total	257] 33	an FEI	Jenuie		31	. 1							

Central Avenue & Lamar Street



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>4-A</u>

SUPERVISORIAL DISTRICT: 4

SUBJECT: Intersection Control

LOCATION: Central Avenue & Lamar Street, SPRING VALLEY

INITIATED BY: DPW Traffic Engineering

REQUEST: All-Way Stop Controls

PROBLEM AS STATED BY REQUESTER:

The intersection of Central Avenue and Lamar Street has been identified by Traffic Engineering as meeting Option B, an intersection where there is a desire to control vehicle/pedestrian conflicts near locations that generate high pedestrian volumes, and Option D, at an intersection of two residential collectors of similar design and the all-way stop would enhance the traffic operations of said intersection, of the Multi-Way Stop Application optional criteria as described in the California Manual on Uniform Traffic Control Devices (CA MUTCD), Section 2B.07, therefore an all-way stop control should be considered.

Existing Traffic Devices

Central Avenue is a striped two-lane, 28 to 40-foot wide, undivided highway. The roadway is striped with a no passing centerline. Central Avenue is signed with an intersection ahead warning sign. The road is unclassified on the County General Plan Mobility Element Network. The road has no posted speed limit.

Lamar Street is a striped two-lane, 30-foot wide, undivided highway. The roadway is striped with a no passing centerline. Lamar Street is stop controlled at the intersection with Central Avenue. The road is unclassified on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Average Daily Traffic Volumes	<u>10/24</u>
Central Avenue:	
N/o Lamar Street	723 SB
S/o Lamar Street	687 NB
Lamar Street:	
E/o Central Avenue	540 WB
W/o Central Avenue	762 EB

Collision Data

There have been 4 reported collisions along this segment of roadway, 3 of which involved

injury, in a 3-year period (2022-01-01 to 2024-12-31). 2 of these collisions are susceptible to correction by an all-way stop installation. These collisions result in an intersection accident rate of 1.35 collisions per million vehicles entering. The statewide average is 0.36 collisions per million vehicle miles for similar four-legged intersections with stop signs (excluding 4-way stops).



PUBLIC WORKS

WILLIAM MORGAN, P.E.
INTERIM DIRECTOR OF PUBLIC
WORKS

5510 OVERLAND AVENUE, SUITE 410, SAN DIEGO, CALIFORNIA 92123-1237 (858) 694-2212

COUNTY TRAFFIC ENGINEER RECOMMENDATION

Date: January 17, 2025

Item Title: All-Way Stop Control

Location: Central Avenue and Lamar Street, Spring Valley

The County Traffic Engineer recommends installing all-way stop controls at the intersection of Central Avenue and Lamar Street, pursuant to the following conditions:

- Section 21354 "Stop Signs on Local Highways" of the California Vehicle Code (CVC) authorizes local agencies to designate any intersection under its exclusive jurisdiction as a stop intersection.
- Section 2B.07 "Multi-Way Stop Applications" of the California Manual on Uniform Traffic Control Devices (MUTCD) provides guidelines that should and/or may be considered in an engineering study when evaluating an intersection for an all-way stop control.
- Option B of Section 2B.07 An All-Way Stop Control may be considered to control vehicle/pedestrian conflicts near locations that generate high pedestrian volumes, such as parks. The subject intersection of Central Avenue and Lamar Street is located within Lamar County Park.
- Option D of Section 2B.07 An intersection of two Residential Collectors, indicates allway stop controls may be considered at an intersection of two residential collectors of similar design and the all-way stop control would enhance the traffic operations of said intersection. Both Central Avenue and Lamar Street are considered Residential Collectors with similar traffic operation.
- During the period of January 1, 2022, to December 31, 2024, there were 4 collisions at the intersection. These collisions resulted in an intersection accident rate of 1.35 vs the statewide average for similar intersections of 0.36 collision per million vehicles entering.

Michael L. Kenney, TE 2045 & CE 56661		
Michael Kenney	1/17/25	

VOLUME

Central Ave & Lamar St

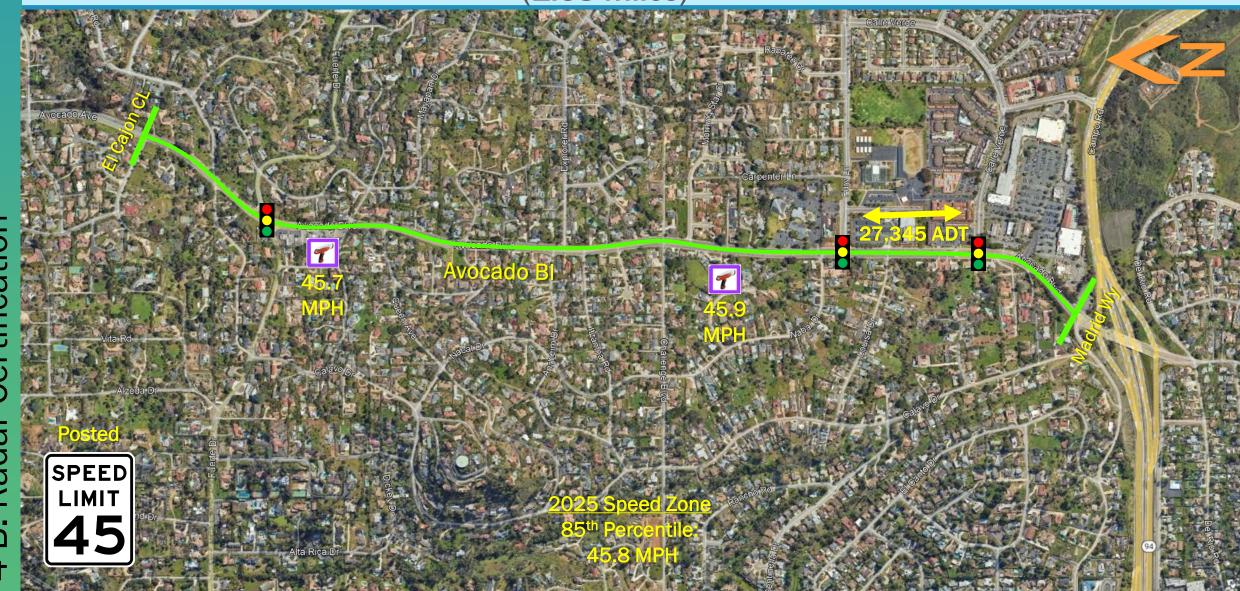
City: Spring Valley
Project #: CA24_040033_001

Day: Thursday
Date: 3/7/2024

	D	AILY T	OTA	ALS		-	NB 687		SB 723		EB 762		WB 540								otal 712
AM Period	NB		SB		ЕВ		WB			OTAL	PM Period	NB		SB		EB		WB			TAL
00:00	0		0		1		1		2		12:00	12		8		8		6		34	
00:15 00:30	0		1 1		2 1		2 0		5 2		12:15 12:30	4 6		7 13		10 11		7 6		28 36	
00:45	1	1	2	4	4	8	1	4	8	17	12:45	7	29	4	32	10	39	9	28	30	128
01:00 01:15	0		1 1		0 0		0 1		1 2		13:00 13:15	9 6		14 9		14 10		7 4		44 29	
01:30	1		2		1		1		5		13:30	9		6		21		14		50	
01:45	0	1	0	4	0	2	0	3	2	10	13:45 14:00	8	32	11	40	10 14	55	7	32	36 41	159
02:00 02:15	2		1		0		0		3		14:15	14 11		10 9		16		9		41	
02:30	0	2	1	2	1		0	2	2	•	14:30	10	40	18		17	F.0	5	22	50	100
02:45 03:00	1	3	0	3	0	1	<u>2</u>	2	2	9	14:45 15:00	13 21	48	20 13	57	12 23	59	<u>5</u> 8	22	50 65	186
03:15	1		0		0		0		1		15:15	15		11		19		4		49	
03:30 03:45	1 1	4	0 1	1	2 0	2	1 1	3	4 3	10	15:30 15:45	11 15	62	11 18	53	14 16	72	9 12	33	45 61	220
04:00	2	•	0		1		0	J	3		16:00	14		23	- 55	16		8	- 55	61	
04:15 04:30	0 1		0 1		0 2		0		0 4		16:15 16:30	17 9		16 20		15 16		12 10		60 55	
04:45	1	4	1	2	0	3	2	2	4	11	16:45	14	54	24	83	16	63	10	40	64	240
05:00 05:15	4 4		1 2		0 2		1 1		6 9		17:00 17:15	17 16		18 19		16 19		8 7		59 61	
05:30	3		2		3		4		12		17:30	7		22		24		12		65	
05:45	5 7	16	3	8	<u>4</u> 5	9	<u>6</u> 2	12	18 17	45	17:45 18:00	10	50	19 11	78	13 10	72	16 5	43	58 40	243
06:00 06:15	8		3 7		3		7		25		18:00	14 8		5		10 14		2		40 29	
06:30	9		6		9		4		28	.=	18:30	8		14		13		7		42	
06:45 07:00	8	32	<u>6</u> 8	22	<u>4</u> 8	21	9	22	27 34	97	18:45 19:00	5 10	35	12 8	42	9 7	46	10 8	24	36 33	147
07:15	21		10		10		15		56		19:15	7		11		7		7		32	
07:30 07:45	13 24	66	7 15	40	12 12	42	23 16	64	55 67	212	19:30 19:45	6 4	27	5 4	28	6 5	25	1 6	22	18 19	102
08:00	22	- 00	19	-10	18		13	01	72		20:00	5		4		8		1		18	102
08:15 08:30	15 22		18 10		12 15		10 12		55 59		20:15 20:30	5 4		6 10		7 12		8 4		26 30	
08:45	7	66	14	61	15	60	10	45	46	232	20:45	6	20	5	25	4	31	4	17	19	93
09:00 09:15	15 9		12 7		18 9		11 10		56 35		21:00 21:15	5 3		2		8 5		4 5		19 16	
09:30	8		1		6		7		22		21:30	4		3		4		6		17	
09:45	17 9	49	8	28	6	39	12	40	43	156	21:45 22:00	3	15	9	17	3	19	3	18	17 9	69
10:00 10:15	3		12 6		13 6		6 7		22		22:15	3 1		1 3		5 5		1		10	
10:30	7	24	12	20	9	26	11	22	39	420	22:30	1	_	1	_	3	45	1	•	6	22
10:45 11:00	12 8	31	9	39	8 10	36	<u>8</u>	32	37 31	138	22:45 23:00	<u>0</u> 2	5	3	7	3	15	2	6	8 10	33
11:15	6		17		7		6		36		23:15	0		1		3		5		9	
11:30 11:45	9	29	9 8	43	2 13	32	2 5	17	22 32	121	23:30 23:45	3 3	8	0 2	6	1 4	11	1 1	9	5 10	34
TOTALS		302		255		255		246		1058	TOTALS		385		468		507		294		1654
SPLIT %		28.5%		24.1%		24.1%		23.3%		39.0%	SPLIT %		23.3%		28.3%		30.7%		17.8%		61.0%
	_D	AILY T	OTA	ALS _			NB		SB		ЕВ		WB								tal
			 /				687		723		762		540							2,	712
AM Peak Hour		07:45		07:45		08:00		07:15		07:45	PM Peak Hour		15:00		16:00		16:45		17:00		16:45
AM Pk Volume Pk Hr Factor		83 0.865		62 0.816		60 0.833		67 0.728		253 0.878	PM Pk Volume Pk Hr Factor		62 0.738		83 0.865		75 0.781		43 0.672		249 0.958
7 - 9 Volume		132		101		102		109		444	4 - 6 Volume		104		161		135		83		483
7 - 9 Peak Hour		07:45		07:45		08:00		07:15		07:45	4 - 6 Peak Hour		16:15		16:00		16:45		17:00		16:45
7 - 9 Pk Volume		83		62		60		67		253	4 - 6 Pk Volume		57		83		75		43		249
Pk Hr Factor		0.865		0.816		0.833		0.728		0.878	Pk Hr Factor		0.838		0.865		0.781		0.672		0.958

Avocado Boulevard

El Cajon city limit (at Dewitt Court) to Madrid Way (1.58 miles)



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>4-B</u>

SUPERVISORIAL DISTRICT: 4

SUBJECT: Radar Certification

LOCATION: Avocado Boulevard from El Cajon city limit (at Dewitt

Court) to Madrid Way (a distance of 1.58 miles)

MOUNT HELIX/CALAVO GARDENS

INITIATED BY: DPW Traffic Engineering

REQUEST: Radar Certification

PROBLEM AS STATED BY REQUESTER:

Avocado Boulevard from El Cajon city limit to Madrid Way is posted 45 MPH Radar Enforced. Preliminary review of prevailing speeds and roadway conditions could support radar recertification of the existing 45 MPH speed limit.

Existing Traffic Devices

Avocado Boulevard is a striped four-lane, 64 to 86 foot wide, through highway. The roadway is striped with a two-way turn lane, lane lines and bike lane. The road is posted with intersection ahead warning and signal ahead warning signs. The road is signalized at Calle Verde, Fury Lane, and Fuerte Drive. Avocado Boulevard is classified as a Major Road on the County General Plan Mobility Element Network. The roadway is posted 45 MPH/Radar Enforced.

Average Daily Traffic Volumes	<u>11/24</u>	<u>04/16</u>
Avocado Boulevard:		
250' N/o Puebla Drive	27,345	
S/o Fuerte Drive		27,070

		85th	10 MPH	% in
Speed Data		<u>Percentile</u>	<u>Pace</u>	<u>Pace</u>
Avocado Boulevard:				
250' N/o Puebla Drive	(2024)	45.7 MPH	37-46	65%
100' S/o Dewitt Court	(2018)	51.6 MPH	39-48	58%
280' S/o Morning Star Drive	(2024)	45.9 MPH	37-46	66%
	(2018)	52.4 MPH	43-52	77%
Speed Zone	(2024)	45.8 MPH	37-46	66%
-	(2018)	52.0 MPH	41-50	68%

<u>Collision Data</u>
There have been 58 reported collisions along this segment of roadway, 26 of which involved injury, of which 2 included a serious injury and of which 1 included a fatality, in a 3-year period (2021-10-01 to 2024-09-30). These collisions result in a segment accident rate of 1.23 collisions per million vehicle miles. The statewide average is 1.24 collisions per million vehicle miles for similar suburban conventional 2 lanes or less with speeds between 45 and 55 MPH.

Prepared by NDS/ATD

Prepared by National Data & Surveying Services

VOLUME

Avocado Blvd 250' N/O Puebla Dr

Day: Wednesday Date: 11/6/2024 City: La Mesa
Project #: CA24_040209_002

	D	AILY 1	ГОТА	LS		NB		SB		EB		WB							otal
						12,520		14,825		0		0						27,	,345
AM Period	NB		SB		EB	WB		то	TAL	PM Period	NB		SB		EB	WI	3		TAL
0:00 0:15	27 19		18 21		0 0	0 0		45 40		12:00 12:15	199 192		174 227		0	0		373 419	
0:30	12		20		0	0		32		12:30	157		223		0	0		380	
0:45	11	69	14	73	0	0		25	142	12:45	174	722	205	829	0	0		379	1551
1:00 1:15	15 12		8 8		0 0	0 0		23 20		13:00 13:15	184 166		209 221		0	0		393 387	
1:30	6		12		Ö	0		18		13:30	159		217		Ö	0		376	
1:45	8	41	3	31	0	0		11	72	13:45	179	688	253	900	0	0		432	1588
2:00 2:15	4 9		5 1		0 0	0 0		9 10		14:00 14:15	214 225		296 280		0	0		510 505	
2:30	6		4		0	0		10		14:30	248		284		0	0		532	
2:45 3:00	<u>7</u> 5	26	<u>2</u> 5	12	0	0		9 10	38	14:45 15:00	240 251	927	306 341	1166	0	0		546 592	2093
3:15	9		4		0	0		13		15:15	245		356		0	0		601	
3:30	9		3		0	0		12		15:30	296		356		0	0		652	
3:45 4:00	16 6	39	<u>8</u> 4	20	0	0		24 10	59	15:45 16:00	329 274	1121	379 409	1432	0	0		708 683	2553
4:00 4:15	11		15		0	0		26		16:15	256		440		0	0		696	
4:30	18		16		0	0		34		16:30	269		383		0	0		652	
4:45 5:00	29 15	64	20 17	55	0	0		49 32	119	16:45 17:00	247 244	1046	393 392	1625	0	0		640 636	2671
5:15	39		33		0	0		72		17:15	264		395		0	0		659	
5:30	41		48		0	0		89		17:30	208		351		0	0		559	
5:45 6:00	54 65	149	60 59	158	0	0		114 124	307	17:45 18:00	182 166	898	388 275	1526	0	0		570 441	2424
6:15	96		59 67		0	0		163		18:15	149		247		0	0		396	
6:30	138		106		0	0		244		18:30	121		202		0	0		323	
6:45 7:00	200 218	499	116 167	348	0	0		316 385	847	18:45 19:00	110 112	546	186 124	910	0	0		296 236	1456
7:00 7:15	283		213		0	0		385 496		19:15	91		167		0	0		258	
7:30	316		255		0	0		571		19:30	99		109		0	0		208	
7:45 8:00	297 294	1114	310 292	945	0	0		607 586	2059	19:45 20:00	74 88	376	109 138	509	0	0		183 226	885
8:15	318		233		0	0		551		20:15	90		94		0	0		184	
8:30	257		255		0	0		512		20:30	78		93		0	0		171	
8:45 9:00	272 226	1141	255 223	1035	0	0		527 449	2176	20:45 21:00	72 62	328	95 69	420	0	0		167 131	748
9:15	184		193		0	0		377		21:15	68		73		0	0		141	
9:30	186		178		0	0		364		21:30	72		73		0	0		145	
9:45 10:00	162 155	758	186 164	780	0	0		348 319	1538	21:45 22:00	65 81	267	64 51	279	0	0		129 132	546
10:15	153		178		0	0		331		22:15	69		44		0	0		113	
10:30	159		176		0	0		335		22:30	49		42		0	0		91	
10:45 11:00	150 173	617	197 182	715	0	0		347 355	1332	22:45 23:00	44	243	41 29	178	0	0		85 72	421
11:15	172		194		0	0		366		23:15	43 48		28		0	0		76	
11:30	189		192		0	0		381	4.470	23:30	24	4.62	30	40-	0	0		54	2
11:45 TOTALS	165	699 5216	206	774 4946	0	0		371	1473 10162	23:45 TOTALS	27	7304	18	105 9879	0	0		45	247 17183
SPLIT %		51.3%		4946					37.2%	SPLIT %		42.5%		57.5%					62.8%
JI LII /0		J1.J/0		70.770					37.2/0					37.370					
	D	AILY 1	ОТА	LS		NB		SB		EB		WB							otal
						12,520		14,825		0		0						-27,	,345
AM Peak Hour		7:30		7:30					7:30	PM Peak Hour		15:30		16:00					15:30
AM Pk Volume		1225		1090					2315	PM Pk Volume		1155		1625					2739
Pk Hr Factor 7 - 9 Volume		0.963 2255		0.879 1980		0	0		0.953 4235	Pk Hr Factor 4 - 6 Volume		0.878 1944		0.923 3151		0	n		0.967 5095
7 - 9 Peak Hour		7:30		7:30					7:30	4 - 6 Peak Hour		16:00		16:00					16:00
7 - 9 Pk Volume		1225		1090					2315	4 - 6 Pk Volume		1046		1625					2671
Pk Hr Factor		0.963		0.879	0	.000	0.000		0.953	Pk Hr Factor		0.954		0.923		0.000	0.000		0.959



RADAR SPEED SURVEY SAN DIEGO COUNTY DEPT OF PUBLIC WORKS

116	DCCCL												
Road Na	ame:	Avocado	BI		From:	El Cajon	CL			То:	Madrid	Wy	
Position	:	250' N/c) Puel	ola Dr	•					Direction:	NB/SB		
									- i				
Date:		12/12/2	024		Weath	er:	Clea	r		Project Nu	mber:	24-040210-0	 002
Time Sta	art:	9:00 AM	1		Road Co	ondition:	Dry			Observer:		Contractor	
Time En	d:	11:00 Al	M		Posted	Speed:	45 N	1PH		Calibration	Test:	Υ	
Speed	Num.	Cum.				-			ber of Ve	h:-l			
(mph) 15	Veh.	Pct.			0	10	20	30	iber of ve	50	60	70	80
16			1	15	+							,,	
17			ł		1								
18 19			ł	20	=								
20			1	25	_								
21			1	30									
22			<u>ج</u>										
23			요	35									
24 25			느	40									
26	2	0.2%	e l	45									
27	2	0.2%	Speed (mph)	45									
28	4	0.8%	1 ~	50		_							
29	9	1.8%		55		_							
30	12	3.1%			=								
31	11	4.2%		60	3								
32	27	7.1%		65	1								
33 34	21 31	9.3% 12.6%	ł		3								
35	43	17.1%	ł	70	_								
36	47	22.1%											
37	51	27.5%		100%	7								
38	57	33.5%		90%									
39	72	41.1%											
40	66	48.0%		80%									
41 42	73 64	55.8% 62.5%	겉	70%	+					/			
43	59	68.7%	ulative Percent	60%						/			
44	67	75.8%	- Je	50%					/				
45	51	81.2%	<u>.</u>										
46	51	86.6%] <u>÷</u>	40%	+				/				
47	39	90.7%] =	30%	1				_/				
48	27	93.6%	Cum						/ /				
49	22	95.9%	₹	20%	1								
50 51	16 11	97.6% 98.7%	1	10%	+								
52	9	99.7%	1	0%	1			_/		$\perp \perp \perp \perp$			
53	<u> </u>	33.770	1	2,3	0	10	20	30	40	50	60	70	80
54	2	99.9%	1		J	10	20				00	,0	30
55	1	100.0%							Speed (m	ph)			
56					Data Plot	•		50	th Percen	tile		85th Percent	tile
57			ļ									Court Creen	
58 50			ł	——9	90th Per	centile		 95	th Percen	tile			
59 60	 	+											
61			1					DATA	ANALYSI	S			
62			_		C l		40.0		_			26 55	
63			L A	verage :	speed	<u> </u>	40.6		$oldsymbol{oldsymbol{oldsymbol{L}}^{R}}$	ange		26 - 55	
64			50	Oth Pero	entile		40.3		10 m	ph Pace		37 - 46	
65			ر	טנוו רפונ	Jennie	ļ	40.3		1011	יףוו רמנפ		37 - 40	
66			85	5th Pero	centile		45.7		Numb	er in Pace		611	
67 68	 					+			-		+		
69	 	 	90	Oth Pero	centile		46.8		Perce	nt in Pace		65%	
70	 	 				 	46.5		 		†		
Total	947		95	5th Pero	centile		48.6						
		•							•				



RADAR SPEED SURVEY SAN DIEGO COUNTY DEPT OF PUBLIC WORKS

William Control	Docall													
Road Na	me:	Avocado	ВІ		From:	El Cajor	n CL			To:	N	∕ladrid V	Vy	
Position	:	280' S/o	Morr	ning Sta	r Dr					Direct	tion: N	NB/SB		
Date:		12/12/20	024		Weathe	er:	Clea	r		Proje	ct Numb	er:	24-040210-0	002
Time Sta	art:	11:00 AN	V		Road Co	ondition	Dry			Obsei	rver:		Contractor	
Time En	d:	1:00 PM			Posted	Speed:	45 N	ЛРH		Calibr	ration Te	st:	Υ	
Speed (mph)	Num. Veh.	Cum. Pct.			•			Nun	nber of \	/ehicles				
15	ven.	1 00.			0	10	20		30	40		50	60	70
16 17				15	+									
18				20	3									
19 20				25	3									
21		1		30	<u></u>									
22			(ر											
23			ldu	35						_				
24 25		\vdash	Speed (mph)	40									_	
26			eec	45										
27			Sp	50										
28 29	2	0.3%												
30		0.570		55	Ī									
31	8	1.3%		60]									
32 33	11 21	2.6% 5.3%		65										
34	29	8.9%		70]									
35	38	13.7%		70										
36	44	19.2%		100%	_									
37 38	39 47	24.2% 30.1%												
39	57	37.2%		90%										
40	55	44.2%		80%										
41 42	52 56	50.7% 57.7%	Ħ	70%	+									
43	55	64.7%	lative Percent	60%	+									
44	55	71.6%	Ь	50%						/				
45 46	61 50	79.2% 85.5%	<u>×</u>	40%										
47	33	89.7%	<u> a</u>	30%					/					
48	25	92.8%	Cumu											
49	20	95.3%	5	20%										
50 51	16 12	97.4% 98.9%		10%	+									
52	6	99.6%		0%	+	-			<u> </u>					
53	2	99.9%			0	10	20	30	4	0	50	60	70	80
54 55	1	100.0%							Speed (mph)				
56					Data Plot				th Perce				35th Percent	tilo
57													ooui reiceni	uie
58 59		++		——9	90th Pero	centile		 95	th Perc	entile				
60								DATA	ANIALY	cic				
61						_		DAIA	ANALY	313				
62 63			A٠	verage :	Speed		41.3			Range			29 - 54	
64				Oth Pero			40.9		10) ₅₀		37 - 46	
65			50	ui rero	entile	<u> </u>	40.9		10	mph Pa	ice		37 - 40	
66 67			85	th Perd	entile		45.9		Nun	nber in I	Pace		527	
68 69			90	th Pero	entile		47.1		Pero	cent in F	Pace		66%	
70				oth Pero			48.9							
Total	795		95	oui rero	entile	<u> </u>	48.9		ļ					

Old Highway 395 & Canoita Dr/Stewart Canyon Rd



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>5-A</u>

SUPERVISORIAL DISTRICT: 5

SUBJECT: Intersection Control

LOCATION: Old Highway 395 & Canonita Drive/Stewart Canyon

Road, MONSERATE

INITIATED BY: DPW Traffic Engineering

REQUEST: Traffic Control Signal

PROBLEM AS STATED BY REQUESTER:

The intersection of Old Highway 395 and Canonita Drive/Stewart Canyon Road has been identified by Traffic Engineering as meeting 2 warrants – 1, eight-hour vehicular volume warrant, and 2, four-hour vehicular volume warrant – of the 9 traffic signal warrants as described in the California Manual on Uniform Traffic Control Devices (CA MUTCD), Chapter 4C, therefore a traffic control signal should be considered

Existing Traffic Devices

Old Highway 395 is a striped two-lane, 40 to 85-foot wide, undivided highway. The roadway is striped with a passing centerline, lane lines, bike lanes, and a left turn lane in both directions at the intersection with Canonita Drive/Stewart Canyon Road. Old Highway 395 is signed with an intersection ahead warning sign. The road is classified as a Community Collector Road on the County General Plan Mobility Element Network. The road has no posted speed limit.

Canonita Drive is a striped two-lane, 24 to 30-foot wide, undivided highway. The roadway is striped with a no passing centerline approaching the intersection. Canonita Drive is stop controlled at the intersection with Old Highway 395. The road is unclassified on the County General Plan Mobility Element Network. The road has no posted speed limit.

Stewart Canyon Road is a striped two-lane, 42-foot wide, undivided highway. The roadway is striped with a passing centerline and white edgeline. Stewart Canyon Road is stop controlled at the intersection with Old Highway 395. The road is classified as a Major Road on the County General Plan Mobility Element Network. The road has no posted speed limit.

Average Daily Traffic Volumes 10/24

Old Highway 395:

N/o Canonita Drive/Stewart Canyon Road 4,257 SB S/o Canonita Drive/Stewart Canyon Road 4,137 NB

Canonita Drive:

W/o Old Highway 395 283 EB

Stewart Canyon Road:

E/o Old Highway 395 1,775 WB

Collision Data

There have been 7 reported collisions along this segment of roadway, 4 of which involved an injury, and 1 of which included a fatality, in a 3-year period (2022-01-01 to 2024-12-31). 5 of these collisions are susceptible to correction by a signal installation. These collisions result in an intersection accident rate of 0.61 collisions per million vehicles entering. The statewide average is 0.36 collisions per million vehicle miles for similar four legged intersections with stop signs (excluding 4-way stops).



PUBLIC WORKS

WILLIAM MORGAN, P.E.
INTERIM DIRECTOR OF PUBLIC
WORKS

5510 OVERLAND AVENUE, SUITE 410, SAN DIEGO, CALIFORNIA 92123-1237 (858) 694-2212

COUNTY TRAFFIC ENGINEER RECOMMENDATION

Date: January 17, 2025

Item Title: Traffic Control Signal

Location: Old Highway 395 and Canonita Drive/Stewart Canyon Road

The County Traffic Engineer recommends placing the intersection of Old Highway 395 and Canonita Drive/Stewart Canyon Road on the DPW traffic signal list, pursuant to the following conditions:

- Section 21351 of the California Vehicle Code (CVC) authorizes a local agency to place and maintain or cause to be placed and maintained traffic signs, signals and other traffic control devices upon streets and highways within their jurisdiction as may be necessary to warn and guide traffic.
- Chapter 4C "Traffic Control Signal Needs Studies" of the California Manual on Uniform Traffic Control Devices (MUTCD), provides guidance for the preparation of an engineering study of traffic conditions to determine whether a traffic control signal is justified.
- The intersection total approach ADT is 10,452 vehicles/day.
- During the period of January 1, 2022, to December 31, 2024, there were 7 collisions at the intersection. These collisions resulted in an intersection accident rate of 0.61 vs the statewide average for similar intersections of 0.36 collision per million vehicles entering.
- An engineering study following Chapter 4C of the California MUTCD guidelines, shows
 that the subject intersection meets the Eight Hour Vehicular Volume Warrant and Four
 Hour Vehicular Volume Warrant based on the intersection traffic conditions. Hence, a
 traffic signal control can be considered for the intersection of Old Highway 395 and
 Canonita Drive/Stewart Canyon Road.

Michael L. Kenney, TE 2045 & CE 56661

Michael Kenney

1/17/25

Date

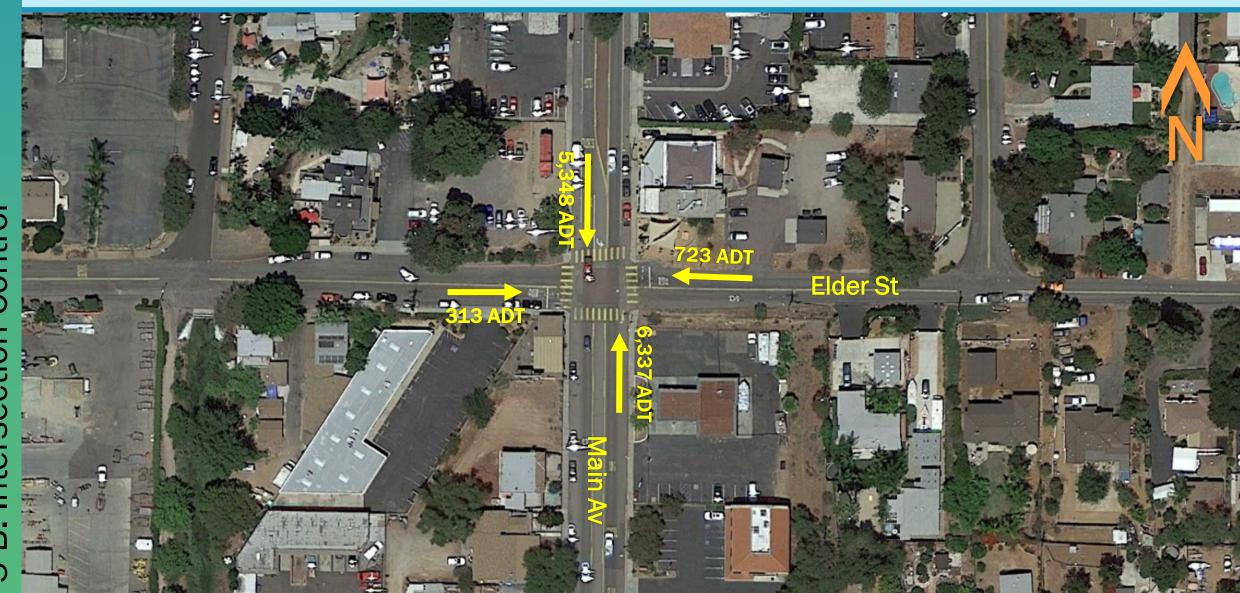
VOLUME

Old Hwy 395 & Canonita Dr

Day: Tuesday Date: 12/12/2023 City: Fallbrook
Project #: CA23_040262_001

	D	AILY T	OT/	AI S			NB		SB		EB		WB							To	otal
		AILI I	017	1LJ			4,137	,	4,257		283		1,775							10,	,452
AM Period	NB		SB		EB		WB		TC	TAL	PM Period	NB		SB		EB		WB		ТО	TAL
0:00 0:15	6		7 1		0		1		14 4		12:00 12:15	34 56		50 68		2 5		39 43		125 172	
0:30	2		1		0		3		6		12:30	61		55		1		31		148	
0:45	4	15	2	11	0		0	4	6	30	12:45	48	199	69	242	4	12	21	134	142	587
1:00 1:15	1 1		3 2		0		0 1		4 4		13:00 13:15	67 54		57 60		7 8		28 26		159 148	
1:30	0		2		0		2		4		13:30	74		67		3		33		177	
1:45	2	4	0	7	0		1	4	3	15	13:45	57	252	70	254	6	24	29	116	162	646
2:00 2:15	2		3 2		0		1 0		6 2		14:00 14:15	59 69		68 40		1 5		39 31		167 145	
2:30	1		2		0		0		3		14:30	76		74		6		27		183	
2:45	3	6	1	8	0		0	1	4	15	14:45	106	310	78	260	4	16	32	129	220	715
3:00	1		2		0		1		4		15:00 15:15	104		81		2		37		224	
3:15 3:30	3 1		4 3		1 0		3 4		11 8		15:30	115 126		62 110		2 5		29 32		208 273	
3:45	0	5	0	9	Ö	1	3	11	3	26	15:45	132	477	79	332	6	15	24	122	241	946
4:00	2		5		1		1		9		16:00	123		90		3		25		241	
4:15 4:30	4 5		8 19		0 3		4 5		16 32		16:15 16:30	122 143		67 66		3 6		28 28		220 243	
4:45	1	12	28	60	0	4	1	11	30	87	16:45	143	535	72	295	5	17	31	112	255	959
5:00	4		48		1		9		62		17:00	161		67		5		31		264	
5:15	5		57		3		8		73		17:15	144		62		3		43		252	
5:30 5:45	9 10	28	57 85	247	4 3	11	8 16	41	78 114	327	17:30 17:45	139 146	590	63 73	265	3 4	15	20 19	113	225 242	983
6:00	16		86	247	6		15		123	327	18:00	102	330	42	203	2	13	21	113	167	303
6:15	18		91		3		20		132		18:15	103		52		5		10		170	
6:30 6:45	26 28	88	78 83	338	8 5	22	21 32	88	133 148	E26	18:30 18:45	86 62	252	34 36	164	5 1	12	15 22	68	140 121	EOR
7:00	15	88	91	338	7	22	40	88	153	536	19:00	43	353	31	164	3	13	19	80	96	598
7:15	40		86		6		38		170		19:15	36		29		1		24		90	
7:30	41		92		4		63		200		19:30	43		29		1	_	20		93	
7:45 8:00	54 39	150	97 93	366	<u>8</u> 6	25	57 45	198	216 183	739	19:45 20:00	34 30	156	30 27	119	2	5	10 14	73	74 73	353
8:15	53		88		4		29		174		20:15	34		27		1		10		72	
8:30	41		77		5		27		150		20:30	27		25		4		10		66	
8:45 9:00	27 31	160	64 68	322	<u>3</u> 7	18	23 29	124	117 135	624	20:45 21:00	22 26	113	24 28	103	1	8	<u>7</u> 4	41	54 59	265
9:15	37		54		8		29 37		135		21:00 21:15	26		28 18		0		7		52	
9:30	46		48		10		29		133		21:30	23		21		3		7		54	
9:45	35	149	61	231	4	29	27	122	127	531	21:45	24	100	25	92	1	5	7	25	57	222
10:00 10:15	35 37		56 49		5 6		36 24		132 116		22:00 22:15	20 9		10 8		0 2		5 3		35 22	
10:30	48		82		5		24		159		22:30	15		15		1		2		33	
10:45	46	166	62	249	4	20	20	104	132	539	22:45	14	58	5	38	0	3	2	12	21	111
11:00	52		55 40		10		33		150		23:00 23:15	10		6		0		2		18	
11:15 11:30	33 45		49 70		4 2		33 23		119 140		23:15 23:30	11 13		4 4		0 0		3 2		18 19	
11:45	41	171	53	227	4	20	25	114	123	532	23:45	6	40	4	18	0		1	8	11	66
TOTALS		954		2075		150		822		4001	TOTALS		3183		2182		133		953		6451
SPLIT %		23.8%		51.9%		3.7%		20.5%		38.3%	SPLIT %		49.3%		33.8%		2.1%		14.8%		61.7%
							NB		SB		EB		WB							To	otal
	D	AILY T	OTA	ALS		•	4,137	,	4,257		283		1,775								,452
ANA Destruction		11.45		7,20							DM Dock Have				15.20		12.00		12.00		16:20
AM Peak Hour AM Pk Volume		11:45 192		7:30 370		9:00 29		7:15 203		7:30 773	PM Peak Hour PM Pk Volume		16:30 595		15:30 346		13:00 24		12:00 134		16:30 1014
Pk Hr Factor		0.787		0.954		0.725		0.806		0.895	Pk Hr Factor		0.924		0.786		0.750		0.779		0.960
7 - 9 Volume		310		688		43		322		1363	4 - 6 Volume		1125		560		32		225		1942
7 - 9 Peak Hour		7:30		7:30		7:00		7:15		7:30	4 - 6 Peak Hour		16:30		16:00		16:15		16:30		16:30
7 - 9 Pk Volume		187		370		25		203		773	4 - 6 Pk Volume		595		295		19		133		1014
Pk Hr Factor		0.866		0.954		0.781		0.806		0.895	Pk Hr Factor		0.924		0.819		0.792		0.773		0.960

Main Avenue & Elder Street



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>5-B</u>

SUPERVISORIAL DISTRICT: 5

SUBJECT: Intersection Control

LOCATION: Main Avenue & Elder Street, FALLBROOK

INITIATED BY: DPW Traffic Engineering

REQUEST: All-Way Stop Controls

PROBLEM AS STATED BY REQUESTER:

The intersection of Main Avenue and Elder Street has been identified by Traffic Engineering as meeting Option B, an intersection where there is the need to control vehicle/pedestrian conflicts near locations that generate high pedestrian volumes, and Option C, an intersection where motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection, of the Multi-Way Stop Application optional criteria as described in the California Manual on Uniform Traffic Control Devices (CA MUTCD), Section 2B.07, therefore an all-way stop control should be considered.

Existing Traffic Devices

Main Avenue is a striped two-lane, 50-foot wide, undivided highway. The roadway is striped with a two way left turn lane and an uncontrolled school crossing. The road is posted with a school crossing assembly sign. The road is unclassified on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Elder Street is a striped two-lane, 30 to 38-foot wide, undivided highway. The roadway is striped with a no passing centerline and controlled crossing at Main Avenue. Elder Street is stop controlled at the intersection with Main Avenue. The road is unclassified on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Average Daily Traffic Volumes	<u>10/24</u>
Main Avenue:	
N/o Elder Street	5,348 SB
S/o Elder Street	6,337 NB
Elder Street:	
E/o Main Avenue	723 WB
W/o Main Avenue	313 EB

Collision Data

There has been 1 reported collision along this segment of roadway, in a 3-year period

(2022-01-01 to 2024-12-31). This collision is susceptible to correction by an all-way stop installation. This collision resulted in a segment accident rate of 0.07 collision per million vehicles entering. The statewide average is 0.36 collisions per million vehicle miles for similar four-legged intersections with stop signs (excluding 4-way stops).



PUBLIC WORKS

WILLIAM MORGAN, P.E.
INTERIM DIRECTOR OF PUBLIC
WORKS

5510 OVERLAND AVENUE, SUITE 410, SAN DIEGO, CALIFORNIA 92123-1237 (858) 694-2212

COUNTY TRAFFIC ENGINEER RECOMMENDATION

Date: January 17, 2025

Item Title: All-Way Stop Control

Location: Main Avenue and Elder Street

The County Traffic Engineer recommends installing all-way stop controls at the intersection of Main Avenue and Elder Street, pursuant to the following conditions:

- Section 21354 "Stop Signs on Local Highways" of the California Vehicle Code (CVC) authorizes local agencies to designate any intersection under its exclusive jurisdiction as a stop intersection.
- Section 2B.07 "Multi-Way Stop Applications" of the California Manual on Uniform Traffic Control Devices (MUTCD) provides guidelines that should and/or may be considered in an engineering study when evaluating an intersection for an all-way stop control.
- Option B of Section 2B.07 An All-Way Stop Control may be considered to control vehicle/pedestrian conflicts near locations that generate high pedestrian volumes, such as schools. The subject intersection of Main Avenue and Elder Street is located within Maie Ellis Elementary School.
- Option C of Section 2B.07 Lack of sight distance, indicates all-way stop controls can be considered when motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection.
- The operational sight distance for the eastbound approach of Elder Street, looking north and south, does not meet the minimum required operational sight distance per County Public Road Standards.

chael L. Kenney, TE 2045 & CE 56661	Date	
lichael Kenney	1/17/25	
1: has Kana		

per County Public Road Standards.

• The operational sight distance for the westbound approach of Elder Street, looking north, does not meet the minimum required operational sight distance

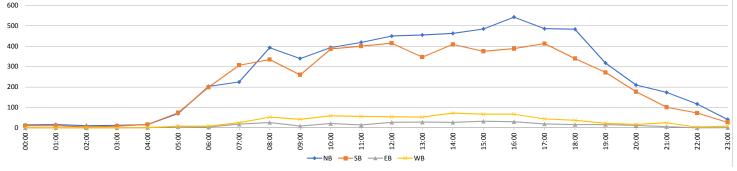
VOLUME

Main Ave & Elder St

 Day: Thursday
 City: Fallbrook

 Date: 09/07/2023
 Project #: CA23_040172_001

TIME 0:00 0:15 0:30 0:45 1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15 3:30	NB 2 4 5 3 4 4 3 5 0 4 4 2 4	SB 5 0 3 2 2 5 3 0 1 2 0 1	EB 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		5-Minutt TOTAL 8 4 8 5 6 9 6 5		6,337 NB 118 112 130 90 98 121 105	\$\frac{\sqrt{5,348}}{\sqrt{8}}\$ \$\frac{135}{92}\$ \$87\$ \$101\$ \$70\$ \$86\$	EB 4 6 9 8 7	723 WB 22 7 9	12,721 TOTAL 279 217	TIME 00:00 01:00	Hour NB	rly Inte		WB 1 0	TOTAL 25
0:00 0:15 0:30 0:45 1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	2 4 5 3 4 4 3 5 0 4 4 2	5 0 3 2 2 5 3 0 1 2 0 1	0 0 0 0 0 0 0 0	WB 1 0 0 0 0 0 0 0 0 0 0 0 0 0	8 4 8 5 6 9 6 5	12:00 12:15 12:30 12:45 13:00 13:15 13:30	NB 118 112 130 90 98 121	135 92 87 101 70	4 6 9 8	22 7 9	279	00:00 01:00	NB 14	SB 10	EB	1	25
0:00 0:15 0:30 0:45 1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	2 4 5 3 4 4 3 5 0 4 4 2	5 0 3 2 2 5 3 0 1 2 0 1	0 0 0 0 0 0 0 0	1 0 0 0 0 0 0 0 0	8 4 8 5 6 9 6 5	12:00 12:15 12:30 12:45 13:00 13:15 13:30	118 112 130 90 98 121	135 92 87 101 70	4 6 9 8	22 7 9	279	00:00 01:00	14	10	0	1	25
0:15 0:30 0:45 1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	4 5 3 4 4 3 5 0 4 4 2	0 3 2 2 5 3 0 1 2 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	4 8 5 6 9 6 5	12:15 12:30 12:45 13:00 13:15 13:30	112 130 90 98 121	92 87 101 70	6 9 8	7 9							
0:30 0:45 1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	5 3 4 4 3 5 0 4 4 2	3 2 2 5 3 0 1 2 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	8 5 6 9 6 5	12:30 12:45 13:00 13:15 13:30	130 90 98 121	87 101 70	9 8	9	217		16	10	0	0	20
0:45 1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	3 4 4 3 5 0 4 4 2	2 2 5 3 0 1 2 0 1	0 0 0 0 0 0	0 0 0 0 0 0	5 6 9 6 5	12:45 13:00 13:15 13:30	90 98 121	101 70	8			01:00 02:00	10				26
1:00 1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	4 4 3 5 0 4 4 2	2 5 3 0 1 2 0 1	0 0 0 0 0	0 0 0 0 0	6 9 6 5	13:00 13:15 13:30	98 121	70			235	02:00 03:00	10	4	0	0	14
1:15 1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	4 3 5 0 4 4 2	5 3 0 1 2 0 1	0 0 0 0 0	0 0 0 0	9 6 5	13:15 13:30	121		7	16	215	03:00 04:00	12	8	0	0	20
1:30 1:45 2:00 2:15 2:30 2:45 3:00 3:15	3 5 0 4 4 2	3 0 1 2 0 1	0 0 0 0 0	0 0 0 0	6 5	13:30		86		16	191	04:00 05:00		17	1	1	35
1:45 2:00 2:15 2:30 2:45 3:00 3:15	5 0 4 4 2	0 1 2 0 1	0 0 0 0	0 0 0	5		105		7	14	228	05:00 06:00	70	74	6	8	158
2:00 2:15 2:30 2:45 3:00 3:15	0 4 4 2 4	1 2 0 1	0 0 0	0	_	13:45		100	7	11	223	06:00 07:00		199	4	8	414
2:15 2:30 2:45 3:00 3:15	4 4 2 4	2 0 1	0	0	1	44.00	131	91	7	12	241	07:00 08:00	225	307	18	25	575
2:30 2:45 3:00 3:15	4 2 4	0 1	0		_	14:00	116	105	5	11	237	08:00 09:00	393	335	26	53	807
2:45 3:00 3:15	2	1		U	6	14:15	118	100	4	15	237	09:00 10:00	340	260	9	42	651
3:00 3:15	4				4	14:30	109	107	3	29	248	10:00 11:00		387	21	59	861
3:15			0	0	3 4	14:45	120	97 84	15 10	17 13	249 223	11:00 12:00 12:00 13:00	419	400	14 27	56	889
ll ll		0				15:00	116				_		450	415		54 52	946
3:3U	3 2	2 2	0 0	0 0	5 4	15:15 15:30	114 122	92 95	6 7	14 23	226	13:00 14:00	455 463	347 409	28 27	53 72	883 971
2.45	3	4	0	0	7	15:30 15:45	133	95 104	9	23 17	247	14:00 15:00 15:00 16:00	463 485	409 375	32	72 67	959
3:45	1	2	0	0	3	16:00	147	115	5	22	263 289		543		30		
4:00 4:15	4	6	1	1	12	16:00	147	86	5 9	16	259	16:00 17:00 17:00 18:00	486	388 413	30 19	67 44	1028 962
4:30	4	3	0	0	7	16:30	125	99	5	11	240	18:00 19:00	483	340	16	37	876
4:45	7	6	0	0	13	16:45	131	88	5 11	18	240	19:00 20:00	318	272	16	22	628
5:00	13	15	2	0	30	17:00	130	109	8	15	262	20:00 21:00	210	177	12	22 17	416
5:00 5:15	11	8	1	0	20	17:00	131	109	4	10	253	21:00 22:00		101	6	25	306
5:30	23	20	0	6	49	17:15	119	108	5	13	239	22:00 23:00		73	0	25 4	194
5:45	23	31	3	2	59	17:45	106	94	2	6	208	23:00 00:00	41	73 27	1	8	77
			1	4					5	8		23.00 00.00		ATISTI		8	
6:00	35	39			79	18:00	132	94			239					MAZD	TOTAL
6:15	38	42	0	1	81	18:15	123	95	1	9	228		NB	SB	EB	WB	TOTAL
6:30	62	55	2	3	122	18:30	120	70	5	14	209	Peak Perio		to	12:00		
6:45	68	63	1	0	132	18:45	108	81	5	6	200	Volum		2011	99	253	4475
7:00	43	58	3	1	105	19:00	80	88	3	7	178	Peak Hou		11:00	7:45	10:15	11:00
7:15	44	68	5	6	123	19:15	83	69	1	7	160	Peak Volum		400	31	61	889
7:30	62	91	3	10	166	19:30	80	67	6	3	156	Peak Hour Facto	r 0.895	0.833	0.861	0.953	0.875
7:45	76	90	7	8	181	19:45	75	48	6	5	134						
8:00	99	105	9	20	233	20:00	58	58	4	8	128	Peak Perio		to	00:00	470	0246
	106	88	9	10	213	20:15	67	40	2	2	111	Volum		3337	214	470	8246
8:30 8:45	88 100	73 60	6	12	179	20:30	34 51	41	4	7 0	86	Peak Hou		12:00	14:45	15:30	15:30
	100	69	0	11	182	20:45	51	38	2	5	91	Peak Volum		415	38	78	1050
9:00	76	54 EE		12	142	21:00	47	30			84	Peak Hour Facto	r 0.927	0.769	0.633	0.848	0.908
9:15	81 86	55 66	2 4	9 10	147	21:15	41	30 22	2 2	9 7	82 72	Deed Deed	4 67.00		00.00		
9:30	86			10	166	21:30 21:45	41 45	22		4	72 68	Peak Perio		to	09:00	70	1202
9:45 10:00	97 85	85	3	11 12	196 212		45 35	19 27	0	2		Volum Poak Hou		642 7:20	44 7:45	78 8:00	1382
10:00	85 95	111 90	4 3	16	204	22:00 22:15	35 37	27 29	0	2	64 68	Peak Hou		7:30 374	7:45	8:00 53	8:00 807
ll ll	95 100	90 85	3 8	15	204	22:15	37 28	29 13	0	0	68 41	Peak Volum Peak Hour Facto		374 0.890	31 0.861	0.663	
ll ll	114	85 101	6	16	208	22:30	28 17	4	0	0	21	reak Hour Facto	0.927	0.890	0.801	0.003	0.866
11:00	99	101	1	14	223	23:00	15	9	0	3	27	Peak Perio	d 16:00	to	18:00		
ll ll	102	77	6	12	197	23:00	15 14	9 12	0	0	26	Volum		to 801	49	111	1990
ll ll	102	94	5	15	215	23:15	14 5	5	1	4	26 15			17:00		16:00	16:00
	101	94 120	2	15 15	254	23:30	5 7	5 1	0	1	9	Peak Hou Peak Volum			16:15 33	67	1028
											_			413			
	2112	2011	99	253	4475	TOTALS	4225	3337	214	470	8246	Peak Hour Facto	r 0.923	0.947	0.750	0.761	0.889
SPLIT %	47%	45%	2%	6%	35%	SPLIT %	51%	40%	3%	6%	65%						<u> </u>



Main Avenue & Ivy Street



SAN DIEGO COUNTY TRAFFIC ADVISORY COMMITTEE

COMMITTEE REPORT OF: February 7, 2025 Item <u>5-C</u>

SUPERVISORIAL DISTRICT: 5

SUBJECT: Intersection Control

LOCATION: Main Avenue & Ivy Street, FALLBROOK

INITIATED BY: DPW Traffic Engineering

REQUEST: All-Way Stop Controls

PROBLEM AS STATED BY REQUESTER:

The intersection of Main Avenue and Ivy Street has been identified by Traffic Engineering as meeting Option B, an intersection where there is the need to control vehicle/pedestrian conflicts near locations that generate high pedestrian volumes, and Option C, an intersection where motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection, of the Multi-Way Stop Application optional criteria as described in the California Manual on Uniform Traffic Control Devices (CA MUTCD), Section 2B.07, therefore an all-way stop control should be considered.

Existing Traffic Devices

Main Avenue is a striped two-lane, 50-foot wide, undivided highway. The roadway is striped with a two way left turn lane and an uncontrolled crossing. Main Avenue is signed with a 7-ton truck weight restriction. The road is unclassified on the County General Plan Mobility Element Network. The road has a posted 25 MPH speed limit.

Ivy Street is a two-lane, 24 to 38-foot wide, undivided highway. The roadway is striped with a controlled crossing at Main Avenue. Ivy Street is stop controlled at the intersection with Main Avenue. The road is unclassified on the County General Plan Mobility Element Network. The road has no posted speed limit.

Average Daily Traffic Volumes	<u>10/24</u>
Main Avenue:	
N/o Ivy Street	2,470 SB
S/o Ivy Street	3,952 NB
Ivy Street:	
E/o Main Avenue	554 WB
W/o Main Avenue	232 EB

Collision Data

There have been 1 reported collision along this segment of roadway, which involved an injury, in a 3-year period (2022-01-01 to 2024-12-31). This collision is susceptible to

correction by an all-way stop installation. This collision result in an intersection accident rate of 0.13 collisions per million vehicles entering. The statewide average is 0.36 collisions per million vehicle miles for similar four-legged intersections with stop signs (excluding 4-way stops).



PUBLIC WORKS

WILLIAM MORGAN, P.E.
INTERIM DIRECTOR OF PUBLIC
WORKS

5510 OVERLAND AVENUE, SUITE 410, SAN DIEGO, CALIFORNIA 92123-1237 (858) 694-2212

COUNTY TRAFFIC ENGINEER RECOMMENDATION

Date: January 17, 2025

Item Title: All-Way Stop Control

Location: Main Avenue and Ivy Street

The County Traffic Engineer recommends installing all-way stop controls at the intersection of Main Avenue and Ivy Street, pursuant to the following conditions:

- Section 21354 "Stop Signs on Local Highways" of the California Vehicle Code (CVC) authorizes local agencies to designate any intersection under its exclusive jurisdiction as a stop intersection.
- Section 2B.07 "Multi-Way Stop Applications" of the California Manual on Uniform Traffic Control Devices (MUTCD) provides guidelines that should and/or may be considered in an engineering study when evaluating an intersection for an all-way stop control.
- Option B of Section 2B.07 An All-Way Stop Control may be considered to control vehicle/pedestrian conflicts near locations that generate high pedestrian volumes, such as commercial areas. The subject intersection of Main Avenue and Ivy Street is located within Fallbrook Downtown District.
- Option C of Section 2B.07 Lack of sight distance, indicates all-way stop controls can be considered when motorists are unable to see conflicting traffic to determine when it is safe to enter the intersection.
- The operational sight distance for the eastbound approach of Ivy Street, looking north, does not meet the minimum required operational sight distance per County Public Road Standards.

Michael Kenney	1/17/25	
Michael L. Kenney, TE 2045 & CE 56661	Date	

VOLUME

Main Ave & Ivy St

 Day: Thursday
 City: Fallbrook

 Date: 09/07/2023
 Project #: CA23_040172_005

TIME							3.952	2.470	232	554	7,208		DAIL		17123		
TIDAE				11	5-Minute	sa lukaw		2,470	232	334	7,200		Haur	b. Inte	meda		
	NB	SB	ЕВ	WB I:	TOTAL	TIME	vai NB	SB	ЕВ	WB	TOTAL	TIME	NB	ly Inte SB	ervais EB	WB	TOTAL
0:00	2	2	0	0	4	12:00	73	50	5	12	140	00:00 01:00	10	6	0	2	18
0:15	1	0	0	1	2	12:15	50	40	6	6	102	01:00 02:00	9	4	1	1	15
0:30	5	2	0	0	7	12:30	68	41	4	8	121	02:00 03:00	5	3	0	1	9
0:45	2	2	0	1	5	12:45	53	44	3	7	107	03:00 04:00	7	2	0	0	9
1:00	4	1	0	0	5	13:00	52	29	0	8	89	04:00 05:00	15	7	3	3	28
1:15	2	3	0	0	5	13:15	63	40	4	4	111	05:00 06:00	44	35	6	10	95
1:30	0	0	1	1	2	13:30	52	53	6	7	118	06:00 07:00	146	72	6	37	261
1:45	3	0	0	0	3	13:45	64	36	7	9	116	07:00 08:00	156	104	10	31	301
2:00 2:15	0 2	1 1	0	0 0	1	14:00 14:15	68 57	37 40	2	5 12	112 111	08:00 09:00 09:00 10:00	216 154	130 149	15 6	45 20	406 329
2:15	2	0	0	1	3	14:15	66	40 58	3	8	135	10:00 11:00	194	189	12	20 25	420
2:45	1	1	0	0	2	14:45	64	37	3	9	113	11:00 12:00	207	179	12	29	420
3:00	0	0	0	0	0	15:00	80	40	2	9	131	12:00 13:00	244	175	18	33	470
3:15	2	0	0	0	2	15:15	73	47	4	12	136	13:00 14:00	231	158	17	28	434
3:30	2	0	0	0	2	15:30	82	39	6	12	139	14:00 15:00	255	172	10	34	471
3:45	3	2	0	0	5	15:45	83	42	10	13	148	15:00 16:00	318	168	22	46	554
4:00	0	1	1	1	3	16:00	90	43	9	19	161	16:00 17:00	371	184	22	63	640
4:15	3	3	1	2	9	16:15	91	46	2	16	155	17:00 18:00	340	185	31	53	609
4:30	5	2	0	0	7	16:30	105	46	5	14	170	18:00 19:00	337	171	23	38	569
4:45	7	1	1	0	9	16:45	85	49	6	14	154	19:00 20:00	260	148	8	24	440
5:00	10	6	2	3	21	17:00	99	49	11	10	169	20:00 21:00	160	107	7	14	288
5:15	9	5	0	0	14	17:15	76	49	8	14	147	21:00 22:00	141	63	1	13	218
5:30	13	9	3	4	29	17:30	88	35	7	17	147	22:00 23:00	97	42	2	4	145
5:45	12	15	1	3	31	17:45	77	52	5	12	146	23:00 00:00	35	17	0	0	52
6:00	28	14	0	4	46	18:00	91	42	9	10	152			ATIST			
6:15	31	20	3	9	63	18:15	92	51	8	7	158		NB	SB	EB	WB	TOTAL
6:30	46	17	1	12	76	18:30	80	35	1	11	127	Peak Period	00:00	to	12:00		
6:45	41 35	21 16	2	12 2	76 55	18:45 19:00	74 75	43	5 3	10	132 124	Volume	1163 8:00	880 10:00	71 7:30	204 7:30	2318 11:00
7:00 7:15	35 35	26	2	10	73	19:00	75 68	40	3 1	6 7	119	Peak Hour Peak Volume	216	189	7:30 15	7:30 52	427
7:30	42	33	3	9	73 87	19:30	52	43	0	5	100	Peak Hour Factor	0.900	0.892	0.750	0.619	0.828
7:45	44	29	3	10	86	19:45	65	22	4	6	97	reak Hour ractor	0.300	0.032	0.730	0.013	0.020
8:00	57	35	5	21	118	20:00	42	33	1	4	80	Peak Period	12:00	to	00:00		
8:15	60	37	4	12	113	20:15	51	23	1	6	81	Volume	2789	1590	161	350	4890
8:30	53	29	2	9	93	20:30	29	29	4	2	64	Peak Hour	16:15	16:30	16:45	16:00	16:15
8:45	46	29	4	3	82	20:45	38	22	1	2	63	Peak Volume	380	193	32	63	648
9:00	34	27	2	5	68	21:00	46	17	0	5	68	Peak Hour Factor	0.905	0.985	0.727	0.829	0.953
9:15	35	33	2	5	75	21:15	34	16	1	4	55						
9:30	45	44	1	8	98	21:30	29	13	0	1	43	Peak Period	07:00	to	09:00		
9:45	40	45	1	2	88	21:45	32	17	0	3	52	Volume	372	234	25	76	707
10:00	49	50	0	8	107	22:00	38	15	2	1	56	Peak Hour	8:00	7:30	7:30	7:30	7:45
10:15	48 52	40 46	2 4	6 4	96 106	22:15 22:30	31	13 9	0 0	2 1	46	Peak Volume	216	134	15	52	410
10:30 10:45	52 45	46 53	4 6	4 7	106	22:30	18 10	5	0	0	28 15	Peak Hour Factor	0.900	0.905	0.750	0.619	0.869
11:00	40	36	2	8	86	23:00	15	7	0	0	22	Peak Period	16:00	to	18:00		
11:15	50	35	1	4	90	23:15	9	5	0	0	14	Volume	711	369	53	116	1249
11:30	58	60	4	7	129	23:30	6	3	0	0	9	Peak Hour	16:15	16:30	16:45	16:00	16:15
11:45	59	48	5	10	122	23:45	5	2	0	0	7	Peak Volume	380	193	32	63	648
TOTALS	1163	880	71	204	2318	TOTALS	2789	1590	161	350	4890	Peak Hour Factor	0.905	0.985	0.727	0.829	0.953
SPLIT %	50%	38%	3%	9%	32%	SPLIT %	57%	33%	3%	7%	68%						

