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Sediment Load Analysis Report Cottonwood Sand Mine Jamacha, California

Submitted to

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A WEPP Hillslope Profiles

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1. Introduction and Background

Geo-Logic Associates (GLA) has prepared this report for the Cottonwood Sand Mine Project (Project) Environmental Impact Report (EIR) as part of the requirements for obtaining a Major Use Permit (MUP) for the extraction of aggregate on the existing Cottonwood Golf Club property. The purpose of this report is to evaluate worst-case sediment erosion and transport at the property and potential impacts to surface water quality as a result of proposed sand mining operations with the worst-case assumption that no best-management practices (BMPs) will be employed. The scope of work is detailed in the Geo-Logic work plan dated 1/4/2020 (Geo-Logic, 2020).

The Cottonwood Sand Mine Project is proposed on the current Cottonwood Golf Club property, replacing the two 18-hole golf courses (referred to as the Lakes and Ivanhoe courses). Specifically, the Project is located in Jamacha Valley at 3121 Willow Glen Drive in southern San Diego County, California. The approximately 280-acre site is situated within the Sweetwater River valley and in the floodplain of the Sweetwater River, which flows in a northeast-to-southwest direction through the site.

The proposed mining operations would extract, process, and transport sand using conventional earth moving and processing equipment. Approximately 4.3 million cubic yards (CY; 6.40 million tons) of material are proposed to be extracted. Mining and extraction activities are expected to produce approximately 3.8 million cy (5.7 million tons) of sand and gravel for market use, with a 10 percent waste factor from the total amount extracted that includes wash fines and materials undesirable for processing. Material extracted and processed at the site would be suitable for construction uses and would be available to customers in San Diego County. Approximately 214 acres of the approximately 280-acre Project site are proposed for extractive use under a phased extraction program. Surface areas not disturbed by mining would be subject to removal of invasive species in the river channel on the southwest portion of the site or be left in their current condition. The existing Sweetwater River channel and the majority of native habitat that currently exists on the site would be retained.

The Project will be fully completed within 12 years. Native habitat will be planted within the channel and slopes. Vegetation monitoring will continue for a minimum of two years and until performance standards are met. At the conclusion of the project, when the plant community is established within the reclaimed area (two years after mining operations are complete), the property will become open space along the Sweetwater channel.

The project proposes to convert two golf courses to a sand mining operation that would be conducted in three phases over 10 years. The project's mining operations would extract,

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process, and transport sand using conventional earth moving and processing equipment.

Approximately 4.3 million cubic yards (CY; 6.40 million tons) of material are proposed to be extracted. Mining and extraction activities are expected to produce approximately 3.8 million CY (5.7 million tons) of sand and gravel for market use. Extraction operations would be limited to a maximum production of 380,000 CY (570,000 tons) of construction grade aggregate (sand) per calendar year, with a 10 percent waste factor from the total amount extracted that includes wash fines and materials undesirable for processing.

Since the release of the Draft EIR, the project description has been updated to include backfilling of the site in order to achieve final elevations. Backfilling would be accomplished using a combination of wash fines and overburden produced from the mining operations and imported fill. Approximately 2.5 million cubic yards would be needed to be imported to the site to fulfill the backfill requirements. The imported material would consist of inert debris transported to the project site at an estimated rate of 250,000 cubic yards per year for the 10-year duration of mining activities. Backfill material import operations would occur from 9:00 a.m. to 3:30 p.m. Monday through Friday to avoid peak traffic periods. In addition to the 88 trucks necessary for daily export of the saleable material, 58 trucks are assumed to commute to the construction site on a daily basis for the import of backfill material. A comparison of the project description information presented in the Public Review Draft EIR and the current Project Description is presented below:

PROJECT DESCRIPTION COMPARISON (VOLUMES AND TRUCKING)

Project Component	Public Review Draft EIR Project Description	Current Project Description
Excavated Sand	4,266,900	4,266,900
Saleable Sand	3,840,210	3,840,210
Waste Sand (10 percent of Excavated Sand)	426,690	426,690
Total Backfill Required	2,928,700	2,928,700
Total Imported Backfill (Backfill – Waste Sand)	0	2,502,010
Export Trucks (Average/Day)	88.6	88.6
Import Trucks (Average/Day)	0	57.7

^{*}All volumes are reported in cubic yards



2. Sediment Erosion Modeling

Sediment erosion analysis was conducted with the Water Erosion Prediction Project (WEPP) model. WEPP was created to replace the Universal Soil Loss Equation (USLE) and it has been continuously improved upon since its release in 1995. WEPP has been shown to provide sufficient accuracy at predicting total soil erosion and sediment load leaving a hillslope (Saghafian et al., 2015; Pandey et al., 2008). WEPP considers climate, topography, soil, and land use data and generates soil detachment, deposition, total sediment exiting in runoff, and the size distribution of the exiting sediment.

2.1 Planned Stormwater Pollution Prevention Measures

A Storm Water Pollution Prevention Plan (SWPPP) will be prepared and submitted to the State Water Resources Control Board (SWRCB) prior to construction in accordance with the Industrial General Permit Order 2014-0057-DWQ, effective July 1, 2015. The SWPPP and erosion control plan will define best management practices (BMPs) to prevent erosion and the discharge of sediment to surface waters.

During mining, the Project site will establish temporary de-siltation basins that will be utilized to capture runoff from existing culverts within Willow Glen Drive and to prevent sediment from leaving the site while allowing water to pass through to existing drainage features. Mining and reclamation grading will direct runoff from the disturbed areas towards the basins, as necessary, to allow for desiltation and infiltration. Typical soil stabilization BMPs include preservation of existing vegetation, mulch, hydroseeding, soil binders, geotextiles, lining of drainage ditches, and/or velocity control structures if needed. Erosion and sedimentation control measures, at a minimum, will be designed for the 20-year, 1-hour storm event in accordance with Surface Mining and Reclamation Act (SMARA) guidelines. Silt fences will be installed five feet from the outer edge of each side of the existing Sweetwater River channel and may be installed in other areas. Other erosion control measures in accordance with set criteria to reduce on- and off-site erosion will include monitoring soil movement, arresting gullies or rills using straw mulch and hay bales, compacting soils with equipment, and re-grading as necessary. Vehicle track out and dust related BMPs may include paved or stabilized roadway surfaces, tire washes, use of grates at vehicle entrances or exits, soil stabilizers, and water spray. Temporary erosion control measures will be retained until vegetation becomes sufficiently established to serve as an effective erosion control measure. Recommended erosion and sedimentation control measures will be described in detail in the Project SWPPP.



2.2 Erosion Modeling Methodology

WEPP was run in a two-dimensional mode based on available hillslope data for the current and proposed Project conditions. Total soil erosion was estimated by extrapolating results for representative hillslope profiles to the surrounding Project acreage. Chang (2021) completed a hydraulic analysis for the Cottonwood sand mining project in which HEC-RAS models were created of the current 100-year floodplain and of the Proposed project condition. Six representative cross sections used in the HEC-RAS model were selected for the purpose of sediment erosion modeling. Selected cross-section locations are shown on Figure 1. These cross sections (six section locations considering pre and post-mining project conditions, for twelve sections in total) were then digitized for input into the erosion modeling software. WEPP is not capable of simulating erosion on uphill slopes (cross-sections must be downhill orflat).

Therefore, the cross sections were broken into segments where necessary and uphill sections were assumed to be flat. Hillslope profiles used in WEPP modeling are presented in Appendix A.

The Cottonwood Sand Mining Project has plans to implement various BMPs to ensure no significant mining impacts on water transfers during extraction activities as described above. The presence of these BMPs was not accounted for in WEPP modeling, and therefore simulation results represent a conservative "worst-case" erosion estimate.

Soil properties were generated for WEPP model runs based on sediment gradation curves presented in Geocon (2017) with 7% clay, 7% silt (14% fines total) and 86% sand. A fraction organic matter of 0.001 was assumed. Sediment size fractions were determined based on the average gradation results presented in Geocon (2017) for samples taken within the top 6 feet. A sensitivity test was conducted and the model was not sensitive to changes in the percent of silt and clay within the range of the measured samples.

Land use management type was assigned as "grass-lawn" for the current condition and "fallow" for the proposed Project condition. This simplification ignores the heterogeneity of vegetation on the current Cottonwood site, but because the WEPP model isn't capable of representing numerous land uses on a single hillslope, it was decided this simplification was appropriate and conservative for modeling purposes.

Erosion was simulated for a 10 year period using Cligen generated climate data (USDA, 2016). Cligen is a stochastic weather generator that produces daily estimates of precipitation, temperature, dewpoint, wind, and solar radiation for a single geographic point, using monthly parameters derived from historical measurements. Unlike other climate generators, it produces



individual storm parameter estimates, including time to peak, peak intensity, and storm duration, which are required to run the WEPP soil erosion models.

2.3 Erosion Modeling Results

WEPP modeling results are summarized in Table 1 through Table 5. Table 1 and Table 2 provide a summary of simulated erosion for each cross-section for the current and proposed project conditions, respectively. Table 3 provides simulated erosion broken down by WEPP particle class (clay, silt, sand, small aggregates and large aggregates). Table 4 summarizes the size fraction composition of the exiting sediment (percent sand, silt, clay and total mass of each).

Table 5 summarizes the tons of sediment exiting the project for each individual Phase (Phase 1 through Phase 3).

WEPP modeling indicates that under the current condition there is approximately an average of 2.2 tons of sediment exiting the Cottonwood golf course per year (Table 1). Under the proposed Project condition, WEPP modeling indicates (Table 5):

- Phase 1 area: Erosion increases from 0.71 tons/year to 25 tons/year.
- Phase 2 area: Erosion increases from 1 ton/year to 1.9 ton/year.
- Phase 3 area: Erosion increases from 0.48 to 1.6 tons/year.

Phase 1 is the largest of the 3 areas. The increase in erosion is due to increased slope and the larger area. It is assumed that the project will proceed to each individual Phase at one time and then start reclamation immediately as they move to the next Phase. Therefore, increased potential erosion will depend on which Phase of the project is ongoing. As indicated above increased erosion is primarily estimated to occur during Phase 1, with a net increase of 24.3 tons/year (assuming the conservative worst-case scenario modeled of no BMPs). For context, 24.3 tons/year averaged over the area of Phase 1 (115 acres) is estimated to be about 0.03 mm/year of sediment erosion. Given this minor amount of increased erosion averaged over the Phase 1 area even under very conservative assumptions of no BMPs this is considered to be less than significant.

WEPP simulates five different particle types in exiting flow: clay, silt, small aggregates, large aggregates, and sand (Table 4). Sediment exiting the hillslope profiles is estimated to consist of 21% clay, 12% silt (33% total fines), and 67% sand. Although the simulated hillslope profiles were assigned 14% fines, the amount of fines exiting the profiles is relatively enriched due to preferential erosion of fine sediments compared to coarse sediments (Flanagan and Nearing, 2000).

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3. Sediment Transport Analysis

Sediment eroded from the Project footprint that exits the hillslope profiles is assumed to be loaded to Sweetwater River. Standard methods were used to evaluate the fraction of sediment capable of being transported by the river downstream and the fraction that is estimated to deposit on the riverbed prior to Sweetwater reservoir. Sediment can be transported as suspended sediment in the water column ("suspended load"), or in direct contact with the riverbed via saltation and rolling ("bed load"). Suspended load consists of the finer-grained sediments (clay, silt, fine sands), whereas bed-load consists of coarser-grained sediments. The largest-grained sediment fractions that cannot be transported by suspension, saltation or rolling are deposited on the riverbed.

Sediment transport for each grain-size fraction was evaluated based on properties of the river (i.e., slope and water depth) and sediment grain-size diameter (Southard, 2006; Crone, 2004). Shields stress ($\tau\tau\tau\tau$) is used to estimate transport stages (no movement, suspension, saltation and rolling):

$$\tau \tau_0 = \frac{\tau \tau}{\int_{\mathbb{R}^d} \Phi \rho \rho_{pp} - \rho \rho_{ff} \Phi D}$$
 (1)

where τ = shear stress g = acceleration due to gravity $\rho\rho_{pp}$ =particle density (assumed 2.65 g/cm³) $\rho\rho_{ff}$ = water density D = particle diameter

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Shear stress (τ) is given by Crone (2004), as follows:

$$\tau \tau = \rho \rho_{ff} u u *^2 \tag{2}$$

where u^* = the characteristic velocity scale within turbulent flow, given by the following equation:

where h = river depth

S = river slope

As displayed on Figure 2, the Sweetwater River was delineated between the Project and Sweetwater Reservoir based on review of aerial imagery, 1-meter resolution digital elevation data (USGS, 2017), and the National Hydrography Dataset (USGS, 2019). River slope was estimated for two sections, from the Project to 1.5 miles downstream within the San Diego National Wildlife Refuge, and from 1.5 miles to the location of a pond above Jeep Trail (3.5 miles downstream from the Project). Slope from the Project to 1.5 miles downstream is estimated to be 0.0029, and from 1.5 miles downstream to the Sweetwater Reservoir to be 0.0069.

River depth was assumed to be 3.5 feet, which is within the upper range of values within the Project footprint given by Chang (2020) during reservoir transfers. Downstream of the Project, Sweetwater River appears to form a braided river channel, and flow depths may decrease in certain reaches. For this reason, a river depth of 1-ft was also considered.

Figure 3 displays the various transport stages (no movement, saltation, rolling and suspension) as a function of the Shields stress, river slope and particle diameter (Southard, 2006). The fine sediment fractions (clays, silts; 33% of total eroded sediments as discussed above), are estimated to remain suspended and be transported in the river under flow conditions of 1 foot and 3.5 feet. For the sand fraction:

- From the Project to 1.5 miles downstream (river slope of 0.0029) under 3.5-ft of flow sand greater than 0.6 mm will be deposited (not transported), which is about 25 to 50% of the sediment sand fraction based on gradation analysis of Project sediments (Geocon, 2017). If the river widens and depth decreases to 1 ft, sand greater than 0.3 mm will be deposited, about 45 to 70% of the sand based on the gradation analysis.
- The remaining reach to Jeep Trail has a steeper slope (0.0069) so any sediment remaining at that point will be carried to the Sweetwater Reservoir, specifically the most upstream area of the reservoir.



At the Sweetwater Reservoir at Jeep Trail the river widens from about 15 feet to 350 feet (Figure 2). It is expected that much of the remaining coarse-grained sediment would be deposited in this pond and not reach the main reservoir.

4. Pollutant Loading Analysis

Chemicals bound to eroded soils may be transported into Sweetwater Reservoir. As discussed above, it is assumed that fines (33% of eroded sediments) may be transported to the reservoir, whereas sands and coarser sediments would be deposited prior to reaching Sweetwater Reservoir. As much as 70% of sands may be deposited in the reach south of the Project due to relatively low river slope and a braided river channel, and the remaining sand fraction likely would deposit in the pond located at Jeep Trail. Estimated incremental pollutant loading into the reservoir was estimated assuming a conservative scenario that the annual loading occurred during a single storm event, and therefore all fine sediments (clays and silts) estimated for an entire year (and bound contaminants) were transported into the reservoir at once. Phase I estimated loading for each compound was estimated as follows:

$$LL_{cc} = CC_{ss}LL_{ss} \tag{4}$$

where LL_{cc} = chemical loading (mg/yr)

 CC_{ss} = average soil concentration of the chemical (mg/kg)

 LL_{ss} = incremental mass of fine sediments loaded during Phase 1 (8.02 ton/yr, or 7,272 kg/yr)

Table 6 presents resulting incremental added contaminant concentrations in the reservoir for Project Phase 1, assuming a reservoir volume of 6,000 acre-feet (approximately 20% of reservoir capacity; Sweetwater Authority, 2020), and complete mixing of eroded sediments within the reservoir (per Chapra, 1997). A higher reservoir stage or increased reservoir volume would further reduce potential contaminant concentrations. Assumed soil pollutant soil concentrations are based on the average of Site sampling results. Resulting incremental added contaminant concentrations within the reservoir are at least two orders-of-magnitude less than applicable water quality criteria where listed, and in most cases several orders-of-magnitude less. Importantly, these estimated concentrations represent total water concentrations (dissolved and suspended), and sediment-bound contaminants would not be observed in dissolved-phase water sampling results (i.e., water samples filtered prior to analysis). Pollutant loading from Project Phases 2 and 3 would be less than Phase 1, due to less erosion compared to Phase 1 (Table 5).

Over time, sediments and sediment-bound contaminants would settle to the bottom of the reservoir and water-column concentrations would decrease. Given the area of the reservoir

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(approximately 640 acres), the incremental added sediment thickness at the bottom of the reservoir from erosion at the Project is de minimis (i.e., much less than 1 mm over the 10-year period of the project), as illustrated below:

The worst case erosion scenario produces 24 tons per year of sediment existing the project or 240 tons over 10 years. With a bulk density of 106 pounds per cubic foot (lbs/ft3) (equivalent to 1.7 grams per cubic centimeter), 240 tons (480,000 lbs) equates to 4,528 ft3 of sediment. The area of the reservoir is assumed to be 640 acres or 27,878,400 square feet (ft2). A total of 4,528 ft3 of sediment distributed over an area of 640 acres equates to 0.0019 inches or approximately 0.05 millimeters.

5. Conclusions

Conclusions from this analysis are:

- Erosion modeling indicates that, as a worst-case, the project condition will result in increased erosion of approximately 24 tons/year during Phase 1, with lower increased erosion during other project phases. This level of erosion is equivalent to 0.03 mm/year averaged over the area of Phase 1.
- Modeling does not account for the construction of BMPs that will be used to prevent impacts to the river and reduce erosion where present (Chang, 2021).
- Eroded sediment is estimated to be 33% fines, which represents an enrichment compared to the hillslope soil properties because fine sediments are preferentially eroded.
- Sediment transport analysis indicates that under typical reservoir transfer operations and
 major storms, fines (clays and silts) will be transported by Sweetwater River. A significant
 amount of the sand fraction (between 25 to 70%) will be deposited within the river reach
 south of the Project based on variations in water depth (the river becomes braided in this
 area), slope, and particle size. The remaining sand fraction will be transported by the river
 downstream.
- A pond is present at Jeep Trail prior to the Sweetwater Reservoir.. At the pond the river widens from about 15 feet to 350 feet (Figure 2). It is expected that much of the remaining coarse-grained sediment would be deposited in this pond and not reach the main reservoir.
- Under a conservative scenario that all sediments estimated for an entire year during Phase 1 reach Sweetwater Reservoir in a single storm event, resulting additional water- column pollutant concentrations are expected to be much less than applicable water quality criteria.
- Incremental added sediment thickness at the bottom of the reservoir from erosion at the Project is de minimis.

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• Given the minor amount of increased erosion, sediment loading to the reservoir, and impact on reservoir water quality even under very conservative assumptions of no BMPs, this is considered to be less than significant.

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Figures







Explanation

HEC-RAS Polygons
HEC-RAS Cross Sections

2018 Google Earth Imagery

0 600 1200 Feet

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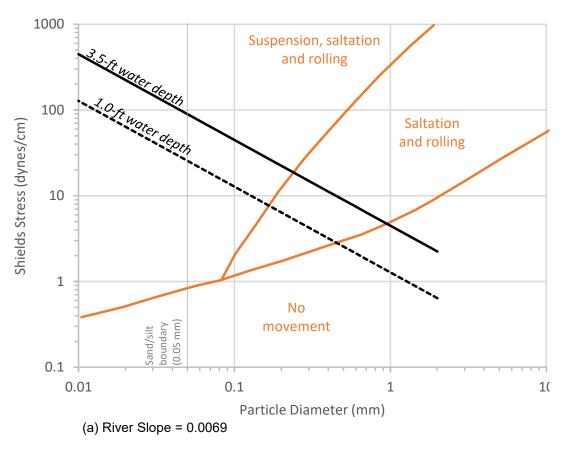
COTTONWOOD MINE Cross-Sections used in WEPP Modeling

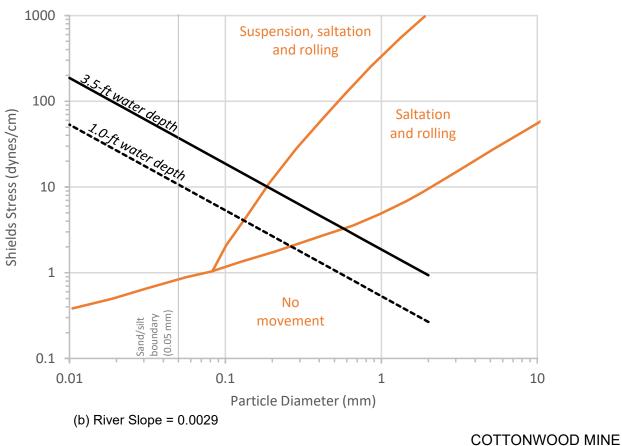
2018 Google Earth Imagery



2000 4000 Feet

COTTONWOOD MINE Sweetwater River between Project and Sweetwater Reservoir





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Critical Shields Stress Diagram

Tables



Table 1. Annual sediment leaving hillslopes under Current conditions.									
Section	Size in acres	Current Slopes	Segment length ft	% of Total cross section length	Acres attributed to each hillslope	tons/acre Exiting per year	Tons of Soil Exiting per year		
50	71.5	50-2	343	23%	16.3	3.0E-04	4.9E-03		
50	71.5	50-11	1161	77%	55.2	2.0E-04	1.1E-02		
100	43.7	100-2	124	12%	5.3	1.7E-02	9.1E-02		
100	43.7	100-11	248	24%	10.6	1.5E-02	1.5E-01		
100	43.7	100-111	533	52%	22.8	1.2E-02	2.7E-01		
100	43.7	100-22	115	11%	4.9	3.7E-02	1.8E-01		
150	34.5	150-1	307	49%	17.0	1.7E-02	2.9E-01		
150	34.5	150-2	317	51%	17.5	1.8E-02	3.2E-01		
180	58.5	180-1	556.8	40%	23.4	1.1E-02	2.6E-01		
180	58.5	180-2	83	6%	3.5	3.8E-02	1.3E-01		
260	39.6	260-1	111.3	12%	4.8	1.0E-03	4.8E-03		
260	39.6	260-2	248	16%	6.3	3.7E-02	2.4E-01		
280	32.8	280-1	64	7%	2.3	7.1E-02	1.6E-01		
280	32.8	280-2	67	7%	2.3	3.5E-02	8.0E-02		
		Total	2.2E+00						

Table 2. Annual sediment leaving hillslopes under Proposed conditions.								
Section	Size in acres	Proposed Slopes	Segment length ft	% of Total cross section length	Acres attributed to each hillslope	tons/acre Exiting per year	Tons of Soil Exiting per year	
50	71.5	50-3	961	71%	51.0	9.9E-02	5.1E+00	
50	71.5	50-4	200	15%	10.6	9.7E-01	1.0E+01	
50	71.5	50-31	186	14%	9.9	5.5E-01	5.5E+00	
100	43.7	100-3	528	61%	26.9	4.5E-02	1.2E+00	
100	43.7	100-4	331	39%	16.8	1.6E-01	2.7E+00	
150	34.5	150-3	357	36%	12.4	1.1E-02	1.3E-01	
150	34.5	150-4	636	64%	22.1	3.3E-02	7.2E-01	
180	58.5	180-3	603	42%	24.5	2.9E-03	7.1E-02	
180	58.5	180-4	835	58%	34.0	3.0E-02	1.0E+00	
260	39.6	260-3	409.9	39%	15.3	4.4E-02	6.8E-01	
260	39.6	260-4	649	61%	24.3	1.4E-02	3.5E-01	
280	32.8	280-3	402	30%	9.9	5.7E-03	5.7E-02	
280	32.8	280-4	928	70%	22.9	2.3E-02	5.3E-01	

Table 3. Distribution of sediment exiting based on grain size								
		%Sand	%Silt		Current	Proposed		
Class	Diameter			%Clay	tons	Tons exiting		
Class	(mm)			70Clay	exiting	of		
					of	each class		
					each			
					class			
1 (clay)	0.002	0%	0%	100%	5.1E-02	3.0E+00		
2 (silt)	0.01	0%	100%	0%	3.5E-05	3.2E-03		
3 (aggregate 1)	0.03	0%	50%	50%	1.6E-01	4.6E+00		
4 (aggregate 2)	0.3	84%	11%	5%	6.9E-01	1.1E+01		
5 (sand)	0.2	100%	0%	0%	1.3E+00	1.0E+01		

Table 4. Composition of sediment exiting									
	Current		Proposed						
Cadinaant	Tons	Current % of	Tons exiting	Proposed					
Sediment	exiting of	total exiting	of each	% of total					
type	each	tons	sediment	exiting tons					
	sediment		type/yr						
	type/yr								
Clay	1.7E-01	8%	5.9E+00	21%					
Silt	1.6E-01	7%	3.5E+00	12%					
Sand	1.9E+00	85%	1.9E+01	67%					

Tab					
Phase	Sections	Total acreage	Current tons exiting annually	Proposed tons exiting annually	Difference
1	50 and 100	115.2	7.1E-01	2.5E+01	24.29
2	150 and 180	93	1.0E+00	1.9E+00	0.9
3	260 and 280	72.4	4.8E-01	1.6E+00	1.12

ft = Feet mm = Millimeters yr = Year

Table 6 - Pollutant Loading Estimate

	Average Soil Concentration	Surface Water Water Quality	Water Quality Criteria Source	Phase 1 Estimated Loading (mg/yr)	Estimated Incremental Additional Total Reservoir	Estimated Incremental Additional Total Reservoir
Analyte	(mg/kg)	Criteria (mg/L)			Concentration (mg/L) ^a	Concentration (mg/L) ^a
General Chemistry						
Nitrate as N	1.97	10	1	14301	0.00000190	1.9E-06
Total Kjeldahl Nitrogen	196.67	NV	1	1430105	0.00019000	1.9E-04
Total Nitrogen	200.00	0.25	3	1454345	0.00020000	2.0E-04
Total Organic Carbon	2978.57	NV	1	21659346	0.00290000	2.9E-03
Total Phosphorus	313.33	0.025	3	2278473	0.00031000	3.1E-04
Metals	•					
Antimony	NC	0.006	1	NC	<u>NC</u>	NC
Arsenic	0.60	0.01	1	4387	0.00000059	5.9E-07
Barium	45.67	1	1	332075	0.00004500	4.5E-05
Beryllium	0.08	0.004	1	582	0.00000008	7.9E-08
Cadmium	NC	0.005	1	NC	<u>NC</u>	NC
Chromium	7.80	0.05	1	56719	0.00000770	7.7E-06
Cobalt	2.83	NV	1	20603	0.00000280	2.8E-06
Copper	6.23	1.3	1	45327	0.00000610	6.1E-06
Iron	10333.33	0.3	1	75141136	0.01000000	1.0E-02
Lead	3.59	0.015	1	26105	0.00000350	3.5E-06
Magnesium	1733.33	NA	NA	12604320	0.00170000	1.7E-03
Mercury	NC	0.002	1	NC	<u>NC</u>	NC
Molybdenum	0.16	NV	1	1178	0.00000016	1.6E-07
Nickel	3.03	0.1	1	22058	0.00000300	3.0E-06
Selenium	NC	0.05	1	NC	<u>NC</u>	NC
Silver	NC	0.1	1	NC	<u>NC</u>	NC
Thallium	0.09	0.002	1	684	0.00000009	9.2E-08
Vanadium	26.33	NV	1	191489	0.00002600	2.6E-05
Zinc	16.03	5	1	116590	0.00001600	1.6E-05
Chlorinated Herbicides (mg/kg) : ND						
Organochlorine Pesticides						
4,4'-DDD	0.00	1.20E-07	2	2.7	0.000000004	3.6E-10
4,4'-DDE	0.0006	1.80E-08	2	4.1	0.000000006	5.5E-10
4,4'-DDT	0.0004	3.00E-08	2	3.1	0.000000004	4.2E-10
Petroleum Hydrocarbons						
Oil and Grease	16	NA	NA	116347.5649	0.00001600	1.6E-05
SVOC (mg/kg) : ND				_		
VOC (mg/kg) : ND						

Sources

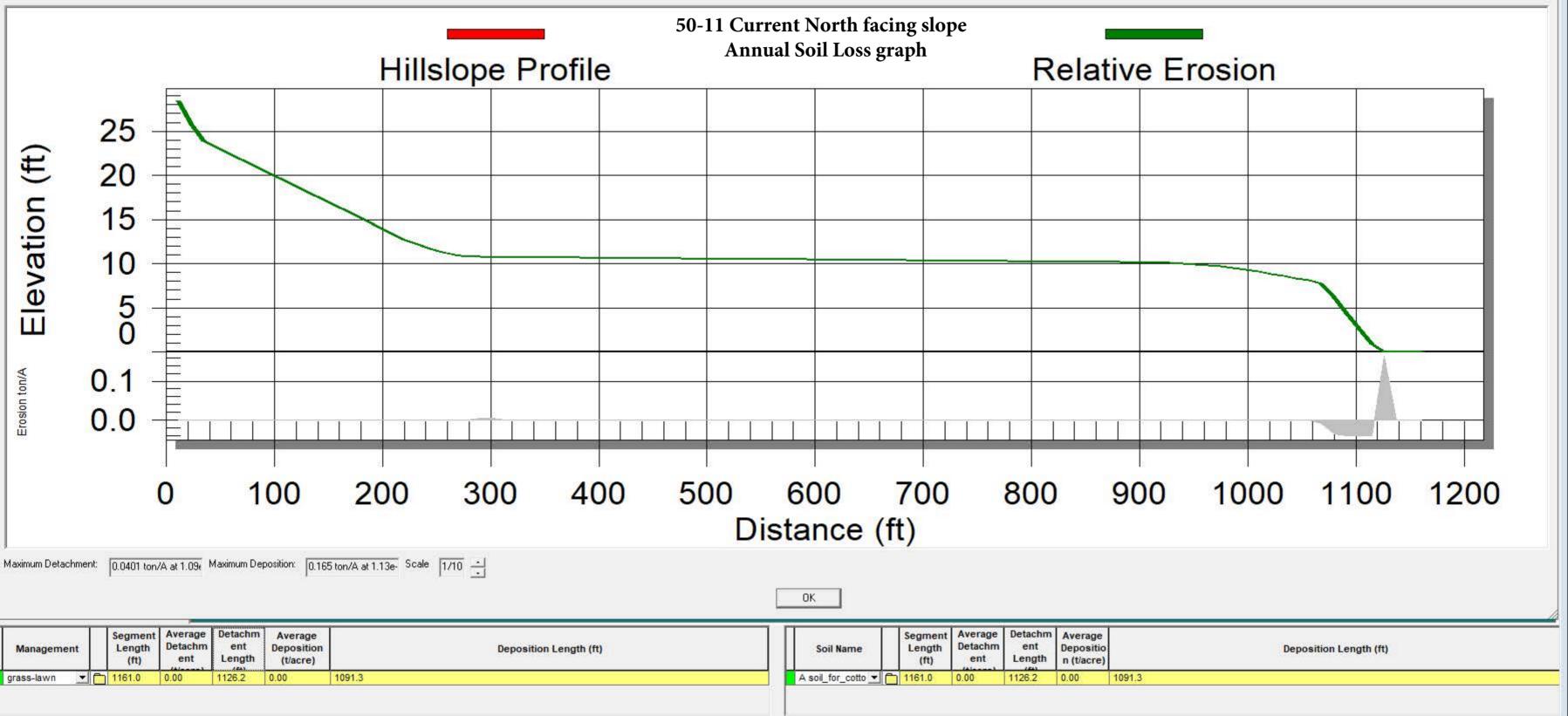
^{1:} Table 3.0 - Water Quality Comparison - Surface Water

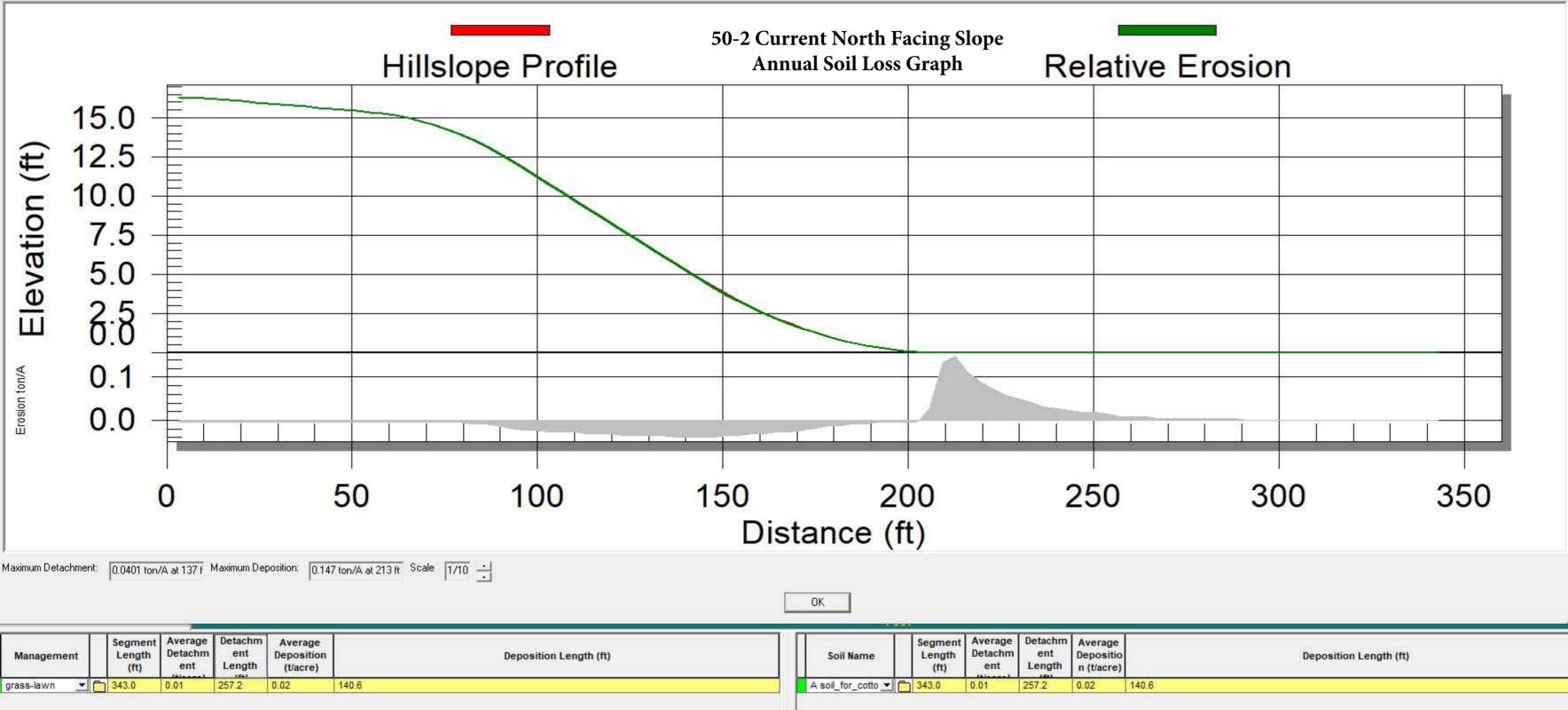
 $^{2:} National\ Recommended\ Water\ Quality\ Criteria\ -\ Human\ Health\ Criteria\ Table; Human\ Health\ for\ the\ consumption\ of\ Water\ +\ Organism$

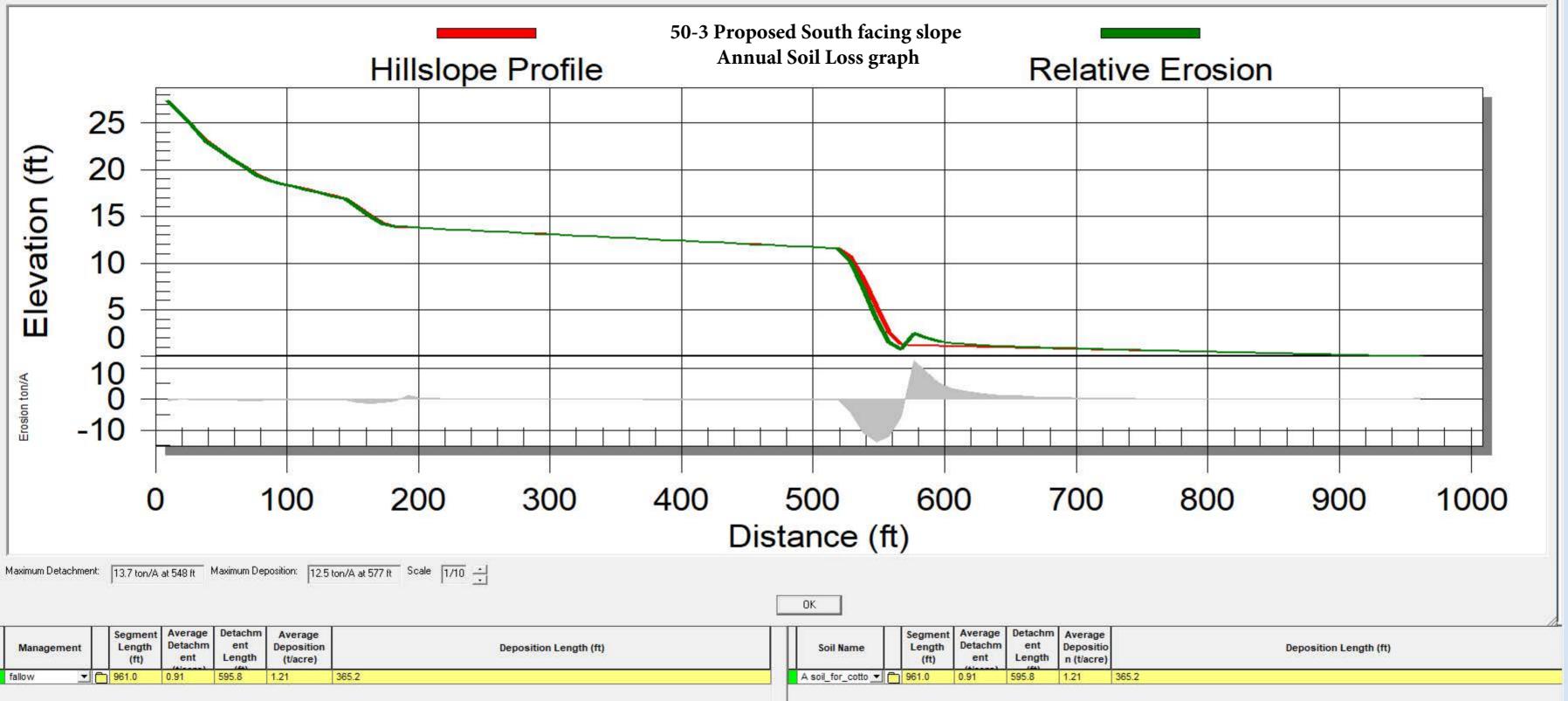
⁽https://www.epa.gov/wqc/national-recommended-water-quality-criteria-human-health-criteria-table)
3: Water Quality Control Plan for the San Diego Basin - California Regional Water Quality Control Board, San Diego Region. a: Assumes all loading occurs within a single event and complete mixing within the reservoir

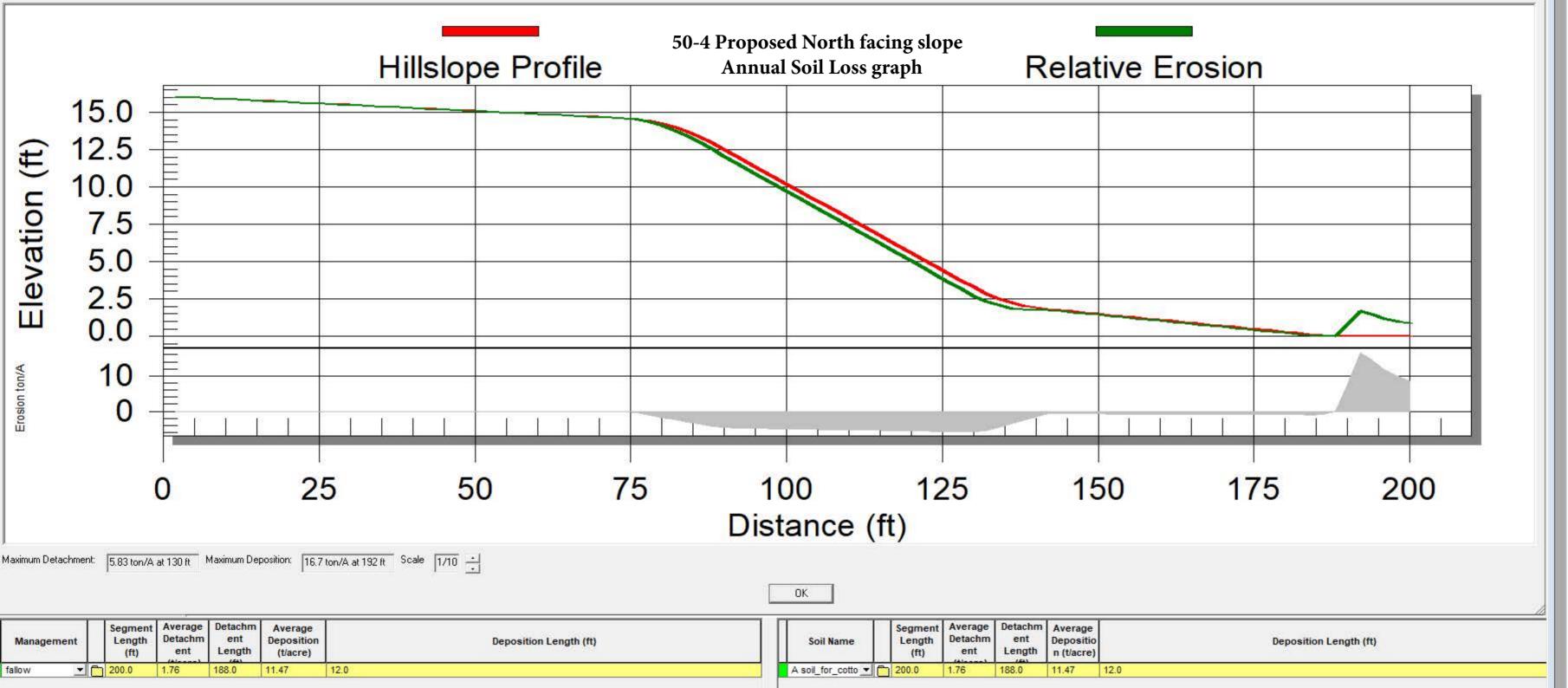
Appendix A WEPP Hillslope Profiles

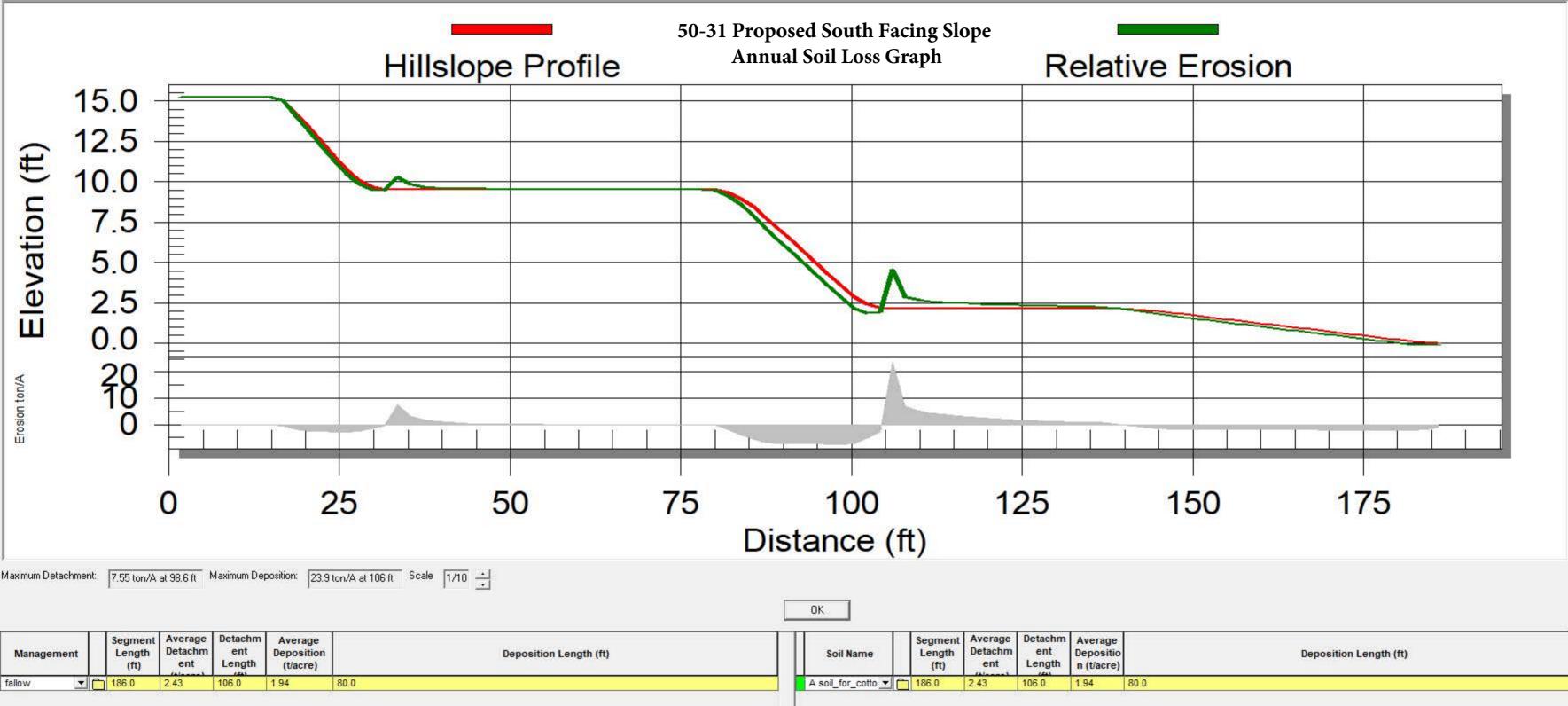


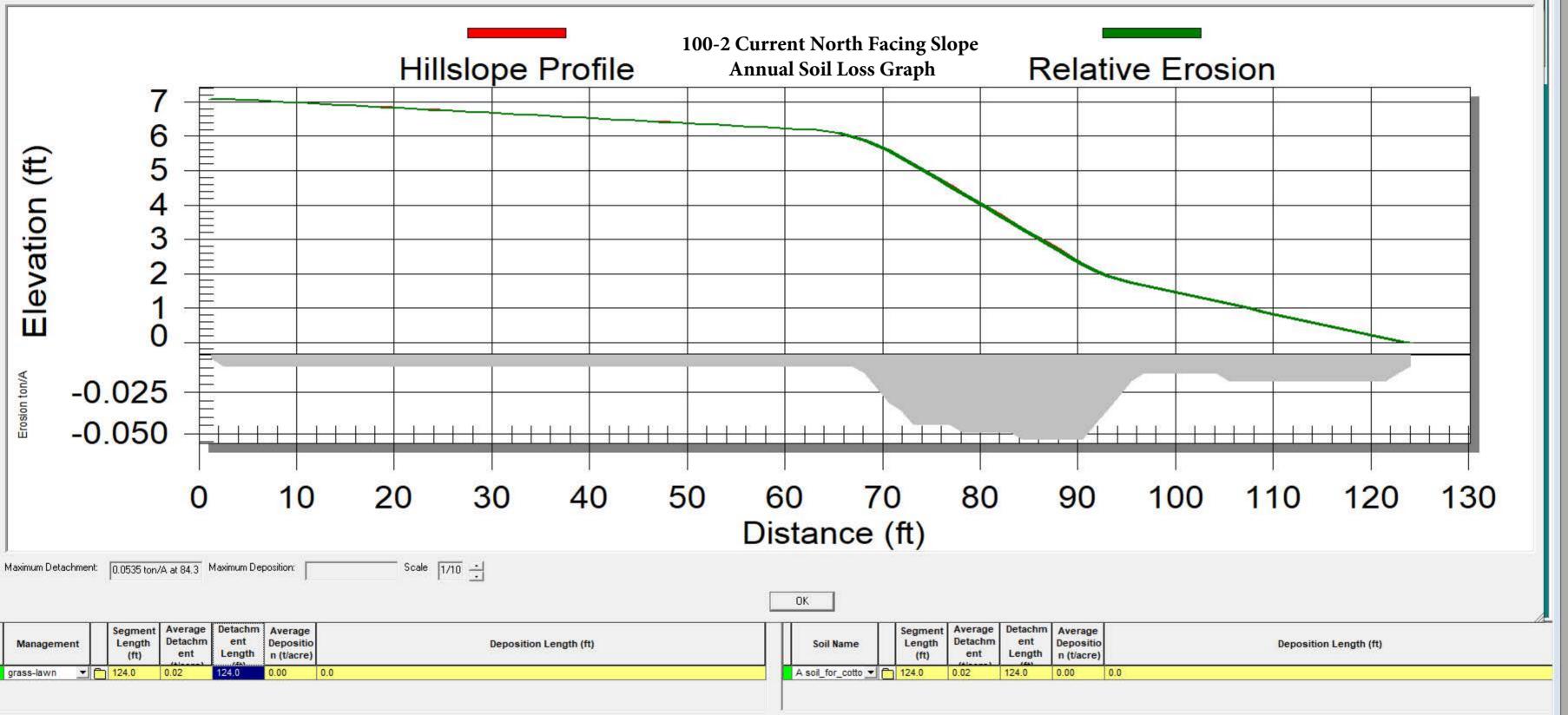


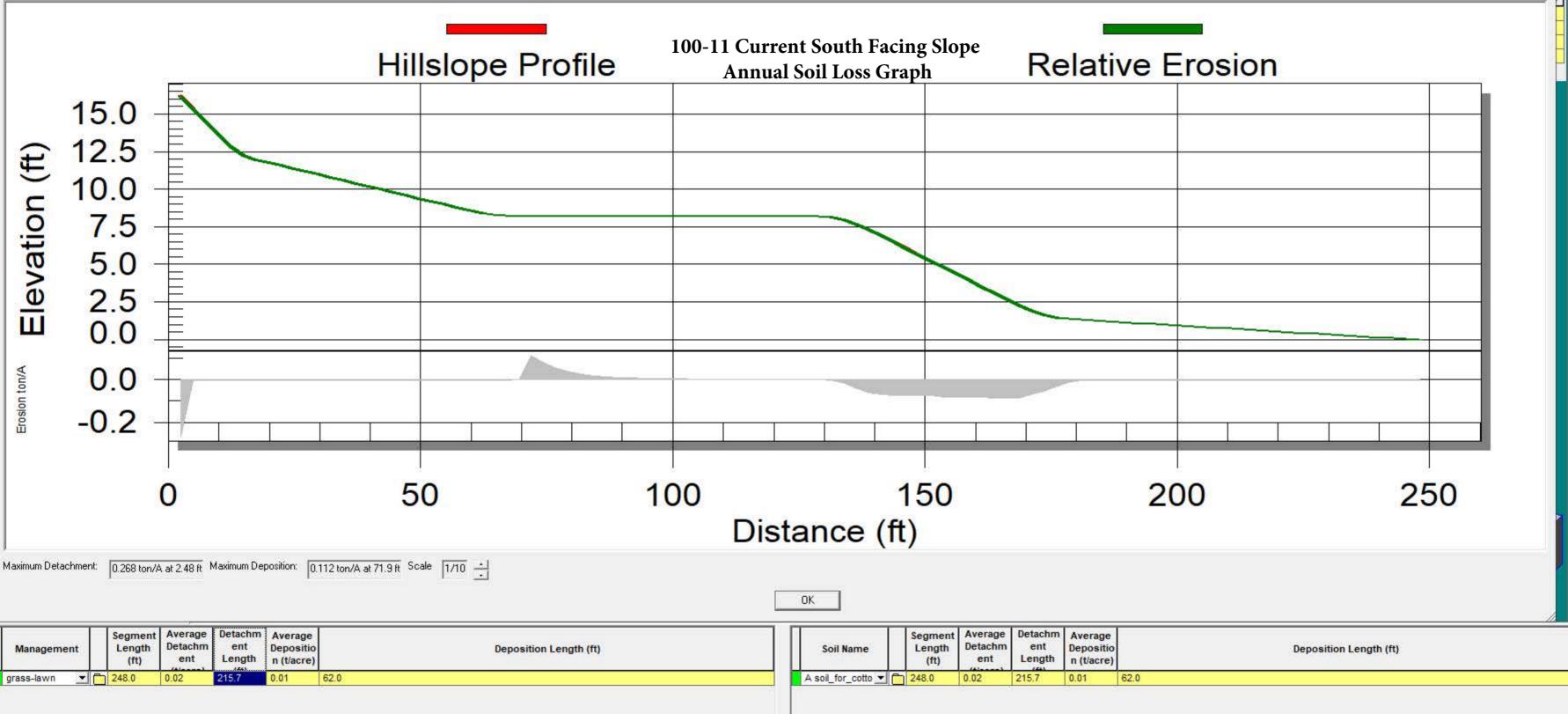












100-22 Proposed South Facing Slope Annual Soil Loss Graph Hillslope Profile **Relative Erosion** 12.5 Elevation (ft) 10.0 7.5 5.0 2.5 0.0 -0.1020 30 40 50 60 70 80 100 110 120 10 90 Distance (ft) Scale 1/10 -Maximum Detachment: 0.112 ton/A at 88.5 ft Maximum Deposition: OK Detachm Detachm Average Average Average Average Segment Segment Detachm Depositio Length Detachment Depositio Length Management Deposition Length (ft) Soil Name Deposition Length (ft) Length Length n (t/acre) n (t/acre) A soil_for_cotto ▼ 115.0

grass-lawn

