DEXTER WILSON ENGINEERING, INC.

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MASTER PLAN OF WATER FOR THE NEWLAND SIERRA PROJECT

January 30, 2017

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TABLE OF CONTENTS

		PAGE NO.
CHAPTER 1	INTRODUCTION	1-1
	PROJECT OVERVIEW	
	DEVELOPMENT PLAN	
	STUDY AREA	
	PURPOSE OF STUDY	
CHAPTER 2	DESIGN CRITERIA	2-1
	WATER SYSTEM	2-1
	Water Duty Factors and Peaking Factors	2-1
	System Pressure	2-2
	Reservoir Storage	2-2
CHAPTER 3	PROJECTED WATER DEMANDS AND	
	RESERVOIR STORAGE	3-1
	Newland Sierra Water Demand	3-1
	Water Storage	3-5
CHAPTER 4	EXISTING WATER FACILITIES	4-1
	Existing Water Facilities	4-1
	Twin Oaks 1028 Zone	4-1
	North 1228 Zone	4-1
	Deer Springs 1235 Zone	4-2
	Coggan 1608 Zone	4-2
CHAPTER 5	RECOMMENDED WATER FACILITIES	5-1
	Water System Computer Model	5-1
	Twin Oaks 1028 Zone	5-3
	North 1228 Zone	5-3
	Deer Springs 1235 Zone	5-3
	Coggan 1608/Proposed 1475 Zones	5-4

TABLE OF CONTENTS

PAGE NO.

APPENDIX A VWD MASTER PLAN FIGURE 3-1

APPENDIX B 1235 ZONE COMPUTER MODELING OUTPUT

APPENDIX C 1608/1475 ZONE COMPUTER MODELING OUTPUT

LIST OF TABLES

TABLE 1-1	NEWLAND SIERRA PROPOSED LAND USE SUMMARY1-2
	USE SUMMARY1-2
TABLE 2-1	RECOMMENDED UNIT WATER DUTY FACTORS2-2
TABLE 3-1	NEWLAND SIERRA WATER DEMAND ESTIMATE3-2
TABLE 3-2	NEWLAND SIERRA PROJECT
	WATER DEMAND SUMMARY3-3
TABLE 3-3	NEWLAND SIERRA WATER STORAGE REQUIREMENTS 3-5
TABLE 5-1	NEWLAND SIERRA PROJECT
	WATER SERVICE SUMMARY5-1

LIST OF FIGURES

		PAGE NO.
FIGURE 1-1	LOCATION MAP	1-4
FIGURE 1-2	PROPOSED LAND USE MAP	1-5
FIGURE 3-1	PRESSURE ZONE MAP	3-4
FIGURE 4-1	EXISTING WATER FACILITIES	4-3
FIGURE 5-1	RECOMMENDED WATER FACILITIES	5-2

CHAPTER 1

INTRODUCTION

This report provides a master plan of water for the Newland Sierra project. This report will provide information concerning projected water demands, existing facilities, proposed major facilities and onsite improvements necessary to provide adequate water service to the project.

PROJECT OVERVIEW

The Newland Sierra project is located within an unincorporated portion of the County of San Diego. The project is located on the north side of Deer Springs Road, just west of Interstate 15. Figure 1-1 provides a location map of the project. The project application includes a general plan amendment, specific plan, rezone, EIR, and tentative map.

DEVELOPMENT PLAN

The Newland Sierra project encompasses a total of approximately 1,985 acres. The development plan for the project includes seven neighborhoods (also called planning areas). The land planning for the project includes a variety of housing types with a total of 2,135 residential units. The project will also include parks, commercial, school, and open space land uses. Table 1-1 summarizes the proposed land use plan for the project and Figure 1-2 graphically shows the land use plan.

TABLE 1-1 NEWLAND SIERRA PROPOSED LAND USE SUMMARY

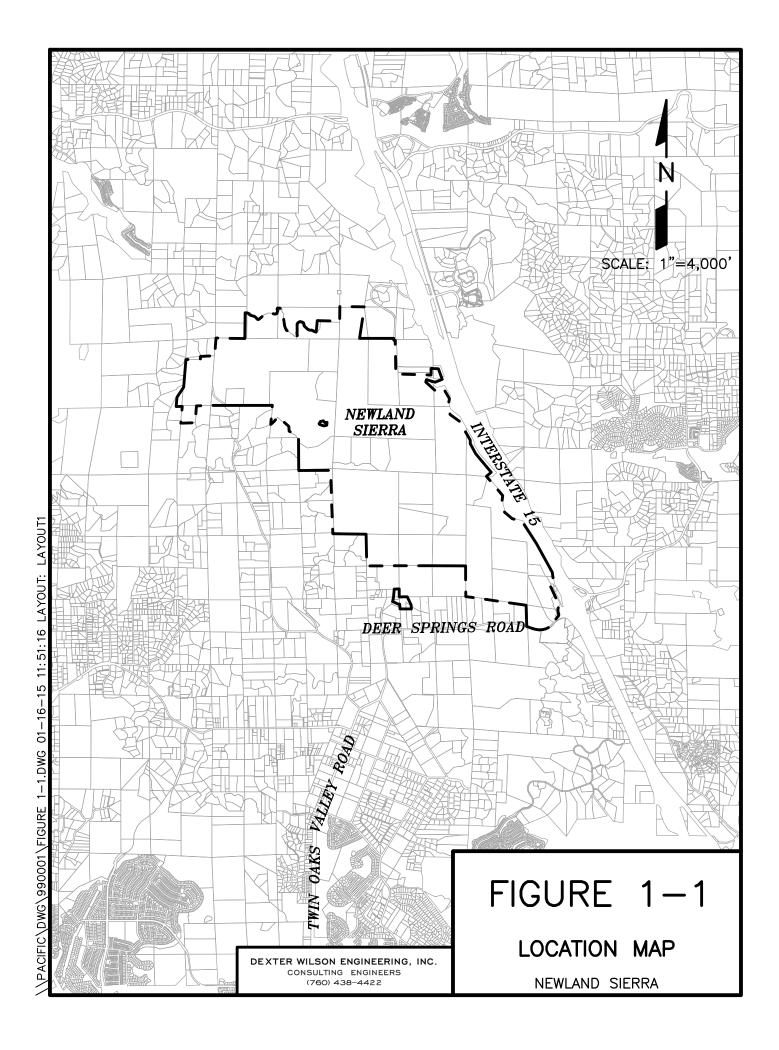
Neighborhood	Land Use	Acres	Units	
	Commercial	7.4	55E2	
Town Center	School	3.6		
	Park	5.7	***	
	Multi-Family	7.2	95	
Subtotal		23.9	95	
	Multi-Family	23.8	316	
Valley	Single Family	32	189	
	Park	12.3	***	
Subtotal		68.1	505	
m	Multi-Family	28.4	446	
Terraces	Water Tank	1.3	***	
Subtotal		29.7	446	
TT:11 · 1	Single Family	36.5	241	
Hillside	Park	2.3	775E)	
Subtotal		38.8	241	
	Multi-Family	6.1	60	
Mesa	Single Family	53.6	265	
	Park	4.1	###D	
Subtotal		63.8	325	
	Multi-Family	4.8	29	
Lower Knoll	Single Family	44.5	203	
	Park	8.9	***	
Subtotal		58.2	232	
	Single Family	26.1	140	
Upper Knoll	Park	0.6	**	
Subtotal	No State Land State of the	26.7	140	
	Multi-Family	14.9	49	
~	Single Family	35.4	102	
Summit	Water Tank	2.9	###)	
	Park	2.0	9.116.	
Subtotal		55.2	151	
-212	Backbone Roads	34.0	222	
	Fuel Modification	378.2	P.T.	
	Open Space	1209.0	255	
TOTAL	Sport Space	1,985.6	2,135	

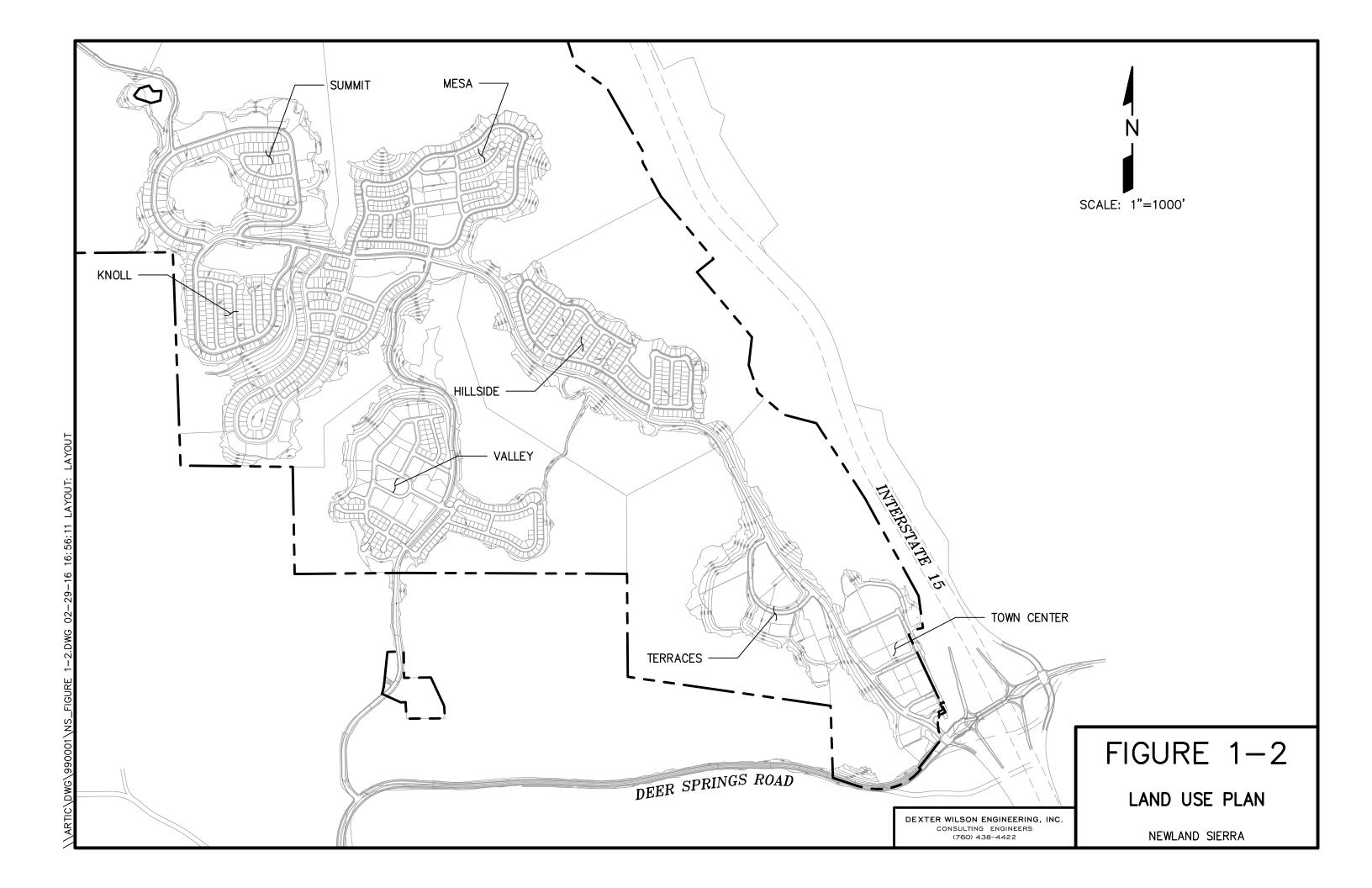
STUDY AREA

The entire project is within the boundaries of the Vallecitos Water District for water service. A series of reservoirs and pipelines were constructed on the property from an assessment district that was formed in the 1970s.

PURPOSE OF STUDY

The purpose of this report is to provide the recommended water facilities that are necessary to provide water service to Newland Sierra. This plan will be compatible with the Vallecitos Water District Master Plan.





CHAPTER 2

DESIGN CRITERIA

This chapter presents the design criteria used to evaluate recommended water system improvements for the Newland Sierra project. The criteria utilized in this study are in accordance with the Vallecitos Water District 2008 Water, Wastewater, and Recycled Water Master Plan, unless otherwise noted. This report was prepared by PBS&J and is dated November 2010. The design criteria are used for evaluating the existing system as well as for design and sizing of proposed improvements to accommodate development in the study area.

WATER SYSTEM

Water Duty Factors and Peaking Factors

Table 2-1 presents the water duty factors from the Draft 2014 VWD Master Plan that were used in projecting water demands for the Newland Sierra project. Table 2-1 also provides the required fire flow and duration for each land use category. To determine the required fire flow from the Uniform Fire Code requires specific information on the building type and square footage which is not typically available at the planning level. The fire flow values used in this report are consistent with the Vallecitos Water District's Master Plan. To convert average day water demands to maximum day demands and peak hour demands, Figure 3-1 from the District Master Plan was utilized. This figure has been included in Appendix A for reference.

TABLE 2-1
RECOMMENDED UNIT WATER DUTY FACTORS¹

Land Use Category	Water Duty Factor, gpd/ac	Required Fire Flow ² , gpm	Fire Flow Duration, Hours
Residential (2-4 du/ac)	1,800	1,500	2
Residential (4-8 du/ac)	2,500	1,500	2
Residential (8-12 du/ac)	2,800	2,500	2
Residential (12-15 du/ac)	4,500	2,500	2
Residential (15-20 du/ac)	5,000	2,500	2
Residential (20-30 du/ac)	6,000	2,500	2
Commercial	1,500	2,500	2
Schools and Public Facilities	1,000	3,500	4
Parks	1,500		
Open Space	200		555
Right of Way	200		H+H)

¹ Per Draft 2014 VWD Master Plan.

System Pressure

The water distribution system has been designed to maintain static pressures between 65 psi and 150 psi. This criteria is used to initially divide a project between water service zones. Computer modeling is then performed to ensure that adequate residual pressures are obtained under all demand conditions. The system has been designed to yield minimum residual pressures of 40 psi during peak hour demands and 20 psi during maximum day demand plus fire flow conditions. Water lines were sized for a maximum velocity of 7.0 feet per second during any demand condition. Headloss in water lines are calculated using the Hazen-Williams equation with a "C" value of 130.

Reservoir Storage

Treated water storage reservoirs, or water storage tanks, within the District are sized for operational storage, emergency storage (within the District), and fire flow storage. The District requires storage tanks to be sized for the greater of 450 percent of average daily demand plus fire flow storage or 500 percent of average daily demand.

² Final determination of fire flow will be made by the Fire Department.

CHAPTER 3

PROJECTED WATER DEMANDS AND RESERVOIR STORAGE

This chapter provides the projected water demands and storage tank requirements by service zone for the Newland Sierra project. Storage tank requirements are provided for development within the project. The final sizing of new storage tanks will be determined by the District Master Plan.

Newland Sierra Water Demand

Due to the range of topography on the project, the project falls within four of the Vallecitos Water District's water service zones and a new pressure zone with a gradeline of 1,475 feet is being proposed by pressure reducing off the Coggan 1608 Zone. Table 3-1 provides the projected water demands by water service zone. As evidenced by Table 3-1, no residential development is proposed to be served by the Twin Oaks 1028 or North 1228 pressure zones. Table 3-2 provides a summary of the average and maximum daily water demands within each pressure zone for the Newland Sierra project. Figure 3-1 provides a pressure zone map for the project.

TABLE 3-1 NEWLAND SIERRA WATER DEMAND ESTIMATE

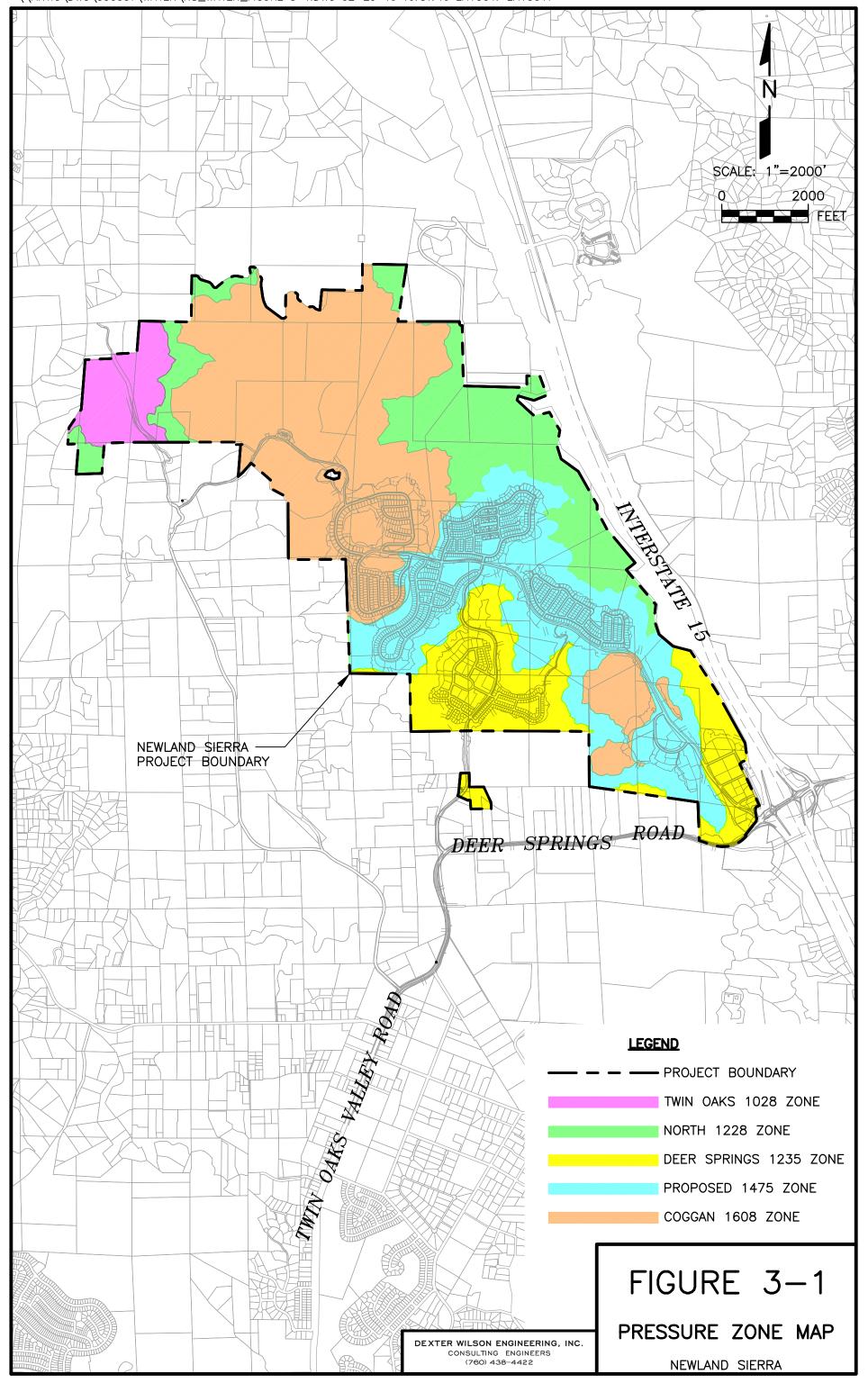
Neighborhood	Land Use	Acres	Dwelling Units	Density,	Water Duty Factor, gpd/ac	Average Water Demand, gpd
Twin Oaks 1028	Zone	,				
	Open Space	195.7	10000		200	39,140
	Backbone Roads	4.5	1000	###.I	200	900
Twin Oaks 1028	Zone Subtotal	200.2				40,040
North 1228 Zone		9.				
	Open Space	349	3222	222	200	69,800
North 1228 Zone	Subtotal	349			200	69,800
Deer Springs 123	5 Zone					
	Commercial	7.4	***	577	1,500	11,100
m	School	3.6	***	***	1,000	3,600
Town Center	Park	5.7			1,500	8,550
	Multi-Family	7.2	95	13.2	4,500	32,400
	Multi-Family	23.8	316	13.3	4,500	107,100
Valley	Single Family	32	189	5.9	2,500	80,000
	Park	12.3	7200	222	1,500	18,450
HTH.	Backbone Roads	9	1555	7.F.	200	1,800
	Fuel Modification - Irrigated	32.4	3889	***	1,500	48,600
:555	Fuel Modification - Non Irrigated	60.5	1557	######################################	200	12,100
Deer Springs 123		193.9	600			323,700
Proposed 1475 Zo						
	Multi-Family	28.4	446	15.7	5,000	142,000
Terraces	Water Tank	1.3	5484	484)	1,000	1,300
TT.11	Single Family	36.5	241	6.6	2,500	91,250
Hillside	Park	2.3	1484	2227	1,500	3,450
	Multi-Family	6.1	60	9.8	2,800	17,080
Mesa	Single Family	53.6	265	4.9	2,500	134,000
	Park	4.1	700	555	1,500	6,150
	Multi-Family	4.8	29	6.0	2,500	12,000
Lower Knoll	Single Family	44.5	203	4.6	2,500	111,250
	Park	8.9		***	1,500	13,350
	Backbone Roads	15.8	1 1 1		200	3,160
:555	Fuel Modification - Irrigated	72.4	3505	200 pt	1,500	108,600
	Fuel Modification - Non Irrigated	147.8		200	200	29,560
	Open Space	200.9	707	+++1	200	40,180
Proposed 1475 Zo		627.4	1,244	THE LANG		713,330

TABLE 3-1 NEWLAND SIERRA WATER DEMAND ESTIMATE

Neighborhood	Land Use	Acres	Dwelling Units	Density, du/ac	Water Duty Factor, gpd/ac	Average Water Demand, gpd
Coggan 1608 Zon	e					
Umman Vmall	Single Family	26.1	140	5.3	2,500	65,250
Upper Knoll	Park	0.6	722		1,500	900
	Multi-Family	14.9	49	3.4	1,800	26,820
Summit	Single Family	35.4	102	2.9	1,800	63,720
Summit	Water Tank	2.9	555	315	1,000	2,900
	Park	2.0	1222	(44)	1,500	3,000
) 111 /	Backbone Roads	4.7		35.5E	200	940
S###	Fuel Modification - Irrigated	26.2		***	1,500	39,300
	Fuel Modification - Non Irrigated	38.9			200	7,780
J aar ji	Open Space	463.4	7,555		200	92,680
Coggan 1608 Zon	e Subtotal	615.1	291			303,290
TOTAL		1,985.6	2,135	N INGS IVE	7. 7.	1,450,160

TABLE 3-2 NEWLAND SIERRA PROJECT WATER DEMAND SUMMARY

Pressure Zone	Average Daily Demand, mgd	Maximum Daily, Demand, mgd	Peak Hour Demand, mgd	
Twin Oaks 1028	0.040	0.120	0.260	
North 1228	0.070	0.209	0.454	
Deer Springs 1235	0.324	0.906	1.845	
Proposed 1475	0.713	1.926	3.638	
Coggan 1608	0.303	0.864	1.759	



Water Storage

Based on the water demand projections provided in Table 3-2. Table 3-3 summarizes the required water storage for development within Newland Sierra. Water storage requirements for the Twin Oaks 1028 and North 1228 Zones were not evaluated as part of this study since the project is not proposing any development within these pressure zones.

TABLE 3-3 NEWLAND SIERRA WATER STORAGE REQUIREMENTS								
Pressure Zone	Average Demand, mgd	Operational & Emergency Storage, MG	Fire Flow Storage, MG	Total Storage Required, MG				
Deer Springs 1235 0.324		1.46^{3}	0.30^{1}	1.763				
Coggan 1608 ²	1.086^{2}	5.43^{3}	0.30^{1}	5.43^{3}				

¹ From VWD 2008 Master Plan.

² Includes demands within the North 1228 Zone and proposed 1475 Zone.

³ 500 Percent of average daily demand governs in the Coggan 1608 Zone. There is 1.3 MG of existing water storage capacity in the Coggan 1608 Zone. 450 percent of average daily demand plus fire flow governs in the Deer Springs 1235 Zone.

CHAPTER 4

EXISTING WATER FACILITIES

This chapter provides a description of existing water facilities in the vicinity of the Newland Sierra project. Figure 4-1 graphically shows the location of existing major facilities.

Existing Water Facilities

The Newland Sierra project is within the boundaries of the Vallecitos Water District. There are numerous existing major water facilities within and around the project. A number of major facilities within the project were constructed in 1975 as part of Assessment District 75-1. A description of major water facilities by water service zone is provided below.

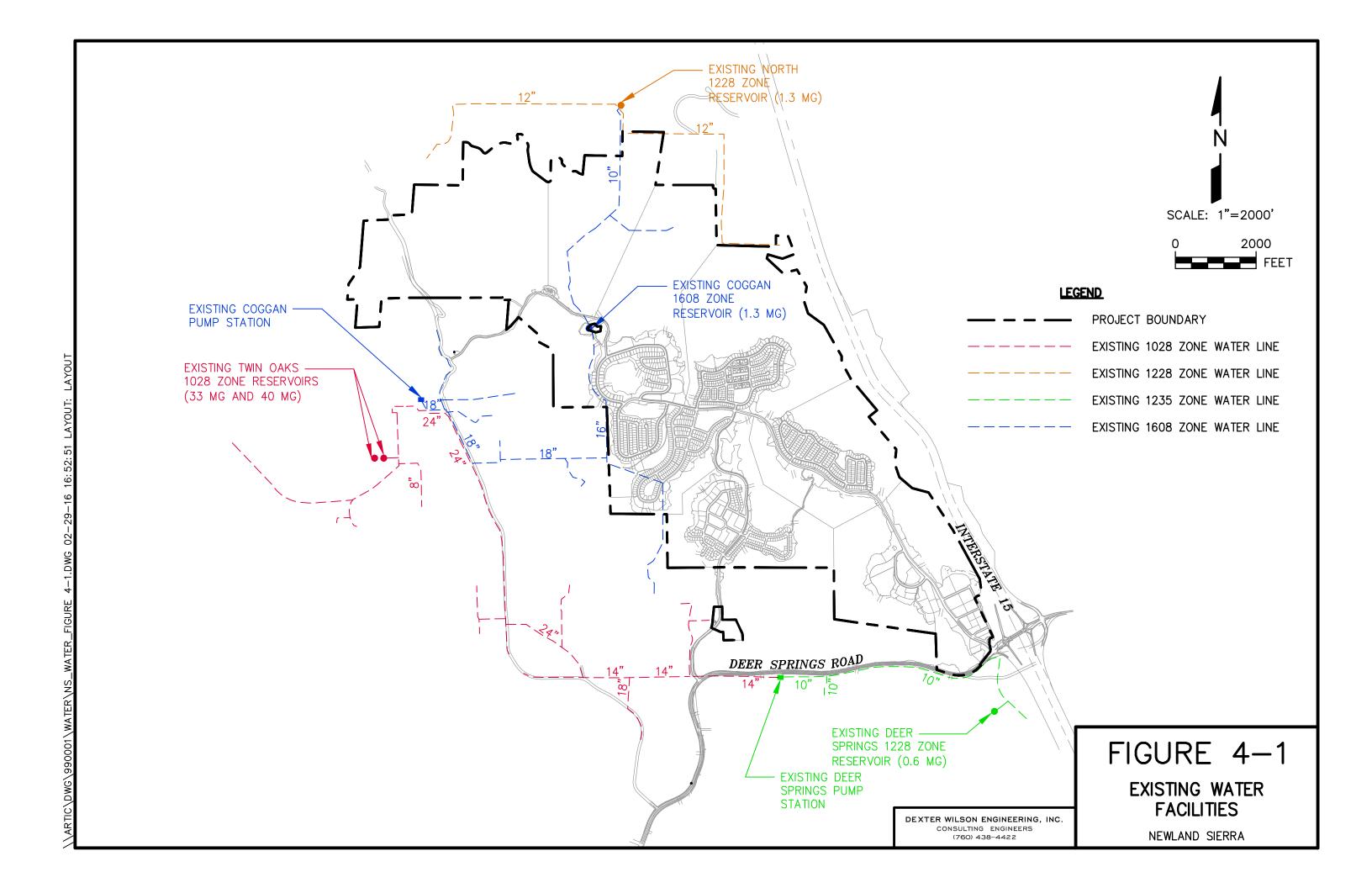
Twin Oaks 1028 Zone. The Vallecitos Water District has historically relied on the purchase of treated water from the San Diego County Water Authority (CWA) for its entire supply of water. Starting in 2016, VWD began receiving water from the Carlsbad Desalination project. This supply can provide up to 3,500 acre-feet per year to VWD which represents approximately 25 percent of the current overall supply.

The VWD Number 10 turnout off the CWA aqueduct is located just west of Twin Oaks Valley Road and provides the District with a capacity of 38.8 million gallons per day at this location. This aqueduct connection allows the adjacent Twin Oaks 1028 Reservoirs to be filled by gravity. There is a 33 million gallon tank and 40 million gallon tank in this zone. Although the Twin Oaks 1028 Zone will not provide direct service to any of the developed area within Newland Sierra project, it will provide the source of water supply for the entire project.

North 1228 Zone. The North 1228 Zone consists of a 1.3 million gallon reservoir just north of the Newland Sierra project that was constructed as part of Assessment District 75-1. The North tank is supplied by the Coggan 1608 Zone via the North Pressure Reducing Station which has an outlet hydraulic gradeline setting of 1,243 feet. This tank has recently been abandoned such that the limited connections in this zone can be served directly from the pressure reducing station. There are some existing 8-inch through 12-inch water lines that have been constructed in the North 1228 Zone.

Deer Springs 1235 Zone. The Deer Springs 1235 Zone is formed by the Deer Springs Pump Station and Deer Springs Water Storage Tank. The Deer Springs Pump Station is located along the south side of Deer Springs Road and is supplied by the Twin Oaks 1028 Zone. This pump station has a firm capacity of 1,200 gpm. The Deer Springs Tank is located just southeast of the Newland Sierra project and has a capacity of 0.6 million gallons. This reservoir does not have current capacity to serve existing system demands, including emergency and fire flow storage requirements, and the need for construction of additional storage in this zone has been identified by the District.

Coggan 1608 Zone. The Coggan 1608 Zone Reservoir is located within the Newland Sierra project and was constructed as part of Assessment District 75-1. The Coggan Reservoir has a capacity of 1.3 million gallons and provides service to a few customers west and north of the Newland Sierra project. The Coggan Pump Station is located near the Twin Oaks 1028 Zone Reservoirs and pumps water from the 1028 Zone to the Coggan Reservoir. This pump station has three pumps, each with a rated capacity of 2,000 gpm. There are numerous 1608 Zone transmission and distribution lines within the Newland Sierra project.



CHAPTER 5

RECOMMENDED WATER FACILITIES

This chapter provides the recommended water system improvements necessary to provide service to the Newland Sierra project. A new 1475 Zone is proposed via pressure reducing stations off the Coggan 1608 Zone system. The 1475 Zone will provide service to development that could not receive sufficient pressure from the Deer Springs 1235 Zone and would receive pressures of greater than 150 psi from the Coggan 1608 Zone. Table 5-1 summarizes the anticipated range of static pressures on the project. Figure 5-1 graphically shows the location of proposed facilities. A brief discussion of facilities by water service zone as well as a description of the computer modeling is provided below.

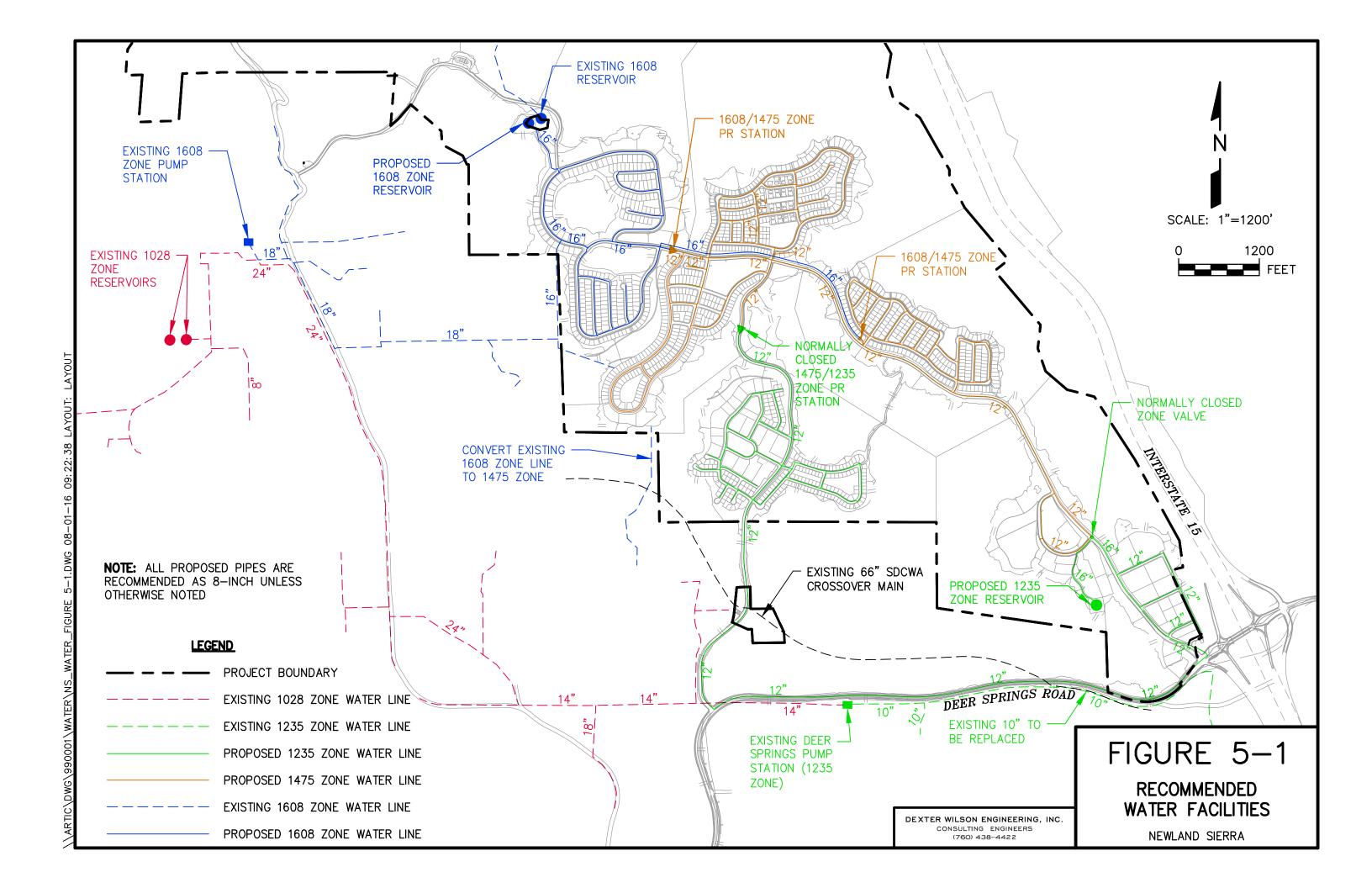
TABLE 5-1 NEWLAND SIERRA PROJECT WATER SERVICE SUMMARY						
D	Pad Elev	ation, Ft.	Static Pres	ssure¹, psi		
Pressure Zone	Minimum	Maximum	Minimum	Maximum		
Deer Springs 1235	875	1080	67	170		
Proposed 1475	1175	1295	78	130		
Coggan 1608	1320	1495	49	142		

¹Maximum static pressure is calculated with reservoir full. Minimum static pressure is calculated with reservoir empty. Maximum static pressures are for development pads only and higher pressures may occur in sections of the piping where there are no service connections.

Water System Computer Model

The University of Kentucky KYPIPE computer program was used to conduct a hydraulic model of the proposed water system within the study area. This computer program utilizes the Hazen-Williams equation for determining headloss in pipes; the Hazen-Williams "C" value used for all pipes is 130.

Computer modeling was only applied to portions of the project where there are expected improvements. Two computer models were created, one for the 1235 Deer Springs Zone, and one for the 1608 and 1475 Zones combined. Each model will be further discussed in its respective section.



<u>Twin Oaks 1028 Zone</u>. The District has recently completed the construction of 33 milliongallon and 40 million gallon reservoirs in this zone. The VWD Number 10 turnout that supplies the Twin Oaks 1028 Zone has a capacity of 38.8 mgd. Improvements in this zone will not be required to be constructed by the Newland Sierra project, but the project will share in the cost of storage facilities through the payment of capacity fees.

North 1228 Zone. Approximately 349 acres within the Newland Sierra project are within the North 1228 Zone, but all of this property is proposed to be dedicated open space. Since Newland Sierra is not proposing any development in this zone, no improvements are proposed for this project. The water demands associated with the open space are included in the Coggan 1608 Zone water storage requirements.

Deer Springs 1235 Zone. The Vallecitos Water District Master Plan identifies capital improvement program projects to provide additional storage in the Deer Springs Zone and construction of a new transmission line between the Deer Springs Pump Station and the Reservoir. The Deer Springs Pump Station was upgraded to a capacity of 1,200 gpm by the District. Further evaluation will be required by the District to determine if the pump station will be adequate to accommodate ultimate development, including additional demands from the Newland Sierra project.

Construction of the other facilities will be required prior to or concurrent with the development of the Newland Sierra project, as described below.

The existing Deer Springs Reservoir has a capacity of 0.6 million gallons and has been determined by the District to be undersized to serve existing development in this zone. The District Master Plan identifies Deer Springs Reservoir No. 2 as a 1.0 million gallon reservoir (Capital Improvement Project R-4) to meet storage deficits in the 1235 and 1549 Zones and to allow the existing Deer Springs Reservoir to be abandoned. Table 3-3 identified the need for a 1.76 MG mg tank to serve the Newland Sierra project only. The final sizing of this reservoir will need to be determined by VWD based on master plan data and additional projected demands for Newland Sierra. For the purposes of this study, a 2.0 MG tank has been assumed. This reservoir is proposed to be located within the Newland Sierra project and shall have a low water line of 1,235 feet and a high water line of 1,267 feet to match the existing reservoir. Construction of this reservoir will need to be completed prior to occupancy of any development within this zone.

There is an existing 10-inch water line constructed in 1961 that conveys water from the Deer Springs Pump Station to the Deer Springs Reservoir. The District Master Plan has identified the need for a replacement 12-inch water line (Capital Improvement Project P-56). This pipeline project needs to be completed prior to or concurrent with development in this zone. As previously mentioned, the 1235 Zone was modeled as one zone. The model output is included as Appendix B, with a corresponding Exhibit A that provides the node and pipe diagram for the system model. To provide redundancy, we are recommending that the zone is supplied by both the proposed Deer Springs 1235 Zone Tank as well as a normally closed 1475/1235 Zone pressure reducing station. The hydraulic analysis assumes the 1235 Zone reservoir is half full with a hydraulic grade line of 1251 feet and the normally closed pressure reducing valve is set to open when the hydraulic grade line in the system drops below 1220 feet. The hydraulic model shows that 8-inch through 16-inch water lines can adequately supply the project during maximum day demand and peak hour demand scenarios from the reservoir only. In order to adequately supply the project during maximum day demand plus fire flow scenarios, the normally closed 1475/1235 Zone pressure reducing valve will need to open to allow the 1475 Zone to supplement supply to the 1235 Zone.

Coggan 1608/Proposed 1475 Zones. The Coggan Pump Station has a firm capacity of 4,000 gpm and is not anticipated to require upgrades to serve ultimate development. Water from the pump station to the Coggan Tank is conveyed through 16-inch and 18-inch pipelines. These pipelines do not require upsizing to accommodate ultimate development, but portions may be re-aligned during construction of the Newland Sierra project. The existing Coggan Tank has a capacity of 1.3 million gallons, but the District Master Plan recommends replacement of this reservoir with a 6.0 MG Coggan No. 2 Tank. A future Coggan No. 3 Tank is also shown in the District Master Plan within the open space portion of the Newland Sierra property.

Based on the information contained in Table 3-3, the total capacity required for Newland Sierra is 5.43 MG. With the existing 1.3 MG tank, Newland Sierra would need to construct an additional 4.13 MG of storage to meet their project needs only. This reservoir will need to be oversized to meet District Master Plan requirements. The Coggan No. 2 Tank will be located at the same site as the existing reservoir and will have a low water line of 1608 feet and a high water line of 1648 feet. The VWD Master Plan identifies a 6.0 MG tank at this site that allows the existing reservoir to be abandoned and allows service to future development. This reservoir is identified as a District Capital Improvement project (Project R-5) and construction will be concurrent with development of the Newland Sierra project. The potential future construction of the Future 6.1 MG Coggan No. 3 Tank (District CIP Project R-11) will not be required with development of Newland Sierra, but additional land may be needed from the

project for VWD to develop this future site.

The 1475 Zone will be formed by pressure reducing off the 1608 Zone. Two pressure reducing stations are proposed to provide redundancy. Each pressure reducing station typically has one small (2-inch or 3-inch) and one larger (6-inch or 8-inch) pressure reducing valve to accommodate the full range of anticipated demands. The final determination of valve sizing and setpoints is generally made prior to initiation of final engineering for the project. The piping in the 1475 Zone is recommended to range from 8-inch to 12-inch.

The 1608 and 1475 Zones are modeled as one system. The model output is included as Appendix C, with a corresponding Exhibit B to provide the node and pipe diagram for the system model. The model assumes the 1608 Zone reservoir is half full with a hydraulic grade line of 1628 feet. The 1475 Zone models both pressure reducing stations with an output hydraulic grade line of 1475 feet. The model demonstrates that the zones can be adequately supplied during maximum day demand plus fire flow conditions with 8-inch through 16-inch water lines. Average day demand and peak hour demand scenarios were also modeled.

APPENDIX A

VWD MASTER PLAN FIGURE 3-1

The peaking factors for maximum day and peak hour demands are based on the District's 2007 average demand of 18.3 MGD and are representative of District-wide demands. To size individual development infrastructure, the District will use Figure 3-1 to determine the appropriate peaking factors. Figure 3-1 displays peaking factor curves used in this Master Plan. The corresponding peaking factors obtained from these curves are multiplied by the average water demands to determine the peak flows for individual developments.

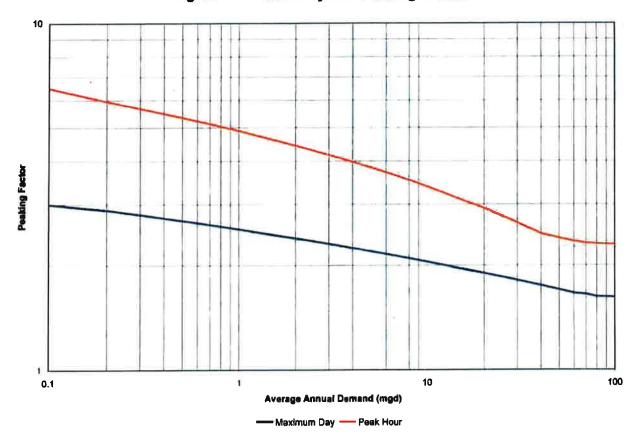


Figure 3-1 Water System Peaking Curves

Peaking Factors for Individual Projects/Developments

Peaking factors for new developments are determined based on the average daily demand of the new development multiplied by the appropriate peaking curve factor, as shown in Figure 3-1. Analysis of major infrastructure may use reduced peaking factors (at the District's discretion) based on the average flows of the tributary area served by the facility.



APPENDIX B

1235 ZONE COMPUTER MODELING OUTPUT

The following conditions were modeled:

- 1. Average day demand
- 2. Peak hour demand
- 3. Maximum day demand plus 3,500 gpm fire flow at node 8
- 4. Maximum day demand plus 1500 gpm fire flow at node 28
- 5. Maximum day demand plus 2500 gpm fire flow at node 44

FLOWRATE IS EXPRESSED IN GPM AND PRESSURE IN PSIG

A SUMMARY OF THE ORIGINAL DATA FOLLOWS

P	IPE	NO.	NOD	E NOS.		DIAMETER (INCHES)	ROUGHNESS	MINOR LOSS K	FIXED GRADE
1	Λ1		0	2	1850.0		130.0	.00	1251.00
	01		0						1231.00
	05		2	4	600.0	12.0		.00	
	09		2	8	1300.0			.00	
	13		4	8	650.0		130.0	.00	
	17		8	12	1050.0		130.0	.00	
1	21		4	12	600.0	12.0	130.0	.00	
1:	25		12	16	11700.0	12.0	130.0	.00	
1:	29		16	20	650.0	12.0	130.0	.00	
1	33		20	22	500.0	12.0	130.0	.00	
1:	35		20	24	150.0	8.0	130.0	.00	
13	37		16	32	700.0	8.0	130.0	.00	
1	41		32	28	1800.0	8.0	130.0	.00	
1	43		32	24	150.0	8.0	130.0	.00	
1	45		24	28	1450.0	8.0	130.0	.00	
1	47		20	36	350.0	8.0	130.0	.00	
	49		16	40	1200.0	8.0	130.0	.00	
	53		40	44	300.0	8.0	130.0	.00	
	57		44	48	600.0	8.0	130.0	.00	
	61		36	40	650.0	8.0	130.0	.00	
	65		36	44	750.0	8.0	130.0	.00	
	69		22	48	1050.0	8.0	130.0	.00	
	73		00	22	2200.0	12.0		.00	
	77	_		200	10.0	12.0		.00	1220.00
						ID MIMPED 1			1220.00

THERE IS A CHECK VALVE IN LINE NUMBER177

JUNCTION NUMBER	DEMAND	ELEVATION	CONNEC	TING	PIPES	
2	5.00	1080.00	101	105	109	
4	5.00	1055.00	105	113	121	
8	40.00	1080.00	109	113	117	
12	5.00	1060.00	117	121	125	
16	5.00	875.00	125	129	137	149
20	5.00	900.00	129	133	135	147
22	5.00	925.00	133	169	173	
24	5.00	900.00	135	143	145	
28	25.00	925.00	141	145		
32	25.00	900.00	137	141	143	
36	25.00	890.00	147	161	165	
40	25.00	900.00	149	153	161	
44	25.00	905.00	153	157	165	
48	25.00	915.00	157	169		
200	.00	1100.00	173	177		

OUTPUT SELECTION: ALL RESULTS ARE OUTPUT EACH PERIOD 5 VALUES ARE OUTPUT FOR MAXIMUM AND MINIMUM PRESSURES

THIS SYSTEM HAS 23 PIPES WITH 15 JUNCTIONS , 7 LOOPS AND 2 FGNS

THE RESULTS ARE OBTAINED AFTER 6 TRIALS WITH AN ACCURACY = .00295

AVERAGE DAY DEMAND

PIPE N	NO.	NODE	NOS.		FLOWRATE	HEAD	LOSS	PUMP	HEAD	MINOR	LOSS	VELO	CITY	HL/100	00
101		0	2		225.00		07	L,	.00	. (00		36	.04	4
105		2	4		128.12		03		.00	. (00		36	.00	6
109		2	8		91.88		04		.00	. (0		26	.03	3
113		4	8		16.28		01		.00	.0	0		10	.03	L
117		8	12		68.16		02		.00	.0	0		19	.02	2
121		4	12		106.84		02		.00	. (0		30	.04	4
125		12	16		170.00	1.	11		.00	.0	0		48	.10)
129		16	20		97.38		02		.00	. (0		28	.03	3
133		20	22		29.21		00	I.	.00	. (0		80	.00	C
135		20	24		21.08		00	L,	.00	. (0		13	.03	1
137		16	32		33.92		02		.00	.0	0		22	.03	3
141		32	28		11.75		01		.00	. (0 -	•	07	.00)
143		32	24		-2.83		00		.00	. (0		02	.00	C
145		24	28		13.25		01		.00	. (0	•	80	.0	1
147		20	36		42.09		02	i	.00	. (0		27	.0!	5
149		16	40		33.70		04		.00	. (0		22	.03	3
153		40	44		14.84		00	L.	.00	. (0		09	.0	1
157		44	48		.79		00	l,	.00	. (0		01	-00	2
161		36	40		6.14		00		.00	. (0		04	.00)
165		36	44		10.95		00		.00	. (0		07	.00	0
169		22	48		24.21		02		.00	. (0		15	.02	2
173	2	00	22		.00		00	1	.00	. (0		00	.00	С
CD T T T	OUT.	OTE 577	T 7777	TAT	T TAIR MILIMINE	D 177	TC C	T ACET	`						

THE CHECK VALVE IN LINE NUMBER 177 IS CLOSED

JUNCTION	NUMBER	DEMAND	GRADE LINE	ELEVATION	PRESSURE
2		5.00	1250.93	1080.00	74.07
4		5.00	1250.89	1055.00	84.89
8		40.00	1250.89	1080.00	74.05
12		5.00	1250.87	1060.00	82.71
16		5.00	1249.75	875.00	162.39
20		5.00	1249.73	900.00	151.55
22		5.00	1249.73	925.00	140.72
24		5.00	1249.73	900.00	151.55
28		25.00	1249.72	925.00	140.71
32		25.00	1249.73	900.00	151.55
36		25.00	1249.71	890.00	155.88
40		25.00	1249.71	900.00	151.54
44		25.00	1249.71	905.00	149.37
48		25.00	1249.71	915.00	145.04

200	.00	1249.73	1100.00	64.88
	MAXIMUM P	RESSURES		
16	5.00	1249.75	875.00	162.39
36	25.00	1249.71	890.00	155.88
20	5.00	1249.73	900.00	151.55
24	5.00	1249.73	900.00	151.55
32	25.00	1249.73	900.00	151.55
	MINIMUM P	RESSURES		
200	.00	1249.73	1100.00	64.88
8	40.00	1250.89	1080.00	74.05
2	5.00	1250.93	1080.00	74.07
12	5.00	1250.87	1060.00	82.71
4	5.00	1250.89	1055.00	84.89

THE NET SYSTEM DEMAND = 225.00

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 101 225.00

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 225.00
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

A SUMMARY OF CONDITIONS SPECIFIED FOR THE NEXT SIMULATION FOLLOWS

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 5.70

THE RESULTS ARE OBTAINED AFTER 5 TRIALS WITH AN ACCURACY = .00006

PEAK HOUR DEMAND

PIPE	NO.	NODE	NOS.	FLOWRATE	HEAD LOSS	PUMP HEAD	MINOR LOSS	VELOCITY	HL/1000
101		0	2	1267.86	1.79	.00	.00	2.02	.97
105		2	4	721.43	.83	.00	.00	2.05	1.39
109		2	8	517.94	.97	.00	.00	1.47	.75
113		4	8	92.22	.14	.00	.00	.59	.22
117		8	12	382.16	.45	.00	.00	1.08	.43
121		4	12	600.71	.59	.00	.00	1.70	.99
125		12	16	954.36	27.21	.00	.00	2.71	2.33
129		16	20	544.84	.54	.00	.00	1.55	.82
133		20	22	153.02	.04	.00	.00	.43	.08
135		20	24	122.93	.06	.00	.00	.78	.38
137		16	32	190.57	.59	.00	.00	1.22	.85
141		32	28	66.94	.22	.00	.00	.43	.12
143		32	24	-18.87	.00	.00	.00	12	01
145		24	28	75.56	.22	.00	.00	.48	.15
147		20	36	240.39	.46	.00	.00	1.53	1.30

Newland S	Sierra Wat	er Systei	m Analysis – 12	35 Zone			August 3 File: 9900	
149	16	40	190.45	1.02	.00	.00	1.22	.8
1.53	40	44	83.60	.06	.00	.00	.53	.1
157	44	48	3.34	.00	.00	.00	.02	.0
161	36	40	35.66	.02	.00	.00	.23	.0
165	36	44	62.23	.08	.00	.00	.40	.1
169	22	48	139.16	.50	.00	.00	.89	. 4
173	200	22	14.64	.00	.00	.00	.04	.0
177	0	200	14.64	.00	.00	.00	.04	.0
JUNCT	ION NUM	BER	DEMAND	GRADE LINE	ELEVATION	PRESSURE		
	2		28.50	1249.21	1080.00	73.32		
	4		28.50	1248.38	1055.00	83.80		
	8		228.00	1248.23	1080.00	72.90		
	12		28.50	1247.78	1060.00	81.37		
	16		28.50	1220.57	875.00	149.75		
	20		28.50	1220.04	900.00	138.68		
	22		28.50	1220.00	925.00	127.83		
	24		28.50	1219.98	900.00	138.66		
	28		142.50	1219.76	925.00	127.73		
	32		142.50	1219.98	900.00	138.66		
	36		142.50	1219.58	890.00	142.82		
	40		142.50	1219.56	900.00	138.47		
	44		142.50	1219.50	905.00	136.28		
	48		142.50	1219.50	915.00	131.95		
T,	200		.00	1220.00	1100.00	52.00		
			MAXIMUM E	RESSURES				
	16		28.50	1220.57	875.00	149.75		
	36		142.50	1219.58	890.00	142.82		
	20		28.50	1220.04	900.00	138.68		
	24		28.50	1219.98	900.00	138.66		
	32		142.50	1219.98	900.00	138.66		
			MINIMUM E					
- 1	200		.00	1220.00	1100.00	52.00		
	8		228.00	1248.23	1080.00	72.90		
	2		28.50	1249.21	1080.00	73.32		
	12		28.50	1247.78	1060.00	81.37		
	4		28.50	1248.38	1055.00	83.80		

.85 .18 .00 .04 .11 .47 .00 .00

THE NET SYSTEM DEMAND = 1282.50

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

FLOWRATE PIPE NUMBER 101 1267.86 177 14.64

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 1282.50 THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

August 3, 2016 File: 990001a4

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.80

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE :

JUNCTION NUMBER

DEMAND

8

16

20

22

24

28

32

36

40

44

48

200

3610.00

THE RESULTS ARE OBTAINED AFTER 6 TRIALS WITH AN ACCURACY = .00049

MAXIMUM DAY DEMAND PLUS 3500 GPM FIRE FLOW AT NODE 8

PIPE	NO. N	ODE	NOS.	FLOWRATE	HEAD LOSS	PUMP HEAD	MINOR LOSS	VELOCITY	HL/1000
101		0	2	4084.10	15.65	.00	.00	6.52	8.46
105		2	4	2102.81	6.03	.00	.00	5.96	10.05
109		2	8	1967.29	11.54	.00	.00	5.58	8.88
113		4	8	660.99	5.52	.00	.00	4.22	8.49
117		8	12	-981.72	-2.57	.00	.00	-2.78	-2.45
121		4	12	1427.82	2.94	.00	.00	4.05	
125	1	2	16	432.10	6.27	.00	.00	1.23	.54
129	1	6	20	241.64	.12	.00	.00	.69	.18
133	2	0	22	41.01	.00	.00	.00	.12	.01
135	2	0	24	67.15	.02	.00	.00	.43	.12
137	1	6	32	86.85	.14	.00	.00	.55	.20
141	3	2	28	32.76	.06	.00	.00	.21	.03
143	3	2	24	-15.91	.00	.00	.00	10	01
145	2	4	28	37.24	.06	.00	.00	.24	.04
147	2	0	36	119.48	.12	.00	.00	.76	.36
149	1	6	40	89.61	.25	.00	.00	.57	.21
153	4	0	44	38.76	.01	.00	.00	.25	.04
157	4	4	48	91	.00	.00	.00	01	.00
161	3	6	40	19.15	.01	.00	.00	.12	.01
165	3	6	44	30.33	.02	.00	.00	.19	.03
169	2	2	48	70.91	.14	.00	.00	.45	.14
173	20	0	22	43.90	.02	.00	.00	.12	.01
177		0	200	43.90	.00	.00	.00	.12	.01
JUNCT	'ION N	UMBI	ER	DEMAND	GRADE LINE	ELEVATIO			
	2			14.00	1235.35	1080.00			
	4			14.00	1229.32	1055.00	75.5		
	8			3610.00	1223.80	1080.00	62.3		
	12			14.00	1226.38	1060.00	72.1	0	

1220.10

1219.99

1219.98

1219.91

1219.97

1219.86

1219.85

1219.84

1219.84

1219.97 900.00

1220.00 1100.00

875.00

900.00

925.00

925.00

900.00

900.00

905.00

915.00

890.00

149.55

138.66

127.83

138.65

127.79

138.65

142.94

138.60

136.43

132.10

52.00

MAXIMUM PRESSURES

14.00

14.00

14.00

14.00

70.00

70.00

70.00

70.00

70.00

70.00

.00

August 3, 2016 File: 990001a4

16	14.00	1220.10	875.00	149.55
36	70.00	1219.86	890.00	142.94
20	14.00	1219.99	900.00	138.66
24	14.00	1219.97	900.00	138.65
32	70.00	1219.97	900.00	138.65
	MINIMUM	PRESSURES		
200	.00	1220.00	1100.00	52.00
8	3610.00	1223.80	1080.00	62.32
2	14.00	1235.35	1080.00	67.32
12	14.00	1226.38	1060.00	72.10
		1000 30	1055 00	75 54
4	14.00	1229.32	1055.00	75.54

THE NET SYSTEM DEMAND = 4128.00

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 101 4084.10 177 43.90

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 4128.00
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

A SUMMARY OF CONDITIONS SPECIFIED FOR THE NEXT SIMULATION FOLLOWS

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.80

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE:

JUNCTION NUMBER

DEMAND

28

1555.00

THE RESULTS ARE OBTAINED AFTER 4 TRIALS WITH AN ACCURACY = .00100

MAXIMUM DAY DEMAND PLUS 1500 GPM FIRE FLOW AT NODE 28

PIPE NO.	NODE	NOS.	FLOWRATE	HEAD LOSS	PUMP HEAD	MINOR LOSS	VELOCITY	HL/1000
101	0	2	1211.87	1.65	.00	.00	1.93	.89
105	2	4	709.14	.81	.00	.00	2.01	1.34
109	2	8	488.74	.88	.00	.00	1.39	.67
113	4	8	62.74	.07	.00	.00	. 40	.11
117	8	12	439.47	.58	.00	.00	1.25	.55
121	4	12	632.40	.65	.00	.00	1.79	1.09
125	12	16	1057.87	32.93	.00	.00	3.00	2.81
129	16	20	415.12	.32	.00	.00	1.18	.50
133	20	22	-735.06	72	.00	.00	-2.09	-1.43
135	20	24	1111.96	3.34	.00	.00	7.10	22.24
137	16	32	527.04	3.91	.00	.00	3.36	5.58
141	32	28	729.37	18.33	.00	.00	4.66	10.18
143	32	24	-272.33	25	.00	.00	-1.74	-1.64

Newland S	ierra Wat	er Syste	m Analysis – 12	35 Zone			August File: 990	
145	24	28	825.63	18.58	.00	.00	5.27	12.81
147	20	36	24.21	.01	.00	.00	.15	.02
149	16	40	101.72	.32	.00	.00	.65	.27
153	40	44	7.63	.00	.00	.00	.05	.00
157	44	48	-84.07	11	.00	.00	54	19
161	36	40	-24.08	01	.00	.00	15	02
165	36	44	-21.70	01	.00	.00	14	02
169	22	48	154.07	.60	.00	.00	.98	.57
173	200	22	903.13	4.62	.00	.00	2.56	2.10
177	0	200	903.13	.02	.00	.00	2.56	2.10
177	V	200	903.13	.02	.00	•00	2.50	2.10
JUNCT	ION NUM	BER	DEMAND	GRADE LINE	ELEVATION	PRESSURE		
	2		14.00	1249.35	1080.00	73.39		
	4		14.00	1248.55	1055.00	83.87		
	8		112.00	1248.47	1080.00	73.01		
	12		14.00	1247.89	1060.00	81.42		
	16		14.00	1214.97	875.00	147.32		
	20		14.00	1214.64	900.00	136.35		
	22		14.00	1215.36	925.00	125.82		
	24		14.00	1211.31	900.00	134.90		
	28		1555.00	1192.73	925.00	116.02		
	32		70.00	1211.06	900.00	134.79		
	36		70.00	1214.64	890.00	140.68		
	40		70.00	1214.65	900.00	136.35		
	44		70.00	1214.65	905.00	134.18		
	48		70.00	1214.76	915.00	129.90		
2	200		.00	1219.98	1100.00	51.99		
			MAXIMUM P	RESSURES				
	16		14.00	1214.97	875.00	147.32		
	36		70.00	1214.64	890.00	140.68		
	40		70.00	1214.65	900.00	136.35		
	20		14.00	1214.64	900.00	136.35		
	24		14.00	1211.31	900.00	134.90		
			MINIMUM P					
2	200		.00	1219.98	1100.00	51.99		
	8		112.00	1248.47	1080.00	73.01		
	2		14.00	1249.35	1080.00	73.39		
	12		14.00	1247.89	1060.00	81.42		
	4		14.00	1248.55	1055.00	83.87		

THE NET SYSTEM DEMAND = 2115.00

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 101 1211.87 177 903.13

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 2115.00 THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.80

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE:

JUNCTION NUMBER DEMAND

44

2555.00

THE RESULTS ARE OBTAINED AFTER 4 TRIALS WITH AN ACCURACY = .00025

MAXIMUM DAY DEMAND PLUS 2500 GPM FIRE FLOW AT NODE 44

PIPE NO. NODE NOS. FLOWRATE HEAD LOSS PUMP HEAD MINOR LOSS VELOCITY HL/1000 101 0 2 1381.40 2.10 .00 .00 2.20 1.14 811.13 1.03 556.26 1.11 67.22 .08 511.49 .77 729.91 .85 2 4 .00 .00 2.30 1.72 105 .00 2 8 .00 1.58 .86 109 4 8 8 12 8 .00 .12 .00 113 .43 1.45 .00 .73 .00 117 .00 2.07 121 4 12 729.91 .85 .00 1.42 .00 2.07 1.42 .00 3.48 3.71 .00 1.04 .40 .00 -2.60 -2.16 .00 .20 .03 .00 .78 .37 .00 .21 .03 12 16 1227.40 43.36 16 20 368.21 .26 20 22 -917.06 -1.08 20 24 31.50 .00 16 32 122.50 .26 32 28 33.25 .06 .00 125 .00 129 133 135 137 141 .01 19.25 36.75 .00 .00 .00 .12 32 24 143 .06
1239.77 9.52
16 40 722.69 12.01
40 44 1058.44 6.09
44 48 -732.55 -6.16
36 40 405.76 2.23
36 44 764.01 8.32
22 48 802 55 .00 .00 .23 .04 145 .00 .00 7.91 27.20 .00 4.61 10.01 .00 6.76 20.29 .00 -4.68 -10.27 .00 2.59 3.44 147 149 153 157 44 48 161 36 40 -4.60 2.59 3.-. 11.10 .00 4.88 .00 .00 165 .00 12.16 .00 5.12 169 1/3 200 177 ^ 200 22 1733.60 15.46 0 200 1733.60 .07 .00 4.92 7.03 .00 4.92 7.03 .00 .07

JUNCTION	NUMBER	DEMAND	GRADE LINE	ELEVATION	PRESSURE
2		14.00	1248.90	1080.00	73.19
4		14.00	1247.86	1055.00	83.57
8		112.00	1247.79	1080.00	72.71
12		14.00	1247.02	1060.00	81.04
16		14.00	1203.65	875.00	142.42
20		14.00	1203.39	900.00	131.47
22		14.00	1204.47	925.00	121.11
24		14.00	1203.39	900.00	131.47
28		70.00	1203.33	925.00	120.61
32		70.00	1203.39	900.00	131.47
36		70.00	1193.87	890.00	131.68
40		70.00	1191.64	900.00	126.38
44		2555.00	1185.55	905.00	121.57
48		70.00	1191.71	915.00	119.91
200		.00	1219.93	1100.00	51.97

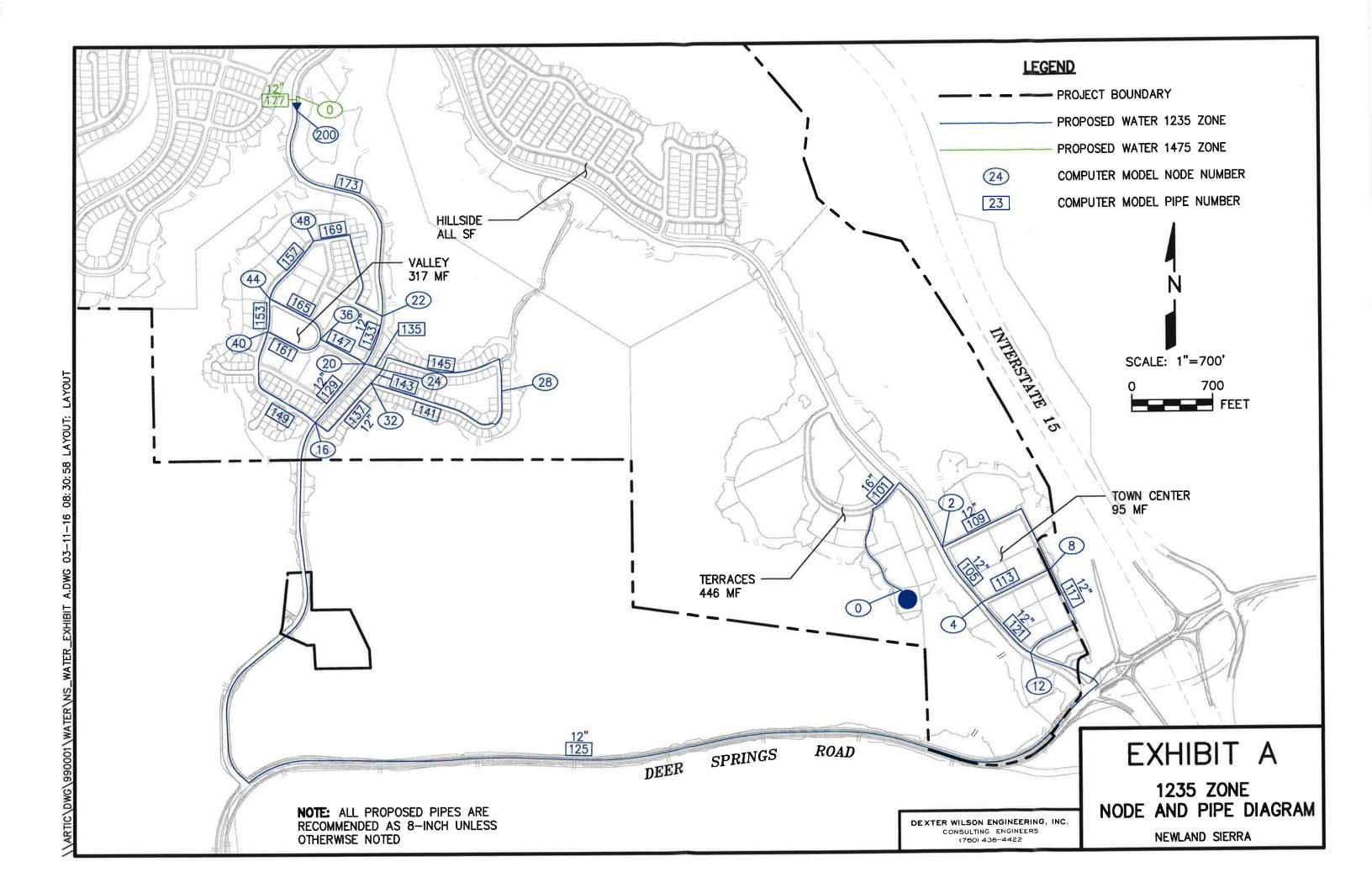
	MAXIMUM	PRESSURES		
16	14.00	1203.65	875.00	142.42
36	70.00	1193.87	890.00	131.68
20	14.00	1203.39	900.00	131.47
32	70.00	1203.39	900.00	131.47
24	14.00	1203.39	900.00	131.47
	MINIMUM	PRESSURES		
200	.00	1219.93	1100.00	51.97
8	112.00	1247.79	1080.00	72.71
2	14.00	1248.90	1080.00	73.19
4.0				
12	14.00	1247.02	1060.00	81.04
12 4	14.00 14.00	1247.02 1247.86	1060.00 1055.00	81.04 83.57

THE NET SYSTEM DEMAND = 3115.00

SUMMARY OF INFLOWS(+) AND OUTFLOWS(-) FROM FIXED GRADE NODES

PIPE NUMBER	FLOWRATE
101	1381.40
177	1733.60

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 3115.00
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00



APPENDIX C

1608/1475 ZONE COMPUTER MODELING OUTPUT

The following conditions were modeled:

- 1. Average day demand
- 2. Peak hour demand
- 3. Maximum day demand plus 1500 gpm fire flow at node 8
- 4. Maximum day demand plus 1500 gpm fire flow at node 24
- 5. Maximum day demand plus 2500 gpm fire flow at node 152
- 6. Maximum day demand plus 2500 gpm fire flow at node 128

FLOWRATE IS EXPRESSED IN GPM AND PRESSURE IN PSIG

A SUMMARY OF THE ORIGINAL DATA FOLLOWS

THERE IS A PRV AT JUNCTION 36 FOR LINE 501 SET AT A GRADE OF 1475.00 THERE IS A PRV AT JUNCTION 40 FOR LINE 527 SET AT A GRADE OF 1475.00

PIPE	NO. NO	ODE NOS.	LENGTH (FEET)	DIAMETER (INCHES)	ROUGHNESS	MINOR LOSS K	FIXED GRADE
401	0	4	800.0		130.0	.00	1628.00
403	4	8	1550.0	8.0	130.0	.00	
405	8	12	2100.0	8.0	130.0	.00	
409	16	12	500.0	16.0	130.0	.00	
413	4	10	800.0	16.0	130.0	.00	
415	10	16	1000.0	16.0	130.0	.00	
417	16	20	500.0	8.0	130.0	.00	
421	20	24	1500.0	8.0	130.0	.00	
425	24	28	2100.0	8.0	130.0	.00	
429	20	24	1300.0	8.0	130.0	.00	
433	12	32	700.0	16.0	130.0	.00	
437	32	28	30.0	12.0	130.0	.00	
441	28	36	100.0	12.0	130.0	.00	
445	32	40	3400.0	16.0	130.0	.00	
501	36	100	200.0	12.0	130.0	.00	
503	100	104	400.0	12.0	130.0	.00	
505	104	108	900.0	12.0	130.0	.00	
509	108	112	1800.0	12.0	130.0	.00	
513	112	116	1450.0	12.0	130.0	.00	
517	116	120	2100.0	12.0	130.0	.00	
521	120	124	900.0		130.0	.00	
525	120	128	1100.0	12.0	130.0	.00	
527	40	112	20.0	16.0	130.0	.00	
529	124	128	900.0	12.0	130.0	.00	
533	100	132	1000.0	8.0	130.0	.00	
537	132	140	1700.0		130.0	.00	
541	136	140	2300.0	8.0	130.0	.00	
545	104	136	1050.0	8.0	130.0	.00	
549	108	136	1900.0	8.0	130.0	.00	
551	104	152	1500.0	8.0	130.0	.00	
553	108	144	200.0	12.0	130.0	.00	
555	152	156	800.0		130.0	.00	
557	144	152	1000.0		130.0	.00	
563	148	156	2800.0	8.0	130.0	.00	
567	148	156	800.0	8.0	130.0	.00	
571	112	160	200.0	8.0	130.0	.00	
573	116	168	200.0	8.0	130.0	.00	
575	160	164	650.0	8.0	130.0	.00	
579	160	164	1750.0	8.0	130.0	.00	
583	160	168	1400.0	8.0	130.0	.00	
587	164	172	1600.0	8.0	130.0	.00	

591 168 172 250.0 8.0 130.0

August 3, 2016 File: 990001b1

.00

595	172	176	800	.0	8.0	130.	0		.00
599	168	176	1500	.0	8.0	130.	0		.00
*									
JUNCTION	NUMBE:	r i	DEMAND	E	LEVATION	CONNEC	TING	PIPES	
4			16.00		1465.00	401	403	413	
8			35.00		1495.00	403	405		
10			14.00		1420.00	413	415		
12			35.00		1450.00	405	409	433	
16			35.00		1425.00	409	415	417	
20			36.00		1380.00	417	421	429	
24			36.00		1400.00	421	425	429	
28			13.00		1315.00	425	437	441	
32			.00		1315.00	433	437	445	
36			.00		1310.00	441	501		
40			.00		1275.00	445	527		
100			7.00		1285.00	501	503	533	
104			7.00		1270.00	503	505	545	551
108			7.00		1250.00	505	509	549	553
112			7.00		1265.00	509	513	527	571
116			7.00		1300.00	513	517	573	
120			7.00		1260.00	517	521	525	
124			47.00		1185.00	521	529		
128			47.00		1300.00	525	529		
132			38.00		1230.00	533	537		
136			38.00		1225.00	541	545	549	
140			38.00		1175.00	537	541	8	
144			34.00		1260.00	553	557		
148			34.00		1275.00	563	567		
152			34.00		1280.00	551	555	557	
156			34.00		1290.00	555	563	567	
160			20.00		1270.00	571	575	579	583

OUTPUT SELECTION: ALL RESULTS ARE OUTPUT EACH PERIOD 5 VALUES ARE OUTPUT FOR MAXIMUM AND MINIMUM PRESSURES

1275.00

1300.00

1295.00

1300.00

575 579 587

595 599

573 583 591 **599** 587 **591** 595

THIS SYSTEM HAS MULTIPLE SUPPLY ZONES

ZONE NO. 1 IS SUPPLIED THROUGH THESE PIPES: 501

20.00

20.00

20.00

20.00

527

THIS SYSTEM HAS 44 PIPES WITH 31 JUNCTIONS , 12 LOOPS AND 3 FGNS

THE RESULTS ARE OBTAINED AFTER 6 TRIALS WITH AN ACCURACY = .00315

164

168

172

176

			FLOWRATE			MINOR LOSS		
401	0	4	706.00	.26	.00	.00	1.13	.33
403	4	8	89.55	.32	.00	.00	.57	.21
405	8	12	54.55	.18	.00	.00	.35	.08
409	16	12	456.87	.07	.00	.00	.73	.15
413	4	10	600.45	.19	.00	.00	.96	.24
415	10	16	586.45	.23	.00	.00	.94	.23
417	16	20	94.58	.12	.00	.00	.60	.23
421	20	24	28.16	.04	.00	.00	.18	.02
425	24	28	22.58	.03	.00	.00	.14	.02
429	20	24	30.42	.04	.00	.00	.19	.03
433	12	32	476.42	.11	.00	.00	.76	
437	32	28	173.82	.00	.00	.00	.49	.10
441	28	36	183.40	.01	.00	.00	.52	.11
445	32	40	302.60	.23	.00	.00	.48	
501	0	100	183.40	.02	.00	.00	.52	.11
503	100	104	122.20	.02	.00	.00	.35	.05
505	104	108	50.29	.01	.00	.00	.14	.01
509	108	112	-87.60	05	.00	.00	25	03
513	112	116	120.16	.07	.00	.00	.34	.05
517	116	120	101.00	.08	.00	.00	.29	.04
521	120	124	49.49	.01	.00	.00	.14	.01
525	120	128	44.51	.01	.00	.00	.13	.01
527	0	112	302.60	.00	.00	.00	.48	.07
529	124	128	2.49	.00	.00	.00	.01	.00
533	100	132	54.20	.08	.00	.00	.35	.08
537	132	140	16.20	.02	.00	.00	.10	.01
541	136	140	21.80	.04	.00	.00	.14	.02
545	104	136	36.53	.04	.00	.00	.23	.04
549	108	136	23.27	.03	.00	.00	.15	.02
551	104	152	28.38	.04	.00	.00	.18	.02
553	108	144	107.62	.01	.00	-00	.31	.04
555	152	156	68.00	.10	.00	.00	.43	.13
557	144		73.62	.02	.00	.00	.21	.02
563	148	156	-11.53	01	.00	.00	07	.00
567	148	156	-22.47	01	.00	.00	14 .56	02
571	112	160	87.84	.04	.00	.00		.20
573	116	168	12.16	.00	.00	.00	.08	.01
575	160	164	25.33	.01	.00	.00	.16	.02 .01
579	160	164	14.84	.01	.00	.00 .00	.09 .18	.01
583	160	168	27.66	.03	.00		.13	.02
587	164	172	20.17 10.80	.02 .00	.00 .00	.00	.13	.00
591	168	172	10.80			.00	.07	.00
595	172	176		.00	.00	.00		
599	168	176	9.02	.00	.00	.00	.06	.00
JUNCI	ION NUMB	ΞR	DEMAND	GRADE LINE	ELEVATIO	ON PRESSUE	RE	
	4		16.00	1627.74	1465.00	70.52	2	
	8		35.00	1627.41	1495.00	57.38	3	
	10		14.00	1627.54	1420.00	89.94	1	
	12		35.00	1627.24	1450.00	76. 80)	
	16		35.00	1627.31	1425.00	87.67	7	
	20		36.00	1627.19	1380.00	107.12	2	

24	36.00	1627.16	1400.00	98.44
28	13.00	1627.12	1315.00	135.25
32	.00	1627.13	1315.00	135.26
36	.00	1627.11	1310.00	137.42
40	.00	1626.89	1275.00	152.49
100	7.00	1474.98	1285.00	82.32
104	7.00	1474.96	1270.00	88.81
108	7.00	1474.95	1250.00	97.48
112	7.00	1475.00	1265.00	91.00
116	7.00	1474.93	1300.00	75.80
120	7.00	1474.85	1260.00	93.10
124	47.00	1474.84	1185.00	125.60
128	47.00	1474.84	1300.00	75.76
132	38.00	1474.90	1230.00	106.12
136	38.00	1474.92	1225.00	108.30
140	38.00	1474.88	1175.00	129.95
144	34.00	1474.94	1260.00	93.14
148	34.00	1474.81	1275.00	86.58
152	34.00	1474.92	1280.00	84.47
156	34.00	1474.82	1290.00	80.09
160	20.00	1474.96	1270.00	88.82
164	20.00	1474.95	1275.00	86.64
168	20.00	1474.92	1300.00	75.80
172	20.00	1474.92	1295.00	77.97
176	20.00	1474.92	1300.00	75.80
	MAXIMUM	PRESSURES		
40	.00	1626.89	1275.00	152.49
36	.00	1627.11	1310.00	137.42
32	.00	1627.13	1315.00	135.26
28	13.00	1627.12	1315.00	135.25
140	38.00	1474.88	1175.00	129.95
	MINIMUM	PRESSURES		
8	35.00	1627.41	1495.00	57.38
4	16.00	1627.74	1465.00	70.52
128	47.00	1474.84	1300.00	75.76
176	20.00	1474.92	1300.00	75.80
168	20.00	1474.92	1300.00	75.80

THE NET SYSTEM DEMAND = 706.00

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 401 706.00

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 706.00
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

A SUMMARY OF CONDITIONS SPECIFIED FOR THE NEXT SIMULATION FOLLOWS

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 4.90

THE RESULTS ARE OBTAINED AFTER 2 TRIALS WITH AN ACCURACY = .00002

PEAK HOUR DEMAND

PIPE		NOS.				MINOR LOSS		
401	0	4	3459.40	4.98	.00	.00	5.52	6.22
403	4	8	438.77	6.16	.00	.00	2.80	3.97
405	8	12	267.27	3.33	.00	.00	1.71	1.59
409	16	12	2238.69	1.39	.00	.00	3.57	2.78
413	4	10	2942.23	3.69	.00	-00	4.69	4.61
415	10	16	2873.63	4.41	.00	.00	4.59	4.41
417	16	20	463.44	2.20	.00	.00	2.96	4.40
421	20	24	137.98	.70	.00	.00	.88	.47
425	24	28	110.64	.65	.00	.00	.71	.31
429	20	24	149.06	.70	.00	.00	.95	.54
433	12	32	2334.46	2.10	.00	.00	3.72	3.00
437	32	28	851.73	.06	.00	.00	2.42	1.88
441	28	36	898.67	.21	.00	.00	2.55	2.08
445	32	40	1482.73	4.41	.00	.00	2.37	1.30
501	0	100	898.67	.42	.00	.00	2.55	2.08
503	100	104	598.78	.39	.00	.00	1.70	.98
505	104	108	246.44	.17	.00	.00	.70	.19
509	108	112	-429.23	95	.00	.00	-1.22	53
513	112	116	588.80	1.38	.00	.00	1.67	.95
517	116	120	494.90	1.45	.00	.00	1.40	.69
521	120	124	242.52	.17	.00	.00	.69	.18
525	120	128	218.08	.17	.00	.00	.62	.15
527	0	112	1482.73	.03	.00	.00	2.37	1.30
529	124	128	12.22	.00	.00	.00	.03	.00
533	100	132	265.58	1.57	.00	.00	1.70	1.57
537	132	140	79.38	.28	.00	.00	.51	.17
541	136	140	106.82	. 67	.00	.00	.68	.29
545	104	136	179.00	.79	.00	.00	1.14	.76
549	108	136	114.02	. 62	.00	.00	.73	.33
551	104	152	139.04	.71	.00	.00	.89	.47
553	108	144	527.36	.16	.00	.00	1.50	.78
555	152	156	333.20	1.91	.00	.00	2.13	2.39
557	144	152	360.76	.38	.00	.00	1.02	.38
563	148	156	-56.15	25	.00	.00	36	09
567	148	156	-110.45	25	.00	.00	70	31
571	112	160	430.40	.77	.00	.00	2.75	3.83
573	116	168	59.60	.02	.00	.00	.38	.10
575	160	164	124.13	.25	.00	.00	.79	.38
579	160	164	72.72	.25	.00	.00	.46	.14
583	160	168	135.55	.63	.00	.00	.87	.45
587	164	172	98.85	.40	.00	.00	.63	.25
591	168	172	52.94	.02	.00	.00	.34	.08
595	172	176	53.79	.07	.00	.00	.34	.08
599	168	176	44.21	.08	.00	.00	.28	.06

JUNCTION NUMBER	DEMAND	GRADE LINE	ELEVATION	PRESSURE
4	78.40	1623.02	1465.00	68.48
8	171.50	1616.86	1495.00	52 - 81

10	68.60	1619.33	1420.00	86.38
12	171.50	1613.53	1450.00	70.86
16	171.50	1614.92	1425.00	82.30
20	176.40	1612.72	1380.00	100.85
24	176.40	1612.02	1400.00	91.88
28	63.70	1611.37	1315.00	128.43
32	.00	1611.43	1315.00	128.45
36	.00	1611.17	1310.00	130.51
40	.00	1607.03	1275.00	143.88
100	34.30	1474.58	1285.00	82.15
104	34.30	1474.19	1270.00	88.48
108	34.30	1474.02	1250.00	97.08
112	34.30	1474.97	1265.00	90.99
116	34.30	1473.60	1300.00	75.22
120	34.30	1472.15	1260.00	91.93
124	230.30	1471.98	1185.00	124.36
128	230.30	1471.98	1300.00	74.53
132	186.20	1473.02	1230.00	105.31
136	186.20	1473.40	1225.00	107.64
140	186.20	1472.73	1175.00	129.02
144	166.60	1473.87	1260.00	92.68
148	166.60	1471.33	1275.00	85.07
152	166.60	1473.48	1280.00	83.84
156	166.60	1471.57	1290.00	78.68
160	98.00	1474.21	1270.00	88.49
164	98.00	1473.96	1275.00	86.22
168	98.00	1473.58	1300.00	75.22
172	98.00	1473.56	1295.00	77.37
176	98.00	1473.49	1300.00	75.18
	MAXIMUM	PRESSURES		
40	.00	1607.03	1275.00	143.88
36	.00	1611.17	1310.00	130.51
140	186.20	1472.73	1175.00	129.02
32	.00	1611.43	1315.00	128.45
28	63.70	1611.37	1315.00	128.43
	MTNTMIIM	PRESSURES		
8	171.50	1616.86	1495.00	52.81
4	78.40	1623.02	1465.00	68.48
12	171.50	1613.53	1450.00	70.86
128	230.30	1471.98	1300.00	74.53
176	98.00	1473.49	1300.00	75.18
1,0	30.00			

THE NET SYSTEM DEMAND = 3459.40

SUMMARY OF INFLOWS(+) AND OUTFLOWS(-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 401 3459.40

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 3459.40
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.60

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE:

JUNCTION NUMBER DEMAND

8

1590.00

THE RESULTS ARE OBTAINED AFTER 3 TRIALS WITH AN ACCURACY = .00032

MAXIMUM DAY DEMAND PLUS 1500 GPM FIRE FLOW AT NODE 8

PIPE NO. NODE NOS. FLOWRATE HEAD LOSS PUMP HEAD MINOR LOSS VELOCITY HL/1000 401 0 4 3334.60 4.65 .00 .00 5.32 5.81 925.56 24.54 -664.44 -17.99 1956.62 1.08 2367.44 2.47 .00 15.83 .00 5.91 8 403 .00 -4.24 .00 3.12 .00 3.78 -8.57 .00 8 12 405 .00 2.17 16 12 409 3.08 4 10 413 2.99 2.99 2331.04 .00 3.72 10 16 415

 10
 16
 2331.04
 2.39

 16
 20
 283.42
 .88

 20
 24
 91.25
 .33

 24
 28
 96.22
 .50

 20
 24
 98.58
 .33

 12
 32
 1201.18
 .61

 32
 28
 414.42
 .01

 28
 36
 476.84
 .06

 .00 1.81 1.77 .00 .00 1.81 .00 .58 .00 .61 .00 .63 .00 1.92 .00 1.18 417 .22 421 .24 425 .25 429 .88 433 .50 437 .64 441

 445
 32
 40
 786.76
 1.36

 501
 0
 100
 476.84
 .13

 503
 100
 104
 317.72
 .12

 505
 104
 108
 130.77
 .05

 509
 108
 112
 -227.76
 -.29

 513
 112
 116
 312.42
 .43

 517
 116
 120
 262.60
 .45

 .00 .40 -00 1.26 .00 .00 1.35 .64 .00 .90 .30 .00 .37 .06 .00 -.65 -.16 .00 .89 .29 128.68 .05 115.72 .05 786.76 .01 6.48 .00 140.92 .48 42.12 .09 56.68 .21 .37
 521
 120
 124

 525
 120
 128
 .06 .00 .00 .00 .00 .33 .05 .40 0 112 527 .00 529 124 128 .48 100 132 533 .05 132 140 537 .09 136 140 541 .00 .00 .61 .23 94.98 .25 104 136 545 60.50 .19 73.78 .22 279.82 .05 176.80 .59 191.42 .12 -29.80 -.08 .00 .39 .00 .47 .00 .79 .00 1.13 .00 .54 .00 .00 108 136 .39 .10 549 .00 .15 551 104 152 .24 553 108 144 .74 152 156 555 .12 144 152 557 .00 -.03 -.19 563 148 156 -.08 .00 -.37 -.10 148 156 -58.60 .00 567

 -58.60
 -.00

 228.38
 .24

 31.62
 .01

 65.87
 .08

 38.58
 .08

 71.93
 .20

 52.45
 .12

 .00 .00 1.46 1.19 571 112 160 .00 .00 1.46 1.19 .00 .00 .20 .03 .00 .00 .42 .12 .00 .00 .25 .04 .00 .00 .46 .14 .00 .00 .33 .08 116 168 573 575 160 164 160 579 164 160 168 583 587 164 172

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.03

.02

.18 .02

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.15

591 168 595 172 599 168	172 28.09 176 28.54 176 23.46	.01 .02 .03	.00	.00
JUNCTION NUMB: 4 8 10 12 16 20 24 28 32 36 40 100 104 108 112 116 120 124 128 132 136 140 144 148 152 156	41.60 1590.00 36.40 91.00 91.00 93.60 93.60 33.80 .00 .00 .00 18.20 18.20 18.20 18.20 18.20 12.20 122.20 98.80 98.80 98.80 98.80 88.40 88.40 88.40 88.40	GRADE LINE 1623.35 1598.81 1620.88 1616.81 1617.01 1616.68 1616.18 1616.19 1616.11 1614.83 1474.87 1474.75 1474.75 1474.70 1474.99 1474.57 1474.07 1474.07 1474.07 1474.07 1474.30 1474.50 1474.30 1474.53 1473.86 1473.94	ELEVATION 1465.00 1495.00 1420.00 1450.00 1450.00 1380.00 1400.00 1315.00 1315.00 1315.00 1275.00 1285.00 1270.00 1250.00 1265.00 1300.00 1265.00 1300.00 1260.00 1185.00 1300.00 1275.00 1280.00 1275.00 1280.00 1290.00	PRESSURE 68.62 44.99 87.05 72.28 83.59 102.70 93.89 130.51 130.52 132.65 147.26 82.28 88.72 97.37 91.00 75.65 92.78 125.26 75.43 105.90 108.12 129.70 93.01 86.17 84.30 79.71
160 164 168	52.00 52.00 52.00	1474.75 1474.68 1474.56	1270.00 1275.00 1300.00	88.73 86.53 75.64
172 176	52.00 52.00	1474.55 1474.53	1295.00 1300.00	77.81 75.63
40 36 32 28 140	MAXIMUM E .00 .00 .00 .33.80 98.80	PRESSURES 1614.83 1616.11 1616.19 1616.18 1474.30	1275.00 1310.00 1315.00 1315.00 1175.00	147.26 132.65 130.52 130.51 129.70
8 4 12 128 176	MINIMUM E 1590.00 41.60 91.00 122.20 52.00	PRESSURES 1598.81 1623.35 1616.81 1474.07 1474.53	1495.00 1465.00 1450.00 1300.00	44.99 68.62 72.28 75.43 75.63

THE NET SYSTEM DEMAND = 3334.60

SUMMARY OF INFLOWS(+) AND OUTFLOWS(-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 401 3334.60

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 3334.60
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

A SUMMARY OF CONDITIONS SPECIFIED FOR THE NEXT SIMULATION FOLLOWS

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.60

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE :

JUNCTION NUMBER

DEMAND

24

1593.00

THE RESULTS ARE OBTAINED AFTER 4 TRIALS WITH AN ACCURACY = .00119

MAXIMUM DAY DEMAND PLUS 1500 GPM FIRE FLOW AT NODE 24

PIPE	NO. NODE	NOS.	FLOWRATE			MINOR LOSS		
401	0	4	3335.00	4.65	.00	.00	5.32	5.81
403	4	8	385.82	4.85	.00	.00	2.46	3.13
405	8	12	294.82	4.00	.00	.00	1.88	1.90
409	16	12	1698.23	.83	.00	.00	2.71	1.67
413	4	10	2907.58	3.61	.00	.00	4.64	4.51
415	10	16	2871.18	4.41	.00	.00	4.58	4.41
417	16	20	1081.96	10.57	.00	.00	6.91	21.14
421	20	24	475.10	6.91	.00	.00	3.03	4.60
425	24	28	-604.64	-15.11	.00	.00	-3.86	-7.19
429	20	24	513.26	6.91	.00	.00	3.28	5.31
433	12	32	1902.04	1.44	.00	.00	3.03	2.05
437	32	28	1115.29	.09	.00	.00	3.16	3.10
441	28	36	476.84	.06	.00	.00	1.35	.64
445	32	40	786.76	1.36	.00	.00	1.26	.40
501	0	100	476.84	.13	.00	.00	1.35	.64
503	100	104	317.72	.12	.00	.00	.90	.30
505	104	108	130.77	.05	.00	.00	.37	.06
509	108	112	-227.76	29	.00	.00	65	16
513	112	116	312.42	.43	.00	.00	.89	.29
517	116	120	262.60	.45	.00	.00	.74	.21
521	120	124	128.68	.05	.00	.00	.37	.06
525	120	128	115.72	.05	.00	.00	.33	.05
527	0	112	786.76	.01	.00	.00	1.26	.40
529	124	128	6.48	.00	.00	.00	.02	.00
533	100	132	140.92	.48	.00	.00	.90	.48
537	132	140	42.12	.09	.00	.00	.27	.05
541	136	140	56.68	.21	.00	.00	.36	.09
545	104	136	94.98	.25	.00	.00	.61	.23
549	108	136	60.50	.19	.00	.00	.39	.10
551	104	152	73.78	.22	.00	.00	.47	.15
553	108	144	279.82	.05	.00	.00	.79	.24
555	152	156	176.80	.59	.00	-00	1.13	.74
557	144	152	191.42	.12	.00	.00	.54	.12

Newland Sierra Water System Analysis – 1608/1475 Zone							
563	148	156	-29.80	08	.00	.00	
567	148	156	-58.60		.00	.00	
571	112	160	228.38		.00	.00	
573	116	168	31.62		.00	.00	
575	160	164	65.87		.00	.00	
579	160	164	38.58		.00	.00	
583	160	168	71.93		.00	.00	
587	164	172	52.45		.00	.00	
591	168	172	28.09		.00	.00	
595	172	176	28.54		.00	.00	
599	168	176	23.46		.00	.00	
399	100	170	23.40	.03	.00	.00	
JUNCTI	ON NUM	BER	DEMAND	GRADE LINE	ELEVATION	PRESSURE	
	4		41.60	1623.35	1465.00	68.62	
	8		91.00	1618.50	1495.00	53.52	
	10		36.40	1619.74	1420.00	86.55	
•	12		91.00	1614.50	1450.00	71.28	
	16		91.00	1615.34	1425.00	82.48	
	20		93.60	1604.77	1380.00	97.40	
:	24		1593.00	1597.86	1400.00	85.74	
	28		33.80	1612.97	1315.00	129.12	
	32		.00	1613.06	1315.00	129.16	
	36		.00	1612.91	1310.00	131.26	
4	40		.00	1611.70	1275.00	145.90	
10	00		18.20	1474.87	1285.00	82.28	
10	04		18.20	1474.75	1270.00	88.72	
10	08		18.20	1474.70	1250.00	97.37	
13	12		18.20	1474.99	1265.00	91.00	
1:	16		18.20	1474.57	1300.00	75.65	
12	20		18.20	1474.12	1260.00	92.78	
12	24		122.20	1474.07	1185.00	125.26	
12	28		122.20	1474.07	1300.00	75.43	
13	32		98.80	1474.39	1230.00	105.90	
13	36		98.80	1474.50	1225.00	108.12	
14	40		98.80	1474.30	1175.00	129.70	
14	44		88.40	1474.65	1260.00	93.01	
14	48		88.40	1473.86	1275.00	86.17	
	52		88.40	1474.53	1280.00	84.30	
	56		88.40	1473.94	1290.00	79.71	
	60		52.00	1474.75	1270.00	88.73	
	54		52,00	1474.68	1275.00	86.53	
	68		52.00	1474.56	1300.00	75.64	
	72		52.00	1474.55	1295.00	77.81	
17	76		52.00	1474.53	1300.00	75.63	
	4.0			PRESSURES	1075 00	145 00	
	10		.00	1611.70	1275.00	145.90	
	36		.00	1612.91	1310.00	131.26	
	10		98.80	1474.30	1175.00	129.70	
	32		.00	1613.06	1315.00	129.16	
2	28		33.80	1612.97	1315.00	129.12	
				PRESSURES			
	8		91.00	1618.50	1495.00	53.52	
	4		41.60	1623.35	1465.00	68.62	
	L2		91.00	1614.50	1450.00	71.28	
12	28		122.20	1474.07	1300.00	75.43	

August 3, 2016

File: 990001b1

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52.00 1474.53 1300.00

75.63

THE NET SYSTEM DEMAND = 3335.00

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

FLOWRATE PIPE NUMBER

401 3335.00

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 3335.00 THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

A SUMMARY OF CONDITIONS SPECIFIED FOR THE NEXT SIMULATION FOLLOWS

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.60

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE:

JUNCTION NUMBER

DEMAND

152

2590.00

THE RESULTS ARE OBTAINED AFTER 4 TRIALS WITH AN ACCURACY = .00468

MAXIMUM DAY DEMAND PLUS 2500 GPM FIRE FLOW AT NODE 152

PIPE	NO. NOD	E NOS.	FLOWRATE	HEAD LOSS	PUMP HEAD	MINOR LOSS	VELOCITY	HL/1000
401	0	4	4337.20	7.57	.00	.00	6.92	9.46
403	4	8	509.56	8.12	.00	.00	3.25	5.24
405	8	12	418.56	7.65	.00	.00	2.67	3.64
409	16	12	3181.65	2.66	.00	.00	5.08	5.33
413	4	10	3786.04	5.88	.00	.00	6.04	7.35
415	10	16	3749.64	7.22	.00	.00	5.98	7.22
417	16	20	476.98	2.32	.00	.00	3.04	4.64
421	20	24	184.29	1.20	.00	.00	1.18	.80
425	24	28	289.78	3.87	.00	.00	1.85	1.84
429	20	24	199.09	1.20	.00	.00	1.27	.92
433	12	32	3509.22	4.47	.00	.00	5.60	6.39
437	32	28	1761.82	.22	.00	.00	5.00	7.24
441	28	36	2017.80	.93	.00	.00	5.72	9.31
445	32	40	1747.40	5.9 7	.00	.00	2.79	1.76
501	0	100	2017.80	1.86	.00	.00	5.72	9.31
503	100	104	1719.75	2.77	.00	.00	4.88	6.92
505	104	108	850.03	1.69	.00	.00	2.41	1.88
509	108	112	-1188.40	-6.28	.00	.00	-3.37	-3.49
513	112	116	312.42	.43	.00	.00	.89	.29
517	116	120	262.60	.45	.00	.00	.74	.21
521	120	124	128.68	.05	.00	.00	.37	.06
525	120	128	115.72	.05	.00	.00	.33	.05
527	0	112	1747.40	.04	.00	.00	2.79	1.76
529	124	128	6.48	.00	.00	.00	.02	.00
533	100	132	279.85	1.73	.00	.00	1.79	1.73

Newland Sierra Water System Analysis – 1608/1475 Zone							
537 132 541 136 545 104 549 108 551 104 553 108 555 152 557 144 563 148 567 148 571 112 573 116 575 160 579 160 583 160 587 164 591 168 595 172 599 168	140 140 136 136 152 144 156 156 160 168 164 164 168 172 172 176	181.05 -82.25 164.82 -148.27 686.70 2168.50 176.80 2080.10 -29.80 -58.60 228.38 31.62 65.87 38.58 71.93 52.45 28.09 28.54 23.46	1.3141 .68 -1.01 13.66 2.13 .59 9.85080808 .24 .01 .08 .08 .20 .12 .01 .02 .03	.00 .00 .00 .00 .00 .00 .00 .00 .00 .00	.00 .00 .00 .00 .00 .00 .00 .00 .00 .00		
JUNCTION NUM 4 8	IBER	DEMAND 41.60 91.00	GRADE LINE 1620.43 1612.31	ELEVATION 1465.00 1495.00	PRESSURE 67.35 50.83		
10 12 16 20 24 28 32		36.40 91.00 91.00 93.60 93.60 33.80	1614.55 1604.66 1607.33 1605.01 1603.81 1599.98 1600.19	1420.00 1450.00 1425.00 1380.00 1400.00 1315.00	84.31 67.02 79.01 97.50 88.32 123.49 123.58		
36 40 100 104 108 112		.00 .00 18.20 18.20 18.20	1599.04 1594.22 1473.14 1470.37 1468.68 1474.96	1310.00 1275.00 1285.00 1270.00 1250.00 1265.00	125.25 138.33 81.53 86.83 94.76 90.98		
116 120 124 128 132 136		18.20 18.20 122.20 122.20 98.80 98.80	1474.54 1474.09 1474.04 1474.04 1471.41 1469.69	1300.00 1260.00 1185.00 1300.00 1230.00 1225.00	75.63 92.77 125.25 75.42 104.61 106.03		
140 144 148 152 156 160		98.80 88.40 88.40 2590.00 88.40 52.00	1470.10 1466.55 1456.04 1456.71 1456.12 1474.73	1175.00 1260.00 1275.00 1280.00 1290.00 1270.00	127.88 89.51 78.45 76.57 71.98 88.72		
164 168 172 176		52.00 52.00 52.00 52.00	1474.73 1474.65 1474.53 1474.53	1275.00 1300.00 1295.00 1300.00	86.52 75.63 77.79 75.62		
40 140 36		MAXIMUM P .00 98.80 .00	RESSURES 1594.22 1470.10 1599.04	1275.00 1175.00 1310.00	138.33 127.88 125.25		

August 3, 2016 File: 990001b1

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4.38 6.15

1.13 5.90 -.19

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1.46 .20

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122.20 .00	1474.04 1600.19	1185.00 1315.00	125.25 123.58
	RESSURES		
91.00	1612.31	1495.00	50.83
91.00	1604.66	1450.00	67.02
41.60	1620.43	1465.00	67.35
88.40	1456.12	1290.00	71.98
122.20	1474.04	1300.00	75.42
	.00 MINIMUM F 91.00 91.00 41.60 88.40	.00 1600.19 MINIMUM PRESSURES 91.00 1612.31 91.00 1604.66 41.60 1620.43 88.40 1456.12	.00 1600.19 1315.00 MINIMUM PRESSURES 91.00 1612.31 1495.00 91.00 1604.66 1450.00 41.60 1620.43 1465.00 88.40 1456.12 1290.00

THE NET SYSTEM DEMAND = 4337.20

SUMMARY OF INFLOWS(+) AND OUTFLOWS(-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 401 4337.20

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 4337.20
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

A SUMMARY OF CONDITIONS SPECIFIED FOR THE NEXT SIMULATION FOLLOWS

THE DEMANDS ARE CHANGED FROM ORIGINAL VALUES BY A FACTOR = 2.60

THE FOLLOWING SPECIFIC DEMAND CHANGES ARE MADE :

JUNCTION NUMBER

DEMAND

128

2620.00

THE RESULTS ARE OBTAINED AFTER 3 TRIALS WITH AN ACCURACY = .00115

MAXIMUM DAY DEMAND PLUS 2500 GPM FIRE FLOW AT NODE 128

PIPE	NO. N	ODE	NOS.	FLOWRATE	HEAD LOSS	PUMP HEAD	MINOR LOSS	VELOCITY	HL/1000
401		0	4	4333.40	7.55	.00	.00	6.91	9.44
403		4	8	509.27	8.12	.00	.00	3.25	5.24
405		8	12	418.27	7.64	.00	.00	2.67	3.64
409	1	6	12	3184.09	2.67	.00	.00	5.08	5.34
413		4	10	3782.53	5.87	.00	.00	6.04	7.34
415	1	0	16	3746.13	7.21	.00	.00	5.98	7.21
417	1	6	20	471.05	2.27	.00	.00	3.01	4.53
421	2	0	24	181.44	1.16	.00	.00	1.16	.77
425	2	4	28	283.85	3.72	.00	.00	1.81	1.77
429	2	0	24	196.01	1.16	.00	.00	1.25	.89
433	1	2	32	3511.35	4.48	.00	.00	5.60	6.40
437	3	2	28	251.76	.01	.00	.00	.71	.20
441	2	8	36	501.81	.07	.00	.00	1.42	.71
445	3	2	40	3259.59	18.95	.00	.00	5.20	5.57
501		0	100	501.81	.14	.00	.00	1.42	.71
503	10	0	104	341.44	.14	.00	.00	.97	.35
505	10	4	10.8	151.32	.07	.00	.00	.43	.08

				,			File	: 990001b1
509	108	112	-202.79	24	.00	.00	58	13
513	112	116	2010.80	13.41	.00	.00	5.70	9.25
517	116	120	2760.40	34.92	.00	.00	7.83	16.63
521	120	124	1223.68	3.32	.00	.00	3.47	3.69
525	120	128	1518.52	6.05	.00	.00	4.31	5.50
527	0	112	3259.59	.11	.00	.00	5.20	5.57
529	124	128	1101.48	2.73	.00	.00	3.12	3.03
533	100	132	142.17	.49	.00	.00	.91	.49
537	132	140	43.37	.09	.00	.00	.28	.05
541	136	140	55.43	.20	.00	.00	.35	.09
545	104	136	95.82	.25	.00	.00	.61	.24
549	108	136	58.41	.18	.00	.00	.37	.09
551	104	152	76.10	.23	.00	.00	.49	.15
553	108	144	277.50	.05	.00	.00	.79	.24
555	152	156	176.80	.59	.00	.00	1.13	.74
557	144	152	189.10	.12	.00	.00	.54	.12
563	148	156	-29.80	08	.00	.00	19	03
567	148	156	-58.60	08	.00	.00	37	10
571	112	160	1027.80	3.84	.00	.00	6.56	19.22
573	116	168	-767.80	-2.24	.00	.00	-4.90	-11.20
575	160	164	294.34	1.23	.00	.00	1.88	1.90
579	160	164	172.42	1.23	.00	.00	1.10	.70
583	160	168	509.04	7.32	.00	.00	3.25	5.23
587	164	172	414.76	5.73	.00	.00	2.65	3.58
591	168	172	-254.93	36	.00	.00	-1.63	-1.45
595	172	176	107.83	.24	.00	.00	.69	.30
599	168	176	-55.83	13	.00	.00	36	09

JUNCTION	NUMBER	DEMAND	GRADE LINE	ELEVATION	PRESSURE
4		41.60	1620.45	1465.00	67.36
8		91.00	1612.33	1495.00	50.84
10		36.40	1614.57	1420.00	84.32
12		91.00	1604.70	1450.00	67.03
16		91.00	1607.36	1425.00	79.02
20		93.60	1605.10	1380.00	97.54
24		93.60	1603.94	1400.00	88.37
28		33.80	1600.21	1315.00	123.59
32		.00	1600.22	1315.00	123.59
36		.00	1600.14	1310.00	125.73
40		.00	1581.27	1275.00	132.72
100		18.20	1474.86	1285.00	82.27
104		18.20	1474.72	1270.00	88.71
108		18.20	1474.65	1250.00	97.35
112		18.20	1474.89	1265.00	90.95
116		18.20	1461.48	1300.00	69.98
120		18.20	1426.57	1260.00	72.18
124		122.20	1423.25	1185.00	103.24
128		2620.00	1420.52	1300.00	52.22
132		98.80	1474.37	1230.00	105.89
136		98.80	1474.47	1225.00	108.10
140		98.80	1474.27	1175.00	129.68
144		88.40	1474.60	1260.00	92.99
148		88.40	1473.82	1275.00	86.16
152		88.40	1474.49	1280.00	84.28
156		88.40	1473.90	1290.00	79.69
160		52.00	1471.04	1270.00	87.12
164	36	52.00	1469.81	1275.00	84.42

168 172	52.00 52.00	1463.72 1464.08	1300.00 1295.00	70.95 73.27
176	52.00	1463.85	1300.00	71.00
	MAXIMUM	PRESSURES		
40	.00	1581.27	1275.00	132.72
140	98.80	1474.27	1175.00	129.68
36	.00	1600.14	1310.00	125.73
32	.00	1600.22	1315.00	123.59
28	33.80	1600.21	1315.00	123.59
	MINIMUM	PRESSURES		
8	91.00	1612.33	1495.00	50.84
128	2620.00	1420.52	1300.00	52.22
12	91.00	1604.70	1450.00	67.03
4	41.60	1620.45	1465.00	67.36
116	18.20	1461.48	1300.00	69.98

THE NET SYSTEM DEMAND = 4333.40

SUMMARY OF INFLOWS (+) AND OUTFLOWS (-) FROM FIXED GRADE NODES

PIPE NUMBER FLOWRATE 401 4333.40

THE NET FLOW INTO THE SYSTEM FROM FIXED GRADE NODES = 4333.40
THE NET FLOW OUT OF THE SYSTEM INTO FIXED GRADE NODES = .00

