

PREPARED BY:
HUNSAKER & ASSOCIATES
SAN DIEGO, CALIF.

PLANNING: 3707 Wadsworth Street
ENGINEERING: San Diego, CA 92121
SURVEYING: PH085658-4500 / PH085658-1414

EXHIBIT 4.1
PROPOSED CONDITION HEC-HMS HYDROLOGY MAP
OTAY RANCH VILLAGE14 & PA 16/19
COUNTY OF SAN DIEGO, CALIFORNIA

CHAPTER 5

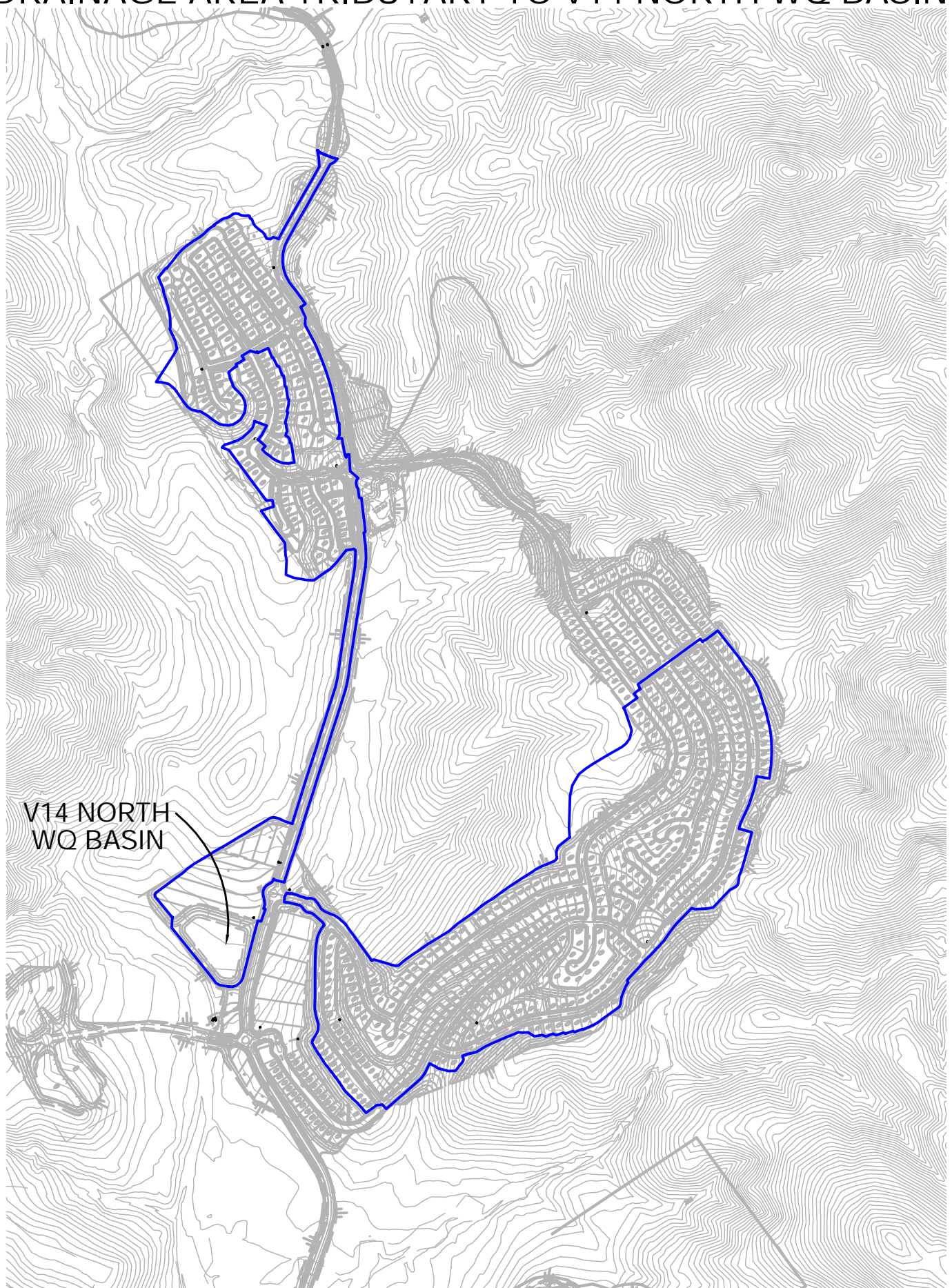
Rational Method Hydrologic Model for Proposed Condition

CHAPTER 5

5.1.1 – Rational Method Hydrologic Analysis (AES 2015)

Drainage Area Tributary to V14 North WQ Basin

OTAY RANCH VILLAGE 14 & PA 16/19 DRAINAGE AREA TRIBUTARY TO V14 NORTH WQ BASIN




```

*****>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
          2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1239

Analysis prepared by:

Hunsaker & Associates San Diego, Inc.
9707 Waples Street
San Diego CA 92121

=====
FILE NAME: R:\1235\HYD\CALCS\AES\SRP\V14N.DAT
TIME/DATE OF STUDY: 11:59 10/07/2016

=====
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
=====
2003 SAN DIEGO MANUAL CRITERIA

=====
USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.100
SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
=====
NO.    WIDTH  CROSSFALL  IN- / OUT-/PARK-  HEIGHT  WIDTH  LIP  HIKE  FACTOR
      (FT)    (FT)    SIDE / SIDE/ WAY  (FT)    (FT)  (FT)  (FT)  (n)
=====
1      16.0      8.0    0.020/0.020/0.020  0.50    2.00  0.0312  0.125  0.0150
2      12.0      6.0    0.020/0.020/0.020  0.50    1.50  0.0312  0.125  0.0130
=====
GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
   as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/SEC)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

=====
FLOW PROCESS FROM NODE    400.00 TO NODE    401.00 IS CODE = 21
=====
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 1000.05
DOWNSTREAM ELEVATION(FEET) = 999.35
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.59
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.59

=====
FLOW PROCESS FROM NODE    401.00 TO NODE    402.00 IS CODE = 61
=====
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 997.00 DOWNSTREAM ELEVATION(FEET) = 946.50
STREET LENGTH(FEET) = 709.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.71
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.27
HALFSTREET FLOOD WIDTH(FEET) = 7.33
AVERAGE FLOW VELOCITY(FT/SEC.) = 5.12
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.40
STREET FLOW TRAVEL TIME(MIN.) = 2.31 Tc(MIN.) = 11.04
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.899
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.78 SUBAREA RUNOFF(CFS) = 12.18
TOTAL AREA(ACRES) = 5.0 PEAK FLOW RATE(CFS) = 12.69

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.74
FLOW VELOCITY(FT/SEC.) = 5.95 DEPTH*VELOCITY(FT*FT/SEC.) = 1.91
LONGEST FLOWPATH FROM NODE    400.00 TO NODE    402.00 = 779.00 FEET.

=====
FLOW PROCESS FROM NODE    402.00 TO NODE    407.00 IS CODE = 31
=====
*****RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 941.50 DOWNSTREAM(FEET) = 916.00
FLOW LENGTH(FEET) = 628.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.2 INCHES
PIPE-FLOW VELOCITY(FT/SEC.) = 12.27
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.69
PIPE TRAVEL TIME(MIN.) = 0.85 Tc(MIN.) = 11.90
LONGEST FLOWPATH FROM NODE    400.00 TO NODE    407.00 = 1407.00 FEET.

=====
FLOW PROCESS FROM NODE    407.00 TO NODE    407.00 IS CODE = 1
=====
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.90
RAINFALL INTENSITY(INCH/HR) = 4.67
TOTAL STREAM AREA(ACRES) = 4.98
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.69

=====
FLOW PROCESS FROM NODE    405.00 TO NODE    406.00 IS CODE = 21
=====
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 950.15
DOWNSTREAM ELEVATION(FEET) = 949.45
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.50
TOTAL AREA(ACRES) = 0.17 TOTAL RUNOFF(CFS) = 0.50

=====
FLOW PROCESS FROM NODE    406.00 TO NODE    407.00 IS CODE = 61
=====
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 948.00 DOWNSTREAM ELEVATION(FEET) = 916.00
STREET LENGTH(FEET) = 620.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.61
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.26
HALFSTREET FLOOD WIDTH(FEET) = 6.57
AVERAGE FLOW VELOCITY(FT/SEC.) = 4.20
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.08
STREET FLOW TRAVEL TIME(MIN.) = 2.46 Tc(MIN.) = 11.20
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.856
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.24 SUBAREA RUNOFF(CFS) = 8.18
TOTAL AREA(ACRES) = 3.4 PEAK FLOW RATE(CFS) = 8.61

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.30 HALFSTREET FLOOD WIDTH(FEET) = 8.85
FLOW VELOCITY(FT/SEC.) = 4.77 DEPTH*VELOCITY(FT*FT/SEC.) = 1.45
LONGEST FLOWPATH FROM NODE    405.00 TO NODE    407.00 = 690.00 FEET.

=====
FLOW PROCESS FROM NODE    407.00 TO NODE    407.00 IS CODE = 1
=====
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.20
RAINFALL INTENSITY(INCH/HR) = 4.86
TOTAL STREAM AREA(ACRES) = 3.41
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.61

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER   (CFS)    (MIN.)  (INCH/HOUR)  (ACRE)
1         12.69    11.90      4.670      4.98
2          8.61    11.20      4.856      3.41

=====
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
=====

```



```

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           20.55      11.20      4.856
2           20.97      11.90      4.670

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 20.97      Tc(MIN.) = 11.90
TOTAL AREA(ACRES) = 8.4
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 407.00 = 1407.00 FEET.

*****
FLOW PROCESS FROM NODE 407.00 TO NODE 412.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 911.00 DOWNSTREAM(FEET) = 866.50
FLOW LENGTH(FEET) = 1241.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.22
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 20.97
PIPE TRAVEL TIME(MIN.) = 1.56 Tc(MIN.) = 13.46
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 412.00 = 2648.00 FEET.

*****
FLOW PROCESS FROM NODE 412.00 TO NODE 412.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.46
RAINFALL INTENSITY(INCH/HR) = 4.31
TOTAL STREAM AREA(ACRES) = 8.39
PEAK FLOW RATE(CFS) AT CONFLUENCE = 20.97

*****
FLOW PROCESS FROM NODE 410.00 TO NODE 411.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 915.85
DOWNSTREAM ELEVATION(FEET) = 915.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.50
TOTAL AREA(ACRES) = 0.17 TOTAL RUNOFF(CFS) = 0.50

*****
FLOW PROCESS FROM NODE 411.00 TO NODE 412.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(FEET) = 914.00 DOWNSTREAM ELEVATION(FEET) = 871.50
STREET LENGTH(FEET) = 1191.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.22
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.00
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.21
STREET FLOW TRAVEL TIME(MIN.) = 4.96 Tc(MIN.) = 13.70
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.264
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.98 SUBAREA RUNOFF(CFS) = 13.26
TOTAL AREA(ACRES) = 6.2 PEAK FLOW RATE(CFS) = 13.64

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.36 HALFSTREET FLOOD WIDTH(FEET) = 11.64
FLOW VELOCITY(FEET/SEC.) = 4.63 DEPTH*VELOCITY(FT*FT/SEC.) = 1.66
LONGEST FLOWPATH FROM NODE 410.00 TO NODE 412.00 = 1261.00 FEET.

*****
FLOW PROCESS FROM NODE 412.00 TO NODE 412.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.70
RAINFALL INTENSITY(INCH/HR) = 4.26
TOTAL STREAM AREA(ACRES) = 6.15
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.64

*****
FLOW PROCESS FROM NODE 415.00 TO NODE 416.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 879.00
DOWNSTREAM ELEVATION(FEET) = 877.50
ELEVATION DIFFERENCE(FEET) = 1.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.060
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.003
SUBAREA RUNOFF(CFS) = 0.27
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.27

*****
FLOW PROCESS FROM NODE 416.00 TO NODE 417.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 477.50 DOWNSTREAM(FEET) = 473.00
FLOW LENGTH(FEET) = 191.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.35
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.27
PIPE TRAVEL TIME(MIN.) = 0.95 Tc(MIN.) = 9.01
LONGEST FLOWPATH FROM NODE 415.00 TO NODE 417.00 = 261.00 FEET.

*****
FLOW PROCESS FROM NODE 416.00 TO NODE 417.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.586
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 1.09 SUBAREA RUNOFF(CFS) = 2.50
TOTAL AREA(ACRES) = 1.2 TOTAL RUNOFF(CFS) = 2.75
Tc(MIN.) = 9.01

*****
FLOW PROCESS FROM NODE 417.00 TO NODE 412.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 868.00 DOWNSTREAM(FEET) = 866.00
FLOW LENGTH(FEET) = 70.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.17
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.75
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 9.17
LONGEST FLOWPATH FROM NODE 415.00 TO NODE 412.00 = 331.00 FEET.

*****
FLOW PROCESS FROM NODE 412.00 TO NODE 412.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 9.17
RAINFALL INTENSITY(INCH/HR) = 5.52
TOTAL STREAM AREA(ACRES) = 1.20
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.75

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           20.97      13.46      4.312      8.39
2           13.64      13.70      4.264      6.15
3           2.75      9.17      5.522      1.20

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           28.25      9.17      5.522
2           36.52      13.46      4.312
3           36.49      13.70      4.264

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 36.52      Tc(MIN.) = 13.46

```



```

TOTAL AREA(ACRES) = 15.7
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 412.00 = 2648.00 FEET.
2 9.81 13.71 4.260 4.43

*****
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
*****
FLOW PROCESS FROM NODE 412.00 TO NODE 422.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 866.00 DOWNSTREAM(FEET) = 859.00
FLOW LENGTH(FEET) = 701.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 22.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.23
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 36.52
PIPE TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 14.73
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 422.00 = 3349.00 FEET.
*****
FLOW PROCESS FROM NODE 422.00 TO NODE 427.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 859.00 DOWNSTREAM(FEET) = 739.00
FLOW LENGTH(FEET) = 986.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 25.15
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 45.89
PIPE TRAVEL TIME(MIN.) = 0.65 Tc(MIN.) = 15.38
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 427.00 = 4335.00 FEET.
*****
FLOW PROCESS FROM NODE 420.00 TO NODE 421.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 883.95
DOWNSTREAM ELEVATION(FEET) = 883.25
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.59
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.59
*****
FLOW PROCESS FROM NODE 421.00 TO NODE 422.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 882.00 DOWNSTREAM ELEVATION(FEET) = 864.00
STREET LENGTH(FEET) = 895.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.34
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.79
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.00
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.91
STREET FLOW TRAVEL TIME(MIN.) = 4.98 Tc(MIN.) = 13.71
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.260
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.23 SUBAREA RUNOFF(CFS) = 9.37
TOTAL AREA(ACRES) = 4.4 PEAK FLOW RATE(CFS) = 9.81

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.43
FLOW VELOCITY(FEET/SEC.) = 3.45 DEPTH*VELOCITY(FT*FT/SEC.) = 1.22
LONGEST FLOWPATH FROM NODE 420.00 TO NODE 422.00 = 965.00 FEET.
*****
FLOW PROCESS FROM NODE 422.00 TO NODE 422.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.71
RAINFALL INTENSITY(INCH/HR) = 4.26
TOTAL STREAM AREA(ACRES) = 4.43
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.81

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 36.52 14.73 4.069 15.74

*****
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
*****
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 44.69 13.71 4.260
2 45.89 14.73 4.069

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 45.89 Tc(MIN.) = 14.73
TOTAL AREA(ACRES) = 20.2
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 422.00 = 3349.00 FEET.
*****
FLOW PROCESS FROM NODE 422.00 TO NODE 427.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 859.00 DOWNSTREAM(FEET) = 739.00
FLOW LENGTH(FEET) = 986.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 25.15
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 45.89
PIPE TRAVEL TIME(MIN.) = 0.65 Tc(MIN.) = 15.38
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 427.00 = 4335.00 FEET.
*****
FLOW PROCESS FROM NODE 427.00 TO NODE 427.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.38
RAINFALL INTENSITY(INCH/HR) = 3.96
TOTAL STREAM AREA(ACRES) = 20.17
PEAK FLOW RATE(CFS) AT CONFLUENCE = 45.89
*****
FLOW PROCESS FROM NODE 425.00 TO NODE 426.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 880.00
DOWNSTREAM ELEVATION(FEET) = 866.00
ELEVATION DIFFERENCE(FEET) = 14.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.055
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.89
TOTAL AREA(ACRES) = 0.21 TOTAL RUNOFF(CFS) = 0.89
*****
FLOW PROCESS FROM NODE 426.00 TO NODE 427.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 865.00 DOWNSTREAM ELEVATION(FEET) = 745.00
STREET LENGTH(FEET) = 983.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.57
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 7.97
AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.01
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 2.00
STREET FLOW TRAVEL TIME(MIN.) = 2.34 Tc(MIN.) = 6.39
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.970
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.30 SUBAREA RUNOFF(CFS) = 19.21
TOTAL AREA(ACRES) = 5.5 PEAK FLOW RATE(CFS) = 19.97

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.58
FLOW VELOCITY(FEET/SEC.) = 8.07 DEPTH*VELOCITY(FT*FT/SEC.) = 2.73
LONGEST FLOWPATH FROM NODE 425.00 TO NODE 427.00 = 1053.00 FEET.
*****
FLOW PROCESS FROM NODE 427.00 TO NODE 427.00 IS CODE = 1

```



```

-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 6.39
RAINFALL INTENSITY(INCH/HR) = 6.97
TOTAL STREAM AREA(ACRES) = 5.51
PEAK FLOW RATE(CFS) AT CONFLUENCE = 19.97

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 45.89 15.38 3.957 20.17
2 19.97 6.39 6.970 5.51

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 46.02 6.39 6.970
2 57.23 15.38 3.957

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 57.23 Tc(MIN.) = 15.38
TOTAL AREA(ACRES) = 25.7
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 427.00 = 4335.00 FEET.

*****
FLOW PROCESS FROM NODE 427.00 TO NODE 428.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 740.00 DOWNSTREAM(FEET) = 724.00
FLOW LENGTH(FEET) = 251.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 21.04
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 57.23
PIPE TRAVEL TIME(MIN.) = 0.20 Tc(MIN.) = 15.58
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 428.00 = 4586.00 FEET.

*****
FLOW PROCESS FROM NODE 428.00 TO NODE 428.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====
*****
FLOW PROCESS FROM NODE 440.00 TO NODE 441.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 989.00
DOWNSTREAM ELEVATION(FEET) = 976.00
ELEVATION DIFFERENCE(FEET) = 13.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.055
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.19
TOTAL AREA(ACRES) = 0.28 TOTAL RUNOFF(CFS) = 1.19

*****
FLOW PROCESS FROM NODE 441.00 TO NODE 442.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 976.00 DOWNSTREAM ELEVATION(FEET) = 931.50
STREET LENGTH(FEET) = 611.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.29
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.31
HALFSTREET FLOOD WIDTH(FEET) = 9.25
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.80
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.80
STREET FLOW TRAVEL TIME(MIN.) = 1.76 Tc(MIN.) = 5.81
100 YEAR RAINFALL INTENSITY(INCH/HR) = 7.413
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.22 SUBAREA RUNOFF(CFS) = 20.12

TOTAL AREA(ACRES) = 5.5 PEAK FLOW RATE(CFS) = 21.20

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.37 HALFSTREET FLOOD WIDTH(FEET) = 12.06
FLOW VELOCITY(FEET/SEC.) = 6.74 DEPTH*VELOCITY(FT*FT/SEC.) = 2.48
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 442.00 = 681.00 FEET.

*****
FLOW PROCESS FROM NODE 442.00 TO NODE 447.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 926.00 DOWNSTREAM(FEET) = 883.00
FLOW LENGTH(FEET) = 612.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.99
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 21.20
PIPE TRAVEL TIME(MIN.) = 0.60 Tc(MIN.) = 6.41
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 447.00 = 1293.00 FEET.

*****
FLOW PROCESS FROM NODE 447.00 TO NODE 447.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 6.41
RAINFALL INTENSITY(INCH/HR) = 6.96
TOTAL STREAM AREA(ACRES) = 5.50
PEAK FLOW RATE(CFS) AT CONFLUENCE = 21.20

*****
FLOW PROCESS FROM NODE 445.00 TO NODE 446.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 931.25
DOWNSTREAM ELEVATION(FEET) = 930.55
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.71
TOTAL AREA(ACRES) = 0.24 TOTAL RUNOFF(CFS) = 0.71

*****
FLOW PROCESS FROM NODE 446.00 TO NODE 447.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 930.00 DOWNSTREAM ELEVATION(FEET) = 888.50
STREET LENGTH(FEET) = 615.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.54
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.27
HALFSTREET FLOOD WIDTH(FEET) = 7.33
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.99
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.36
STREET FLOW TRAVEL TIME(MIN.) = 2.06 Tc(MIN.) = 10.79
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.973
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.49 SUBAREA RUNOFF(CFS) = 11.61
TOTAL AREA(ACRES) = 4.7 PEAK FLOW RATE(CFS) = 12.23

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.74
FLOW VELOCITY(FEET/SEC.) = 5.73 DEPTH*VELOCITY(FT*FT/SEC.) = 1.84
LONGEST FLOWPATH FROM NODE 445.00 TO NODE 447.00 = 685.00 FEET.

*****
FLOW PROCESS FROM NODE 447.00 TO NODE 447.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.79
RAINFALL INTENSITY(INCH/HR) = 4.97
TOTAL STREAM AREA(ACRES) = 4.73
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.23

```



```

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            21.20      6.41      6.958      5.50
2            12.23      10.79     4.973      4.73

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            28.47      6.41      6.958
2            27.39      10.79     4.973

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 28.47 Tc(MIN.) = 6.41
TOTAL AREA(ACRES) = 10.2
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 447.00 = 1293.00 FEET.

LONGEST FLOWPATH FROM NODE 450.00 TO NODE 452.00 = 784.00 FEET.

*****
FLOW PROCESS FROM NODE 447.00 TO NODE 452.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 883.00 DOWNSTREAM(FEET) = 877.50
FLOW LENGTH(FEET) = 507.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.96
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 28.47
PIPE TRAVEL TIME(MIN.) = 0.94 Tc(MIN.) = 7.35
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 452.00 = 1800.00 FEET.

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            28.47      7.35      6.368      10.23
2            9.94      9.50      5.397      3.54

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            36.16      7.35      6.368
2            34.07      9.50      5.397

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 36.16 Tc(MIN.) = 7.35
TOTAL AREA(ACRES) = 13.8
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 452.00 = 1800.00 FEET.

*****
FLOW PROCESS FROM NODE 452.00 TO NODE 452.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 877.50 DOWNSTREAM(FEET) = 875.00
FLOW LENGTH(FEET) = 253.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 22.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.17
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 36.16
PIPE TRAVEL TIME(MIN.) = 0.46 Tc(MIN.) = 7.81
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 457.00 = 2053.00 FEET.

*****
FLOW PROCESS FROM NODE 457.00 TO NODE 457.00 IS CODE = 1
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 910.00
DOWNSTREAM ELEVATION(FEET) = 900.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.846
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.70
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 1.70

*****
FLOW PROCESS FROM NODE 451.00 TO NODE 452.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(FEET) = 890.00 DOWNSTREAM ELEVATION(FEET) = 882.50
STREET LENGTH(FEET) = 684.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.21
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.34
HALFSTREET FLOOD WIDTH(FEET) = 10.72
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.45
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.83
STREET FLOW TRAVEL TIME(MIN.) = 4.66 Tc(MIN.) = 9.50
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.397
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.14 SUBAREA RUNOFF(CFS) = 8.81
TOTAL AREA(ACRES) = 3.5 PEAK FLOW RATE(CFS) = 9.94

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.39 HALFSTREET FLOOD WIDTH(FEET) = 13.04
FLOW VELOCITY(FEET/SEC.) = 2.73 DEPTH*VELOCITY(FT*FT/SEC.) = 1.06

*****
FLOW PROCESS FROM NODE 456.00 TO NODE 457.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(FEET) = 885.00 DOWNSTREAM ELEVATION(FEET) = 880.00
STREET LENGTH(FEET) = 409.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.74
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.33

```



```

HALFSTREET FLOOD WIDTH(FEET) = 10.16
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.50
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.82
STREET FLOW TRAVEL TIME(MIN.) = 2.73 Tc(MIN.) = 7.58
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.247
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 2.59 SUBAREA RUNOFF(CFS) = 8.41
TOTAL AREA(ACRES) = 2.9 PEAK FLOW RATE(CFS) = 9.55

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.38 HALFSTREET FLOOD WIDTH(FEET) = 12.55
FLOW VELOCITY(FEET/SEC.) = 2.82 DEPTH*VELOCITY(FT*FT/SEC.) = 1.06
LONGEST FLOWPATH FROM NODE 450.00 TO NODE 457.00 = 509.00 FEET.

*****
FLOW PROCESS FROM NODE 457.00 TO NODE 457.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.58
RAINFALL INTENSITY(INCH/HR) = 6.25
TOTAL STREAM AREA(ACRES) = 2.94
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.55

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 36.16 7.81 6.124 13.77
2 9.55 7.58 6.247 2.94

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 45.00 7.58 6.247
2 45.52 7.81 6.124

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 45.52 Tc(MIN.) = 7.81
TOTAL AREA(ACRES) = 16.7
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 457.00 = 2053.00 FEET.

*****
FLOW PROCESS FROM NODE 457.00 TO NODE 462.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 875.00 DOWNSTREAM(FEET) = 871.50
FLOW LENGTH(FEET) = 294.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.54
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 45.52
PIPE TRAVEL TIME(MIN.) = 0.46 Tc(MIN.) = 8.28
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 462.00 = 2347.00 FEET.

*****
FLOW PROCESS FROM NODE 462.00 TO NODE 462.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 8.28
RAINFALL INTENSITY(INCH/HR) = 5.90
TOTAL STREAM AREA(ACRES) = 16.71
PEAK FLOW RATE(CFS) AT CONFLUENCE = 45.52

*****
FLOW PROCESS FROM NODE 460.00 TO NODE 461.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 900.00
DOWNSTREAM ELEVATION(FEET) = 890.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.846
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.19
TOTAL AREA(ACRES) = 0.28 TOTAL RUNOFF(CFS) = 1.19

*****
FLOW PROCESS FROM NODE 461.00 TO NODE 462.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 888.00 DOWNSTREAM ELEVATION(FEET) = 876.50

STREET LENGTH(FEET) = 961.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.25
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.36
HALFSTREET FLOOD WIDTH(FEET) = 11.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.71
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.98
STREET FLOW TRAVEL TIME(MIN.) = 5.91 Tc(MIN.) = 10.75
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.984
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.28 SUBAREA RUNOFF(CFS) = 13.69
TOTAL AREA(ACRES) = 5.6 PEAK FLOW RATE(CFS) = 14.41

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.42 HALFSTREET FLOOD WIDTH(FEET) = 14.87
FLOW VELOCITY(FEET/SEC.) = 3.09 DEPTH*VELOCITY(FT*FT/SEC.) = 1.31
LONGEST FLOWPATH FROM NODE 460.00 TO NODE 462.00 = 1061.00 FEET.

*****
FLOW PROCESS FROM NODE 462.00 TO NODE 462.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.75
RAINFALL INTENSITY(INCH/HR) = 4.98
TOTAL STREAM AREA(ACRES) = 5.56
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.41

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 45.52 8.28 5.900 16.71
2 14.41 10.75 4.984 5.56

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 56.62 8.28 5.900
2 52.87 10.75 4.984

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 56.62 Tc(MIN.) = 8.28
TOTAL AREA(ACRES) = 22.3
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 462.00 = 2347.00 FEET.

*****
FLOW PROCESS FROM NODE 462.00 TO NODE 467.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 872.00 DOWNSTREAM(FEET) = 871.00
FLOW LENGTH(FEET) = 382.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 45.0 INCH PIPE IS 34.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.21
ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 56.62
PIPE TRAVEL TIME(MIN.) = 1.03 Tc(MIN.) = 9.30
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 467.00 = 2729.00 FEET.

*****
FLOW PROCESS FROM NODE 467.00 TO NODE 467.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 9.30
RAINFALL INTENSITY(INCH/HR) = 5.47
TOTAL STREAM AREA(ACRES) = 22.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 56.62

*****
FLOW PROCESS FROM NODE 465.00 TO NODE 466.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 897.35
DOWNSTREAM ELEVATION(FEET) = 896.65

```



```

ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
         THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
         (Reference: Table 3-1B of Hydrology Manual)
         THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.74
TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.74

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 9.80
RAINFALL INTENSITY(INCH/HR) = 5.29
TOTAL STREAM AREA(ACRES) = 25.46
PEAK FLOW RATE(CFS) AT CONFLUENCE = 62.42

*****
FLOW PROCESS FROM NODE 470.00 TO NODE 471.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
FLOW PROCESS FROM NODE 466.00 TO NODE 467.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
-----
UPSTREAM ELEVATION(FEET) = 896.00 DOWNSTREAM ELEVATION(FEET) = 888.00
STREET LENGTH(FEET) = 530.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.28
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.50
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.54
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.75
STREET FLOW TRAVEL TIME(MIN.) = 3.47 Tc(MIN.) = 12.21
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.593
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 2.94 SUBAREA RUNOFF(CFS) = 7.02
TOTAL AREA(ACRES) = 3.2 PEAK FLOW RATE(CFS) = 7.62

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.93
FLOW VELOCITY(FEET/SEC.) = 2.90 DEPTH*VELOCITY(FT*FT/SEC.) = 1.00
LONGEST FLOWPATH FROM NODE 465.00 TO NODE 467.00 = 600.00 FEET.

*****
FLOW PROCESS FROM NODE 467.00 TO NODE 467.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.21
RAINFALL INTENSITY(INCH/HR) = 4.59
TOTAL STREAM AREA(ACRES) = 3.19
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.62

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 56.62 9.30 5.472 22.27
2 7.62 12.21 4.593 3.19

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 62.42 9.30 5.472
2 55.14 12.21 4.593

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 62.42 Tc(MIN.) = 9.30
TOTAL AREA(ACRES) = 25.5
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 467.00 = 2729.00 FEET.

*****
FLOW PROCESS FROM NODE 467.00 TO NODE 472.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 883.00 DOWNSTREAM(FEET) = 802.40
FLOW LENGTH(FEET) = 765.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 25.67
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 62.42
PIPE TRAVEL TIME(MIN.) = 0.50 Tc(MIN.) = 9.80
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 472.00 = 3494.00 FEET.

*****
FLOW PROCESS FROM NODE 472.00 TO NODE 472.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.04
RAINFALL INTENSITY(INCH/HR) = 4.63
TOTAL STREAM AREA(ACRES) = 6.53
PEAK FLOW RATE(CFS) AT CONFLUENCE = 15.73

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 62.42 9.80 5.291 25.46
2 15.73 12.04 4.634 6.53

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 75.23 9.80 5.291
2 70.40 12.04 4.634

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 75.23 Tc(MIN.) = 9.80
TOTAL AREA(ACRES) = 32.0
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 472.00 = 3494.00 FEET.

*****
FLOW PROCESS FROM NODE 472.00 TO NODE 476.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 801.00 DOWNSTREAM(FEET) = 747.00
FLOW LENGTH(FEET) = 394.00 MANNING'S N = 0.013

```



```

DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 29.48
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 75.23
PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 10.02
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 476.00 = 3888.00 FEET.

*****
FLOW PROCESS FROM NODE 476.00 TO NODE 476.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.02
RAINFALL INTENSITY(INCH/HR) = 5.22
TOTAL STREAM AREA(ACRES) = 31.99
PEAK FLOW RATE(CFS) AT CONFLUENCE = 75.23

*****
FLOW PROCESS FROM NODE 473.00 TO NODE 474.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 890.00
DOWNSTREAM ELEVATION(FEET) = 880.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.765
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION#
100 YEAR RAINFALL INTENSITY(INCH/HR) = 7.451
SUBAREA RUNOFF(CFS) = 0.64
TOTAL AREA(ACRES) = 0.21 TOTAL RUNOFF(CFS) = 0.64

*****
FLOW PROCESS FROM NODE 474.00 TO NODE 475.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 890.00 DOWNSTREAM(FEET) = 770.00
FLOW LENGTH(FEET) = 770.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.45
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.64
PIPE TRAVEL TIME(MIN.) = 1.52 Tc(MIN.) = 7.28
LONGEST FLOWPATH FROM NODE 473.00 TO NODE 475.00 = 870.00 FEET.

*****
FLOW PROCESS FROM NODE 474.00 TO NODE 475.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HR) = 6.408
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 2.30 SUBAREA RUNOFF(CFS) = 6.04
TOTAL AREA(ACRES) = 2.5 TOTAL RUNOFF(CFS) = 6.59
TC(MIN.) = 7.28

*****
FLOW PROCESS FROM NODE 475.00 TO NODE 476.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 765.00 DOWNSTREAM(FEET) = 751.00
FLOW LENGTH(FEET) = 163.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.62
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.59
PIPE TRAVEL TIME(MIN.) = 0.20 Tc(MIN.) = 7.48
LONGEST FLOWPATH FROM NODE 473.00 TO NODE 476.00 = 1033.00 FEET.

*****
FLOW PROCESS FROM NODE 476.00 TO NODE 476.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.48
RAINFALL INTENSITY(INCH/HR) = 6.30
TOTAL STREAM AREA(ACRES) = 2.51
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.59

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 75.23 10.02 5.215 31.99
2 6.59 7.48 6.297 2.51

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 68.90 7.48 6.297
2 80.69 10.02 5.215

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 80.69 Tc(MIN.) = 10.02
TOTAL AREA(ACRES) = 34.5
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 476.00 = 3888.00 FEET.

*****
FLOW PROCESS FROM NODE 476.00 TO NODE 479.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 747.00 DOWNSTREAM(FEET) = 735.00
FLOW LENGTH(FEET) = 304.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.10
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 80.69
PIPE TRAVEL TIME(MIN.) = 0.27 Tc(MIN.) = 10.29
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 479.00 = 4192.00 FEET.

*****
FLOW PROCESS FROM NODE 479.00 TO NODE 479.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.29
RAINFALL INTENSITY(INCH/HR) = 5.13
TOTAL STREAM AREA(ACRES) = 34.50
PEAK FLOW RATE(CFS) AT CONFLUENCE = 80.69

*****
FLOW PROCESS FROM NODE 477.00 TO NODE 478.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 803.45
DOWNSTREAM ELEVATION(FEET) = 802.75
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.68
TOTAL AREA(ACRES) = 0.23 TOTAL RUNOFF(CFS) = 0.68

*****
FLOW PROCESS FROM NODE 478.00 TO NODE 479.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(FEET) = 803.00 DOWNSTREAM ELEVATION(FEET) = 730.00
STREET LENGTH(FEET) = 639.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.83
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.27
HALFSTREET FLOOD WIDTH(FEET) = 7.04
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.39
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.70
STREET FLOW TRAVEL TIME(MIN.) = 1.67 Tc(MIN.) = 10.40
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.092
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.39 SUBAREA RUNOFF(CFS) = 14.27
TOTAL AREA(ACRES) = 5.6 PEAK FLOW RATE(CFS) = 14.88

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.46
FLOW VELOCITY(FEET/SEC.) = 7.35 DEPTH*VELOCITY(FT*FT/SEC.) = 2.32
LONGEST FLOWPATH FROM NODE 477.00 TO NODE 479.00 = 709.00 FEET.

*****
FLOW PROCESS FROM NODE 479.00 TO NODE 479.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 2

```



```

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.40
RAINFALL INTENSITY(INCH/HR) = 5.09
TOTAL STREAM AREA(ACRES) = 5.62
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.88

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            80.69      10.29      5.128          34.50
2            14.88      10.40      5.092          5.62

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            95.41      10.29      5.128
2            95.01      10.40      5.092

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 95.41 Tc(MIN.) = 10.29
TOTAL AREA(ACRES) = 40.1
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 479.00 = 4192.00 FEET.

*****
FLOW PROCESS FROM NODE 479.00 TO NODE 428.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 725.00 DOWNSTREAM(FEET) = 724.00
FLOW LENGTH(FEET) = 60.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 29.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.21
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 95.41
PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 10.36
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 428.00 = 4252.00 FEET.

*****
FLOW PROCESS FROM NODE 428.00 TO NODE 428.00 IS CODE = 11
*****
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
*****
** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            95.41      10.36      5.105          40.12
LONGEST FLOWPATH FROM NODE 440.00 TO NODE 428.00 = 4252.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            57.23      15.58      3.924          25.68
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 428.00 = 4586.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            133.47      10.36      5.105
2            130.56      15.58      3.924

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 133.47 Tc(MIN.) = 10.36
TOTAL AREA(ACRES) = 65.8

*****
FLOW PROCESS FROM NODE 428.00 TO NODE 428.00 IS CODE = 12
*****
>>>>CLEAR MEMORY BANK # 1 <<<<
*****
*****
FLOW PROCESS FROM NODE 428.00 TO NODE 482.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 724.00 DOWNSTREAM(FEET) = 707.00
FLOW LENGTH(FEET) = 547.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 30.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.45
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 133.47
PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 10.83
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 482.00 = 5133.00 FEET.

*****
FLOW PROCESS FROM NODE 482.00 TO NODE 482.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.83
RAINFALL INTENSITY(INCH/HR) = 4.96
TOTAL STREAM AREA(ACRES) = 65.80
PEAK FLOW RATE(CFS) AT CONFLUENCE = 133.47

*****
FLOW PROCESS FROM NODE 480.00 TO NODE 481.00 IS CODE = 21
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 738.35
DOWNSTREAM ELEVATION(FEET) = 737.65
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.65
TOTAL AREA(ACRES) = 0.22 TOTAL RUNOFF(CFS) = 0.65

*****
FLOW PROCESS FROM NODE 481.00 TO NODE 482.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(FEET) = 737.50 DOWNSTREAM ELEVATION(FEET) = 712.00
STREET LENGTH(FEET) = 707.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.98
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.45
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.70
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.02
STREET FLOW TRAVEL TIME(MIN.) = 3.18 Tc(MIN.) = 11.92
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.664
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.55 SUBAREA RUNOFF(CFS) = 8.61
TOTAL AREA(ACRES) = 3.8 PEAK FLOW RATE(CFS) = 9.14

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.81
FLOW VELOCITY(FEET/SEC.) = 4.23 DEPTH*VELOCITY(FT*FT/SEC.) = 1.36
LONGEST FLOWPATH FROM NODE 480.00 TO NODE 482.00 = 777.00 FEET.

*****
FLOW PROCESS FROM NODE 482.00 TO NODE 482.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.92
RAINFALL INTENSITY(INCH/HR) = 4.66
TOTAL STREAM AREA(ACRES) = 3.77
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.14

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            133.47      10.83      4.962          65.80
2            9.14      11.92      4.664          3.77

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            141.77      10.83      4.962
2            134.61      11.92      4.664

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 141.77 Tc(MIN.) = 10.83
TOTAL AREA(ACRES) = 69.6
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 482.00 = 5133.00 FEET.

*****
FLOW PROCESS FROM NODE 482.00 TO NODE 487.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 706.00 DOWNSTREAM(FEET) = 703.00
FLOW LENGTH(FEET) = 100.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.76
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1

```



```

PIPE-FLOW(CFS) = 141.77
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 10.91
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 487.00 = 5233.00 FEET.
*****
FLOW PROCESS FROM NODE 487.00 TO NODE 492.00 IS CODE = 31
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.91
RAINFALL INTENSITY(INCH/HR) = 4.94
TOTAL STREAM AREA(ACRES) = 69.57
PEAK FLOW RATE(CFS) AT CONFLUENCE = 141.77
*****
FLOW PROCESS FROM NODE 485.00 TO NODE 486.00 IS CODE = 21
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 70.00
UPSTREAM ELEVATION(Feet) = 713.75
DOWNSTREAM ELEVATION(Feet) = 713.05
ELEVATION DIFFERENCE(Feet) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.44
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 0.44
*****
FLOW PROCESS FROM NODE 486.00 TO NODE 487.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(Feet) = 713.00 DOWNSTREAM ELEVATION(Feet) = 708.00
STREET LENGTH(Feet) = 257.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(Feet) = 18.00
*****
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(Feet) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
*****
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
*****
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.60
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(Feet) = 0.25
HALFSTREET FLOOD WIDTH(Feet) = 6.28
AVERAGE FLOW VELOCITY(Feet/Sec.) = 2.54
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.64
STREET FLOW TRAVEL TIME(MIN.) = 1.69 Tc(MIN.) = 10.42
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.086
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 1.63 SUBAREA RUNOFF(CFS) = 4.31
TOTAL AREA(ACRES) = 1.8 PEAK FLOW RATE(CFS) = 4.71
*****
END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(Feet) = 0.29 HALFSTREET FLOOD WIDTH(Feet) = 8.38
FLOW VELOCITY(Feet/Sec.) = 2.87 DEPTH*VELOCITY(FT*FT/SEC.) = 0.84
LONGEST FLOWPATH FROM NODE 485.00 TO NODE 487.00 = 327.00 FEET.
*****
FLOW PROCESS FROM NODE 487.00 TO NODE 487.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.42
RAINFALL INTENSITY(INCH/HR) = 5.09
TOTAL STREAM AREA(ACRES) = 1.78
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.71
*****
** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 141.77 10.91 4.937 69.57
2 4.71 10.42 5.086 1.78
*****
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
*****
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 142.32 10.42 5.086
2 146.34 10.91 4.937
*****
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 146.34 Tc(MIN.) = 10.91
TOTAL AREA(ACRES) = 71.3
*****
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 487.00 = 5233.00 FEET.
*****
FLOW PROCESS FROM NODE 487.00 TO NODE 492.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(Feet) = 703.00 DOWNSTREAM(Feet) = 695.00
FLOW LENGTH(Feet) = 676.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 37.8 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 13.80
ESTIMATED PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 146.34
PIPE TRAVEL TIME(MIN.) = 0.82 Tc(MIN.) = 11.73
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 492.00 = 5909.00 FEET.
*****
FLOW PROCESS FROM NODE 492.00 TO NODE 492.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.73
RAINFALL INTENSITY(INCH/HR) = 4.71
TOTAL STREAM AREA(ACRES) = 71.35
PEAK FLOW RATE(CFS) AT CONFLUENCE = 146.34
*****
FLOW PROCESS FROM NODE 490.00 TO NODE 491.00 IS CODE = 21
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 70.00
UPSTREAM ELEVATION(Feet) = 714.05
DOWNSTREAM ELEVATION(Feet) = 713.35
ELEVATION DIFFERENCE(Feet) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.39
TOTAL AREA(ACRES) = 0.13 TOTAL RUNOFF(CFS) = 0.39
*****
FLOW PROCESS FROM NODE 491.00 TO NODE 492.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(Feet) = 712.50 DOWNSTREAM ELEVATION(Feet) = 706.00
STREET LENGTH(Feet) = 348.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(Feet) = 18.00
*****
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(Feet) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
*****
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
*****
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.32
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(Feet) = 0.27
HALFSTREET FLOOD WIDTH(Feet) = 7.21
AVERAGE FLOW VELOCITY(Feet/Sec.) = 2.60
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.70
STREET FLOW TRAVEL TIME(MIN.) = 2.23 Tc(MIN.) = 10.97
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.921
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 2.28 SUBAREA RUNOFF(CFS) = 5.83
TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 6.17
*****
END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(Feet) = 0.32 HALFSTREET FLOOD WIDTH(Feet) = 9.53
FLOW VELOCITY(Feet/Sec.) = 3.01 DEPTH*VELOCITY(FT*FT/SEC.) = 0.95
LONGEST FLOWPATH FROM NODE 490.00 TO NODE 492.00 = 418.00 FEET.
*****
FLOW PROCESS FROM NODE 492.00 TO NODE 492.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.97
RAINFALL INTENSITY(INCH/HR) = 4.92
TOTAL STREAM AREA(ACRES) = 2.41
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.17
*****
FLOW PROCESS FROM NODE 483.00 TO NODE 483.10 IS CODE = 21
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):

```



```

RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 740.00
DOWNSTREAM ELEVATION(FEET) = 730.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.765
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION+
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.451
SUBAREA RUNOFF(CFS) = 0.27
TOTAL AREA(ACRES) = 0.09 TOTAL RUNOFF(CFS) = 0.27

*****
FLOW PROCESS FROM NODE 483.10 TO NODE 484.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 740.00 DOWNSTREAM(FEET) = 720.00
FLOW LENGTH(FEET) = 602.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.78
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.27
PIPE TRAVEL TIME(MIN.) = 2.65 Tc(MIN.) = 8.42
LONGEST FLOWPATH FROM NODE 483.00 TO NODE 484.00 = 702.00 FEET.

*****
FLOW PROCESS FROM NODE 483.10 TO NODE 484.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.836
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 1.86 SUBAREA RUNOFF(CFS) = 4.45
TOTAL AREA(ACRES) = 2.0 TOTAL RUNOFF(CFS) = 4.67
TC(MIN.) = 8.42

*****
FLOW PROCESS FROM NODE 484.00 TO NODE 492.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 705.00 DOWNSTREAM(FEET) = 700.00
FLOW LENGTH(FEET) = 140.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.02
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.67
PIPE TRAVEL TIME(MIN.) = 0.26 Tc(MIN.) = 8.68
LONGEST FLOWPATH FROM NODE 483.00 TO NODE 492.00 = 842.00 FEET.

*****
FLOW PROCESS FROM NODE 492.00 TO NODE 492.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 8.68
RAINFALL INTENSITY(INCH/HR) = 5.72
TOTAL STREAM AREA(ACRES) = 1.95
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.67

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 146.34 11.73 4.713 71.35
2 6.17 10.97 4.921 2.41
3 4.67 8.68 5.723 1.95

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 130.05 8.68 5.723
2 150.32 10.97 4.921
3 156.09 11.73 4.713

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 156.09 Tc(MIN.) = 11.73
TOTAL AREA(ACRES) = 75.7
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 492.00 = 5909.00 FEET.

*****
FLOW PROCESS FROM NODE 492.00 TO NODE 497.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 700.50 DOWNSTREAM(FEET) = 692.50
FLOW LENGTH(FEET) = 789.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 39.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.28

ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 156.09
PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 12.72
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 497.00 = 6698.00 FEET.

*****
FLOW PROCESS FROM NODE 497.00 TO NODE 497.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.72
RAINFALL INTENSITY(INCH/HR) = 4.47
TOTAL STREAM AREA(ACRES) = 75.71
PEAK FLOW RATE(CFS) AT CONFLUENCE = 156.09

*****
FLOW PROCESS FROM NODE 495.00 TO NODE 496.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 706.85
DOWNSTREAM ELEVATION(FEET) = 706.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.71
TOTAL AREA(ACRES) = 0.24 TOTAL RUNOFF(CFS) = 0.71

*****
FLOW PROCESS FROM NODE 496.00 TO NODE 497.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 705.00 DOWNSTREAM ELEVATION(FEET) = 697.00
STREET LENGTH(FEET) = 750.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.67
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.62
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.13
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.64
STREET FLOW TRAVEL TIME(MIN.) = 5.86 Tc(MIN.) = 14.60
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.093
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 2.74 SUBAREA RUNOFF(CFS) = 5.83
TOTAL AREA(ACRES) = 3.0 PEAK FLOW RATE(CFS) = 6.34

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.86
FLOW VELOCITY(FEET/SEC.) = 2.44 DEPTH*VELOCITY(FT*FT/SEC.) = 0.84
LONGEST FLOWPATH FROM NODE 495.00 TO NODE 497.00 = 820.00 FEET.

*****
FLOW PROCESS FROM NODE 497.00 TO NODE 497.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.60
RAINFALL INTENSITY(INCH/HR) = 4.09
TOTAL STREAM AREA(ACRES) = 2.98
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.34

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 156.09 12.72 4.473 75.71
2 6.34 14.60 4.093 2.98

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 161.62 12.72 4.473

```



```

2      149.17    14.60      4.093

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      161.62    Tc(MIN.) =      12.72
TOTAL AREA(ACRES) =      78.7
LONGEST FLOWPATH FROM NODE      400.00 TO NODE      497.00 =      6698.00 FEET.

*****
FLOW PROCESS FROM NODE      497.00 TO NODE      498.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) =      692.50  DOWNSTREAM(FEET) =      691.50
FLOW LENGTH(FEET) =      50.00  MANNING'S N =      0.013
DEPTH OF FLOW IN 45.0 INCH PIPE IS 35.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      17.20
ESTIMATED PIPE DIAMETER(INCH) =      45.00  NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      161.62
PIPE TRAVEL TIME(MIN.) =      0.05    Tc(MIN.) =      12.77
LONGEST FLOWPATH FROM NODE      400.00 TO NODE      498.00 =      6748.00 FEET.

*****
FLOW PROCESS FROM NODE      498.00 TO NODE      498.00 IS CODE =      10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
-----
*****
FLOW PROCESS FROM NODE      500.00 TO NODE      501.00 IS CODE =      21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) =      0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      70.00
UPSTREAM ELEVATION(FEET) =      991.45
DOWNSTREAM ELEVATION(FEET) =      990.75
ELEVATION DIFFERENCE(FEET) =      0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      5.699
SUBAREA RUNOFF(CFS) =      0.68
TOTAL AREA(ACRES) =      0.23  TOTAL RUNOFF(CFS) =      0.68

*****
FLOW PROCESS FROM NODE      501.00 TO NODE      502.00 IS CODE =      61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(FEET) =      990.00  DOWNSTREAM ELEVATION(FEET) =      932.50
STREET LENGTH(FEET) =      616.00  CURB HEIGHT(INCHES) =      6.0
STREET HALFWIDTH(FEET) =      18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =      9.00
INSIDE STREET CROSSFALL(DECIMAL) =      0.020
OUTSIDE STREET CROSSFALL(DECIMAL) =      0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =      2
STREET PARKWAY CROSSFALL(DECIMAL) =      0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) =      0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section =      0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      5.33
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) =      0.25
HALFSTREET FLOOD WIDTH(FEET) =      6.10
AVERAGE FLOW VELOCITY(FEET/SEC.) =      5.44
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =      1.35
STREET FLOW TRAVEL TIME(MIN.) =      1.89    Tc(MIN.) =      10.62
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      5.023
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) =      0
AREA-AVERAGE RUNOFF COEFFICIENT =      0.520
SUBAREA AREA(ACRES) =      3.55  SUBAREA RUNOFF(CFS) =      9.27
TOTAL AREA(ACRES) =      3.8    PEAK FLOW RATE(CFS) =      9.87

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) =      0.29  HALFSTREET FLOOD WIDTH(FEET) =      8.21
FLOW VELOCITY(FEET/SEC.) =      6.23  DEPTH*VELOCITY(FT*FT/SEC.) =      1.81
LONGEST FLOWPATH FROM NODE      500.00 TO NODE      502.00 =      686.00 FEET.

*****
FLOW PROCESS FROM NODE      502.00 TO NODE      506.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) =      928.00  DOWNSTREAM(FEET) =      902.50
FLOW LENGTH(FEET) =      618.00  MANNING'S N =      0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      11.62
ESTIMATED PIPE DIAMETER(INCH) =      18.00  NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      9.87
PIPE TRAVEL TIME(MIN.) =      0.89    Tc(MIN.) =      11.51
LONGEST FLOWPATH FROM NODE      500.00 TO NODE      506.00 =      1304.00 FEET.

*****
FLOW PROCESS FROM NODE      506.00 TO NODE      506.00 IS CODE =      1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =      11.51
RAINFALL INTENSITY(INCH/HR) =      4.77
TOTAL STREAM AREA(ACRES) =      3.78
PEAK FLOW RATE(CFS) AT CONFLUENCE =      9.87

*****
FLOW PROCESS FROM NODE      503.00 TO NODE      504.00 IS CODE =      21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) =      0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      100.00
UPSTREAM ELEVATION(FEET) =      990.00
DOWNSTREAM ELEVATION(FEET) =      980.00
ELEVATION DIFFERENCE(FEET) =      10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      5.765
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      7.451
SUBAREA RUNOFF(CFS) =      0.49
TOTAL AREA(ACRES) =      0.16  TOTAL RUNOFF(CFS) =      0.49

*****
FLOW PROCESS FROM NODE      504.00 TO NODE      505.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) =      980.00  DOWNSTREAM(FEET) =      900.00
FLOW LENGTH(FEET) =      652.00  MANNING'S N =      0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      7.17
ESTIMATED PIPE DIAMETER(INCH) =      18.00  NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      0.49
PIPE TRAVEL TIME(MIN.) =      1.52    Tc(MIN.) =      7.28
LONGEST FLOWPATH FROM NODE      503.00 TO NODE      505.00 =      752.00 FEET.

*****
FLOW PROCESS FROM NODE      504.00 TO NODE      505.00 IS CODE =      81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      6.409
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) =      0
AREA-AVERAGE RUNOFF COEFFICIENT =      0.4100
SUBAREA AREA(ACRES) =      6.48  SUBAREA RUNOFF(CFS) =      17.03
TOTAL AREA(ACRES) =      6.6    TOTAL RUNOFF(CFS) =      17.45
Tc(MIN.) =      7.28

*****
FLOW PROCESS FROM NODE      505.00 TO NODE      506.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) =      905.00  DOWNSTREAM(FEET) =      903.00
FLOW LENGTH(FEET) =      75.00  MANNING'S N =      0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      11.32
ESTIMATED PIPE DIAMETER(INCH) =      21.00  NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      17.45
PIPE TRAVEL TIME(MIN.) =      0.11    Tc(MIN.) =      7.39
LONGEST FLOWPATH FROM NODE      503.00 TO NODE      506.00 =      827.00 FEET.

*****
FLOW PROCESS FROM NODE      506.00 TO NODE      506.00 IS CODE =      1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) =      7.39
RAINFALL INTENSITY(INCH/HR) =      6.35
TOTAL STREAM AREA(ACRES) =      6.64
PEAK FLOW RATE(CFS) AT CONFLUENCE =      17.45

** CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1          9.87    11.51    4.770    3.78
2         17.45    7.39    6.347    6.64

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1         23.79    7.39    6.347
2         22.99    11.51    4.770

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      23.79    Tc(MIN.) =      7.39
TOTAL AREA(ACRES) =      10.4

```



```

LONGEST FLOWPATH FROM NODE      500.00 TO NODE      506.00 =      1304.00 FEET.

*****
FLOW PROCESS FROM NODE      506.00 TO NODE      509.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 903.00 DOWNSTREAM(FEET) = 892.00
FLOW LENGTH(FEET) = 495.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.40
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 23.79
PIPE TRAVEL TIME(MIN.) = 0.72 Tc(MIN.) = 8.12
LONGEST FLOWPATH FROM NODE      500.00 TO NODE      509.00 =      1799.00 FEET.

*****
FLOW PROCESS FROM NODE      509.00 TO NODE      509.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 8.12
RAINFALL INTENSITY(INCH/HR) = 5.98
TOTAL STREAM AREA(ACRES) = 10.42
PEAK FLOW RATE(CFS) AT CONFLUENCE = 23.79

*****
FLOW PROCESS FROM NODE      507.00 TO NODE      508.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 931.55
DOWNSTREAM ELEVATION(FEET) = 930.85
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.89
TOTAL AREA(ACRES) = 0.30 TOTAL RUNOFF(CFS) = 0.89

*****
FLOW PROCESS FROM NODE      508.00 TO NODE      509.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 929.00 DOWNSTREAM ELEVATION(FEET) = 897.00
STREET LENGTH(FEET) = 1010.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.67
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.79
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.74
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.13
STREET FLOW TRAVEL TIME(MIN.) = 4.50 Tc(MIN.) = 13.23
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.360
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.05 SUBAREA RUNOFF(CFS) = 11.45
TOTAL AREA(ACRES) = 5.4 PEAK FLOW RATE(CFS) = 12.13

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.36
FLOW VELOCITY(FEET/SEC.) = 4.31 DEPTH*VELOCITY(FT*FT/SEC.) = 1.52
LONGEST FLOWPATH FROM NODE      507.00 TO NODE      509.00 =      1080.00 FEET.

*****
FLOW PROCESS FROM NODE      509.00 TO NODE      509.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.23
RAINFALL INTENSITY(INCH/HR) = 4.36
TOTAL STREAM AREA(ACRES) = 5.35
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.13

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 23.79 8.12 5.976 10.42
2 12.13 13.23 4.360 5.35

*****
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 31.23 8.12 5.976
2 29.48 13.23 4.360

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 31.23 Tc(MIN.) = 8.12
TOTAL AREA(ACRES) = 15.8
LONGEST FLOWPATH FROM NODE      500.00 TO NODE      509.00 =      1799.00 FEET.

*****
FLOW PROCESS FROM NODE      509.00 TO NODE      512.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 892.00 DOWNSTREAM(FEET) = 842.00
FLOW LENGTH(FEET) = 826.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 17.63
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 31.23
PIPE TRAVEL TIME(MIN.) = 0.78 Tc(MIN.) = 8.90
LONGEST FLOWPATH FROM NODE      500.00 TO NODE      512.00 =      2625.00 FEET.

*****
FLOW PROCESS FROM NODE      512.00 TO NODE      512.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 8.90
RAINFALL INTENSITY(INCH/HR) = 5.63
TOTAL STREAM AREA(ACRES) = 15.77
PEAK FLOW RATE(CFS) AT CONFLUENCE = 31.23

*****
FLOW PROCESS FROM NODE      510.00 TO NODE      511.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 896.65
DOWNSTREAM ELEVATION(FEET) = 895.95
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.74
TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.74

*****
FLOW PROCESS FROM NODE      511.00 TO NODE      512.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 895.00 DOWNSTREAM ELEVATION(FEET) = 847.00
STREET LENGTH(FEET) = 829.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.45
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.09
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.82
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.39
STREET FLOW TRAVEL TIME(MIN.) = 2.87 Tc(MIN.) = 11.60
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.746
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.40 SUBAREA RUNOFF(CFS) = 13.33
TOTAL AREA(ACRES) = 5.7 PEAK FLOW RATE(CFS) = 13.94

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.65
FLOW VELOCITY(FEET/SEC.) = 5.56 DEPTH*VELOCITY(FT*FT/SEC.) = 1.89
LONGEST FLOWPATH FROM NODE      510.00 TO NODE      512.00 =      899.00 FEET.

*****
FLOW PROCESS FROM NODE      512.00 TO NODE      512.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====

```



```

===== SUBAREA AREA(ACRES) = 7.87 SUBAREA RUNOFF(CFS) = 19.46
TOTAL NUMBER OF STREAMS = 2 TOTAL AREA(ACRES) = 8.1 PEAK FLOW RATE(CFS) = 20.10
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.60
RAINFALL INTENSITY(INCH/HR) = 4.75
TOTAL STREAM AREA(ACRES) = 5.65
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.94

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.43
FLOW VELOCITY(FT/SEC.) = 7.06 DEPTH*VELOCITY(FT*FT/SEC.) = 2.50
LONGEST FLOWPATH FROM NODE 515.00 TO NODE 517.00 = 1100.00 FEET.

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 31.23 8.90 5.633 15.77
2 13.94 11.60 4.746 5.65

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 41.92 8.90 5.633
2 40.25 11.60 4.746

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 41.92 Tc(MIN.) = 8.90
TOTAL AREA(ACRES) = 21.4
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 512.00 = 2625.00 FEET.

*****
FLOW PROCESS FROM NODE 512.00 TO NODE 517.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) = 838.00 DOWNSTREAM( FEET) = 748.00
FLOW LENGTH( FEET) = 1030.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.8 INCHES
PIPE-FLOW VELOCITY( FEET/SEC.) = 21.53
ESTIMATED PIPE DIAMETER( INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS) = 41.92
PIPE TRAVEL TIME( MIN.) = 0.80 Tc( MIN.) = 9.69
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 517.00 = 3655.00 FEET.

*****
FLOW PROCESS FROM NODE 517.00 TO NODE 517.00 IS CODE = 1
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) = 748.00 DOWNSTREAM( FEET) = 695.00
FLOW LENGTH( FEET) = 857.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.0 INCHES
PIPE-FLOW VELOCITY( FEET/SEC.) = 20.89
ESTIMATED PIPE DIAMETER( INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS) = 58.76
PIPE TRAVEL TIME( MIN.) = 0.68 Tc( MIN.) = 10.38
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 522.00 = 4512.00 FEET.

*****
FLOW PROCESS FROM NODE 517.00 TO NODE 517.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 9.69
RAINFALL INTENSITY(INCH/HR) = 5.33
TOTAL STREAM AREA(ACRES) = 21.42
PEAK FLOW RATE(CFS) AT CONFLUENCE = 41.92

*****
FLOW PROCESS FROM NODE 515.00 TO NODE 516.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH( FEET) = 70.00
UPSTREAM ELEVATION( FEET) = 843.55
DOWNSTREAM ELEVATION( FEET) = 842.85
ELEVATION DIFFERENCE( FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW( MIN.) = 8.735
100 YEAR RAINFALL INTENSITY( INCH/HOUR) = 5.699
SUBAREA RUNOFF( CFS) = 0.77
TOTAL AREA( ACRES) = 0.26 TOTAL RUNOFF( CFS) = 0.77

*****
FLOW PROCESS FROM NODE 516.00 TO NODE 517.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION( FEET) = 840.00 DOWNSTREAM ELEVATION( FEET) = 753.00
STREET LENGTH( FEET) = 1030.00 CURB HEIGHT( INCHES) = 6.0
STREET HALFWIDTH( FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK( FEET) = 9.00
INSIDE STREET CROSSFALL( DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL( DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL( DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.56
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH( FEET) = 0.30
HALFSTREET FLOOD WIDTH( FEET) = 8.68
AVERAGE FLOW VELOCITY( FEET/SEC.) = 6.06
PRODUCT OF DEPTH&VELOCITY( FT*FT/SEC.) = 1.82
STREET FLOW TRAVEL TIME( MIN.) = 2.83 Tc( MIN.) = 11.57
100 YEAR RAINFALL INTENSITY( INCH/HOUR) = 4.755
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520

*****
FLOW PROCESS FROM NODE 517.00 TO NODE 517.00 IS CODE = 1
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) = 748.00 DOWNSTREAM( FEET) = 695.00
FLOW LENGTH( FEET) = 857.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.0 INCHES
PIPE-FLOW VELOCITY( FEET/SEC.) = 20.89
ESTIMATED PIPE DIAMETER( INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS) = 58.76
PIPE TRAVEL TIME( MIN.) = 0.68 Tc( MIN.) = 10.38
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 522.00 = 4512.00 FEET.

*****
FLOW PROCESS FROM NODE 517.00 TO NODE 522.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) = 748.00 DOWNSTREAM( FEET) = 695.00
FLOW LENGTH( FEET) = 857.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.0 INCHES
PIPE-FLOW VELOCITY( FEET/SEC.) = 20.89
ESTIMATED PIPE DIAMETER( INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS) = 58.76
PIPE TRAVEL TIME( MIN.) = 0.68 Tc( MIN.) = 10.38
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 522.00 = 4512.00 FEET.

*****
FLOW PROCESS FROM NODE 522.00 TO NODE 522.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.38
RAINFALL INTENSITY(INCH/HR) = 5.10
TOTAL STREAM AREA(ACRES) = 29.55
PEAK FLOW RATE(CFS) AT CONFLUENCE = 58.76

*****
FLOW PROCESS FROM NODE 520.00 TO NODE 521.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH( FEET) = 70.00
UPSTREAM ELEVATION( FEET) = 758.83
DOWNSTREAM ELEVATION( FEET) = 758.15
ELEVATION DIFFERENCE( FEET) = 0.68
SUBAREA OVERLAND TIME OF FLOW( MIN.) = 8.747
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 68.86
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY( INCH/HOUR) = 5.694
SUBAREA RUNOFF( CFS) = 0.98
TOTAL AREA( ACRES) = 0.33 TOTAL RUNOFF( CFS) = 0.98

*****
FLOW PROCESS FROM NODE 521.00 TO NODE 522.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION( FEET) = 757.00 DOWNSTREAM ELEVATION( FEET) = 700.00
STREET LENGTH( FEET) = 755.00 CURB HEIGHT( INCHES) = 6.0
STREET HALFWIDTH( FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK( FEET) = 9.00
INSIDE STREET CROSSFALL( DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL( DECIMAL) = 0.020

```



```

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

*****
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.25
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.09
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.50
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.59
STREET FLOW TRAVEL TIME(MIN.) = 2.29 Tc(MIN.) = 11.03
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.902
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 2.56 SUBAREA RUNOFF(CFS) = 6.53
TOTAL AREA(ACRES) = 2.9 PEAK FLOW RATE(CFS) = 7.37

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.33 HALFSTREET FLOOD WIDTH(FEET) = 10.30
FLOW VELOCITY(FEET/SEC.) = 6.25 DEPTH*VELOCITY(FT*FT/SEC.) = 2.08
LONGEST FLOWPATH FROM NODE 520.00 TO NODE 522.00 = 825.00 FEET.

*****
FLOW PROCESS FROM NODE 522.00 TO NODE 522.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.03
RAINFALL INTENSITY(INCH/HR) = 4.90
TOTAL STREAM AREA(ACRES) = 2.89
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.37

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 58.76 10.38 5.100 29.55
2 7.37 11.03 4.902 2.89

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 65.69 10.38 5.100
2 63.85 11.03 4.902

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 65.69 Tc(MIN.) = 10.38
TOTAL AREA(ACRES) = 32.4
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 522.00 = 4512.00 FEET.

*****
FLOW PROCESS FROM NODE 522.00 TO NODE 498.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 695.00 DOWNSTREAM(FEET) = 693.00
FLOW LENGTH(FEET) = 114.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 26.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.09
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 65.69
PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 10.52
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 498.00 = 4626.00 FEET.

*****
FLOW PROCESS FROM NODE 498.00 TO NODE 498.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 65.69 10.52 5.054 32.44
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 498.00 = 4626.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 161.62 12.77 4.462 78.69
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 498.00 = 6748.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 198.89 10.52 5.054
2 219.60 12.77 4.462

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 219.60 Tc(MIN.) = 12.77
TOTAL AREA(ACRES) = 111.1

*****
FLOW PROCESS FROM NODE 498.00 TO NODE 498.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====
*****
FLOW PROCESS FROM NODE 498.00 TO NODE 527.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 692.00 DOWNSTREAM(FEET) = 689.50
FLOW LENGTH(FEET) = 407.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 63.0 INCH PIPE IS 50.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.93
ESTIMATED PIPE DIAMETER(INCH) = 63.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 219.60
PIPE TRAVEL TIME(MIN.) = 0.57 Tc(MIN.) = 13.34
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 527.00 = 7155.00 FEET.

*****
FLOW PROCESS FROM NODE 527.00 TO NODE 527.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.34
RAINFALL INTENSITY(INCH/HR) = 4.34
TOTAL STREAM AREA(ACRES) = 111.13
PEAK FLOW RATE(CFS) AT CONFLUENCE = 219.60

*****
FLOW PROCESS FROM NODE 525.00 TO NODE 526.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 754.75
DOWNSTREAM ELEVATION(FEET) = 754.05
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.74
TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.74

*****
FLOW PROCESS FROM NODE 526.00 TO NODE 527.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 753.00 DOWNSTREAM ELEVATION(FEET) = 694.00
STREET LENGTH(FEET) = 1294.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.74
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.56
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.40
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.31
STREET FLOW TRAVEL TIME(MIN.) = 4.91 Tc(MIN.) = 13.64
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.275
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.418
SUBAREA AREA(ACRES) = 3.35 SUBAREA RUNOFF(CFS) = 5.87
TOTAL AREA(ACRES) = 3.6 PEAK FLOW RATE(CFS) = 6.43

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.79
FLOW VELOCITY(FEET/SEC.) = 5.01 DEPTH*VELOCITY(FT*FT/SEC.) = 1.71
LONGEST FLOWPATH FROM NODE 525.00 TO NODE 527.00 = 1364.00 FEET.

*****
FLOW PROCESS FROM NODE 527.00 TO NODE 527.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.64
RAINFALL INTENSITY(INCH/HR) = 4.28
TOTAL STREAM AREA(ACRES) = 3.60
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.43

*****
FLOW PROCESS FROM NODE 530.00 TO NODE 531.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

```



```

=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 700.00
DOWNSTREAM ELEVATION(FEET) = 695.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.396
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.776
SUBAREA RUNOFF(CFS) = 0.19
TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.19

*****
FLOW PROCESS FROM NODE 531.00 TO NODE 532.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 695.00 DOWNSTREAM(FEET) = 690.00
FLOW LENGTH(FEET) = 180.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.21
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.19
PIPE TRAVEL TIME(MIN.) = 0.93 Tc(MIN.) = 6.33
LONGEST FLOWPATH FROM NODE 530.00 TO NODE 532.00 = 250.00 FEET.

*****
FLOW PROCESS FROM NODE 531.00 TO NODE 532.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.015
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 0.79 SUBAREA RUNOFF(CFS) = 2.27
TOTAL AREA(ACRES) = 0.9 TOTAL RUNOFF(CFS) = 2.44
Tc(MIN.) = 6.33

*****
FLOW PROCESS FROM NODE 532.00 TO NODE 527.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 6.33
RAINFALL INTENSITY(INCH/HR) = 7.01
TOTAL STREAM AREA(ACRES) = 0.85
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.44

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 219.60 13.34 4.338 111.13
2 6.43 13.64 4.275 3.60
3 2.44 6.33 7.015 0.85

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 109.67 6.33 7.015
2 227.40 13.34 4.338
3 224.35 13.64 4.275

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 227.40 Tc(MIN.) = 13.34
TOTAL AREA(ACRES) = 115.6
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 527.00 = 7155.00 FEET.

*****
FLOW PROCESS FROM NODE 527.00 TO NODE 533.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 690.00 DOWNSTREAM(FEET) = 689.00
FLOW LENGTH(FEET) = 117.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 60.0 INCH PIPE IS 47.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.62
ESTIMATED PIPE DIAMETER(INCH) = 60.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 227.40
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 13.48
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 533.00 = 7272.00 FEET.

*****
FLOW PROCESS FROM NODE 533.00 TO NODE 533.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<
=====
FLOW PROCESS FROM NODE 600.00 TO NODE 601.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 888.85
DOWNSTREAM ELEVATION(FEET) = 888.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.77
TOTAL AREA(ACRES) = 0.26 TOTAL RUNOFF(CFS) = 0.77

*****
FLOW PROCESS FROM NODE 601.00 TO NODE 602.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 887.00 DOWNSTREAM ELEVATION(FEET) = 869.00
STREET LENGTH(FEET) = 565.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.52
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.68
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.74
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.12
STREET FLOW TRAVEL TIME(MIN.) = 2.52 Tc(MIN.) = 11.25
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.841
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.55 SUBAREA RUNOFF(CFS) = 11.45
TOTAL AREA(ACRES) = 4.8 PEAK FLOW RATE(CFS) = 12.11

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.36
FLOW VELOCITY(FEET/SEC.) = 4.30 DEPTH*VELOCITY(FT*FT/SEC.) = 1.52
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 602.00 = 635.00 FEET.

*****
FLOW PROCESS FROM NODE 602.00 TO NODE 608.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 864.00 DOWNSTREAM(FEET) = 863.50
FLOW LENGTH(FEET) = 151.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.60
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.11
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 11.80
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 608.00 = 786.00 FEET.

*****
FLOW PROCESS FROM NODE 608.00 TO NODE 608.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.80
RAINFALL INTENSITY(INCH/HR) = 4.69
TOTAL STREAM AREA(ACRES) = 4.81
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.11

*****
FLOW PROCESS FROM NODE 605.00 TO NODE 606.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 886.00
DOWNSTREAM ELEVATION(FEET) = 882.00
ELEVATION DIFFERENCE(FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.813
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.412
SUBAREA RUNOFF(CFS) = 0.49
TOTAL AREA(ACRES) = 0.16 TOTAL RUNOFF(CFS) = 0.49

*****
FLOW PROCESS FROM NODE 606.00 TO NODE 607.00 IS CODE = 31
-----

```



```

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 882.00 DOWNSTREAM(FEET) = 869.00
FLOW LENGTH(FEET) = 430.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.38
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.49
PIPE TRAVEL TIME(MIN.) = 1.64 Tc(MIN.) = 7.45
LONGEST FLOWPATH FROM NODE 605.00 TO NODE 607.00 = 500.00 FEET.

*****
FLOW PROCESS FROM NODE 606.00 TO NODE 607.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.316
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 2.06 SUBAREA RUNOFF(CFS) = 5.33
TOTAL AREA(ACRES) = 2.2 TOTAL RUNOFF(CFS) = 5.75
TC(MIN.) = 7.45

*****
FLOW PROCESS FROM NODE 607.00 TO NODE 608.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 865.00 DOWNSTREAM(FEET) = 864.00
FLOW LENGTH(FEET) = 63.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.09
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.75
PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 7.60
LONGEST FLOWPATH FROM NODE 605.00 TO NODE 608.00 = 563.00 FEET.

*****
FLOW PROCESS FROM NODE 608.00 TO NODE 608.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.60
RAINFALL INTENSITY(INCH/HR) = 6.24
TOTAL STREAM AREA(ACRES) = 2.22
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.75

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 12.11 11.80 4.695 4.81
2 5.75 7.60 6.237 2.22

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 13.55 7.60 6.237
2 16.44 11.80 4.695

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 16.44 Tc(MIN.) = 11.80
TOTAL AREA(ACRES) = 7.0
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 608.00 = 786.00 FEET.

*****
FLOW PROCESS FROM NODE 608.00 TO NODE 613.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 864.00 DOWNSTREAM(FEET) = 850.00
FLOW LENGTH(FEET) = 284.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.01
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.44
PIPE TRAVEL TIME(MIN.) = 0.34 Tc(MIN.) = 12.14
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 613.00 = 1070.00 FEET.

*****
FLOW PROCESS FROM NODE 613.00 TO NODE 613.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.14
RAINFALL INTENSITY(INCH/HR) = 4.61
TOTAL STREAM AREA(ACRES) = 7.03
PEAK FLOW RATE(CFS) AT CONFLUENCE = 16.44

*****
FLOW PROCESS FROM NODE 613.00 TO NODE 614.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 850.00 DOWNSTREAM(FEET) = 845.00
FLOW LENGTH(FEET) = 71.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.88

*****
FLOW PROCESS FROM NODE 611.00 TO NODE 612.00 IS CODE = 21
=====
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 877.85
DOWNSTREAM ELEVATION(FEET) = 877.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.74
TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.74

*****
FLOW PROCESS FROM NODE 612.00 TO NODE 613.00 IS CODE = 61
=====
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 877.00 DOWNSTREAM ELEVATION(FEET) = 855.00
STREET LENGTH(FEET) = 683.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.50
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.23
HALFSTREET FLOOD WIDTH(FEET) = 5.34
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.10
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.72
STREET FLOW TRAVEL TIME(MIN.) = 3.67 Tc(MIN.) = 12.41
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.544
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 1.48 SUBAREA RUNOFF(CFS) = 3.50
TOTAL AREA(ACRES) = 1.7 PEAK FLOW RATE(CFS) = 4.09

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.27 HALFSTREET FLOOD WIDTH(FEET) = 6.98
FLOW VELOCITY(FEET/SEC.) = 3.38 DEPTH*VELOCITY(FT*FT/SEC.) = 0.90
LONGEST FLOWPATH FROM NODE 611.00 TO NODE 613.00 = 753.00 FEET.

*****
FLOW PROCESS FROM NODE 613.00 TO NODE 613.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.41
RAINFALL INTENSITY(INCH/HR) = 4.54
TOTAL STREAM AREA(ACRES) = 1.73
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.09

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 16.44 12.14 4.610 7.03
2 4.09 12.41 4.544 1.73

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 20.43 12.14 4.610
2 20.29 12.41 4.544

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 20.43 Tc(MIN.) = 12.14
TOTAL AREA(ACRES) = 8.8
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 613.00 = 1070.00 FEET.

*****
FLOW PROCESS FROM NODE 613.00 TO NODE 614.00 IS CODE = 31
=====
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 850.00 DOWNSTREAM(FEET) = 845.00
FLOW LENGTH(FEET) = 71.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.88

```



```

ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1    STREAM    RUNOFF    Tc    INTENSITY    AREA
PIPE-FLOW(CFS) = 20.43    NUMBER    (CFS)    (MIN.)    (INCH/HR)    (ACRE)
PIPE TRAVEL TIME(MIN.) = 0.07    Tc(MIN.) = 12.21    1    20.43    12.21    4.593    8.76
LONGEST FLOWPATH FROM NODE    600.00 TO NODE    614.00 = 1141.00 FEET.    2    12.38    9.68    5.334    4.34

***** RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
FLOW PROCESS FROM NODE    614.00 TO NODE    614.00 IS CODE = 1    CONFLUENCE FORMULA USED FOR 2 STREAMS.
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.21
RAINFALL INTENSITY(INCH/HR) = 4.59
TOTAL STREAM AREA(ACRES) = 8.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 20.43

*****
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 31.10    Tc(MIN.) = 12.21
TOTAL AREA(ACRES) = 13.1
LONGEST FLOWPATH FROM NODE    600.00 TO NODE    614.00 = 1141.00 FEET.

*****
FLOW PROCESS FROM NODE    617.00 TO NODE    618.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 895.00
DOWNSTREAM ELEVATION(FEET) = 893.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.156
100 YEAR RAINFALL INTENSITY(INCH/HR) = 7.142
SUBAREA RUNOFF(CFS) = 0.74
TOTAL AREA(ACRES) = 0.20    TOTAL RUNOFF(CFS) = 0.74

*****
FLOW PROCESS FROM NODE    618.00 TO NODE    619.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
*****
UPSTREAM ELEVATION(FEET) = 893.00    DOWNSTREAM ELEVATION(FEET) = 865.00
STREET LENGTH(FEET) = 745.00    CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.72
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.50
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.99
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.18
STREET FLOW TRAVEL TIME(MIN.) = 3.11    Tc(MIN.) = 9.26
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.487
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.14    SUBAREA RUNOFF(CFS) = 11.81
TOTAL AREA(ACRES) = 4.3    PEAK FLOW RATE(CFS) = 12.38

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35    HALFSTREET FLOOD WIDTH(FEET) = 11.07
FLOW VELOCITY(FEET/SEC.) = 4.60    DEPTH*VELOCITY(FT*FT/SEC.) = 1.60
LONGEST FLOWPATH FROM NODE    617.00 TO NODE    619.00 = 815.00 FEET.

*****
FLOW PROCESS FROM NODE    619.00 TO NODE    614.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 860.00    DOWNSTREAM(FEET) = 848.00
FLOW LENGTH(FEET) = 302.00    MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.10
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.38
PIPE TRAVEL TIME(MIN.) = 0.42    Tc(MIN.) = 9.68
LONGEST FLOWPATH FROM NODE    617.00 TO NODE    614.00 = 1117.00 FEET.

*****
FLOW PROCESS FROM NODE    614.00 TO NODE    614.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.68
RAINFALL INTENSITY(INCH/HR) = 5.33
TOTAL STREAM AREA(ACRES) = 4.34
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.38

** CONFLUENCE DATA **

*****
FLOW PROCESS FROM NODE    614.00 TO NODE    620.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 848.00    DOWNSTREAM(FEET) = 833.00
FLOW LENGTH(FEET) = 321.00    MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 16.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.77
ESTIMATED PIPE DIAMETER(INCH) = 21.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 31.10
PIPE TRAVEL TIME(MIN.) = 0.34    Tc(MIN.) = 12.55
LONGEST FLOWPATH FROM NODE    600.00 TO NODE    620.00 = 1462.00 FEET.

*****
FLOW PROCESS FROM NODE    620.00 TO NODE    620.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.55
RAINFALL INTENSITY(INCH/HR) = 4.51
TOTAL STREAM AREA(ACRES) = 13.10
PEAK FLOW RATE(CFS) AT CONFLUENCE = 31.10

*****
FLOW PROCESS FROM NODE    623.00 TO NODE    624.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 895.85
DOWNSTREAM ELEVATION(FEET) = 895.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.86
TOTAL AREA(ACRES) = 0.29    TOTAL RUNOFF(CFS) = 0.86

*****
FLOW PROCESS FROM NODE    624.00 TO NODE    625.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
*****
UPSTREAM ELEVATION(FEET) = 894.00    DOWNSTREAM ELEVATION(FEET) = 854.00
STREET LENGTH(FEET) = 555.00    CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.92
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.26
HALFSTREET FLOOD WIDTH(FEET) = 6.86
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.02
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.32
STREET FLOW TRAVEL TIME(MIN.) = 1.84    Tc(MIN.) = 10.58
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.038
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.85    SUBAREA RUNOFF(CFS) = 10.09
TOTAL AREA(ACRES) = 4.1    PEAK FLOW RATE(CFS) = 10.85

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.31    HALFSTREET FLOOD WIDTH(FEET) = 9.11

```



```

FLOW VELOCITY(FEET/SEC.) = 5.72 DEPTH*VELOCITY(FT*FT/SEC.) = 1.77
LONGEST FLOWPATH FROM NODE 623.00 TO NODE 625.00 = 625.00 FEET.
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 862.00 DOWNSTREAM ELEVATION(FEET) = 836.00
STREET LENGTH(FEET) = 643.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.42
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.21
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.05
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.18
STREET FLOW TRAVEL TIME(MIN.) = 2.65 Tc(MIN.) = 11.38
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.805
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.40 SUBAREA RUNOFF(CFS) = 10.99
TOTAL AREA(ACRES) = 4.7 PEAK FLOW RATE(CFS) = 11.74
END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.65
FLOW VELOCITY(FEET/SEC.) = 4.69 DEPTH*VELOCITY(FT*FT/SEC.) = 1.59
LONGEST FLOWPATH FROM NODE 628.00 TO NODE 630.00 = 713.00 FEET.
=====
FLOW PROCESS FROM NODE 630.00 TO NODE 630.00 IS CODE = 1
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.01
RAINFALL INTENSITY(INCH/HR) = 4.91
TOTAL STREAM AREA(ACRES) = 4.14
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.85
** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 31.10 12.55 4.513 13.10
2 10.85 11.01 4.909 4.14
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 39.43 11.01 4.909
2 41.07 12.55 4.513
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 41.07 Tc(MIN.) = 12.55
TOTAL AREA(ACRES) = 17.2
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 620.00 = 1462.00 FEET.
=====
FLOW PROCESS FROM NODE 620.00 TO NODE 630.00 IS CODE = 31
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 833.00 DOWNSTREAM(FEET) = 831.00
FLOW LENGTH(FEET) = 89.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.89
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 41.07
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 12.66
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 630.00 = 1551.00 FEET.
=====
FLOW PROCESS FROM NODE 620.00 TO NODE 630.00 IS CODE = 31
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 833.00 DOWNSTREAM(FEET) = 831.00
FLOW LENGTH(FEET) = 89.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.89
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 41.07
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 12.66
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 630.00 = 1551.00 FEET.
=====
FLOW PROCESS FROM NODE 630.00 TO NODE 630.00 IS CODE = 1
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.66
RAINFALL INTENSITY(INCH/HR) = 4.49
TOTAL STREAM AREA(ACRES) = 17.24
PEAK FLOW RATE(CFS) AT CONFLUENCE = 41.07
=====
FLOW PROCESS FROM NODE 628.00 TO NODE 629.00 IS CODE = 21
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 866.85
DOWNSTREAM ELEVATION(FEET) = 866.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.699
SUBAREA RUNOFF(CFS) = 0.89
TOTAL AREA(ACRES) = 0.30 TOTAL RUNOFF(CFS) = 0.89
=====
FLOW PROCESS FROM NODE 629.00 TO NODE 630.00 IS CODE = 61
=====
UPSTREAM ELEVATION(FEET) = 862.00 DOWNSTREAM ELEVATION(FEET) = 836.00
STREET LENGTH(FEET) = 643.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.42
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.21
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.05
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.18
STREET FLOW TRAVEL TIME(MIN.) = 2.65 Tc(MIN.) = 11.38
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.805
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.40 SUBAREA RUNOFF(CFS) = 10.99
TOTAL AREA(ACRES) = 4.7 PEAK FLOW RATE(CFS) = 11.74
END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.65
FLOW VELOCITY(FEET/SEC.) = 4.69 DEPTH*VELOCITY(FT*FT/SEC.) = 1.59
LONGEST FLOWPATH FROM NODE 628.00 TO NODE 630.00 = 713.00 FEET.
=====
FLOW PROCESS FROM NODE 630.00 TO NODE 630.00 IS CODE = 1
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.38
RAINFALL INTENSITY(INCH/HR) = 4.81
TOTAL STREAM AREA(ACRES) = 4.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.74
** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 41.07 12.66 4.486 17.24
2 11.74 11.38 4.805 4.70
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 50.08 11.38 4.805
2 52.03 12.66 4.486
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 52.03 Tc(MIN.) = 12.66
TOTAL AREA(ACRES) = 21.9
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 630.00 = 1551.00 FEET.
=====
FLOW PROCESS FROM NODE 630.00 TO NODE 631.00 IS CODE = 31
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 831.00 DOWNSTREAM(FEET) = 830.00
FLOW LENGTH(FEET) = 88.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 25.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.52
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 52.03
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 12.80
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 631.00 = 1639.00 FEET.
=====
FLOW PROCESS FROM NODE 631.00 TO NODE 631.00 IS CODE = 10
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====
FLOW PROCESS FROM NODE 634.00 TO NODE 635.00 IS CODE = 21
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 85.00
UPSTREAM ELEVATION(FEET) = 897.00
DOWNSTREAM ELEVATION(FEET) = 896.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 10.028

```



```

WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 72.65
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.214
SUBAREA RUNOFF(CFS) = 0.24
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.24

*****
FLOW PROCESS FROM NODE 635.00 TO NODE 636.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 894.00 DOWNSTREAM(FEET) = 860.00
FLOW LENGTH(FEET) = 907.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.77
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.24
PIPE TRAVEL TIME(MIN.) = 4.01 Tc(MIN.) = 14.04
LONGEST FLOWPATH FROM NODE 634.00 TO NODE 636.00 = 992.00 FEET.

*****
FLOW PROCESS FROM NODE 635.00 TO NODE 636.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.197
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 1.33 SUBAREA RUNOFF(CFS) = 2.29
TOTAL AREA(ACRES) = 1.4 TOTAL RUNOFF(CFS) = 2.48
Tc(MIN.) = 14.04

*****
FLOW PROCESS FROM NODE 636.00 TO NODE 637.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 855.00 DOWNSTREAM(FEET) = 853.00
FLOW LENGTH(FEET) = 43.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.25
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.48
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 14.12
LONGEST FLOWPATH FROM NODE 634.00 TO NODE 637.00 = 1035.00 FEET.

*****
FLOW PROCESS FROM NODE 637.00 TO NODE 642.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 855.00 DOWNSTREAM(FEET) = 835.00
FLOW LENGTH(FEET) = 350.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.89
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.48
PIPE TRAVEL TIME(MIN.) = 0.66 Tc(MIN.) = 14.78
LONGEST FLOWPATH FROM NODE 634.00 TO NODE 642.00 = 1385.00 FEET.

*****
FLOW PROCESS FROM NODE 642.00 TO NODE 642.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.78
RAINFALL INTENSITY(INCH/HR) = 4.06
TOTAL STREAM AREA(ACRES) = 1.44
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.48

*****
FLOW PROCESS FROM NODE 640.00 TO NODE 641.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 912.00
DOWNSTREAM ELEVATION(FEET) = 911.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.204
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 64.29
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.32
TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 1.32

*****
FLOW PROCESS FROM NODE 641.00 TO NODE 642.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 911.00 DOWNSTREAM ELEVATION(FEET) = 840.00
STREET LENGTH(FEET) = 2.13 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.71
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.16
HALFSTREET FLOOD WIDTH(FEET) = 1.50
AVERAGE FLOW VELOCITY(FEET/SEC.) = 108.89
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 17.01
STREET FLOW TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 3.20
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 2.13 SUBAREA RUNOFF(CFS) = 14.79
TOTAL AREA(ACRES) = 2.3 PEAK FLOW RATE(CFS) = 16.11

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.16 HALFSTREET FLOOD WIDTH(FEET) = 1.50
FLOW VELOCITY(FEET/SEC.) = ***** DEPTH*VELOCITY(FT*FT/SEC.) = 17.01
LONGEST FLOWPATH FROM NODE 640.00 TO NODE 642.00 = 72.13 FEET.

*****
FLOW PROCESS FROM NODE 642.00 TO NODE 642.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 3.20
RAINFALL INTENSITY(INCH/HR) = 8.17
TOTAL STREAM AREA(ACRES) = 2.32
PEAK FLOW RATE(CFS) AT CONFLUENCE = 16.11

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 2.48 14.78 4.059 1.44
2 16.11 3.20 8.168 2.32

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 16.64 3.20 8.168
2 10.48 14.78 4.059

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 16.64 Tc(MIN.) = 3.20
TOTAL AREA(ACRES) = 3.8
LONGEST FLOWPATH FROM NODE 634.00 TO NODE 642.00 = 1385.00 FEET.

*****
FLOW PROCESS FROM NODE 642.00 TO NODE 631.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 835.00 DOWNSTREAM(FEET) = 833.00
FLOW LENGTH(FEET) = 89.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.47
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.64
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 3.35
LONGEST FLOWPATH FROM NODE 634.00 TO NODE 631.00 = 1474.00 FEET.

*****
FLOW PROCESS FROM NODE 631.00 TO NODE 631.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 16.64 3.35 8.168 3.76
LONGEST FLOWPATH FROM NODE 634.00 TO NODE 631.00 = 1474.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA

```



```

NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1           52.03      12.80         4.454          21.94
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 631.00 = 1639.00 FEET.

*****
** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1           30.24         3.35         8.168
2           61.11         12.80         4.454

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 61.11 Tc(MIN.) = 12.80
TOTAL AREA(ACRES) = 25.7

*****
FLOW PROCESS FROM NODE 631.00 TO NODE 631.00 IS CODE = 12
*****
>>>>CLEAR MEMORY BANK # 1 <<<<
*****
FLOW PROCESS FROM NODE 631.00 TO NODE 648.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 832.00 DOWNSTREAM(FEET) = 774.00
FLOW LENGTH(FEET) = 963.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.81
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 61.11
PIPE TRAVEL TIME(MIN.) = 0.77 Tc(MIN.) = 13.57
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 648.00 = 2602.00 FEET.

*****
FLOW PROCESS FROM NODE 648.00 TO NODE 648.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.57
RAINFALL INTENSITY(INCH/HR) = 4.29
TOTAL STREAM AREA(ACRES) = 25.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 61.11

*****
FLOW PROCESS FROM NODE 645.00 TO NODE 646.00 IS CODE = 21
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 854.00
DOWNSTREAM ELEVATION(FEET) = 850.00
ELEVATION DIFFERENCE(FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.886
100 YEAR RAINFALL INTENSITY(INCH/HR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.51
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.51

*****
FLOW PROCESS FROM NODE 646.00 TO NODE 647.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(FEET) = 854.00 DOWNSTREAM ELEVATION(FEET) = 788.00
STREET LENGTH(FEET) = 1005.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.30
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.32
HALFSTREET FLOOD WIDTH(FEET) = 9.81
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.69
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.84
STREET FLOW TRAVEL TIME(MIN.) = 2.94 Tc(MIN.) = 7.83
100 YEAR RAINFALL INTENSITY(INCH/HR) = 6.116
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 7.31 SUBAREA RUNOFF(CFS) = 23.25
TOTAL AREA(ACRES) = 7.4 PEAK FLOW RATE(CFS) = 23.63

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.38 HALFSTREET FLOOD WIDTH(FEET) = 12.90
FLOW VELOCITY(FEET/SEC.) = 6.63 DEPTH*VELOCITY(FT*FT/SEC.) = 2.55
LONGEST FLOWPATH FROM NODE 645.00 TO NODE 647.00 = 1075.00 FEET.

*****
FLOW PROCESS FROM NODE 647.00 TO NODE 648.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 781.00 DOWNSTREAM(FEET) = 770.00
FLOW LENGTH(FEET) = 184.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.11
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 23.63
PIPE TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) = 8.02
LONGEST FLOWPATH FROM NODE 645.00 TO NODE 648.00 = 1259.00 FEET.

*****
FLOW PROCESS FROM NODE 648.00 TO NODE 648.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.02
RAINFALL INTENSITY(INCH/HR) = 6.02
TOTAL STREAM AREA(ACRES) = 7.43
PEAK FLOW RATE(CFS) AT CONFLUENCE = 23.63

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1           61.11      13.57      4.289          25.70
2           23.63      8.02       6.022          7.43

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1           59.74         8.02         4.289
2           77.94      13.57         4.289

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 77.94 Tc(MIN.) = 13.57
TOTAL AREA(ACRES) = 33.1
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 648.00 = 2602.00 FEET.

*****
FLOW PROCESS FROM NODE 648.00 TO NODE 653.00 IS CODE = 31
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 770.00 DOWNSTREAM(FEET) = 765.00
FLOW LENGTH(FEET) = 131.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 24.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 18.14
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 77.94
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 13.69
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 653.00 = 2733.00 FEET.

*****
FLOW PROCESS FROM NODE 653.00 TO NODE 653.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.69
RAINFALL INTENSITY(INCH/HR) = 4.27
TOTAL STREAM AREA(ACRES) = 33.13
PEAK FLOW RATE(CFS) AT CONFLUENCE = 77.94

*****
FLOW PROCESS FROM NODE 651.00 TO NODE 652.00 IS CODE = 21
*****
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
*****
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 839.00
DOWNSTREAM ELEVATION(FEET) = 837.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.653
100 YEAR RAINFALL INTENSITY(INCH/HR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.46
TOTAL AREA(ACRES) = 0.21 TOTAL RUNOFF(CFS) = 1.46

*****
FLOW PROCESS FROM NODE 652.00 TO NODE 653.00 IS CODE = 61
*****
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
*****
UPSTREAM ELEVATION(FEET) = 837.00 DOWNSTREAM ELEVATION(FEET) = 768.50
STREET LENGTH(FEET) = 1029.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

```



```

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.74
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.40
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.64
STREET FLOW TRAVEL TIME(MIN.) = 3.18 Tc(MIN.) = 5.83
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.399
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 2.61 SUBAREA RUNOFF(CFS) = 16.41
TOTAL AREA(ACRES) = 2.8 PEAK FLOW RATE(CFS) = 17.73

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.43
FLOW VELOCITY(FEET/SEC.) = 6.23 DEPTH*VELOCITY(FT*FT/SEC.) = 2.21
LONGEST FLOWPATH FROM NODE 651.00 TO NODE 653.00 = 1099.00 FEET.

*****
FLOW PROCESS FROM NODE 653.00 TO NODE 653.00 IS CODE = 1
*****
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.83
RAINFALL INTENSITY(INCH/HR) = 7.40
TOTAL STREAM AREA(ACRES) = 2.82
PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.73

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 77.94 13.69 4.265 33.13
2 17.73 5.83 7.399 2.82

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 62.66 5.83 7.399
2 88.16 13.69 4.265

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 88.16 Tc(MIN.) = 13.69
TOTAL AREA(ACRES) = 36.0
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 653.00 = 2733.00 FEET.

*****
FLOW PROCESS FROM NODE 653.00 TO NODE 654.00 IS CODE = 31
*****
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 763.50 DOWNSTREAM(FEET) = 760.50
FLOW LENGTH(FEET) = 132.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.69
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 88.16
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 13.83
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 654.00 = 2865.00 FEET.

*****
FLOW PROCESS FROM NODE 654.00 TO NODE 654.00 IS CODE = 10
*****
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
*****
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 854.85
DOWNSTREAM ELEVATION(FEET) = 854.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.65

TOTAL AREA(ACRES) = 0.22 TOTAL RUNOFF(CFS) = 0.65

*****
FLOW PROCESS FROM NODE 658.00 TO NODE 659.00 IS CODE = 61
*****
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
*****
UPSTREAM ELEVATION(FEET) = 852.00 DOWNSTREAM ELEVATION(FEET) = 812.00
STREET LENGTH(FEET) = 650.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.31
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.27
HALFSTREET FLOOD WIDTH(FEET) = 7.33
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.81
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.31
STREET FLOW TRAVEL TIME(MIN.) = 2.25 Tc(MIN.) = 10.99
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.916
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.41 SUBAREA RUNOFF(CFS) = 11.27
TOTAL AREA(ACRES) = 4.6 PEAK FLOW RATE(CFS) = 11.84

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.81
FLOW VELOCITY(FEET/SEC.) = 5.48 DEPTH*VELOCITY(FT*FT/SEC.) = 1.77
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 659.00 = 720.00 FEET.

*****
FLOW PROCESS FROM NODE 659.00 TO NODE 659.00 IS CODE = 1
*****
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
*****
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.99
RAINFALL INTENSITY(INCH/HR) = 4.92
TOTAL STREAM AREA(ACRES) = 4.63
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.84

*****
FLOW PROCESS FROM NODE 662.00 TO NODE 663.00 IS CODE = 21
*****
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 825.00
DOWNSTREAM ELEVATION(FEET) = 820.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.396
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.776
SUBAREA RUNOFF(CFS) = 0.48
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 0.48

*****
FLOW PROCESS FROM NODE 663.00 TO NODE 664.00 IS CODE = 31
*****
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
*****
ELEVATION DATA: UPSTREAM(FEET) = 820.00 DOWNSTREAM(FEET) = 816.00
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.00
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.33
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.48
PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 5.46
LONGEST FLOWPATH FROM NODE 662.00 TO NODE 664.00 = 100.00 FEET.

*****
FLOW PROCESS FROM NODE 663.00 TO NODE 664.00 IS CODE = 81
*****
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
*****
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.713
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 0.29 SUBAREA RUNOFF(CFS) = 0.92
TOTAL AREA(ACRES) = 0.4 TOTAL RUNOFF(CFS) = 1.39
Tc(MIN.) = 5.46

*****
FLOW PROCESS FROM NODE 664.00 TO NODE 659.00 IS CODE = 31
*****

```



```

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 816.00 DOWNSTREAM(FEET) = 812.00
FLOW LENGTH(FEET) = 45.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.75
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.39
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 5.55
LONGEST FLOWPATH FROM NODE 662.00 TO NODE 659.00 = 145.00 FEET.

*****
FLOW PROCESS FROM NODE 659.00 TO NODE 659.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.55
RAINFALL INTENSITY(INCH/HR) = 7.64
TOTAL STREAM AREA(ACRES) = 0.44
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.39

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 11.84 10.99 4.916 4.63
2 1.39 5.55 7.636 0.44

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 7.37 5.55 7.636
2 12.73 10.99 4.916

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 12.73 Tc(MIN.) = 10.99
TOTAL AREA(ACRES) = 5.1
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 659.00 = 720.00 FEET.

*****
FLOW PROCESS FROM NODE 659.00 TO NODE 665.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 807.00 DOWNSTREAM(FEET) = 784.00
FLOW LENGTH(FEET) = 356.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.64
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.73
PIPE TRAVEL TIME(MIN.) = 0.41 Tc(MIN.) = 11.39
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 665.00 = 1076.00 FEET.

*****
FLOW PROCESS FROM NODE 665.00 TO NODE 665.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.39
RAINFALL INTENSITY(INCH/HR) = 4.80
TOTAL STREAM AREA(ACRES) = 5.07
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.73

*****
FLOW PROCESS FROM NODE 666.00 TO NODE 667.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 820.00
DOWNSTREAM ELEVATION(FEET) = 810.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.765
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION#
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.451
SUBAREA RUNOFF(CFS) = 0.52
TOTAL AREA(ACRES) = 0.17 TOTAL RUNOFF(CFS) = 0.52

*****
FLOW PROCESS FROM NODE 667.00 TO NODE 668.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 810.00 DOWNSTREAM(FEET) = 800.00
FLOW LENGTH(FEET) = 471.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.97

ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.52
PIPE TRAVEL TIME(MIN.) = 1.98 Tc(MIN.) = 7.74
LONGEST FLOWPATH FROM NODE 666.00 TO NODE 668.00 = 571.00 FEET.

*****
FLOW PROCESS FROM NODE 667.00 TO NODE 668.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.159
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 0.90 SUBAREA RUNOFF(CFS) = 2.27
TOTAL AREA(ACRES) = 1.1 TOTAL RUNOFF(CFS) = 2.70
Tc(MIN.) = 7.74

*****
FLOW PROCESS FROM NODE 668.00 TO NODE 665.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 800.00 DOWNSTREAM(FEET) = 790.00
FLOW LENGTH(FEET) = 260.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.93
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.70
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 8.29
LONGEST FLOWPATH FROM NODE 666.00 TO NODE 665.00 = 831.00 FEET.

*****
FLOW PROCESS FROM NODE 665.00 TO NODE 665.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.29
RAINFALL INTENSITY(INCH/HR) = 5.89
TOTAL STREAM AREA(ACRES) = 1.07
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.70

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 12.73 11.39 4.802 5.07
2 2.70 8.29 5.894 1.07

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 13.07 8.29 5.894
2 14.93 11.39 4.802

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 14.93 Tc(MIN.) = 11.39
TOTAL AREA(ACRES) = 6.1
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 665.00 = 1076.00 FEET.

*****
FLOW PROCESS FROM NODE 665.00 TO NODE 671.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 784.50 DOWNSTREAM(FEET) = 760.00
FLOW LENGTH(FEET) = 464.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.10
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.93
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 11.94
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 671.00 = 1540.00 FEET.

*****
FLOW PROCESS FROM NODE 671.00 TO NODE 671.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.94
RAINFALL INTENSITY(INCH/HR) = 4.66
TOTAL STREAM AREA(ACRES) = 6.14
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.93

*****
FLOW PROCESS FROM NODE 669.00 TO NODE 670.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .7900
S.C.S. CURVE NUMBER (AMC II) = 0

```



```

INITIAL SUBAREA FLOW-LENGTH(FEET) = 85.00
UPSTREAM ELEVATION(FEET) = 812.00
DOWNSTREAM ELEVATION(FEET) = 807.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.850
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.77
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.77

*****
FLOW PROCESS FROM NODE 670.00 TO NODE 671.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
-----
UPSTREAM ELEVATION(FEET) = 806.00 DOWNSTREAM ELEVATION(FEET) = 765.00
STREET LENGTH(FEET) = 685.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.35
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.56
AVERAGE FLOW VELOCITY(FT*FT/SEC.) = 5.11
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.52
STREET FLOW TRAVEL TIME(MIN.) = 2.24 Tc(MIN.) = 5.09
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.079
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .7900
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.790
SUBAREA AREA(ACRES) = 1.12 SUBAREA RUNOFF(CFS) = 7.15
TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 7.91

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.14
FLOW VELOCITY(FT/SEC.) = 5.82 DEPTH*VELOCITY(FT*FT/SEC.) = 2.03
LONGEST FLOWPATH FROM NODE 669.00 TO NODE 671.00 = 770.00 FEET.

*****
FLOW PROCESS FROM NODE 271.00 TO NODE 271.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.09
RAINFALL INTENSITY(INCH/HR) = 8.08
TOTAL STREAM AREA(ACRES) = 1.24
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.91

*****
FLOW PROCESS FROM NODE 673.00 TO NODE 674.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 85.00
UPSTREAM ELEVATION(FEET) = 790.00
DOWNSTREAM ELEVATION(FEET) = 780.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.315
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.852
SUBAREA RUNOFF(CFS) = 0.45
TOTAL AREA(ACRES) = 0.14 TOTAL RUNOFF(CFS) = 0.45

*****
FLOW PROCESS FROM NODE 674.00 TO NODE 675.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 790.00 DOWNSTREAM(FEET) = 765.00
FLOW LENGTH(FEET) = 285.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.5 INCHES
PIPE-FLOW VELOCITY(FT/SEC.) = 6.20
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.45
PIPE TRAVEL TIME(MIN.) = 0.77 Tc(MIN.) = 6.08
LONGEST FLOWPATH FROM NODE 673.00 TO NODE 675.00 = 370.00 FEET.

*****
FLOW PROCESS FROM NODE 674.00 TO NODE 675.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.198
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100

S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 1.40 SUBAREA RUNOFF(CFS) = 4.13
TOTAL AREA(ACRES) = 1.5 TOTAL RUNOFF(CFS) = 4.54
TC(MIN.) = 6.08

*****
FLOW PROCESS FROM NODE 675.00 TO NODE 671.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 760.00 DOWNSTREAM(FEET) = 759.00
FLOW LENGTH(FEET) = 60.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.3 INCHES
PIPE-FLOW VELOCITY(FT/SEC.) = 6.78
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.54
PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 6.23
LONGEST FLOWPATH FROM NODE 673.00 TO NODE 671.00 = 430.00 FEET.

*****
FLOW PROCESS FROM NODE 671.00 TO NODE 671.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 6.23
RAINFALL INTENSITY(INCH/HR) = 7.09
TOTAL STREAM AREA(ACRES) = 1.54
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.54

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 14.93 11.94 4.659 6.14
2 7.91 5.09 8.079 1.24
3 4.54 6.23 7.088 1.54

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 20.24 5.09 8.079
2 21.30 6.23 7.088
3 22.48 11.94 4.659

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 22.48 Tc(MIN.) = 11.94
TOTAL AREA(ACRES) = 8.9
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 671.00 = 1540.00 FEET.

*****
FLOW PROCESS FROM NODE 671.00 TO NODE 654.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 765.00 DOWNSTREAM(FEET) = 764.00
FLOW LENGTH(FEET) = 66.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.7 INCHES
PIPE-FLOW VELOCITY(FT/SEC.) = 9.66
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 22.48
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 12.05
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 654.00 = 1606.00 FEET.

*****
FLOW PROCESS FROM NODE 654.00 TO NODE 654.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 22.48 12.05 4.630 8.92
LONGEST FLOWPATH FROM NODE 657.00 TO NODE 654.00 = 1606.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 88.16 13.83 4.237 35.95
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 654.00 = 2865.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 99.31 12.05 4.630
2 108.74 13.83 4.237

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 108.74 Tc(MIN.) = 13.83
TOTAL AREA(ACRES) = 44.9

*****
FLOW PROCESS FROM NODE 654.00 TO NODE 654.00 IS CODE = 12
-----

```



```

>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====
*****
***** FLOW PROCESS FROM NODE 681.00 TO NODE 681.00 IS CODE = 1
*****
FLOW PROCESS FROM NODE 654.00 TO NODE 681.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 765.00 DOWNSTREAM(FEET) = 742.00
FLOW LENGTH(FEET) = 578.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 25.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.55
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 108.74
PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 14.30
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 681.00 = 3443.00 FEET.
*****
***** FLOW PROCESS FROM NODE 681.00 TO NODE 681.00 IS CODE = 1
*****
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.30
RAINFALL INTENSITY(INCH/HR) = 4.15
TOTAL STREAM AREA(ACRES) = 44.87
PEAK FLOW RATE(CFS) AT CONFLUENCE = 108.74
*****
***** FLOW PROCESS FROM NODE 678.00 TO NODE 679.00 IS CODE = 21
*****
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 795.35
DOWNSTREAM ELEVATION(FEET) = 794.65
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.59
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.59
*****
***** FLOW PROCESS FROM NODE 679.00 TO NODE 680.00 IS CODE = 61
*****
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 794.00 DOWNSTREAM ELEVATION(FEET) = 744.00
STREET LENGTH(FEET) = 1376.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.10
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.35
HALFSTREET FLOOD WIDTH(FEET) = 11.07
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.50
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.56
STREET FLOW TRAVEL TIME(MIN.) = 5.10 Tc(MIN.) = 13.83
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.237
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 10.28 SUBAREA RUNOFF(CFS) = 22.65
TOTAL AREA(ACRES) = 10.5 PEAK FLOW RATE(CFS) = 23.09
END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.41 HALFSTREET FLOOD WIDTH(FEET) = 14.38
FLOW VELOCITY(FEET/SEC.) = 5.28 DEPTH*VELOCITY(FT*FT/SEC.) = 2.19
LONGEST FLOWPATH FROM NODE 678.00 TO NODE 680.00 = 1446.00 FEET.
*****
***** FLOW PROCESS FROM NODE 680.00 TO NODE 681.00 IS CODE = 31
*****
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 744.00 DOWNSTREAM(FEET) = 742.00
FLOW LENGTH(FEET) = 184.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.65
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 23.09
PIPE TRAVEL TIME(MIN.) = 0.35 Tc(MIN.) = 14.19
*****
***** FLOW PROCESS FROM NODE 681.00 TO NODE 681.00 IS CODE = 1
*****
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.19
RAINFALL INTENSITY(INCH/HR) = 4.17
TOTAL STREAM AREA(ACRES) = 10.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 23.09
** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 108.74 14.30 4.147 44.87
2 23.09 14.19 4.169 10.48
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 130.96 14.19 4.169
2 131.71 14.30 4.147
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 131.71 Tc(MIN.) = 14.30
TOTAL AREA(ACRES) = 55.3
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 681.00 = 3443.00 FEET.
*****
***** FLOW PROCESS FROM NODE 681.00 TO NODE 686.00 IS CODE = 31
*****
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 742.00 DOWNSTREAM(FEET) = 727.00
FLOW LENGTH(FEET) = 500.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 30.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.11
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 131.71
PIPE TRAVEL TIME(MIN.) = 0.44 Tc(MIN.) = 14.74
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 686.00 = 3943.00 FEET.
*****
***** FLOW PROCESS FROM NODE 686.00 TO NODE 686.00 IS CODE = 1
*****
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.74
RAINFALL INTENSITY(INCH/HR) = 4.07
TOTAL STREAM AREA(ACRES) = 55.35
PEAK FLOW RATE(CFS) AT CONFLUENCE = 131.71
*****
***** FLOW PROCESS FROM NODE 684.00 TO NODE 685.00 IS CODE = 21
*****
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 765.00
DOWNSTREAM ELEVATION(FEET) = 763.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.653
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.67
TOTAL AREA(ACRES) = 0.24 TOTAL RUNOFF(CFS) = 1.67
*****
***** FLOW PROCESS FROM NODE 685.00 TO NODE 686.00 IS CODE = 61
*****
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 763.00 DOWNSTREAM ELEVATION(FEET) = 727.00
STREET LENGTH(FEET) = 648.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.75
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.31
HALFSTREET FLOOD WIDTH(FEET) = 9.18

```

```

    AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.08
    PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.57
    STREET FLOW TRAVEL TIME(MIN.) = 2.13 Tc(MIN.) = 4.78
    100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
    NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
    *USER SPECIFIED(SUBAREA):
    LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
    S.C.S. CURVE NUMBER (AMC II) = 0
    AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
    SUBAREA AREA(ACRES) = 2.33 SUBAREA RUNOFF(CFS) = 16.18
    TOTAL AREA(ACRES) = 2.6 PEAK FLOW RATE(CFS) = 17.84

    END OF SUBAREA STREET FLOW HYDRAULICS:
    DEPTH(FEET) = 0.36 HALFSTREET FLOOD WIDTH(FEET) = 11.92
    FLOW VELOCITY(FEET/SEC.) = 5.80 DEPTH*VELOCITY(FT*FT/SEC.) = 2.11
    LONGEST FLOWPATH FROM NODE 684.00 TO NODE 686.00 = 718.00 FEET.

*****
FLOW PROCESS FROM NODE 686.00 TO NODE 686.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.78
RAINFALL INTENSITY(INCH/HR) = 8.17
TOTAL STREAM AREA(ACRES) = 2.57
PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.84

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 131.71 14.74 4.067 55.35
2 17.84 4.78 8.168 2.57

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 83.43 4.78 8.168
2 140.59 14.74 4.067

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 140.59 Tc(MIN.) = 14.74
TOTAL AREA(ACRES) = 57.9
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 686.00 = 3943.00 FEET.

*****
FLOW PROCESS FROM NODE 686.00 TO NODE 691.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 727.00 DOWNSTREAM(FEET) = 709.00
FLOW LENGTH(FEET) = 970.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 45.0 INCH PIPE IS 32.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.39
ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 140.59
PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 15.72
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 691.00 = 4913.00 FEET.

*****
FLOW PROCESS FROM NODE 691.00 TO NODE 691.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.72
RAINFALL INTENSITY(INCH/HR) = 3.90
TOTAL STREAM AREA(ACRES) = 57.92
PEAK FLOW RATE(CFS) AT CONFLUENCE = 140.59

*****
FLOW PROCESS FROM NODE 689.00 TO NODE 690.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 726.00
DOWNSTREAM ELEVATION(FEET) = 724.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.653
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.04
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 1.04

*****
FLOW PROCESS FROM NODE 690.00 TO NODE 691.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 724.00 DOWNSTREAM ELEVATION(FEET) = 709.00
STREET LENGTH(FEET) = 970.00 CURB HEIGHT(INCHES) = 6.0

STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.51
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.32
HALFSTREET FLOOD WIDTH(FEET) = 9.46
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.72
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.86
STREET FLOW TRAVEL TIME(MIN.) = 5.94 Tc(MIN.) = 8.60
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.759
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 1.78 SUBAREA RUNOFF(CFS) = 8.71
TOTAL AREA(ACRES) = 1.9 PEAK FLOW RATE(CFS) = 9.45

    END OF SUBAREA STREET FLOW HYDRAULICS:
    DEPTH(FEET) = 0.36 HALFSTREET FLOOD WIDTH(FEET) = 11.92
    FLOW VELOCITY(FEET/SEC.) = 3.07 DEPTH*VELOCITY(FT*FT/SEC.) = 1.12
    LONGEST FLOWPATH FROM NODE 689.00 TO NODE 691.00 = 1040.00 FEET.

*****
FLOW PROCESS FROM NODE 691.00 TO NODE 691.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.60
RAINFALL INTENSITY(INCH/HR) = 5.76
TOTAL STREAM AREA(ACRES) = 1.93
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.45

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 140.59 15.72 3.901 57.92
2 9.45 8.60 5.759 1.93

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 104.68 8.60 5.759
2 146.99 15.72 3.901

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 146.99 Tc(MIN.) = 15.72
TOTAL AREA(ACRES) = 59.8
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 691.00 = 4913.00 FEET.

*****
FLOW PROCESS FROM NODE 691.00 TO NODE 697.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 709.00 DOWNSTREAM(FEET) = 699.00
FLOW LENGTH(FEET) = 925.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 36.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.56
ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 146.99
PIPE TRAVEL TIME(MIN.) = 1.14 Tc(MIN.) = 16.86
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 697.00 = 5838.00 FEET.

*****
FLOW PROCESS FROM NODE 697.00 TO NODE 697.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 16.86
RAINFALL INTENSITY(INCH/HR) = 3.73
TOTAL STREAM AREA(ACRES) = 59.85
PEAK FLOW RATE(CFS) AT CONFLUENCE = 146.99

*****
FLOW PROCESS FROM NODE 694.00 TO NODE 695.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 721.00
DOWNSTREAM ELEVATION(FEET) = 710.00
ELEVATION DIFFERENCE(FEET) = 11.00

```



```

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.765
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION#
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.451
SUBAREA RUNOFF(CFS) = 0.86
TOTAL AREA(ACRES) = 0.28 TOTAL RUNOFF(CFS) = 0.86

*****
FLOW PROCESS FROM NODE 695.00 TO NODE 696.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 708.00 DOWNSTREAM(FEET) = 703.00
FLOW LENGTH(FEET) = 266.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.40
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.86
PIPE TRAVEL TIME(MIN.) = 1.01 Tc(MIN.) = 6.77
LONGEST FLOWPATH FROM NODE 694.00 TO NODE 696.00 = 366.00 FEET.

*****
FLOW PROCESS FROM NODE 695.00 TO NODE 696.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.715
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 2.28 SUBAREA RUNOFF(CFS) = 6.28
TOTAL AREA(ACRES) = 2.6 TOTAL RUNOFF(CFS) = 7.05
Tc(MIN.) = 6.77

*****
FLOW PROCESS FROM NODE 696.00 TO NODE 697.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 703.00 DOWNSTREAM(FEET) = 699.00
FLOW LENGTH(FEET) = 74.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.73
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.05
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 6.88
LONGEST FLOWPATH FROM NODE 694.00 TO NODE 697.00 = 440.00 FEET.

*****
FLOW PROCESS FROM NODE 697.00 TO NODE 697.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 6.88
RAINFALL INTENSITY(INCH/HR) = 6.65
TOTAL STREAM AREA(ACRES) = 2.56
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.05

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 146.99 16.86 3.729 59.85
2 7.05 6.88 6.649 2.56

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 89.50 6.88 6.649
2 150.95 16.86 3.729

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 150.95 Tc(MIN.) = 16.86
TOTAL AREA(ACRES) = 62.4
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 697.00 = 5838.00 FEET.

*****
FLOW PROCESS FROM NODE 697.00 TO NODE 702.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 699.00 DOWNSTREAM(FEET) = 698.00
FLOW LENGTH(FEET) = 131.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 40.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.92
ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 150.95
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 17.04
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 702.00 = 5969.00 FEET.

*****
FLOW PROCESS FROM NODE 702.00 TO NODE 702.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.04
RAINFALL INTENSITY(INCH/HR) = 3.70
TOTAL STREAM AREA(ACRES) = 62.41
PEAK FLOW RATE(CFS) AT CONFLUENCE = 150.95

*****
FLOW PROCESS FROM NODE 700.00 TO NODE 701.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 709.00
DOWNSTREAM ELEVATION(FEET) = 708.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.204
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 64.29
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.04
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 1.04

*****
FLOW PROCESS FROM NODE 701.00 TO NODE 702.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(FEET) = 708.00 DOWNSTREAM ELEVATION(FEET) = 698.00
STREET LENGTH(FEET) = 965.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.00
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.32
HALFSTREET FLOOD WIDTH(FEET) = 9.88
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.29
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.74
STREET FLOW TRAVEL TIME(MIN.) = 7.04 Tc(MIN.) = 10.24
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.143
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 1.75 SUBAREA RUNOFF(CFS) = 7.65
TOTAL AREA(ACRES) = 1.9 PEAK FLOW RATE(CFS) = 8.31

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.37 HALFSTREET FLOOD WIDTH(FEET) = 12.27
FLOW VELOCITY(FEET/SEC.) = 2.56 DEPTH*VELOCITY(FT*FT/SEC.) = 0.95
LONGEST FLOWPATH FROM NODE 700.00 TO NODE 702.00 = 1035.00 FEET.

*****
FLOW PROCESS FROM NODE 702.00 TO NODE 702.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.24
RAINFALL INTENSITY(INCH/HR) = 5.14
TOTAL STREAM AREA(ACRES) = 1.90
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.31

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 150.95 17.04 3.703 62.41
2 8.31 10.24 5.143 1.90

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 117.00 10.24 5.143
2 156.93 17.04 3.703

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 156.93 Tc(MIN.) = 17.04
TOTAL AREA(ACRES) = 64.3
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 702.00 = 5969.00 FEET.

```

```

***** PIPE-FLOW VELOCITY(FEET/SEC.) = 3.29
FLOW PROCESS FROM NODE 702.00 TO NODE 533.00 IS CODE = 31 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 698.00 DOWNSTREAM(FEET) = 696.00
FLOW LENGTH(FEET) = 53.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 22.11
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 156.93
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 17.08
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 533.00 = 6022.00 FEET.
*****
FLOW PROCESS FROM NODE 533.00 TO NODE 533.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<
-----
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 156.93 17.08 3.698 64.31
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 533.00 = 6022.00 FEET.
** MEMORY BANK # 2 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 227.40 13.48 4.308 115.58
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 533.00 = 7272.00 FEET.
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/ HOUR)
1 351.23 13.48 4.308
2 352.11 17.08 3.698
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 352.11 Tc(MIN.) = 17.08
TOTAL AREA(ACRES) = 179.9
*****
FLOW PROCESS FROM NODE 533.00 TO NODE 533.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 2 <<<<
-----
*****
FLOW PROCESS FROM NODE 533.00 TO NODE 708.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 697.00 DOWNSTREAM(FEET) = 686.50
FLOW LENGTH(FEET) = 278.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 41.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 26.64
ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 352.11
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 17.26
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 708.00 = 7550.00 FEET.
*****
FLOW PROCESS FROM NODE 708.00 TO NODE 708.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.26
RAINFALL INTENSITY(INCH/HR) = 3.67
TOTAL STREAM AREA(ACRES) = 179.89
PEAK FLOW RATE(CFS) AT CONFLUENCE = 352.11
*****
FLOW PROCESS FROM NODE 705.00 TO NODE 706.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 704.00
DOWNSTREAM ELEVATION(FEET) = 702.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.323
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.386
SUBAREA RUNOFF(CFS) = 0.52
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.52
*****
FLOW PROCESS FROM NODE 706.00 TO NODE 707.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 702.00 DOWNSTREAM(FEET) = 695.00
FLOW LENGTH(FEET) = 560.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES
*****
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.29
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.52
PIPE TRAVEL TIME(MIN.) = 2.84 Tc(MIN.) = 10.16
LONGEST FLOWPATH FROM NODE 705.00 TO NODE 707.00 = 630.00 FEET.
*****
FLOW PROCESS FROM NODE 706.00 TO NODE 707.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.170
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 6.80 SUBAREA RUNOFF(CFS) = 14.41
TOTAL AREA(ACRES) = 7.0 TOTAL RUNOFF(CFS) = 14.84
Tc(MIN.) = 10.16
*****
FLOW PROCESS FROM NODE 707.00 TO NODE 708.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 695.00 DOWNSTREAM(FEET) = 686.00
FLOW LENGTH(FEET) = 132.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.52
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.84
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 10.30
LONGEST FLOWPATH FROM NODE 705.00 TO NODE 708.00 = 762.00 FEET.
*****
FLOW PROCESS FROM NODE 708.00 TO NODE 708.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.30
RAINFALL INTENSITY(INCH/HR) = 5.12
TOTAL STREAM AREA(ACRES) = 7.00
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.84
*****
** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 352.11 17.26 3.674 179.89
2 14.84 10.30 5.124 7.00
RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.
** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/ HOUR)
1 225.02 10.30 5.124
2 362.75 17.26 3.674
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 362.75 Tc(MIN.) = 17.26
TOTAL AREA(ACRES) = 186.9
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 708.00 = 7550.00 FEET.
*****
FLOW PROCESS FROM NODE 708.00 TO NODE 709.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 686.00 DOWNSTREAM(FEET) = 650.00
FLOW LENGTH(FEET) = 218.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 31.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 46.94
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 362.75
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 17.33
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 709.00 = 7768.00 FEET.
*****
FLOW PROCESS FROM NODE 709.00 TO NODE 709.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.33
RAINFALL INTENSITY(INCH/HR) = 3.66
TOTAL STREAM AREA(ACRES) = 186.89
PEAK FLOW RATE(CFS) AT CONFLUENCE = 362.75
*****
FLOW PROCESS FROM NODE 711.00 TO NODE 712.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500

```



```

S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 665.00
DOWNSTREAM ELEVATION(FEET) = 660.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.695
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 95.00
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.185
SUBAREA RUNOFF(CFS) = 0.32
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 0.32

*****
FLOW PROCESS FROM NODE 712.00 TO NODE 709.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 765.00 DOWNSTREAM(FEET) = 755.00
FLOW LENGTH(FEET) = 100.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.84
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.32
PIPE TRAVEL TIME(MIN.) = 0.29 Tc(MIN.) = 7.98
LONGEST FLOWPATH FROM NODE 711.00 TO NODE 709.00 = 200.00 FEET.

*****
FLOW PROCESS FROM NODE 712.00 TO NODE 709.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.041
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 5.79 SUBAREA RUNOFF(CFS) = 12.24
TOTAL AREA(ACRES) = 5.9 TOTAL RUNOFF(CFS) = 12.56
TC(MIN.) = 7.98

*****
FLOW PROCESS FROM NODE 709.00 TO NODE 709.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.98
RAINFALL INTENSITY(INCH/HR) = 6.04
TOTAL STREAM AREA(ACRES) = 5.94
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.56

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 362.75 17.33 3.663 186.89
2 12.56 7.98 6.041 5.94

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 232.52 7.98 6.041
2 370.37 17.33 3.663

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 370.37 Tc(MIN.) = 17.33
TOTAL AREA(ACRES) = 192.8
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 709.00 = 7768.00 FEET.
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 192.8 TC(MIN.) = 17.33
PEAK FLOW RATE(CFS) = 370.37
=====
END OF RATIONAL METHOD ANALYSIS

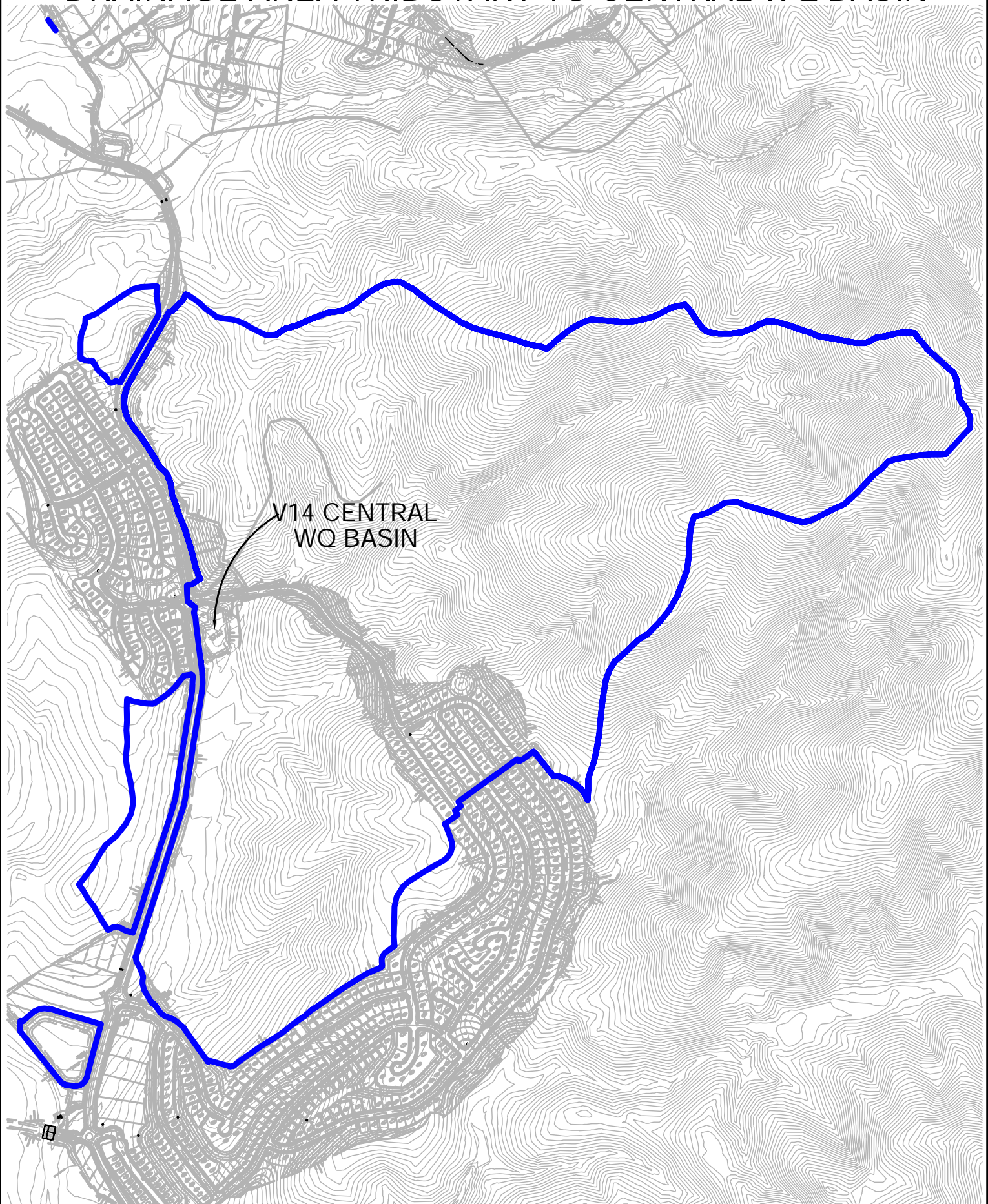
```

CHAPTER 5

5.1.2 – Rational Method Hydrologic Analysis (AES 2015)

Drainage Area Tributary to V14 Central WQ Basin

OTAY RANCH VILLAGE 14 & PA 16/19 DRAINAGE AREA TRIBUTARY TO CENTRAL WQ BASIN



```

*****
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1239

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* V14 Drainage Area tributary to V14 Central Basin and East Offsite
*
*
*****
FILE NAME: R:\1235\HYD\CALCS\AES\SRP\V14E.DAT
TIME/DATE OF STUDY: 13:51 11/03/2016
-----
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
-----
2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.100
SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
  HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
  WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
== =====
1 16.0 8.0 0.020/0.020/0.020 0.50 2.00 0.0313 0.125 0.0150
2 12.0 6.0 0.020/0.020/0.020 0.50 1.50 0.0313 0.125 0.0130

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*****
FLOW PROCESS FROM NODE 750.00 TO NODE 751.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 999.05
DOWNSTREAM ELEVATION(FEET) = 998.35
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.53
TOTAL AREA(ACRES) = 0.18 TOTAL RUNOFF(CFS) = 0.53

*****
FLOW PROCESS FROM NODE 751.00 TO NODE 752.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 998.00 DOWNSTREAM ELEVATION(FEET) = 932.50
STREET LENGTH(FEET) = 775.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.75
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.25
HALFSTREET FLOOD WIDTH(FEET) = 6.33
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.30
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.34
STREET FLOW TRAVEL TIME(MIN.) = 2.44 Tc(MIN.) = 11.17
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.863
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 1.75 SUBAREA RUNOFF(CFS) = 4.42
TOTAL AREA(ACRES) = 1.9 PEAK FLOW RATE(CFS) = 4.88

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.29 HALFSTREET FLOOD WIDTH(FEET) = 8.38
FLOW VELOCITY(FEET/SEC.) = 5.94 DEPTH*VELOCITY(FT*FT/SEC.) = 1.75
LONGEST FLOWPATH FROM NODE 750.00 TO NODE 752.00 = 845.00 FEET.

*****
FLOW PROCESS FROM NODE 752.00 TO NODE 753.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 937.50 DOWNSTREAM(FEET) = 932.50
FLOW LENGTH(FEET) = 264.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.24
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.88
PIPE TRAVEL TIME(MIN.) = 0.61 Tc(MIN.) = 11.78
LONGEST FLOWPATH FROM NODE 750.00 TO NODE 763.00 = 1109.00 FEET.

*****
FLOW PROCESS FROM NODE 763.00 TO NODE 763.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.78
RAINFALL INTENSITY(INCH/HR) = 4.70
TOTAL STREAM AREA(ACRES) = 1.93
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.88

*****
FLOW PROCESS FROM NODE 760.00 TO NODE 761.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 998.00
DOWNSTREAM ELEVATION(FEET) = 992.00
ELEVATION DIFFERENCE(FEET) = 6.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.746
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.467
SUBAREA RUNOFF(CFS) = 0.47
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.47

*****
FLOW PROCESS FROM NODE 761.00 TO NODE 762.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 992.00 DOWNSTREAM ELEVATION(FEET) = 937.00
STREET LENGTH(FEET) = 1024.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.68
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.33
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.73
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.39
STREET FLOW TRAVEL TIME(MIN.) = 3.61 Tc(MIN.) = 9.35
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.453
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.01 SUBAREA RUNOFF(CFS) = 14.21
TOTAL AREA(ACRES) = 5.1 PEAK FLOW RATE(CFS) = 14.55

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.00
FLOW VELOCITY(FEET/SEC.) = 5.47 DEPTH*VELOCITY(FT*FT/SEC.) = 1.90
LONGEST FLOWPATH FROM NODE 760.00 TO NODE 762.00 = 1124.00 FEET.

*****
FLOW PROCESS FROM NODE 762.00 TO NODE 763.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 937.50 DOWNSTREAM(FEET) = 937.00
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.05
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.55
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 9.41
LONGEST FLOWPATH FROM NODE 760.00 TO NODE 763.00 = 1154.00 FEET.

*****
FLOW PROCESS FROM NODE 763.00 TO NODE 763.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====

```



```

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.41
RAINFALL INTENSITY(INCH/HR) = 5.43
TOTAL STREAM AREA(ACRES) = 5.13
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.55

*****
FLOW PROCESS FROM NODE 755.00 TO NODE 756.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 999.75
DOWNSTREAM ELEVATION(FEET) = 999.05
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.59
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.59

*****
FLOW PROCESS FROM NODE 756.00 TO NODE 757.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(FEET) = 999.00 DOWNSTREAM ELEVATION(FEET) = 937.00
STREET LENGTH(FEET) = 1125.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.30
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.74
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.88
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.47
STREET FLOW TRAVEL TIME(MIN.) = 3.84 Tc(MIN.) = 12.57
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.506
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.14 SUBAREA RUNOFF(CFS) = 7.36
TOTAL AREA(ACRES) = 3.3 PEAK FLOW RATE(CFS) = 7.83

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.29
FLOW VELOCITY(FEET/SEC.) = 5.62 DEPTH*VELOCITY(FT*FT/SEC.) = 1.98
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 757.00 = 1195.00 FEET.

*****
FLOW PROCESS FROM NODE 757.00 TO NODE 763.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 937.50 DOWNSTREAM(FEET) = 937.00
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.79
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.83
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 12.64
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 763.00 = 1225.00 FEET.

*****
FLOW PROCESS FROM NODE 763.00 TO NODE 763.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 12.64
RAINFALL INTENSITY(INCH/HR) = 4.49
TOTAL STREAM AREA(ACRES) = 3.34
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.83

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 4.88 11.78 4.699 1.93
2 14.55 9.41 5.433 5.13
3 7.83 12.64 4.491 3.34

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)

1 24.27 9.41 5.433
2 24.76 11.78 4.699
3 24.52 12.64 4.491

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 24.76 Tc(MIN.) = 11.78
TOTAL AREA(ACRES) = 10.4
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 763.00 = 1225.00 FEET.

*****
FLOW PROCESS FROM NODE 763.00 TO NODE 764.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 932.00 DOWNSTREAM(FEET) = 931.00
FLOW LENGTH(FEET) = 99.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.51
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 24.76
PIPE TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) = 11.97
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 764.00 = 1324.00 FEET.

*****
FLOW PROCESS FROM NODE 764.00 TO NODE 764.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
-----
FLOW PROCESS FROM NODE 767.00 TO NODE 768.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 992.65
DOWNSTREAM ELEVATION(FEET) = 991.95
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.65
TOTAL AREA(ACRES) = 0.22 TOTAL RUNOFF(CFS) = 0.65

*****
FLOW PROCESS FROM NODE 768.00 TO NODE 769.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(FEET) = 991.00 DOWNSTREAM ELEVATION(FEET) = 943.00
STREET LENGTH(FEET) = 679.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.97
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.45
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.18
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.43
STREET FLOW TRAVEL TIME(MIN.) = 2.18 Tc(MIN.) = 10.92
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.936
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.91 SUBAREA RUNOFF(CFS) = 12.60
TOTAL AREA(ACRES) = 5.1 PEAK FLOW RATE(CFS) = 13.17

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.33 HALFSTREET FLOOD WIDTH(FEET) = 9.95
FLOW VELOCITY(FEET/SEC.) = 5.94 DEPTH*VELOCITY(FT*FT/SEC.) = 1.93
LONGEST FLOWPATH FROM NODE 767.00 TO NODE 769.00 = 749.00 FEET.

*****
FLOW PROCESS FROM NODE 769.00 TO NODE 769.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.92
RAINFALL INTENSITY(INCH/HR) = 4.94
TOTAL STREAM AREA(ACRES) = 5.13
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.17

*****
FLOW PROCESS FROM NODE 772.00 TO NODE 773.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4100

```

```

S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 955.00
DOWNSTREAM ELEVATION(FEET) = 949.00
ELEVATION DIFFERENCE(FEET) = 6.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.835
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.676
SUBAREA RUNOFF(CFS) = 0.49
TOTAL AREA(ACRES) = 0.18 TOTAL RUNOFF(CFS) = 0.49
*****
FLOW PROCESS FROM NODE 773.00 TO NODE 774.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 449.00 DOWNSTREAM(FEET) = 448.00
FLOW LENGTH(FEET) = 165.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 2.52
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.49
PIPE TRAVEL TIME(MIN.) = 1.09 Tc(MIN.) = 7.93
LONGEST FLOWPATH FROM NODE 772.00 TO NODE 774.00 = 265.00 FEET.
*****
FLOW PROCESS FROM NODE 773.00 TO NODE 774.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.067
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 0.44 SUBAREA RUNOFF(CFS) = 1.09
TOTAL AREA(ACRES) = 0.6 TOTAL RUNOFF(CFS) = 1.54
TC(MIN.) = 7.93
*****
FLOW PROCESS FROM NODE 774.00 TO NODE 769.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 948.00 DOWNSTREAM(FEET) = 943.00
FLOW LENGTH(FEET) = 55.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.12
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.54
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 8.03
LONGEST FLOWPATH FROM NODE 772.00 TO NODE 769.00 = 320.00 FEET.
*****
FLOW PROCESS FROM NODE 769.00 TO NODE 769.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.03
RAINFALL INTENSITY(INCH/HR) = 6.02
TOTAL STREAM AREA(ACRES) = 0.62
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.54

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 13.17 10.92 4.936 5.13
2 1.54 8.03 6.018 0.62

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 11.22 8.03 6.018
2 14.43 10.92 4.936

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 14.43 Tc(MIN.) = 10.92
TOTAL AREA(ACRES) = 5.7
LONGEST FLOWPATH FROM NODE 767.00 TO NODE 769.00 = 749.00 FEET.
*****
FLOW PROCESS FROM NODE 769.00 TO NODE 764.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 937.00 DOWNSTREAM(FEET) = 933.00
FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.97
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.43
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 11.01
LONGEST FLOWPATH FROM NODE 767.00 TO NODE 764.00 = 825.00 FEET.
*****
*****
FLOW PROCESS FROM NODE 764.00 TO NODE 764.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 14.43 11.01 4.909 5.75
LONGEST FLOWPATH FROM NODE 767.00 TO NODE 764.00 = 825.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 24.76 11.97 4.650 10.40
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 764.00 = 1324.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 37.19 11.01 4.909
2 38.43 11.97 4.650

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 38.43 Tc(MIN.) = 11.97
TOTAL AREA(ACRES) = 16.1
*****
FLOW PROCESS FROM NODE 764.00 TO NODE 764.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====
*****
FLOW PROCESS FROM NODE 764.00 TO NODE 775.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 934.00 DOWNSTREAM(FEET) = 918.00
FLOW LENGTH(FEET) = 199.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.54
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 38.43
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 12.14
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 775.00 = 1523.00 FEET.
*****
FLOW PROCESS FROM NODE 775.00 TO NODE 775.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.14
RAINFALL INTENSITY(INCH/HR) = 4.61
TOTAL STREAM AREA(ACRES) = 16.15
PEAK FLOW RATE(CFS) AT CONFLUENCE = 38.43
*****
FLOW PROCESS FROM NODE 778.00 TO NODE 779.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 970.00
DOWNSTREAM ELEVATION(FEET) = 960.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.765
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.451
SUBAREA RUNOFF(CFS) = 0.37
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.37
*****
FLOW PROCESS FROM NODE 779.00 TO NODE 780.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 960.00 DOWNSTREAM(FEET) = 930.00
FLOW LENGTH(FEET) = 639.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.73
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.37
PIPE TRAVEL TIME(MIN.) = 2.25 Tc(MIN.) = 8.02
LONGEST FLOWPATH FROM NODE 778.00 TO NODE 780.00 = 739.00 FEET.
*****
FLOW PROCESS FROM NODE 779.00 TO NODE 780.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.024
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100

```



```

SUBAREA AREA(ACRES) = 1.55 SUBAREA RUNOFF(CFS) = 3.83 STREET HALFWIDTH(FEET) = 18.00
TOTAL AREA(ACRES) = 1.7 TOTAL RUNOFF(CFS) = 4.12
TC(MIN.) = 8.02
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

*****
FLOW PROCESS FROM NODE 780.00 TO NODE 775.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 925.00 DOWNSTREAM(FEET) = 920.00
FLOW LENGTH(FEET) = 40.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.60
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.12
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 8.07
LONGEST FLOWPATH FROM NODE 778.00 TO NODE 775.00 = 779.00 FEET.

*****
FLOW PROCESS FROM NODE 775.00 TO NODE 775.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.07
RAINFALL INTENSITY(INCH/HR) = 6.00
TOTAL STREAM AREA(ACRES) = 1.67
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.12

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 38.43 12.14 4.610 16.15
2 4.12 8.07 6.000 1.67

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 29.66 8.07 6.000 1.67
2 41.60 12.14 4.610 1.67

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 41.60 Tc(MIN.) = 12.14
TOTAL AREA(ACRES) = 17.8
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 775.00 = 1523.00 FEET.

*****
FLOW PROCESS FROM NODE 775.00 TO NODE 785.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 918.00 DOWNSTREAM(FEET) = 858.00
FLOW LENGTH(FEET) = 648.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 22.06
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 41.60
PIPE TRAVEL TIME(MIN.) = 0.49 Tc(MIN.) = 12.63
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 785.00 = 2171.00 FEET.

*****
FLOW PROCESS FROM NODE 785.00 TO NODE 785.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.63
RAINFALL INTENSITY(INCH/HR) = 4.49
TOTAL STREAM AREA(ACRES) = 17.82
PEAK FLOW RATE(CFS) AT CONFLUENCE = 41.60

*****
FLOW PROCESS FROM NODE 783.00 TO NODE 784.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4600
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 941.55
DOWNSTREAM ELEVATION(FEET) = 940.85
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.638
100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.349
SUBAREA RUNOFF(CFS) = 0.62
TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.62

*****
FLOW PROCESS FROM NODE 784.00 TO NODE 785.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 940.00 DOWNSTREAM ELEVATION(FEET) = 863.00
STREET LENGTH(FEET) = 880.00 CURB HEIGHT(INCHES) = 6.0

*****
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.21
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.19
HALFSTREET FLOOD WIDTH(FEET) = 3.40
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.71
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.92
STREET FLOW TRAVEL TIME(MIN.) = 3.11 Tc(MIN.) = 12.75
100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.466
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4600
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.460
SUBAREA AREA(ACRES) = 1.55 SUBAREA RUNOFF(CFS) = 3.18
TOTAL AREA(ACRES) = 1.8 PEAK FLOW RATE(CFS) = 3.70

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.23 HALFSTREET FLOOD WIDTH(FEET) = 5.04
FLOW VELOCITY(FEET/SEC.) = 4.96 DEPTH*VELOCITY(FT*FT/SEC.) = 1.13
LONGEST FLOWPATH FROM NODE 783.00 TO NODE 785.00 = 950.00 FEET.

*****
FLOW PROCESS FROM NODE 785.00 TO NODE 785.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.75
RAINFALL INTENSITY(INCH/HR) = 4.47
TOTAL STREAM AREA(ACRES) = 1.80
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.70

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 41.60 12.63 4.494 17.82
2 3.70 12.75 4.466 1.80

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 45.26 12.63 4.494
2 45.03 12.75 4.466

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 45.26 Tc(MIN.) = 12.63
TOTAL AREA(ACRES) = 19.6
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 785.00 = 2171.00 FEET.

*****
FLOW PROCESS FROM NODE 785.00 TO NODE 786.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 858.00 DOWNSTREAM(FEET) = 767.00
FLOW LENGTH(FEET) = 1269.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.71
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 45.26
PIPE TRAVEL TIME(MIN.) = 1.02 Tc(MIN.) = 13.65
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 786.00 = 3440.00 FEET.

*****
FLOW PROCESS FROM NODE 786.00 TO NODE 786.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.65
RAINFALL INTENSITY(INCH/HR) = 4.27
TOTAL STREAM AREA(ACRES) = 19.62
PEAK FLOW RATE(CFS) AT CONFLUENCE = 45.26

*****
FLOW PROCESS FROM NODE 789.00 TO NODE 790.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .7900
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 863.50
DOWNSTREAM ELEVATION(FEET) = 861.00
ELEVATION DIFFERENCE(FEET) = 2.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.054
100 YEAR RAINFALL INTENSITY(INCH/HR) = 8.168

```

```

NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.71
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.71
*****
*****>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
*****
FLOW PROCESS FROM NODE 790.00 TO NODE 791.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 861.00 DOWNSTREAM ELEVATION(FEET) = 765.50
STREET LENGTH(FEET) = 1296.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.38
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.56
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.35
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.48
STREET FLOW TRAVEL TIME(MIN.) = 4.04 Tc(MIN.) = 7.10
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.517
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .7900
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.790
SUBAREA AREA(ACRES) = 2.56 SUBAREA RUNOFF(CFS) = 13.18
TOTAL AREA(ACRES) = 2.7 PEAK FLOW RATE(CFS) = 13.75

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.33 HALFSTREET FLOOD WIDTH(FEET) = 10.02
FLOW VELOCITY(FEET/SEC.) = 6.13 DEPTH*VELOCITY(FT*FT/SEC.) = 2.00
LONGEST FLOWPATH FROM NODE 789.00 TO NODE 791.00 = 1366.00 FEET.
*****
FLOW PROCESS FROM NODE 791.00 TO NODE 786.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 761.00 DOWNSTREAM(FEET) = 760.00
FLOW LENGTH(FEET) = 125.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 17.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.55
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 13.75
PIPE TRAVEL TIME(MIN.) = 0.32 Tc(MIN.) = 7.41
LONGEST FLOWPATH FROM NODE 789.00 TO NODE 786.00 = 1491.00 FEET.
*****
FLOW PROCESS FROM NODE 786.00 TO NODE 786.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.41
RAINFALL INTENSITY(INCH/HR) = 6.34
TOTAL STREAM AREA(ACRES) = 2.67
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.75

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 45.26 13.65 4.274 19.62
2 13.75 7.41 6.335 2.67

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 44.28 7.41 6.335
2 54.53 13.65 4.274

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 54.53 Tc(MIN.) = 13.65
TOTAL AREA(ACRES) = 22.3
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 786.00 = 3440.00 FEET.
*****
FLOW PROCESS FROM NODE 786.00 TO NODE 795.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 760.00 DOWNSTREAM(FEET) = 730.00
FLOW LENGTH(FEET) = 126.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 34.13
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 54.53
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 13.71
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 795.00 = 3566.00 FEET.
*****
*****
FLOW PROCESS FROM NODE 795.00 TO NODE 795.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.71
RAINFALL INTENSITY(INCH/HR) = 4.26
TOTAL STREAM AREA(ACRES) = 22.29
PEAK FLOW RATE(CFS) AT CONFLUENCE = 54.53

*****
FLOW PROCESS FROM NODE 793.00 TO NODE 794.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 740.00
DOWNSTREAM ELEVATION(FEET) = 730.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 0.14 TOTAL RUNOFF(CFS) = 0.35
*****
FLOW PROCESS FROM NODE 794.00 TO NODE 795.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 730.00 DOWNSTREAM(FEET) = 729.00
FLOW LENGTH(FEET) = 284.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 1.84
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.35
PIPE TRAVEL TIME(MIN.) = 2.57 Tc(MIN.) = 8.83
LONGEST FLOWPATH FROM NODE 793.00 TO NODE 795.00 = 384.00 FEET.
*****
FLOW PROCESS FROM NODE 794.00 TO NODE 795.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.658
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 2.51 SUBAREA RUNOFF(CFS) = 4.97
TOTAL AREA(ACRES) = 2.7 TOTAL RUNOFF(CFS) = 5.25
TC(MIN.) = 8.83
*****
FLOW PROCESS FROM NODE 795.00 TO NODE 795.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.83
RAINFALL INTENSITY(INCH/HR) = 5.66
TOTAL STREAM AREA(ACRES) = 2.65
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.25

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 54.53 13.71 4.262 22.29
2 5.25 8.83 5.658 2.65

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 46.32 8.83 5.658
2 58.48 13.71 4.262

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 58.48 Tc(MIN.) = 13.71
TOTAL AREA(ACRES) = 24.9
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 795.00 = 3566.00 FEET.
*****
FLOW PROCESS FROM NODE 795.00 TO NODE 795.50 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 730.00 DOWNSTREAM(FEET) = 728.00
FLOW LENGTH(FEET) = 120.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.64
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1

```



```

PIPE-FLOW(CFS) = 58.48
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 13.87
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 795.50 = 3686.00 FEET.
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 31.63 SUBAREA RUNOFF(CFS) = 48.18
TOTAL AREA(ACRES) = 87.8 TOTAL RUNOFF(CFS) = 133.81
TC(MIN.) = 13.27
*****
FLOW PROCESS FROM NODE 795.50 TO NODE 795.50 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
-----
*****
FLOW PROCESS FROM NODE 850.00 TO NODE 851.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1880.00
DOWNSTREAM ELEVATION(FEET) = 1870.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 1.61
TOTAL AREA(ACRES) = 0.65 TOTAL RUNOFF(CFS) = 1.61
*****
FLOW PROCESS FROM NODE 851.00 TO NODE 852.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 1870.00 DOWNSTREAM(FEET) = 1430.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1062.00 CHANNEL SLOPE = 0.4143
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 1.61
FLOW VELOCITY(FEET/SEC) = 5.20 (PER LACPCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 3.40 Tc(MIN.) = 9.67
LONGEST FLOWPATH FROM NODE 850.00 TO NODE 852.00 = 1162.00 FEET.
*****
FLOW PROCESS FROM NODE 851.00 TO NODE 852.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.338
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 18.66 SUBAREA RUNOFF(CFS) = 34.86
TOTAL AREA(ACRES) = 19.3 TOTAL RUNOFF(CFS) = 36.08
TC(MIN.) = 9.67
*****
FLOW PROCESS FROM NODE 852.00 TO NODE 853.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 1420.00 DOWNSTREAM(FEET) = 1260.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1033.00 CHANNEL SLOPE = 0.1549
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 36.08
FLOW VELOCITY(FEET/SEC) = 11.15 (PER LACPCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.54 Tc(MIN.) = 11.21
LONGEST FLOWPATH FROM NODE 850.00 TO NODE 853.00 = 2195.00 FEET.
*****
FLOW PROCESS FROM NODE 852.00 TO NODE 853.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.851
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 36.90 SUBAREA RUNOFF(CFS) = 62.66
TOTAL AREA(ACRES) = 56.2 TOTAL RUNOFF(CFS) = 95.45
TC(MIN.) = 11.21
*****
FLOW PROCESS FROM NODE 853.00 TO NODE 854.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 1260.00 DOWNSTREAM(FEET) = 1150.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1030.00 CHANNEL SLOPE = 0.1068
CHANNEL FLOW THRU SUBAREA(CFS) = 95.45
FLOW VELOCITY(FEET/SEC) = 8.35 (PER LACPCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.06 Tc(MIN.) = 13.27
LONGEST FLOWPATH FROM NODE 850.00 TO NODE 854.00 = 3225.00 FEET.
*****
FLOW PROCESS FROM NODE 853.00 TO NODE 854.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.352
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 10.49 SUBAREA RUNOFF(CFS) = 14.16
TOTAL AREA(ACRES) = 168.0 TOTAL RUNOFF(CFS) = 226.73
TC(MIN.) = 16.01
*****
FLOW PROCESS FROM NODE 860.00 TO NODE 861.00 IS CODE = 21
-----

```

```

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1310.00
DOWNSTREAM ELEVATION(FEET) = 1300.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 1.26
TOTAL AREA(ACRES) = 0.51 TOTAL RUNOFF(CFS) = 1.26

*****
FLOW PROCESS FROM NODE 861.00 TO NODE 862.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1300.00 DOWNSTREAM(FEET) = 990.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 14.80 CHANNEL SLOPE = 20.9459
NOTE: CHANNEL SLOPE OF .5 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 1.26
FLOW VELOCITY(FEET/SEC) = 4.28 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 6.32
LONGEST FLOWPATH FROM NODE 860.00 TO NODE 862.00 = 114.80 FEET.
-----
*****
FLOW PROCESS FROM NODE 861.00 TO NODE 862.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.019
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 14.80 SUBAREA RUNOFF(CFS) = 36.36
TOTAL AREA(ACRES) = 15.3 TOTAL RUNOFF(CFS) = 37.61
TC(MIN.) = 6.32

*****
FLOW PROCESS FROM NODE 862.00 TO NODE 863.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 980.00 DOWNSTREAM(FEET) = 870.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1000.00 CHANNEL SLOPE = 0.1100
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 37.61
FLOW VELOCITY(FEET/SEC) = 11.28 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.48 Tc(MIN.) = 7.80
LONGEST FLOWPATH FROM NODE 860.00 TO NODE 863.00 = 1114.80 FEET.
-----
*****
FLOW PROCESS FROM NODE 862.00 TO NODE 863.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.130
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 18.00 SUBAREA RUNOFF(CFS) = 38.62
TOTAL AREA(ACRES) = 33.3 TOTAL RUNOFF(CFS) = 71.47
TC(MIN.) = 7.80

*****
FLOW PROCESS FROM NODE 863.00 TO NODE 865.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 880.00 DOWNSTREAM(FEET) = 780.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 983.00 CHANNEL SLOPE = 0.1017
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 71.47
FLOW VELOCITY(FEET/SEC) = 13.58 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.21 Tc(MIN.) = 9.01
LONGEST FLOWPATH FROM NODE 860.00 TO NODE 857.00 = 2097.80 FEET.
-----
*****
FLOW PROCESS FROM NODE 863.00 TO NODE 857.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.587
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 22.38 SUBAREA RUNOFF(CFS) = 43.77
TOTAL AREA(ACRES) = 55.7 TOTAL RUNOFF(CFS) = 108.91
TC(MIN.) = 9.01

*****
FLOW PROCESS FROM NODE 857.00 TO NODE 857.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.01
RAINFALL INTENSITY(INCH/HR) = 5.59
TOTAL STREAM AREA(ACRES) = 55.69
PEAK FLOW RATE(CFS) AT CONFLUENCE = 108.91

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 226.73 16.01 3.856 168.01
2 108.91 9.01 5.587 55.69

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 236.47 9.01 3.856
2 301.88 16.01 3.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 301.88 Tc(MIN.) = 16.01
TOTAL AREA(ACRES) = 223.7
LONGEST FLOWPATH FROM NODE 850.00 TO NODE 857.00 = 6125.00 FEET.
-----
*****
FLOW PROCESS FROM NODE 867.00 TO NODE 868.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 0.27
UPSTREAM ELEVATION(FEET) = 910.00
DOWNSTREAM ELEVATION(FEET) = 900.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 0.326
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.77
TOTAL AREA(ACRES) = 0.27 TOTAL RUNOFF(CFS) = 0.77

*****
FLOW PROCESS FROM NODE 868.00 TO NODE 869.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 900.00 DOWNSTREAM(FEET) = 860.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 243.00 CHANNEL SLOPE = 0.1646
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.77
FLOW VELOCITY(FEET/SEC) = 2.27 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.78 Tc(MIN.) = 2.11
LONGEST FLOWPATH FROM NODE 867.00 TO NODE 869.00 = 243.27 FEET.
-----
*****
FLOW PROCESS FROM NODE 868.00 TO NODE 869.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 6.64 SUBAREA RUNOFF(CFS) = 18.98
TOTAL AREA(ACRES) = 6.9 TOTAL RUNOFF(CFS) = 19.75
TC(MIN.) = 2.11

*****
FLOW PROCESS FROM NODE 869.00 TO NODE 870.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 860.00 DOWNSTREAM(FEET) = 830.00
FLOW LENGTH(FEET) = 329.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 18.56
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 19.75
PIPE TRAVEL TIME(MIN.) = 0.30 Tc(MIN.) = 2.40
LONGEST FLOWPATH FROM NODE 867.00 TO NODE 870.00 = 572.27 FEET.
-----
*****
FLOW PROCESS FROM NODE 870.00 TO NODE 870.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 2.40
RAINFALL INTENSITY(INCH/HR) = 8.17

```



```

TOTAL STREAM AREA(ACRES) = 6.91
PEAK FLOW RATE(CFS) AT CONFLUENCE = 19.75

TOTAL AREA(ACRES) = 31.5
TC(MIN.) = 11.77

TOTAL RUNOFF(CFS) = 51.89

*****
FLOW PROCESS FROM NODE 871.00 TO NODE 872.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 990.00
DOWNSTREAM ELEVATION(FEET) = 980.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 1.68
TOTAL AREA(ACRES) = 0.68 TOTAL RUNOFF(CFS) = 1.68

*****
FLOW PROCESS FROM NODE 872.00 TO NODE 870.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 980.00 DOWNSTREAM(FEET) = 840.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 670.00 CHANNEL SLOPE = 0.2090
CHANNEL FLOW THRU SUBAREA(CFS) = 1.68
FLOW VELOCITY(FEET/SEC) = 3.04 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 3.67 Tc(MIN.) = 9.94
LONGEST FLOWPATH FROM NODE 871.00 TO NODE 870.00 = 770.00 FEET.

*****
FLOW PROCESS FROM NODE 872.00 TO NODE 870.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.245
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 11.53 SUBAREA RUNOFF(CFS) = 21.16
TOTAL AREA(ACRES) = 12.2 TOTAL RUNOFF(CFS) = 22.41
TC(MIN.) = 9.94

*****
FLOW PROCESS FROM NODE 870.00 TO NODE 870.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.94
RAINFALL INTENSITY(INCH/HR) = 5.24
TOTAL STREAM AREA(ACRES) = 12.21
PEAK FLOW RATE(CFS) AT CONFLUENCE = 22.41

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 19.75 2.40 8.168 6.91
2 22.41 9.94 5.245 12.21

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 25.18 2.40 8.168
2 35.10 9.94 5.245

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 35.10 Tc(MIN.) = 9.94
TOTAL AREA(ACRES) = 19.1
LONGEST FLOWPATH FROM NODE 871.00 TO NODE 870.00 = 770.00 FEET.

*****
FLOW PROCESS FROM NODE 870.00 TO NODE 873.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 840.00 DOWNSTREAM(FEET) = 810.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 762.00 CHANNEL SLOPE = 0.0394
CHANNEL FLOW THRU SUBAREA(CFS) = 35.10
FLOW VELOCITY(FEET/SEC) = 6.94 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.83 Tc(MIN.) = 11.77
LONGEST FLOWPATH FROM NODE 871.00 TO NODE 873.00 = 1532.00 FEET.

*****
FLOW PROCESS FROM NODE 870.00 TO NODE 873.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.703
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 12.40 SUBAREA RUNOFF(CFS) = 20.41

*****
FLOW PROCESS FROM NODE 873.00 TO NODE 874.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 820.00 DOWNSTREAM(FEET) = 815.00
FLOW LENGTH(FEET) = 641.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.20
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 51.89
PIPE TRAVEL TIME(MIN.) = 1.16 Tc(MIN.) = 12.93
LONGEST FLOWPATH FROM NODE 871.00 TO NODE 874.00 = 2173.00 FEET.

*****
FLOW PROCESS FROM NODE 874.00 TO NODE 874.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.93
RAINFALL INTENSITY(INCH/HR) = 4.43
TOTAL STREAM AREA(ACRES) = 31.52
PEAK FLOW RATE(CFS) AT CONFLUENCE = 51.89

*****
FLOW PROCESS FROM NODE 877.00 TO NODE 878.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 960.00
DOWNSTREAM ELEVATION(FEET) = 950.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.86
TOTAL AREA(ACRES) = 0.35 TOTAL RUNOFF(CFS) = 0.86

*****
FLOW PROCESS FROM NODE 878.00 TO NODE 879.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 850.00 DOWNSTREAM(FEET) = 820.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 605.00 CHANNEL SLOPE = 0.0496
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.86
FLOW VELOCITY(FEET/SEC) = 3.34 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 3.02 Tc(MIN.) = 9.29
LONGEST FLOWPATH FROM NODE 877.00 TO NODE 879.00 = 705.00 FEET.

*****
FLOW PROCESS FROM NODE 878.00 TO NODE 879.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.479
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 6.24 SUBAREA RUNOFF(CFS) = 11.97
TOTAL AREA(ACRES) = 6.6 TOTAL RUNOFF(CFS) = 12.64
TC(MIN.) = 9.29

*****
FLOW PROCESS FROM NODE 879.00 TO NODE 874.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 810.00 DOWNSTREAM(FEET) = 805.00
FLOW LENGTH(FEET) = 822.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.98
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.64
PIPE TRAVEL TIME(MIN.) = 2.29 Tc(MIN.) = 11.58
LONGEST FLOWPATH FROM NODE 877.00 TO NODE 874.00 = 1527.00 FEET.

*****
FLOW PROCESS FROM NODE 874.00 TO NODE 874.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.58
RAINFALL INTENSITY(INCH/HR) = 4.75
TOTAL STREAM AREA(ACRES) = 6.59
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.64

** CONFLUENCE DATA **

```

```

STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/ HOUR)      (ACRE)
1            51.89      12.93      4.426      31.52
2            12.64      11.58      4.753      6.59

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/ HOUR)
1            60.95      11.58      4.753
2            63.65      12.93      4.426

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      63.65      Tc(MIN.) =      12.93
TOTAL AREA(ACRES) =      38.1
LONGEST FLOWPATH FROM NODE      871.00 TO NODE      874.00 =      2173.00 FEET.

*****
FLOW PROCESS FROM NODE      874.00 TO NODE      795.50 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) =      805.00 DOWNSTREAM( FEET) =      743.00
FLOW LENGTH( FEET) =      1145.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.1 INCHES
PIPE-FLOW VELOCITY( FEET/ SEC.) = 20.01
ESTIMATED PIPE DIAMETER( INCH) = 27.00      NUMBER OF PIPES = 1
PIPE-FLOW( CFS) =      63.65
PIPE TRAVEL TIME( MIN.) = 0.95      Tc( MIN.) =      13.88
LONGEST FLOWPATH FROM NODE      871.00 TO NODE      795.50 =      3318.00 FEET.

*****
FLOW PROCESS FROM NODE      795.50 TO NODE      795.50 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/ HOUR)      (ACRE)
1            63.65      13.88      4.227      38.11
LONGEST FLOWPATH FROM NODE      871.00 TO NODE      795.50 =      3318.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/ HOUR)      (ACRE)
1            58.48      13.87      4.230      24.94
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      795.50 =      3686.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/ HOUR)
1            122.07      13.87      4.230
2            122.10      13.88      4.227

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      122.10      Tc( MIN.) =      13.88
TOTAL AREA(ACRES) =      63.0

*****
FLOW PROCESS FROM NODE      795.50 TO NODE      795.50 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      795.50 TO NODE      903.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) =      743.00 DOWNSTREAM( FEET) =      695.00
FLOW LENGTH( FEET) =      2502.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 31.1 INCHES
PIPE-FLOW VELOCITY( FEET/ SEC.) = 15.97
ESTIMATED PIPE DIAMETER( INCH) = 42.00      NUMBER OF PIPES = 1
PIPE-FLOW( CFS) =      122.10
PIPE TRAVEL TIME( MIN.) = 2.61      Tc( MIN.) =      16.49
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      903.00 =      6188.00 FEET.

*****
FLOW PROCESS FROM NODE      903.00 TO NODE      903.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====
+-----+
|       |
|       |
|       |
+-----+
*****
FLOW PROCESS FROM NODE      857.00 TO NODE      857.00 IS CODE = 7
-----
>>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<<
=====
USER-SPECIFIED VALUES ARE AS FOLLOWS:
TC( MIN) = 16.01      RAIN INTENSITY( INCH/ HOUR) = 3.86
TOTAL AREA(ACRES) = 223.70      TOTAL RUNOFF( CFS) = 301.88

*****
FLOW PROCESS FROM NODE      857.00 TO NODE      864.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) =      780.00 DOWNSTREAM( FEET) =      760.00
FLOW LENGTH( FEET) =      469.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 37.4 INCHES
PIPE-FLOW VELOCITY( FEET/ SEC.) = 27.06
ESTIMATED PIPE DIAMETER( INCH) = 51.00      NUMBER OF PIPES = 1
PIPE-FLOW( CFS) =      301.88
PIPE TRAVEL TIME( MIN.) = 0.29      Tc( MIN.) =      16.30
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      864.00 =      6657.00 FEET.

*****
FLOW PROCESS FROM NODE      864.00 TO NODE      865.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) =      750.00 DOWNSTREAM( FEET) =      725.00
CHANNEL LENGTH THRU SUBAREA( FEET) =      480.00 CHANNEL SLOPE = 0.0521
CHANNEL FLOW THRU SUBAREA( CFS) =      301.88
FLOW VELOCITY( FEET/ SEC) = 15.22 ( PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME( MIN.) = 0.53      Tc( MIN.) =      16.82
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      865.00 =      7137.00 FEET.

*****
FLOW PROCESS FROM NODE      864.00 TO NODE      865.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY( INCH/ HOUR) = 3.734
*USER SPECIFIED( SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER ( AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA( ACRES) = 6.63      SUBAREA RUNOFF( CFS) = 8.67
TOTAL AREA( ACRES) = 230.3      TOTAL RUNOFF( CFS) = 301.88
TC( MIN.) = 16.82
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****
FLOW PROCESS FROM NODE      865.00 TO NODE      880.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) =      725.00 DOWNSTREAM( FEET) =      700.00
CHANNEL LENGTH THRU SUBAREA( FEET) =      666.00 CHANNEL SLOPE = 0.0375
CHANNEL FLOW THRU SUBAREA( CFS) =      301.88
FLOW VELOCITY( FEET/ SEC) = 12.92 ( PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME( MIN.) = 0.86      Tc( MIN.) =      17.68
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      880.00 =      7803.00 FEET.

*****
FLOW PROCESS FROM NODE      865.00 TO NODE      880.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY( INCH/ HOUR) = 3.616
*USER SPECIFIED( SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER ( AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA( ACRES) = 8.54      SUBAREA RUNOFF( CFS) = 10.81
TOTAL AREA( ACRES) = 238.9      TOTAL RUNOFF( CFS) = 302.33
TC( MIN.) = 17.68

*****
FLOW PROCESS FROM NODE      880.00 TO NODE      880.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION( MIN.) = 17.68
RAINFALL INTENSITY( INCH/ HR) = 3.62
TOTAL STREAM AREA( ACRES) = 238.87
PEAK FLOW RATE( CFS) AT CONFLUENCE = 302.33

*****
FLOW PROCESS FROM NODE      882.00 TO NODE      883.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED( SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER ( AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH( FEET) = 100.00
UPSTREAM ELEVATION( FEET) = 1530.00
DOWNSTREAM ELEVATION( FEET) = 1520.00
ELEVATION DIFFERENCE( FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW( MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY( INCH/ HOUR) = 7.061
SUBAREA RUNOFF( CFS) = 0.49
TOTAL AREA( ACRES) = 0.20      TOTAL RUNOFF( CFS) = 0.49

*****
FLOW PROCESS FROM NODE      883.00 TO NODE      884.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====

```



```

ELEVATION DATA: UPSTREAM(FEET) = 1520.00 DOWNSTREAM(FEET) = 1180.00
CHANNEL LENGTH THRU SUBAREA( FEET) = 987.00 CHANNEL SLOPE = 0.3445
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.49
FLOW VELOCITY(FEET/SEC) = 4.74 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 3.47 Tc(MIN.) = 9.73
LONGEST FLOWPATH FROM NODE 882.00 TO NODE 884.00 = 1087.00 FEET.

*****
FLOW PROCESS FROM NODE 883.00 TO NODE 884.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.315
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 12.73 SUBAREA RUNOFF(CFS) = 23.68
TOTAL AREA(ACRES) = 12.9 TOTAL RUNOFF(CFS) = 24.05
TC(MIN.) = 9.73

*****
FLOW PROCESS FROM NODE 884.00 TO NODE 885.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1180.00 DOWNSTREAM(FEET) = 940.00
CHANNEL LENGTH THRU SUBAREA( FEET) = 1011.00 CHANNEL SLOPE = 0.2374
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 24.05
FLOW VELOCITY(FEET/SEC) = 9.96 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.69 Tc(MIN.) = 11.43
LONGEST FLOWPATH FROM NODE 882.00 TO NODE 885.00 = 2098.00 FEET.

*****
FLOW PROCESS FROM NODE 884.00 TO NODE 885.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.793
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 22.07 SUBAREA RUNOFF(CFS) = 37.02
TOTAL AREA(ACRES) = 35.0 TOTAL RUNOFF(CFS) = 58.71
TC(MIN.) = 11.43

*****
FLOW PROCESS FROM NODE 885.00 TO NODE 886.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 940.00 DOWNSTREAM(FEET) = 830.00
CHANNEL LENGTH THRU SUBAREA( FEET) = 641.00 CHANNEL SLOPE = 0.1716
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 58.71
FLOW VELOCITY(FEET/SEC) = 12.82 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.83 Tc(MIN.) = 12.26
LONGEST FLOWPATH FROM NODE 882.00 TO NODE 886.00 = 2739.00 FEET.

*****
FLOW PROCESS FROM NODE 885.00 TO NODE 886.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.580
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 28.09 SUBAREA RUNOFF(CFS) = 45.03
TOTAL AREA(ACRES) = 63.1 TOTAL RUNOFF(CFS) = 101.14
TC(MIN.) = 12.26

*****
FLOW PROCESS FROM NODE 886.00 TO NODE 890.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 820.00 DOWNSTREAM(FEET) = 780.00
FLOW LENGTH( FEET) = 413.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 20.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 28.24
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 101.14
PIPE TRAVEL TIME(MIN.) = 0.24 Tc(MIN.) = 12.50
LONGEST FLOWPATH FROM NODE 882.00 TO NODE 890.00 = 3152.00 FEET.

*****
FLOW PROCESS FROM NODE 890.00 TO NODE 880.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 780.00 DOWNSTREAM(FEET) = 710.00
CHANNEL LENGTH THRU SUBAREA( FEET) = 1099.00 CHANNEL SLOPE = 0.0637
CHANNEL FLOW THRU SUBAREA(CFS) = 101.14
FLOW VELOCITY(FEET/SEC) = 12.02 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.52 Tc(MIN.) = 14.03

*****
LONGEST FLOWPATH FROM NODE 882.00 TO NODE 880.00 = 4251.00 FEET.
*****
FLOW PROCESS FROM NODE 890.00 TO NODE 880.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.199
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 30.66 SUBAREA RUNOFF(CFS) = 45.06
TOTAL AREA(ACRES) = 93.8 TOTAL RUNOFF(CFS) = 137.78
TC(MIN.) = 14.03

*****
FLOW PROCESS FROM NODE 880.00 TO NODE 880.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.03
RAINFALL INTENSITY(INCH/HR) = 4.20
TOTAL STREAM AREA(ACRES) = 93.75
PEAK FLOW RATE(CFS) AT CONFLUENCE = 137.78

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 302.33 17.68 3.616 238.87
2 137.78 14.03 4.199 93.75

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 377.60 14.03 4.199
2 420.99 17.68 3.616

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 420.99 Tc(MIN.) = 17.68
TOTAL AREA(ACRES) = 332.6
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 880.00 = 7803.00 FEET.

*****
FLOW PROCESS FROM NODE 880.00 TO NODE 895.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 700.00 DOWNSTREAM(FEET) = 680.00
CHANNEL LENGTH THRU SUBAREA( FEET) = 1000.00 CHANNEL SLOPE = 0.0200
CHANNEL FLOW THRU SUBAREA(CFS) = 420.99
FLOW VELOCITY(FEET/SEC) = 10.48 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.59 Tc(MIN.) = 19.27
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 895.00 = 8803.00 FEET.

*****
FLOW PROCESS FROM NODE 880.00 TO NODE 895.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.421
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 22.59 SUBAREA RUNOFF(CFS) = 27.05
TOTAL AREA(ACRES) = 355.2 TOTAL RUNOFF(CFS) = 425.29
TC(MIN.) = 19.27

*****
FLOW PROCESS FROM NODE 895.00 TO NODE 896.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 680.00 DOWNSTREAM(FEET) = 660.00
CHANNEL LENGTH THRU SUBAREA( FEET) = 1260.00 CHANNEL SLOPE = 0.0159
CHANNEL FLOW THRU SUBAREA(CFS) = 425.29
FLOW VELOCITY(FEET/SEC) = 9.37 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.24 Tc(MIN.) = 21.51
LONGEST FLOWPATH FROM NODE 755.00 TO NODE 896.00 = 10063.00 FEET.

*****
FLOW PROCESS FROM NODE 895.00 TO NODE 896.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.187
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 21.20 SUBAREA RUNOFF(CFS) = 23.64
TOTAL AREA(ACRES) = 376.4 TOTAL RUNOFF(CFS) = 425.29
TC(MIN.) = 21.51
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****

```

```

FLOW PROCESS FROM NODE      896.00 TO NODE      896.00 IS CODE =   1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS =   2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =  21.51
RAINFALL INTENSITY(INCH/HR) =   3.19
TOTAL STREAM AREA(ACRES) =   376.41
PEAK FLOW RATE(CFS) AT CONFLUENCE =   425.29

** PEAK FLOW RATE TABLE **
STREAM   RUNOFF      Tc      INTENSITY
NUMBER   (CFS)      (MIN.)  (INCH/HR)
-----
1         414.86    16.21    3.824
2         475.69    21.51    3.187

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =   475.69   Tc(MIN.) =  21.51
TOTAL AREA(ACRES) =   421.6
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      896.00 =  10063.00 FEET.

*****
FLOW PROCESS FROM NODE      893.00 TO NODE      894.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =   0
INITIAL SUBAREA FLOW-LENGTH(FEET) =  100.00
UPSTREAM ELEVATION(FEET) =   980.00
DOWNSTREAM ELEVATION(FEET) =   970.00
ELEVATION DIFFERENCE(FEET) =   10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =   6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) =  7.061
SUBAREA RUNOFF(CFS) =   2.82
TOTAL AREA(ACRES) =   1.14   TOTAL RUNOFF(CFS) =   2.82

*****
FLOW PROCESS FROM NODE      894.00 TO NODE      894.10 IS CODE =  53
-----
>>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   980.00   DOWNSTREAM(FEET) =   750.00
CHANNEL LENGTH THRU SUBAREA(FEET) =  1455.00   CHANNEL SLOPE =  0.1581
CHANNEL FLOW THRU SUBAREA(CFS) =   2.82
FLOW VELOCITY(FEET/SEC) =   3.14 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =   7.71   Tc(MIN.) =  13.98
LONGEST FLOWPATH FROM NODE      893.00 TO NODE      894.10 =  1555.00 FEET.

*****
FLOW PROCESS FROM NODE      894.00 TO NODE      894.10 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HR) =  4.208
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =   0
AREA-AVERAGE RUNOFF COEFFICIENT =  0.3500
SUBAREA AREA(ACRES) =   23.16   SUBAREA RUNOFF(CFS) =   34.11
TOTAL AREA(ACRES) =   24.3    TOTAL RUNOFF(CFS) =   35.79
TC(MIN.) =  13.98

*****
FLOW PROCESS FROM NODE      894.10 TO NODE      896.00 IS CODE =  52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   750.00   DOWNSTREAM(FEET) =   660.00
CHANNEL LENGTH THRU SUBAREA(FEET) =  1260.00   CHANNEL SLOPE =  0.0714
CHANNEL FLOW THRU SUBAREA(CFS) =   35.79
FLOW VELOCITY(FEET/SEC) =   9.40 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =   2.23   Tc(MIN.) =  16.21
LONGEST FLOWPATH FROM NODE      893.00 TO NODE      896.00 =  2815.00 FEET.

*****
FLOW PROCESS FROM NODE      894.10 TO NODE      896.00 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HR) =  3.824
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =   0
AREA-AVERAGE RUNOFF COEFFICIENT =  0.3500
SUBAREA AREA(ACRES) =   20.89   SUBAREA RUNOFF(CFS) =   27.96
TOTAL AREA(ACRES) =   45.2    TOTAL RUNOFF(CFS) =   60.49
TC(MIN.) =  16.21

*****
FLOW PROCESS FROM NODE      896.00 TO NODE      896.00 IS CODE =   1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS =   2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
TIME OF CONCENTRATION(MIN.) =  16.21
RAINFALL INTENSITY(INCH/HR) =   3.82
TOTAL STREAM AREA(ACRES) =   45.19
PEAK FLOW RATE(CFS) AT CONFLUENCE =   60.49

** CONFLUENCE DATA **
STREAM   RUNOFF      Tc      INTENSITY      AREA
NUMBER   (CFS)      (MIN.)  (INCH/HR)    (ACRE)
-----
1         425.29    21.51    3.187        376.41
2          60.49    16.21    3.824         45.19

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR  2 STREAMS.

*****
FLOW PROCESS FROM NODE      896.00 TO NODE      896.00 IS CODE =   1
-----
** PEAK FLOW RATE TABLE **
STREAM   RUNOFF      Tc      INTENSITY
NUMBER   (CFS)      (MIN.)  (INCH/HR)
-----
1         414.86    16.21    3.824
2         475.69    21.51    3.187

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =   475.69   Tc(MIN.) =  21.51
TOTAL AREA(ACRES) =   421.6
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      896.00 =  10063.00 FEET.

*****
FLOW PROCESS FROM NODE      896.00 TO NODE      897.00 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   660.00   DOWNSTREAM(FEET) =   655.00
FLOW LENGTH(FEET) =   406.00   MANNING'S N =  0.013
DEPTH OF FLOW IN  75.0 INCH PIPE IS  57.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  18.91
ESTIMATED PIPE DIAMETER(INCH) =  75.00   NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =   475.69
PIPE TRAVEL TIME(MIN.) =   0.36   Tc(MIN.) =  21.87
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      897.00 =  10469.00 FEET.

*****
FLOW PROCESS FROM NODE      897.00 TO NODE      897.00 IS CODE =  10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<
=====
*****
FLOW PROCESS FROM NODE      900.00 TO NODE      901.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =   0
INITIAL SUBAREA FLOW-LENGTH(FEET) =  100.00
UPSTREAM ELEVATION(FEET) =   790.00
DOWNSTREAM ELEVATION(FEET) =   780.00
ELEVATION DIFFERENCE(FEET) =   10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =   6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HR) =  7.061
SUBAREA RUNOFF(CFS) =   1.43
TOTAL AREA(ACRES) =   0.58   TOTAL RUNOFF(CFS) =   1.43

*****
FLOW PROCESS FROM NODE      901.00 TO NODE      902.00 IS CODE =  52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   730.00   DOWNSTREAM(FEET) =   710.00
CHANNEL LENGTH THRU SUBAREA(FEET) =  1218.00   CHANNEL SLOPE =  0.0164
CHANNEL FLOW THRU SUBAREA(CFS) =   1.43
FLOW VELOCITY(FEET/SEC) =   2.06 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =   9.85   Tc(MIN.) =  16.12
LONGEST FLOWPATH FROM NODE      900.00 TO NODE      902.00 =  1318.00 FEET.

*****
FLOW PROCESS FROM NODE      901.00 TO NODE      902.00 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HR) =  3.839
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =   0
AREA-AVERAGE RUNOFF COEFFICIENT =  0.3500
SUBAREA AREA(ACRES) =   8.67   SUBAREA RUNOFF(CFS) =   11.65
TOTAL AREA(ACRES) =   9.2    TOTAL RUNOFF(CFS) =   12.43
TC(MIN.) =  16.12

*****
FLOW PROCESS FROM NODE      902.00 TO NODE      903.00 IS CODE =  52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   710.00   DOWNSTREAM(FEET) =   705.00
CHANNEL LENGTH THRU SUBAREA(FEET) =  917.00   CHANNEL SLOPE =  0.0055
CHANNEL FLOW THRU SUBAREA(CFS) =   12.43
FLOW VELOCITY(FEET/SEC) =   1.95 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =   7.83   Tc(MIN.) =  23.95
LONGEST FLOWPATH FROM NODE      900.00 TO NODE      903.00 =  2235.00 FEET.

*****
FLOW PROCESS FROM NODE      902.00 TO NODE      903.00 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HR) =  2.974
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =   0
AREA-AVERAGE RUNOFF COEFFICIENT =  0.3500
SUBAREA AREA(ACRES) =   9.25   SUBAREA RUNOFF(CFS) =   9.63
TOTAL AREA(ACRES) =   18.5    TOTAL RUNOFF(CFS) =   19.25
TC(MIN.) =  23.95

*****

```



```

FLOW PROCESS FROM NODE      903.00 TO NODE      903.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           19.25      23.95      2.974      18.50
LONGEST FLOWPATH FROM NODE      900.00 TO NODE      903.00 =      2235.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           122.10      16.49      3.783      63.05
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      903.00 =      6188.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           135.36      16.49      3.783
2           115.23      23.95      2.974

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      135.36      Tc(MIN.) =      16.49
TOTAL AREA(ACRES) =      81.5

*****
FLOW PROCESS FROM NODE      903.00 TO NODE      903.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      903.00 TO NODE      897.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET ) =      710.00 DOWNSTREAM( FEET ) =      690.00
FLOW LENGTH( FEET ) =      566.00 MANNING'S N =      0.013
DEPTH OF FLOW IN      39.0 INCH PIPE IS      28.8 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) =      20.62
ESTIMATED PIPE DIAMETER( INCH ) =      39.00      NUMBER OF PIPES =      1
PIPE-FLOW( CFS ) =      135.36
PIPE TRAVEL TIME( MIN. ) =      0.46      Tc( MIN. ) =      16.95
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      897.00 =      6754.00 FEET.

*****
FLOW PROCESS FROM NODE      897.00 TO NODE      897.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           135.36      16.95      3.717      81.55
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      897.00 =      6754.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           475.69      21.87      3.153      421.60
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      897.00 =      10469.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           503.97      16.95      3.717
2           590.52      21.87      3.153

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      590.52      Tc( MIN. ) =      21.87
TOTAL AREA(ACRES) =      503.1

*****
FLOW PROCESS FROM NODE      897.00 TO NODE      897.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 2 <<<<<
=====

*****
FLOW PROCESS FROM NODE      897.00 TO NODE      904.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET ) =      690.00 DOWNSTREAM( FEET ) =      660.00
FLOW LENGTH( FEET ) =      750.00 MANNING'S N =      0.013
DEPTH OF FLOW IN      66.0 INCH PIPE IS      49.0 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) =      31.19
ESTIMATED PIPE DIAMETER( INCH ) =      66.00      NUMBER OF PIPES =      1
PIPE-FLOW( CFS ) =      590.52
PIPE TRAVEL TIME( MIN. ) =      0.40      Tc( MIN. ) =      22.27
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      904.00 =      11219.00 FEET.

*****
FLOW PROCESS FROM NODE      904.00 TO NODE      904.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      1 ARE:
TIME OF CONCENTRATION( MIN. ) =      22.27

*****
FLOW PROCESS FROM NODE      903.00 TO NODE      903.00 IS CODE = 11
-----
RAINFALL INTENSITY( INCH/HR ) =      3.12
TOTAL STREAM AREA( ACRES ) =      503.15
PEAK FLOW RATE( CFS ) AT CONFLUENCE =      590.52

*****
FLOW PROCESS FROM NODE      905.00 TO NODE      904.00 IS CODE = 7
-----
>>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<<
=====
USER-SPECIFIED VALUES ARE AS FOLLOWS:
TC( MIN ) =      17.33      RAIN INTENSITY( INCH/HOUR ) =      3.66
TOTAL AREA( ACRES ) =      192.83      TOTAL RUNOFF( CFS ) =      370.37

*****
FLOW PROCESS FROM NODE      904.00 TO NODE      904.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      2 ARE:
TIME OF CONCENTRATION( MIN. ) =      17.33
RAINFALL INTENSITY( INCH/HR ) =      3.66
TOTAL STREAM AREA( ACRES ) =      192.83
PEAK FLOW RATE( CFS ) AT CONFLUENCE =      370.37

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           590.52      22.27      3.116      503.15
2           370.37      17.33      3.664      192.83

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR      2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           829.83      17.33      3.664
2           905.54      22.27      3.116

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      905.54      Tc( MIN. ) =      22.27
TOTAL AREA( ACRES ) =      696.0
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      904.00 =      11219.00 FEET.

*****
FLOW PROCESS FROM NODE      904.00 TO NODE      906.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM( FEET ) =      660.00 DOWNSTREAM( FEET ) =      630.00
FLOW LENGTH( FEET ) =      488.00 MANNING'S N =      0.013
DEPTH OF FLOW IN      69.0 INCH PIPE IS      56.0 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) =      40.11
ESTIMATED PIPE DIAMETER( INCH ) =      69.00      NUMBER OF PIPES =      1
PIPE-FLOW( CFS ) =      905.54
PIPE TRAVEL TIME( MIN. ) =      0.20      Tc( MIN. ) =      22.48
LONGEST FLOWPATH FROM NODE      755.00 TO NODE      906.00 =      11707.00 FEET.

=====
END OF STUDY SUMMARY:
TOTAL AREA( ACRES ) =      696.0 TC( MIN. ) =      22.48
PEAK FLOW RATE( CFS ) =      905.54
=====
END OF RATIONAL METHOD ANALYSIS

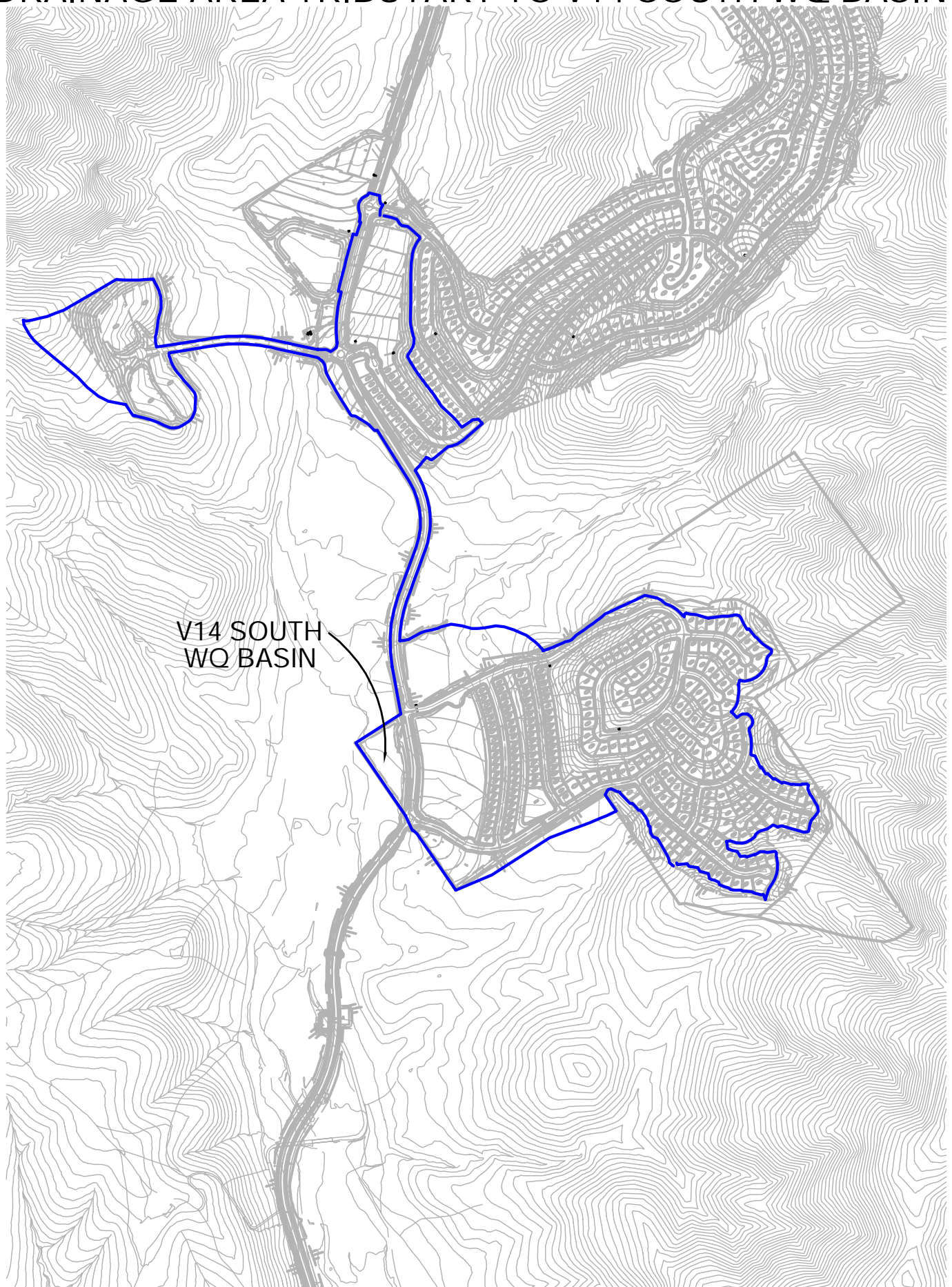
```

CHAPTER 5

5.1.3 – Rational Method Hydrologic Analysis (AES 2015)

Drainage Area Tributary to V14 South WQ Basin

OTAY RANCH VILLAGE 14 & PA 16/19 DRAINAGE AREA TRIBUTARY TO V14 SOUTH WQ BASIN



```

*****
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1239

Analysis prepared by:

HUnsaker & Associates San Diego, Inc.
9707 Waples Street
San Diego CA 92121

-----
FILE NAME: R:\1235\HYD\CALCS\AES\SRP\V14S.DAT
TIME/DATE OF STUDY: 15:28 10/04/2016
-----
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
-----
2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.100
SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
      HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
NO.  WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
    (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
==  =====
1  16.0      8.0   0.020/0.020/0.020  0.50  2.00  0.0313  0.125  0.0150
2  12.0      6.0   0.020/0.020/0.020  0.50  1.50  0.0313  0.125  0.0130

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
   as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*****
FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 783.85
DOWNSTREAM ELEVATION(FEET) = 783.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.92
TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.92

*****
FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 782.00 DOWNSTREAM ELEVATION(FEET) = 765.70
STREET LENGTH(FEET) = 527.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.37
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.68
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.65
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.10
STREET FLOW TRAVEL TIME(MIN.) = 2.40 Tc(MIN.) = 11.14
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.872
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.28 SUBAREA RUNOFF(CFS) = 10.84
TOTAL AREA(ACRES) = 4.6 PEAK FLOW RATE(CFS) = 11.63

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.21
FLOW VELOCITY(FEET/SEC.) = 4.23 DEPTH*VELOCITY(FT*FT/SEC.) = 1.48

LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 597.00 FEET.

*****
FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 760.00 DOWNSTREAM(FEET) = 729.00
FLOW LENGTH(FEET) = 565.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.47
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 11.63
PIPE TRAVEL TIME(MIN.) = 0.70 Tc(MIN.) = 11.84
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 1162.00 FEET.

*****
FLOW PROCESS FROM NODE 103.00 TO NODE 103.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.84
RAINFALL INTENSITY(INCH/HR) = 4.68
TOTAL STREAM AREA(ACRES) = 4.59
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.63

*****
FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 763.85
DOWNSTREAM ELEVATION(FEET) = 763.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.92
TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.92

*****
FLOW PROCESS FROM NODE 106.00 TO NODE 107.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 762.00 DOWNSTREAM ELEVATION(FEET) = 735.00
STREET LENGTH(FEET) = 658.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.50
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.56
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.98
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.11
STREET FLOW TRAVEL TIME(MIN.) = 2.75 Tc(MIN.) = 11.49
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.776
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.67 SUBAREA RUNOFF(CFS) = 9.11
TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 9.88

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.88
FLOW VELOCITY(FEET/SEC.) = 4.52 DEPTH*VELOCITY(FT*FT/SEC.) = 1.46
LONGEST FLOWPATH FROM NODE 105.00 TO NODE 107.00 = 728.00 FEET.

*****
FLOW PROCESS FROM NODE 107.00 TO NODE 103.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.49
RAINFALL INTENSITY(INCH/HR) = 4.78
TOTAL STREAM AREA(ACRES) = 3.98
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.88

```

```

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1            11.63      11.84      4.685            4.59
2            9.88       11.49      4.776            3.98

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1            21.17      11.49      4.776
2            21.32      11.84      4.685

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 21.32 Tc(MIN.) = 11.84
TOTAL AREA(ACRES) = 8.6
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 1162.00 FEET.

*****
FLOW PROCESS FROM NODE 103.00 TO NODE 108.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 730.00 DOWNSTREAM(FEET) = 719.50
FLOW LENGTH(FEET) = 167.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.25
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 21.32
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 12.01
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 108.00 = 1329.00 FEET.

*****
FLOW PROCESS FROM NODE 108.00 TO NODE 108.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.01
RAINFALL INTENSITY(INCH/HR) = 4.64
TOTAL STREAM AREA(ACRES) = 8.57
PEAK FLOW RATE(CFS) AT CONFLUENCE = 21.32

*****
FLOW PROCESS FROM NODE 110.00 TO NODE 111.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 743.85
DOWNSTREAM ELEVATION(FEET) = 743.15
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.83
TOTAL AREA(ACRES) = 0.28 TOTAL RUNOFF(CFS) = 0.83

*****
FLOW PROCESS FROM NODE 111.00 TO NODE 112.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 742.00 DOWNSTREAM ELEVATION(FEET) = 725.00
STREET LENGTH(FEET) = 529.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.19
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.80
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.57
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.01
STREET FLOW TRAVEL TIME(MIN.) = 2.47 Tc(MIN.) = 11.20
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.854
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.44 SUBAREA RUNOFF(CFS) = 8.68
TOTAL AREA(ACRES) = 3.7 PEAK FLOW RATE(CFS) = 9.39

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.33 HALFSTREET FLOOD WIDTH(FEET) = 10.16
FLOW VELOCITY(FEET/SEC.) = 4.08 DEPTH*VELOCITY(FT*FT/SEC.) = 1.34
LONGEST FLOWPATH FROM NODE 110.00 TO NODE 112.00 = 599.00 FEET.

*****
FLOW PROCESS FROM NODE 112.00 TO NODE 108.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.20
RAINFALL INTENSITY(INCH/HR) = 4.85
TOTAL STREAM AREA(ACRES) = 3.72
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.39

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1            21.32      12.01      4.641            8.57
2            9.39       11.20      4.854            3.72

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1            29.78      11.20      4.854
2            30.30      12.01      4.641

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 30.30 Tc(MIN.) = 12.01
TOTAL AREA(ACRES) = 12.3
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 108.00 = 1329.00 FEET.

*****
FLOW PROCESS FROM NODE 108.00 TO NODE 113.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 725.00 DOWNSTREAM(FEET) = 722.50
FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.07
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 30.30
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 12.10
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 113.00 = 1404.00 FEET.

*****
FLOW PROCESS FROM NODE 113.00 TO NODE 113.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.10
RAINFALL INTENSITY(INCH/HR) = 4.62
TOTAL STREAM AREA(ACRES) = 12.29
PEAK FLOW RATE(CFS) AT CONFLUENCE = 30.30

*****
FLOW PROCESS FROM NODE 120.00 TO NODE 121.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4600
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 770.35
DOWNSTREAM ELEVATION(FEET) = 769.65
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.638
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.349
SUBAREA RUNOFF(CFS) = 0.84
TOTAL AREA(ACRES) = 0.34 TOTAL RUNOFF(CFS) = 0.84

*****
FLOW PROCESS FROM NODE 121.00 TO NODE 122.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 769.00 DOWNSTREAM ELEVATION(FEET) = 722.50
STREET LENGTH(FEET) = 1711.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

```



```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      6.19
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.36
HALFSTREET FLOOD WIDTH(FEET) = 11.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.07
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.48
STREET FLOW TRAVEL TIME(MIN.) = 7.01  Tc(MIN.) = 16.65
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.759
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4600
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.460
SUBAREA AREA(ACRES) = 6.08  SUBAREA RUNOFF(CFS) = 10.51
TOTAL AREA(ACRES) = 6.4  PEAK FLOW RATE(CFS) = 11.10

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.43  HALFSTREET FLOOD WIDTH(FEET) = 15.01
FLOW VELOCITY(FEET/SEC.) = 4.68  DEPTH*VELOCITY(FT*FT/SEC.) = 2.00
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 122.00 = 1781.00 FEET.

*****
FLOW PROCESS FROM NODE 122.00 TO NODE 113.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 16.65
RAINFALL INTENSITY(INCH/HR) = 3.76
TOTAL STREAM AREA(ACRES) = 6.42
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.10

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1       30.30     12.10    4.619       12.29
2       11.10     16.65    3.759       6.42

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF      Tc      INTENSITY
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)
1       38.37     12.10    4.619
2       35.76     16.65    3.759

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 38.37  Tc(MIN.) = 12.10
TOTAL AREA(ACRES) = 18.7
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 113.00 = 1781.00 FEET.

*****
FLOW PROCESS FROM NODE 113.00 TO NODE 113.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 125.00 TO NODE 126.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 769.05
DOWNSTREAM ELEVATION(FEET) = 768.35
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.89
TOTAL AREA(ACRES) = 0.30  TOTAL RUNOFF(CFS) = 0.89

*****
FLOW PROCESS FROM NODE 126.00 TO NODE 127.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 767.50  DOWNSTREAM ELEVATION(FEET) = 730.00
STREET LENGTH(FEET) = 935.00  CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      4.69
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.32
HALFSTREET FLOOD WIDTH(FEET) = 9.74
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.40
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.41
STREET FLOW TRAVEL TIME(MIN.) = 3.54  Tc(MIN.) = 12.27

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.576
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.17  SUBAREA RUNOFF(CFS) = 7.54
TOTAL AREA(ACRES) = 3.5  PEAK FLOW RATE(CFS) = 8.26

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.37  HALFSTREET FLOOD WIDTH(FEET) = 12.34
FLOW VELOCITY(FEET/SEC.) = 5.03  DEPTH*VELOCITY(FT*FT/SEC.) = 1.88
LONGEST FLOWPATH FROM NODE 125.00 TO NODE 127.00 = 1005.00 FEET.

*****
FLOW PROCESS FROM NODE 127.00 TO NODE 128.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.27
RAINFALL INTENSITY(INCH/HR) = 4.58
TOTAL STREAM AREA(ACRES) = 3.47
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.26

*****
FLOW PROCESS FROM NODE 130.00 TO NODE 131.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 740.00
DOWNSTREAM ELEVATION(FEET) = 730.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.57
TOTAL AREA(ACRES) = 0.23  TOTAL RUNOFF(CFS) = 0.57

*****
FLOW PROCESS FROM NODE 131.00 TO NODE 132.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 730.00  DOWNSTREAM(FEET) = 726.00
FLOW LENGTH(FEET) = 201.00  MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.96
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.57
PIPE TRAVEL TIME(MIN.) = 0.85  Tc(MIN.) = 7.11
LONGEST FLOWPATH FROM NODE 130.00 TO NODE 132.00 = 301.00 FEET.

*****
FLOW PROCESS FROM NODE 131.00 TO NODE 132.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.507
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.75  SUBAREA RUNOFF(CFS) = 1.71
TOTAL AREA(ACRES) = 1.0  TOTAL RUNOFF(CFS) = 2.23
Tc(MIN.) = 7.11

*****
FLOW PROCESS FROM NODE 132.00 TO NODE 128.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.11
RAINFALL INTENSITY(INCH/HR) = 6.51
TOTAL STREAM AREA(ACRES) = 0.98
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.23

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1       8.26      12.27    4.576       3.47
2       2.23      7.11     6.507       0.98

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF      Tc      INTENSITY
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)
1       7.02      7.11     6.507
2       9.83     12.27    4.576

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 9.83  Tc(MIN.) = 12.27

```

```

TOTAL AREA(ACRES) = 4.4
LONGEST FLOWPATH FROM NODE 125.00 TO NODE 128.00 = 1005.00 FEET.

*****
FLOW PROCESS FROM NODE 128.00 TO NODE 113.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 726.00 DOWNSTREAM(Feet) = 722.50
FLOW LENGTH(Feet) = 228.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.9 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 7.93
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.83
PIPE TRAVEL TIME(Min.) = 0.48 Tc(Min.) = 12.75
LONGEST FLOWPATH FROM NODE 125.00 TO NODE 113.00 = 1233.00 FEET.

*****
FLOW PROCESS FROM NODE 113.00 TO NODE 113.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (Min.) (Inch/Hour) (Acres)
1 9.83 12.75 4.465 4.45
LONGEST FLOWPATH FROM NODE 125.00 TO NODE 113.00 = 1233.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (Min.) (Inch/Hour) (Acres)
1 38.37 12.10 4.619 18.71
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 113.00 = 1781.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (Min.) (Inch/Hour)
1 47.69 12.10 4.619
2 46.91 12.75 4.465

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 47.69 Tc(Min.) = 12.10
TOTAL AREA(ACRES) = 23.2

*****
FLOW PROCESS FROM NODE 113.00 TO NODE 113.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE 113.00 TO NODE 133.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 717.50 DOWNSTREAM(Feet) = 709.50
FLOW LENGTH(Feet) = 240.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.4 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 15.60
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 47.69
PIPE TRAVEL TIME(Min.) = 0.26 Tc(Min.) = 12.35
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 133.00 = 2021.00 FEET.

*****
FLOW PROCESS FROM NODE 133.00 TO NODE 133.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(Min.) = 12.35
RAINFALL INTENSITY(INCH/HR) = 4.56
TOTAL STREAM AREA(ACRES) = 23.16
PEAK FLOW RATE(CFS) AT CONFLUENCE = 47.69

*****
FLOW PROCESS FROM NODE 135.00 TO NODE 136.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 70.00
UPSTREAM ELEVATION(Feet) = 732.15
DOWNSTREAM ELEVATION(Feet) = 731.45
ELEVATION DIFFERENCE(Feet) = 0.70
SUBAREA OVERLAND TIME OF FLOW(Min.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.71
TOTAL AREA(ACRES) = 0.24 TOTAL RUNOFF(CFS) = 0.71

*****
FLOW PROCESS FROM NODE 136.00 TO NODE 137.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(Feet) = 736.00 DOWNSTREAM ELEVATION(Feet) = 714.00

STREET LENGTH(Feet) = 660.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(Feet) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(Feet) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.28
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(Feet) = 0.30
HALFSTREET FLOOD WIDTH(Feet) = 8.62
AVERAGE FLOW VELOCITY(Feet/Sec.) = 3.80
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.14
STREET FLOW TRAVEL TIME(Min.) = 2.89 Tc(Min.) = 11.63
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.739
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 2.07 SUBAREA RUNOFF(CFS) = 5.10
TOTAL AREA(ACRES) = 2.3 PEAK FLOW RATE(CFS) = 5.69

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(Feet) = 0.34 HALFSTREET FLOOD WIDTH(Feet) = 10.93
FLOW VELOCITY(Feet/Sec.) = 4.33 DEPTH*VELOCITY(FT*FT/SEC.) = 1.49
LONGEST FLOWPATH FROM NODE 135.00 TO NODE 137.00 = 730.00 FEET.

*****
FLOW PROCESS FROM NODE 137.00 TO NODE 133.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(Min.) = 11.63
RAINFALL INTENSITY(INCH/HR) = 4.74
TOTAL STREAM AREA(ACRES) = 2.31
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.69

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (Min.) (Inch/Hour) (Acres)
1 47.69 12.35 4.557 23.16
2 5.69 11.63 4.739 2.31

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (Min.) (Inch/Hour)
1 50.57 11.63 4.739
2 53.17 12.35 4.557

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 53.17 Tc(Min.) = 12.35
TOTAL AREA(ACRES) = 25.5
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 133.00 = 2021.00 FEET.

*****
FLOW PROCESS FROM NODE 133.00 TO NODE 138.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 709.50 DOWNSTREAM(Feet) = 706.50
FLOW LENGTH(Feet) = 55.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.5 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 19.48
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 53.17
PIPE TRAVEL TIME(Min.) = 0.05 Tc(Min.) = 12.40
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 138.00 = 2076.00 FEET.

*****
FLOW PROCESS FROM NODE 138.00 TO NODE 138.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(Min.) = 12.40
RAINFALL INTENSITY(INCH/HR) = 4.55
TOTAL STREAM AREA(ACRES) = 25.47
PEAK FLOW RATE(CFS) AT CONFLUENCE = 53.17

*****
FLOW PROCESS FROM NODE 140.00 TO NODE 141.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4900
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 70.00
UPSTREAM ELEVATION(Feet) = 732.95
DOWNSTREAM ELEVATION(Feet) = 732.25

```

```

ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.186
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.517
SUBAREA RUNOFF(CFS) = 1.03
TOTAL AREA(ACRES) = 0.38 TOTAL RUNOFF(CFS) = 1.03

*****
FLOW PROCESS FROM NODE 141.00 TO NODE 142.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 731.00 DOWNSTREAM ELEVATION(FEET) = 711.00
STREET LENGTH(FEET) = 1013.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.82
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.35
HALFSTREET FLOOD WIDTH(FEET) = 11.43
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.39
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.20
STREET FLOW TRAVEL TIME(MIN.) = 4.98 Tc(MIN.) = 14.17
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.172
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4900
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.490
SUBAREA AREA(ACRES) = 3.67 SUBAREA RUNOFF(CFS) = 7.50
TOTAL AREA(ACRES) = 4.1 PEAK FLOW RATE(CFS) = 8.28

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.41 HALFSTREET FLOOD WIDTH(FEET) = 14.24
FLOW VELOCITY(FEET/SEC.) = 3.86 DEPTH*VELOCITY(FT*FT/SEC.) = 1.59
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 142.00 = 1083.00 FEET.

*****
FLOW PROCESS FROM NODE 142.00 TO NODE 138.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 707.00 DOWNSTREAM(FEET) = 706.00
FLOW LENGTH(FEET) = 36.54 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.53
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 8.28
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 14.23
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 138.00 = 1119.54 FEET.

*****
FLOW PROCESS FROM NODE 138.00 TO NODE 138.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.23
RAINFALL INTENSITY(INCH/HR) = 4.16
TOTAL STREAM AREA(ACRES) = 4.05
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.28

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 53.17 12.40 4.546 25.47
2 8.28 14.23 4.159 4.05

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 60.38 12.40 4.546
2 56.92 14.23 4.159

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 60.38 Tc(MIN.) = 12.40
TOTAL AREA(ACRES) = 29.5
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 138.00 = 2076.00 FEET.

*****
FLOW PROCESS FROM NODE 138.00 TO NODE 138.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====
*****
FLOW PROCESS FROM NODE 150.00 TO NODE 151.00 IS CODE = 21
-----

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 753.35
DOWNSTREAM ELEVATION(FEET) = 752.65
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 70.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.68
TOTAL AREA(ACRES) = 0.23 TOTAL RUNOFF(CFS) = 0.68

*****
FLOW PROCESS FROM NODE 151.00 TO NODE 152.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 751.80 DOWNSTREAM ELEVATION(FEET) = 723.50
STREET LENGTH(FEET) = 545.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.38
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.74
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.75
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.43
STREET FLOW TRAVEL TIME(MIN.) = 1.91 Tc(MIN.) = 10.65
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.016
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 5.88 SUBAREA RUNOFF(CFS) = 15.34
TOTAL AREA(ACRES) = 6.1 PEAK FLOW RATE(CFS) = 15.94

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.36 HALFSTREET FLOOD WIDTH(FEET) = 11.50
FLOW VELOCITY(FEET/SEC.) = 5.54 DEPTH*VELOCITY(FT*FT/SEC.) = 1.97
LONGEST FLOWPATH FROM NODE 150.00 TO NODE 152.00 = 615.00 FEET.

*****
FLOW PROCESS FROM NODE 152.00 TO NODE 153.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 718.50 DOWNSTREAM(FEET) = 708.00
FLOW LENGTH(FEET) = 624.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.24
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 15.94
PIPE TRAVEL TIME(MIN.) = 1.13 Tc(MIN.) = 11.77
LONGEST FLOWPATH FROM NODE 150.00 TO NODE 153.00 = 1239.00 FEET.

*****
FLOW PROCESS FROM NODE 153.00 TO NODE 153.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.77
RAINFALL INTENSITY(INCH/HR) = 4.70
TOTAL STREAM AREA(ACRES) = 6.11
PEAK FLOW RATE(CFS) AT CONFLUENCE = 15.94

*****
FLOW PROCESS FROM NODE 155.00 TO NODE 156.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 732.00
DOWNSTREAM ELEVATION(FEET) = 730.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.156
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.142
SUBAREA RUNOFF(CFS) = 0.45
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.45

*****
FLOW PROCESS FROM NODE 156.00 TO NODE 157.00 IS CODE = 61
-----

```



```

-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 730.00 DOWNSTREAM ELEVATION(FEET) = 713.00
STREET LENGTH(FEET) = 942.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.45
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.39
HALFSTREET FLOOD WIDTH(FEET) = 13.11
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.51
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.36
STREET FLOW TRAVEL TIME(MIN.) = 4.47 Tc(MIN.) = 10.63
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.022
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 4.51 SUBAREA RUNOFF(CFS) = 11.78
TOTAL AREA(ACRES) = 4.6 PEAK FLOW RATE(CFS) = 12.09

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.46 HALFSTREET FLOOD WIDTH(FEET) = 16.84
FLOW VELOCITY(FEET/SEC.) = 4.09 DEPTH*VELOCITY(FT*FT/SEC.) = 1.90
LONGEST FLOWPATH FROM NODE 155.00 TO NODE 157.00 = 1012.00 FEET.

*****
FLOW PROCESS FROM NODE 157.00 TO NODE 153.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.63
RAINFALL INTENSITY(INCH/HR) = 5.02
TOTAL STREAM AREA(ACRES) = 4.63
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.09

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 15.94 11.77 4.702 6.11
2 12.09 10.63 5.022 4.63

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 26.48 10.63 5.022
2 27.26 11.77 4.702

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 27.26 Tc(MIN.) = 11.77
TOTAL AREA(ACRES) = 10.7
LONGEST FLOWPATH FROM NODE 150.00 TO NODE 153.00 = 1239.00 FEET.

*****
FLOW PROCESS FROM NODE 153.00 TO NODE 138.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 708.00 DOWNSTREAM(FEET) = 706.50
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.96
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 27.26
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 11.86
LONGEST FLOWPATH FROM NODE 150.00 TO NODE 138.00 = 1303.00 FEET.

*****
FLOW PROCESS FROM NODE 138.00 TO NODE 138.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 27.26 11.86 4.679 10.74
LONGEST FLOWPATH FROM NODE 150.00 TO NODE 138.00 = 1303.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 60.38 12.40 4.546 29.52
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 138.00 = 2076.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 85.00 11.86 4.679
2 86.86 12.40 4.546

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 86.86 Tc(MIN.) = 12.40
TOTAL AREA(ACRES) = 40.3

*****
FLOW PROCESS FROM NODE 138.00 TO NODE 138.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1<<<<
=====

*****
FLOW PROCESS FROM NODE 138.00 TO NODE 158.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 706.50 DOWNSTREAM(FEET) = 692.50
FLOW LENGTH(FEET) = 302.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.68
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 86.86
PIPE TRAVEL TIME(MIN.) = 0.24 Tc(MIN.) = 12.64
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 158.00 = 2378.00 FEET.

*****
FLOW PROCESS FROM NODE 158.00 TO NODE 158.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1<<<<
=====

*****
FLOW PROCESS FROM NODE 170.00 TO NODE 171.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4900
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 0.22
UPSTREAM ELEVATION(FEET) = 798.35
DOWNSTREAM ELEVATION(FEET) = 797.65
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 0.239
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.84
TOTAL AREA(ACRES) = 0.21 TOTAL RUNOFF(CFS) = 0.84

*****
FLOW PROCESS FROM NODE 171.00 TO NODE 172.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 797.00 DOWNSTREAM ELEVATION(FEET) = 734.00
STREET LENGTH(FEET) = 646.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.22
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.92
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.19
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.76
STREET FLOW TRAVEL TIME(MIN.) = 1.74 Tc(MIN.) = 1.98
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4900
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.490
SUBAREA AREA(ACRES) = 4.19 SUBAREA RUNOFF(CFS) = 16.77
TOTAL AREA(ACRES) = 4.4 PEAK FLOW RATE(CFS) = 17.61

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.51
FLOW VELOCITY(FEET/SEC.) = 7.20 DEPTH*VELOCITY(FT*FT/SEC.) = 2.42
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 172.00 = 646.22 FEET.

*****
FLOW PROCESS FROM NODE 172.00 TO NODE 173.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 729.20 DOWNSTREAM(FEET) = 693.00

```

```

FLOW LENGTH(Feet) = 606.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.1 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 15.34
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.61
PIPE TRAVEL TIME(MIN.) = 0.66 Tc(MIN.) = 2.64
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 173.00 = 1252.22 FEET.

*****
FLOW PROCESS FROM NODE 173.00 TO NODE 173.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 2.64
RAINFALL INTENSITY(INCH/HR) = 8.17
TOTAL STREAM AREA(ACRES) = 4.40
PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.61

*****
FLOW PROCESS FROM NODE 175.00 TO NODE 176.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 70.00
UPSTREAM ELEVATION(Feet) = 729.83
DOWNSTREAM ELEVATION(Feet) = 729.15
ELEVATION DIFFERENCE(Feet) = 0.68
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.747
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 68.86
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.694
SUBAREA RUNOFF(CFS) = 0.65
TOTAL AREA(ACRES) = 0.22 TOTAL RUNOFF(CFS) = 0.65

*****
FLOW PROCESS FROM NODE 176.00 TO NODE 177.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
-----
UPSTREAM ELEVATION(Feet) = 729.00 DOWNSTREAM ELEVATION(Feet) = 698.00
STREET LENGTH(Feet) = 571.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(Feet) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(Feet) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.21
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(Feet) = 0.26
HALFSTREET FLOOD WIDTH(Feet) = 6.92
AVERAGE FLOW VELOCITY(Feet/Sec.) = 4.36
PRODUCT OF DEPTH&VELOCITY(Ft*Ft/Sec.) = 1.15
STREET FLOW TRAVEL TIME(MIN.) = 2.18 Tc(MIN.) = 10.93
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.932
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.54 SUBAREA RUNOFF(CFS) = 9.08
TOTAL AREA(ACRES) = 3.8 PEAK FLOW RATE(CFS) = 9.64

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(Feet) = 0.31 HALFSTREET FLOOD WIDTH(Feet) = 9.18
FLOW VELOCITY(Feet/Sec.) = 5.02 DEPTH*VELOCITY(Ft*Ft/Sec.) = 1.56
LONGEST FLOWPATH FROM NODE 175.00 TO NODE 177.00 = 641.00 FEET.

*****
FLOW PROCESS FROM NODE 177.00 TO NODE 173.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.93
RAINFALL INTENSITY(INCH/HR) = 4.93
TOTAL STREAM AREA(ACRES) = 3.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.64

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 17.61 2.64 8.168 4.40
2 9.64 10.93 4.932 3.76

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

```

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	17.61	2.64	8.168	4.40
2	9.64	10.93	4.932	3.76

```

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 20.28 Tc(MIN.) = 10.93
TOTAL AREA(ACRES) = 8.2
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 173.00 = 1252.22 FEET.

*****
FLOW PROCESS FROM NODE 173.00 TO NODE 158.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(Feet) = 693.00 DOWNSTREAM(Feet) = 692.50
FLOW LENGTH(Feet) = 60.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.2 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 7.58
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 20.28
PIPE TRAVEL TIME(MIN.) = 0.13 Tc(MIN.) = 11.06
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 158.00 = 1312.22 FEET.

*****
FLOW PROCESS FROM NODE 158.00 TO NODE 158.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
-----
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 20.28 11.06 4.894 8.16
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 158.00 = 1312.22 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 86.86 12.64 4.490 40.26
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 158.00 = 2378.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 96.26 11.06 4.894
2 105.46 12.64 4.490

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 105.46 Tc(MIN.) = 12.64
TOTAL AREA(ACRES) = 48.4

*****
FLOW PROCESS FROM NODE 158.00 TO NODE 158.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
-----
*****
FLOW PROCESS FROM NODE 158.00 TO NODE 178.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
-----
ELEVATION DATA: UPSTREAM(Feet) = 692.50 DOWNSTREAM(Feet) = 656.60
FLOW LENGTH(Feet) = 449.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 23.0 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 26.17
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 105.46
PIPE TRAVEL TIME(MIN.) = 0.29 Tc(MIN.) = 12.93
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 178.00 = 2827.00 FEET.

*****
FLOW PROCESS FROM NODE 178.00 TO NODE 178.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.93
RAINFALL INTENSITY(INCH/HR) = 4.43
TOTAL STREAM AREA(ACRES) = 48.42
PEAK FLOW RATE(CFS) AT CONFLUENCE = 105.46

*****
FLOW PROCESS FROM NODE 180.00 TO NODE 181.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
-----
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 100.00
UPSTREAM ELEVATION(Feet) = 670.00
DOWNSTREAM ELEVATION(Feet) = 660.00
ELEVATION DIFFERENCE(Feet) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.99
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 0.99

```

```

*****
FLOW PROCESS FROM NODE      181.00 TO NODE      182.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 660.00 DOWNSTREAM(FEET) = 658.00
FLOW LENGTH(FEET) = 545.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 2.57
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.99
PIPE TRAVEL TIME(MIN.) = 3.53 Tc(MIN.) = 9.80
LONGEST FLOWPATH FROM NODE 180.00 TO NODE 182.00 = 645.00 FEET.

*****
FLOW PROCESS FROM NODE      181.00 TO NODE      182.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.291
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 3.33 SUBAREA RUNOFF(CFS) = 6.17
TOTAL AREA(ACRES) = 3.7 TOTAL RUNOFF(CFS) = 6.91
Tc(MIN.) = 9.80

*****
FLOW PROCESS FROM NODE      182.00 TO NODE      178.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.80
RAINFALL INTENSITY(INCH/HR) = 5.29
TOTAL STREAM AREA(ACRES) = 3.73
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.91

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1       105.46    12.93    4.425        48.42
2        6.91     9.80     5.291         3.73

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF      Tc      INTENSITY
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)
1       86.84     9.80     5.291
2      111.24    12.93    4.425

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 111.24 Tc(MIN.) = 12.93
TOTAL AREA(ACRES) = 52.2
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 178.00 = 2827.00 FEET.

*****
FLOW PROCESS FROM NODE      178.00 TO NODE      183.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 656.60 DOWNSTREAM(FEET) = 652.00
FLOW LENGTH(FEET) = 125.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 19.94
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 111.24
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 13.04
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 183.00 = 2952.00 FEET.

*****
FLOW PROCESS FROM NODE      183.00 TO NODE      183.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.04
RAINFALL INTENSITY(INCH/HR) = 4.40
TOTAL STREAM AREA(ACRES) = 52.15
PEAK FLOW RATE(CFS) AT CONFLUENCE = 111.24

*****
FLOW PROCESS FROM NODE      185.00 TO NODE      186.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 712.55
DOWNSTREAM ELEVATION(FEET) = 711.85
ELEVATION DIFFERENCE(FEET) = 0.70

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.735
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.699
SUBAREA RUNOFF(CFS) = 0.56
TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.56

*****
FLOW PROCESS FROM NODE      186.00 TO NODE      187.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 711.00 DOWNSTREAM ELEVATION(FEET) = 656.50
STREET LENGTH(FEET) = 805.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.65
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.25
HALFSTREET FLOOD WIDTH(FEET) = 6.16
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.68
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.17
STREET FLOW TRAVEL TIME(MIN.) = 2.87 Tc(MIN.) = 11.60
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.746
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
SUBAREA AREA(ACRES) = 3.30 SUBAREA RUNOFF(CFS) = 8.14
TOTAL AREA(ACRES) = 3.5 PEAK FLOW RATE(CFS) = 8.61

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.29 HALFSTREET FLOOD WIDTH(FEET) = 8.33
FLOW VELOCITY(FEET/SEC.) = 5.31 DEPTH*VELOCITY(FT*FT/SEC.) = 1.55
LONGEST FLOWPATH FROM NODE 185.00 TO NODE 187.00 = 875.00 FEET.

*****
FLOW PROCESS FROM NODE      187.00 TO NODE      183.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.60
RAINFALL INTENSITY(INCH/HR) = 4.75
TOTAL STREAM AREA(ACRES) = 3.49
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.61

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1      111.24    13.04    4.402        52.15
2        8.61    11.60    4.746         3.49

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF      Tc      INTENSITY
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)
1      111.80    11.60    4.746
2      119.23    13.04    4.402

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 119.23 Tc(MIN.) = 13.04
TOTAL AREA(ACRES) = 55.6
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 183.00 = 2952.00 FEET.

*****
FLOW PROCESS FROM NODE      183.00 TO NODE      188.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 651.50 DOWNSTREAM(FEET) = 647.50
FLOW LENGTH(FEET) = 97.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 21.14
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 119.23
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 13.11
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 188.00 = 3049.00 FEET.

*****
FLOW PROCESS FROM NODE      188.00 TO NODE      188.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      200.00 TO NODE      201.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

```



```

*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 250.00
DOWNSTREAM ELEVATION(FEET) = 240.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.69
TOTAL AREA(ACRES) = 0.28 TOTAL RUNOFF(CFS) = 0.69

*****
FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 740.00 DOWNSTREAM(FEET) = 680.00
FLOW LENGTH(FEET) = 760.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.84
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.69
PIPE TRAVEL TIME(MIN.) = 1.85 Tc(MIN.) = 8.12
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 860.00 FEET.

*****
FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.975
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 3.18 SUBAREA RUNOFF(CFS) = 6.65
TOTAL AREA(ACRES) = 3.5 TOTAL RUNOFF(CFS) = 7.24
TC(MIN.) = 8.12

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 680.00 DOWNSTREAM(FEET) = 665.00
FLOW LENGTH(FEET) = 137.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.25
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.24
PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 8.27
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 203.00 = 997.00 FEET.

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 8.27
RAINFALL INTENSITY(INCH/HR) = 5.90
TOTAL STREAM AREA(ACRES) = 3.46
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.24

*****
FLOW PROCESS FROM NODE 205.00 TO NODE 206.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 769.00
DOWNSTREAM ELEVATION(FEET) = 763.50
ELEVATION DIFFERENCE(FEET) = 5.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.227
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.937
SUBAREA RUNOFF(CFS) = 0.72
TOTAL AREA(ACRES) = 0.22 TOTAL RUNOFF(CFS) = 0.72

*****
FLOW PROCESS FROM NODE 206.00 TO NODE 207.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 766.00 DOWNSTREAM ELEVATION(FEET) = 665.00
STREET LENGTH(FEET) = 1128.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.99
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.23
HALFSTREET FLOOD WIDTH(FEET) = 5.22
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.11
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.18
STREET FLOW TRAVEL TIME(MIN.) = 3.68 Tc(MIN.) = 8.91
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.627
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.410
SUBAREA AREA(ACRES) = 2.80 SUBAREA RUNOFF(CFS) = 6.46
TOTAL AREA(ACRES) = 3.0 PEAK FLOW RATE(CFS) = 6.97

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.27 HALFSTREET FLOOD WIDTH(FEET) = 7.04
FLOW VELOCITY(FEET/SEC.) = 5.68 DEPTH*VELOCITY(FT*FT/SEC.) = 1.52
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 207.00 = 1198.00 FEET.

*****
FLOW PROCESS FROM NODE 207.00 TO NODE 203.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.91
RAINFALL INTENSITY(INCH/HR) = 5.63
TOTAL STREAM AREA(ACRES) = 3.02
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.97

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 7.24 8.27 5.905 3.46
2 6.97 8.91 5.627 3.02

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 13.70 8.27 5.905
2 13.86 8.91 5.627

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 13.86 Tc(MIN.) = 8.91
TOTAL AREA(ACRES) = 6.5
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 203.00 = 1198.00 FEET.

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 208.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 660.00 DOWNSTREAM(FEET) = 651.50
FLOW LENGTH(FEET) = 517.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.91
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 13.86
PIPE TRAVEL TIME(MIN.) = 0.97 Tc(MIN.) = 9.88
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 208.00 = 1715.00 FEET.

*****
FLOW PROCESS FROM NODE 208.00 TO NODE 208.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 9.88
RAINFALL INTENSITY(INCH/HR) = 5.27
TOTAL STREAM AREA(ACRES) = 6.48
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.86

*****
FLOW PROCESS FROM NODE 210.00 TO NODE 211.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 662.05
DOWNSTREAM ELEVATION(FEET) = 661.35
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.691
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 65.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.187
SUBAREA RUNOFF(CFS) = 0.78

```

```

TOTAL AREA(ACRES) =      0.22    TOTAL RUNOFF(CFS) =      0.78

*****
FLOW PROCESS FROM NODE      211.00 TO NODE      212.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 660.50  DOWNSTREAM ELEVATION(FEET) = 656.50
STREET LENGTH(FEET) = 362.00  CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.93
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.79
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.20
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.67
STREET FLOW TRAVEL TIME(MIN.) = 2.74  Tc(MIN.) = 10.43
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.083
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.570
SUBAREA AREA(ACRES) = 2.16  SUBAREA RUNOFF(CFS) = 6.26
TOTAL AREA(ACRES) = 2.4  PEAK FLOW RATE(CFS) = 6.90

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35  HALFSTREET FLOOD WIDTH(FEET) = 11.21
FLOW VELOCITY(FEET/SEC.) = 2.51  DEPTH*VELOCITY(FT*FT/SEC.) = 0.88
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 212.00 = 432.00 FEET.

*****
FLOW PROCESS FROM NODE      212.00 TO NODE      208.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.43
RAINFALL INTENSITY(INCH/HR) = 5.08
TOTAL STREAM AREA(ACRES) = 2.38
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.90

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1       13.86      9.88    5.265        6.48
2       6.90      10.43   5.083        2.38

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF      Tc      INTENSITY
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)
1       20.39      9.88    5.265
2       20.28      10.43   5.083

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 20.39  Tc(MIN.) = 9.88
TOTAL AREA(ACRES) = 8.9
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 208.00 = 1715.00 FEET.

*****
FLOW PROCESS FROM NODE      208.00 TO NODE      213.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 651.50  DOWNSTREAM(FEET) = 650.00
FLOW LENGTH(FEET) = 629.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.72
ESTIMATED PIPE DIAMETER(INCH) = 33.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 20.39
PIPE TRAVEL TIME(MIN.) = 2.22  Tc(MIN.) = 12.10
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 213.00 = 2344.00 FEET.

*****
FLOW PROCESS FROM NODE      213.00 TO NODE      213.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.10
RAINFALL INTENSITY(INCH/HR) = 4.62
TOTAL STREAM AREA(ACRES) = 8.86
PEAK FLOW RATE(CFS) AT CONFLUENCE = 20.39

*****
FLOW PROCESS FROM NODE      215.00 TO NODE      216.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 658.05
DOWNSTREAM ELEVATION(FEET) = 657.35
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.691
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 65.00
(Reference: Table 3-18 of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.187
SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 0.10  TOTAL RUNOFF(CFS) = 0.35

*****
FLOW PROCESS FROM NODE      216.00 TO NODE      217.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 656.50  DOWNSTREAM ELEVATION(FEET) = 645.00
STREET LENGTH(FEET) = 597.00  CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.63
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.33
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.85
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.84
STREET FLOW TRAVEL TIME(MIN.) = 3.49  Tc(MIN.) = 11.18
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.861
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.570
SUBAREA AREA(ACRES) = 3.06  SUBAREA RUNOFF(CFS) = 8.48
TOTAL AREA(ACRES) = 3.2  PEAK FLOW RATE(CFS) = 8.76

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35  HALFSTREET FLOOD WIDTH(FEET) = 11.00
FLOW VELOCITY(FEET/SEC.) = 3.29  DEPTH*VELOCITY(FT*FT/SEC.) = 1.14
LONGEST FLOWPATH FROM NODE 215.00 TO NODE 217.00 = 667.00 FEET.

*****
FLOW PROCESS FROM NODE      217.00 TO NODE      213.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.18
RAINFALL INTENSITY(INCH/HR) = 4.86
TOTAL STREAM AREA(ACRES) = 3.16
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.76

** CONFLUENCE DATA **
STREAM  RUNOFF      Tc      INTENSITY      AREA
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1       20.39      12.10   4.619        8.86
2       8.76      11.18   4.861        3.16

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF      Tc      INTENSITY
NUMBER  (CFS)      (MIN.)  (INCH/HOUR)
1       28.13      11.18   4.861
2       28.71      12.10   4.619

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 28.71  Tc(MIN.) = 12.10
TOTAL AREA(ACRES) = 12.0
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 213.00 = 2344.00 FEET.

*****
FLOW PROCESS FROM NODE      213.00 TO NODE      188.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 645.00  DOWNSTREAM(FEET) = 644.00
FLOW LENGTH(FEET) = 53.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.97
ESTIMATED PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 28.71

```

```

PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 12.18
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 188.00 = 2397.00 FEET.

*****
FLOW PROCESS FROM NODE 188.00 TO NODE 188.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 28.71 12.18 4.600 12.02
LONGEST FLOWPATH FROM NODE 205.00 TO NODE 188.00 = 2397.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 119.23 13.11 4.386 55.64
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 188.00 = 3049.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/ HOUR)
1 139.46 12.18 4.600
2 146.61 13.11 4.386

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 146.61 Tc(MIN.) = 13.11
TOTAL AREA(ACRES) = 67.7

*****
FLOW PROCESS FROM NODE 188.00 TO NODE 188.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE 188.00 TO NODE 218.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 650.00 DOWNSTREAM(FEET) = 638.00
FLOW LENGTH(FEET) = 240.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 26.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 24.18
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 146.61
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 13.28
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 218.00 = 3289.00 FEET.

*****
FLOW PROCESS FROM NODE 218.00 TO NODE 218.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.28
RAINFALL INTENSITY(INCH/HR) = 4.35
TOTAL STREAM AREA(ACRES) = 67.66
PEAK FLOW RATE(CFS) AT CONFLUENCE = 146.61

*****
FLOW PROCESS FROM NODE 220.00 TO NODE 221.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 653.00
DOWNSTREAM ELEVATION(FEET) = 652.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.227
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.502
SUBAREA RUNOFF(CFS) = 0.25
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.25

*****
FLOW PROCESS FROM NODE 221.00 TO NODE 222.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 652.00 DOWNSTREAM(FEET) = 650.00
FLOW LENGTH(FEET) = 223.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 2.34
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.25
PIPE TRAVEL TIME(MIN.) = 1.59 Tc(MIN.) = 10.81
LONGEST FLOWPATH FROM NODE 220.00 TO NODE 222.00 = 293.00 FEET.

*****
FLOW PROCESS FROM NODE 221.00 TO NODE 222.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.966

*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4700
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4635
SUBAREA AREA(ACRES) = 0.91 SUBAREA RUNOFF(CFS) = 2.12
TOTAL AREA(ACRES) = 1.0 TOTAL RUNOFF(CFS) = 2.35
Tc(MIN.) = 10.81

*****
FLOW PROCESS FROM NODE 222.00 TO NODE 218.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 647.00 DOWNSTREAM(FEET) = 638.00
FLOW LENGTH(FEET) = 83.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.97
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.35
PIPE TRAVEL TIME(MIN.) = 0.13 Tc(MIN.) = 10.94
LONGEST FLOWPATH FROM NODE 220.00 TO NODE 218.00 = 376.00 FEET.

*****
FLOW PROCESS FROM NODE 218.00 TO NODE 218.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.94
RAINFALL INTENSITY(INCH/HR) = 4.93
TOTAL STREAM AREA(ACRES) = 1.02
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.35

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 146.61 13.28 4.350 67.66
2 2.35 10.94 4.929 1.02

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/ HOUR)
1 123.15 10.94 4.929
2 148.68 13.28 4.350

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 148.68 Tc(MIN.) = 13.28
TOTAL AREA(ACRES) = 68.7
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 218.00 = 3289.00 FEET.

*****
FLOW PROCESS FROM NODE 218.00 TO NODE 223.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 638.00 DOWNSTREAM(FEET) = 628.00
FLOW LENGTH(FEET) = 175.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 28.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 25.03
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 148.68
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 13.39
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 223.00 = 3464.00 FEET.

*****
FLOW PROCESS FROM NODE 223.00 TO NODE 223.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.39
RAINFALL INTENSITY(INCH/HR) = 4.33
TOTAL STREAM AREA(ACRES) = 68.68
PEAK FLOW RATE(CFS) AT CONFLUENCE = 148.68

*****
FLOW PROCESS FROM NODE 225.00 TO NODE 226.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 650.00
DOWNSTREAM ELEVATION(FEET) = 640.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.59
TOTAL AREA(ACRES) = 0.24 TOTAL RUNOFF(CFS) = 0.59

*****

```



```

FLOW PROCESS FROM NODE      226.00 TO NODE      227.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   640.00 DOWNSTREAM(FEET) =   630.00
FLOW LENGTH(FEET) =  1078.00 MANNING'S N =  0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS   3.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   3.07
ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =   0.59
PIPE TRAVEL TIME(MIN.) =   5.85    Tc(MIN.) =  12.12
LONGEST FLOWPATH FROM NODE      225.00 TO NODE      227.00 =   1178.00 FEET.

*****
FLOW PROCESS FROM NODE      226.00 TO NODE      227.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  4.615
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) =  0
AREA-AVERAGE RUNOFF COEFFICIENT =  0.3500
SUBAREA AREA(ACRES) =   1.48    SUBAREA RUNOFF(CFS) =   2.39
TOTAL AREA(ACRES) =   1.7    TOTAL RUNOFF(CFS) =   2.78
Tc(MIN.) =  12.12

*****
FLOW PROCESS FROM NODE      227.00 TO NODE      223.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   628.00 DOWNSTREAM(FEET) =   627.00
FLOW LENGTH(FEET) =   42.00 MANNING'S N =  0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS   5.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   6.74
ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =   2.78
PIPE TRAVEL TIME(MIN.) =   0.10    Tc(MIN.) =  12.22
LONGEST FLOWPATH FROM NODE      225.00 TO NODE      223.00 =   1220.00 FEET.

*****
FLOW PROCESS FROM NODE      223.00 TO NODE      223.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) =  12.22
RAINFALL INTENSITY(INCH/HR) =  4.59
TOTAL STREAM AREA(ACRES) =   1.72
PEAK FLOW RATE(CFS) AT CONFLUENCE =   2.78

** CONFLUENCE DATA **
STREAM   RUNOFF      Tc      INTENSITY      AREA
NUMBER   (CFS)      (MIN.)   (INCH/HOUR)   (ACRE)
  1      148.68    13.39      4.326      68.68
  2       2.78    12.22      4.589       1.72

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM   RUNOFF      Tc      INTENSITY
NUMBER   (CFS)      (MIN.)   (INCH/HOUR)
  1      142.92    12.22      4.589
  2      151.30    13.39      4.326

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =   151.30    Tc(MIN.) =  13.39
TOTAL AREA(ACRES) =   70.4
LONGEST FLOWPATH FROM NODE      120.00 TO NODE      223.00 =   3464.00 FEET.

*****
FLOW PROCESS FROM NODE      223.00 TO NODE      228.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   630.00 DOWNSTREAM(FEET) =   620.70
FLOW LENGTH(FEET) =  145.00 MANNING'S N =  0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS  27.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  26.42
ESTIMATED PIPE DIAMETER(INCH) =  36.00    NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =  151.30
PIPE TRAVEL TIME(MIN.) =   0.09    Tc(MIN.) =  13.49
LONGEST FLOWPATH FROM NODE      120.00 TO NODE      228.00 =   3609.00 FEET.

*****
FLOW PROCESS FROM NODE      228.00 TO NODE      228.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      240.00 TO NODE      241.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

*****
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) =  0
INITIAL SUBAREA FLOW-LENGTH(FEET) =   70.00
UPSTREAM ELEVATION(FEET) =   634.05

*****
FLOW PROCESS FROM NODE      241.00 TO NODE      242.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) =   650.00 DOWNSTREAM ELEVATION(FEET) =   633.00
STREET LENGTH(FEET) =   506.00 CURB HEIGHT(INCHES) =   6.0
STREET HALFWIDTH(FEET) =   18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =   9.00
INSIDE STREET CROSSFALL(DECIMAL) =  0.020
OUTSIDE STREET CROSSFALL(DECIMAL) =  0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) =  0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =  0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section =  0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =   4.96
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) =  0.28
HALFSTREET FLOOD WIDTH(FEET) =   7.56
AVERAGE FLOW VELOCITY(FEET/SEC.) =   3.59
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =   1.00
STREET FLOW TRAVEL TIME(MIN.) =   2.35    Tc(MIN.) =   6.05
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  7.222
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) =  0
AREA-AVERAGE RUNOFF COEFFICIENT =  0.570
SUBAREA AREA(ACRES) =   2.02    SUBAREA RUNOFF(CFS) =   8.32
TOTAL AREA(ACRES) =   2.2    PEAK FLOW RATE(CFS) =   9.01

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) =  0.32 HALFSTREET FLOOD WIDTH(FEET) =   9.88
FLOW VELOCITY(FEET/SEC.) =  4.12 DEPTH*VELOCITY(FT*FT/SEC.) =  1.33
LONGEST FLOWPATH FROM NODE      240.00 TO NODE      242.00 =   576.00 FEET.

*****
FLOW PROCESS FROM NODE      242.00 TO NODE      229.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =   6.05
RAINFALL INTENSITY(INCH/HR) =   7.22
TOTAL STREAM AREA(ACRES) =   2.19
PEAK FLOW RATE(CFS) AT CONFLUENCE =   9.01

*****
FLOW PROCESS FROM NODE      229.00 TO NODE      243.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   627.50 DOWNSTREAM(FEET) =   623.50
FLOW LENGTH(FEET) =   404.00 MANNING'S N =  0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS  13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   6.51
ESTIMATED PIPE DIAMETER(INCH) =  18.00    NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =   9.01
PIPE TRAVEL TIME(MIN.) =   1.03    Tc(MIN.) =   7.09
LONGEST FLOWPATH FROM NODE      225.00 TO NODE      243.00 =   1624.00 FEET.

*****
FLOW PROCESS FROM NODE      243.00 TO NODE      243.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =   7.09
RAINFALL INTENSITY(INCH/HR) =   6.52
TOTAL STREAM AREA(ACRES) =   2.19
PEAK FLOW RATE(CFS) AT CONFLUENCE =   9.01

*****
FLOW PROCESS FROM NODE      245.00 TO NODE      246.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) =  0
INITIAL SUBAREA FLOW-LENGTH(FEET) =   70.00
UPSTREAM ELEVATION(FEET) =   634.05

```

```

DOWNSTREAM ELEVATION(FEET) = 633.35
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.691
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 65.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.187
SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.35

*****
FLOW PROCESS FROM NODE 246.00 TO NODE 247.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
-----
UPSTREAM ELEVATION(FEET) = 632.50 DOWNSTREAM ELEVATION(FEET) = 628.50
STREET LENGTH(FEET) = 388.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.21
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.15
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.05
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.59
STREET FLOW TRAVEL TIME(MIN.) = 3.15 Tc(MIN.) = 10.84
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.958
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.570
SUBAREA AREA(ACRES) = 2.01 SUBAREA RUNOFF(CFS) = 5.68
TOTAL AREA(ACRES) = 2.1 PEAK FLOW RATE(CFS) = 5.96

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.72
FLOW VELOCITY(FEET/SEC.) = 2.35 DEPTH*VELOCITY(FT*FT/SEC.) = 0.80
LONGEST FLOWPATH FROM NODE 245.00 TO NODE 247.00 = 458.00 FEET.

*****
FLOW PROCESS FROM NODE 247.00 TO NODE 243.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.84
RAINFALL INTENSITY(INCH/HR) = 4.96
TOTAL STREAM AREA(ACRES) = 2.11
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.96

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 9.01 7.09 6.523 2.19
2 5.96 10.84 4.958 2.11

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 12.91 7.09 6.523
2 12.82 10.84 4.958

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 12.91 Tc(MIN.) = 7.09
TOTAL AREA(ACRES) = 4.3
LONGEST FLOWPATH FROM NODE 240.00 TO NODE 243.00 = 576.00 FEET.

*****
FLOW PROCESS FROM NODE 243.00 TO NODE 248.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 623.50 DOWNSTREAM(FEET) = 619.00
FLOW LENGTH(FEET) = 438.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.27
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.91
PIPE TRAVEL TIME(MIN.) = 1.00 Tc(MIN.) = 8.09
LONGEST FLOWPATH FROM NODE 240.00 TO NODE 248.00 = 1014.00 FEET.

*****
FLOW PROCESS FROM NODE 248.00 TO NODE 248.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 8.09
RAINFALL INTENSITY(INCH/HR) = 5.99
TOTAL STREAM AREA(ACRES) = 4.30
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.91

*****
FLOW PROCESS FROM NODE 250.00 TO NODE 251.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 630.15
DOWNSTREAM ELEVATION(FEET) = 629.45
ELEVATION DIFFERENCE(FEET) = 0.70
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.691
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 65.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.187
SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.35

*****
FLOW PROCESS FROM NODE 251.00 TO NODE 252.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
-----
UPSTREAM ELEVATION(FEET) = 628.50 DOWNSTREAM ELEVATION(FEET) = 624.00
STREET LENGTH(FEET) = 425.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.67
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.62
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.13
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.64
STREET FLOW TRAVEL TIME(MIN.) = 3.32 Tc(MIN.) = 11.01
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.908
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (7.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.570
SUBAREA AREA(ACRES) = 2.35 SUBAREA RUNOFF(CFS) = 6.57
TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 6.85

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.29
FLOW VELOCITY(FEET/SEC.) = 2.46 DEPTH*VELOCITY(FT*FT/SEC.) = 0.87
LONGEST FLOWPATH FROM NODE 250.00 TO NODE 252.00 = 495.00 FEET.

*****
FLOW PROCESS FROM NODE 252.00 TO NODE 248.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.01
RAINFALL INTENSITY(INCH/HR) = 4.91
TOTAL STREAM AREA(ACRES) = 2.45
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.85

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 12.91 8.09 5.988 4.30
2 6.85 11.01 4.908 2.45

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 17.95 8.09 5.988
2 17.44 11.01 4.908

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 17.95 Tc(MIN.) = 8.09
TOTAL AREA(ACRES) = 6.8
LONGEST FLOWPATH FROM NODE 240.00 TO NODE 248.00 = 1014.00 FEET.

*****
FLOW PROCESS FROM NODE 248.00 TO NODE 228.00 IS CODE = 31
-----

```

```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 618.00 DOWNSTREAM(FEET) = 617.00
FLOW LENGTH(FEET) = 57.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.59
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.95
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 8.19
LONGEST FLOWPATH FROM NODE 240.00 TO NODE 228.00 = 1071.00 FEET.

*****
FLOW PROCESS FROM NODE 228.00 TO NODE 228.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 17.95 8.19 5.942 6.75
LONGEST FLOWPATH FROM NODE 240.00 TO NODE 228.00 = 1071.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 151.30 13.49 4.307 70.40
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 228.00 = 3609.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/ HOUR)
1 109.83 8.19 5.942
2 164.31 13.49 4.307

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 164.31 Tc(MIN.) = 13.49
TOTAL AREA(ACRES) = 77.2

*****
FLOW PROCESS FROM NODE 228.00 TO NODE 228.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 228.00 TO NODE 253.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 625.70 DOWNSTREAM(FEET) = 608.00
FLOW LENGTH(FEET) = 377.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 30.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 23.90
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 164.31
PIPE TRAVEL TIME(MIN.) = 0.26 Tc(MIN.) = 13.75
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 253.00 = 3986.00 FEET.

*****
FLOW PROCESS FROM NODE 253.00 TO NODE 253.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.75
RAINFALL INTENSITY(INCH/HR) = 4.25
TOTAL STREAM AREA(ACRES) = 77.15
PEAK FLOW RATE(CFS) AT CONFLUENCE = 164.31

*****
FLOW PROCESS FROM NODE 255.00 TO NODE 256.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 625.00
DOWNSTREAM ELEVATION(FEET) = 619.00
ELEVATION DIFFERENCE(FEET) = 6.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.280
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 96.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/ HOUR) = 6.410
SUBAREA RUNOFF(CFS) = 0.58
TOTAL AREA(ACRES) = 0.26 TOTAL RUNOFF(CFS) = 0.58

*****
FLOW PROCESS FROM NODE 256.00 TO NODE 257.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 619.00 DOWNSTREAM(FEET) = 610.00
FLOW LENGTH(FEET) = 923.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000

DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.11
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.58
PIPE TRAVEL TIME(MIN.) = 4.95 Tc(MIN.) = 12.23
LONGEST FLOWPATH FROM NODE 255.00 TO NODE 257.00 = 1023.00 FEET.

*****
FLOW PROCESS FROM NODE 256.00 TO NODE 257.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/ HOUR) = 4.588
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 10.46 SUBAREA RUNOFF(CFS) = 16.80
TOTAL AREA(ACRES) = 10.7 TOTAL RUNOFF(CFS) = 17.21
TC(MIN.) = 12.23

*****
FLOW PROCESS FROM NODE 257.00 TO NODE 253.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 605.00 DOWNSTREAM(FEET) = 604.00
FLOW LENGTH(FEET) = 80.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 17.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.18
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.21
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 12.39
LONGEST FLOWPATH FROM NODE 255.00 TO NODE 253.00 = 1103.00 FEET.

*****
FLOW PROCESS FROM NODE 253.00 TO NODE 253.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.39
RAINFALL INTENSITY(INCH/HR) = 4.55
TOTAL STREAM AREA(ACRES) = 10.72
PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.21

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/ HOUR) (ACRE)
1 164.31 13.75 4.254 77.15
2 17.21 12.39 4.549 10.72

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/ HOUR)
1 165.30 12.39 4.549
2 180.41 13.75 4.254

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 180.41 Tc(MIN.) = 13.75
TOTAL AREA(ACRES) = 87.9
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 253.00 = 3986.00 FEET.

*****
FLOW PROCESS FROM NODE 253.00 TO NODE 258.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 605.00 DOWNSTREAM(FEET) = 585.00
FLOW LENGTH(FEET) = 251.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 29.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 29.59
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 180.41
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 13.89
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 258.00 = 4237.00 FEET.

*****
FLOW PROCESS FROM NODE 258.00 TO NODE 258.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.89
RAINFALL INTENSITY(INCH/HR) = 4.23
TOTAL STREAM AREA(ACRES) = 87.87
PEAK FLOW RATE(CFS) AT CONFLUENCE = 180.41

*****
FLOW PROCESS FROM NODE 260.00 TO NODE 261.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100

```



```

S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 656.00
DOWNSTREAM ELEVATION(FEET) = 651.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.396
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.776
SUBAREA RUNOFF(CFS) = 0.80
TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.80

*****
FLOW PROCESS FROM NODE 261.00 TO NODE 262.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 651.00 DOWNSTREAM ELEVATION(FEET) = 593.00
STREET LENGTH(FEET) = 1124.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.41
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.24
HALFSTREET FLOOD WIDTH(FEET) = 5.57
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.98
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.94
STREET FLOW TRAVEL TIME(MIN.) = 4.71 Tc(MIN.) = 10.11
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.187
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.410
SUBAREA AREA(ACRES) = 2.42 SUBAREA RUNOFF(CFS) = 5.15
TOTAL AREA(ACRES) = 2.7 PEAK FLOW RATE(CFS) = 5.68

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.27 HALFSTREET FLOOD WIDTH(FEET) = 7.27
FLOW VELOCITY(FEET/SEC.) = 4.39 DEPTH*VELOCITY(FT*FT/SEC.) = 1.19
LONGEST FLOWPATH FROM NODE 260.00 TO NODE 262.00 = 1194.00 FEET.

*****
FLOW PROCESS FROM NODE 262.00 TO NODE 258.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 10.11
RAINFALL INTENSITY(INCH/HR) = 5.19
TOTAL STREAM AREA(ACRES) = 2.67
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.68

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 180.41 13.89 4.226 87.87
2 5.68 10.11 5.187 2.67

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 152.65 10.11 5.187
2 185.03 13.89 4.226

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 185.03 Tc(MIN.) = 13.89
TOTAL AREA(ACRES) = 90.5
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 258.00 = 4237.00 FEET.

*****
FLOW PROCESS FROM NODE 258.00 TO NODE 258.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 270.00 TO NODE 271.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 670.00
DOWNSTREAM ELEVATION(FEET) = 660.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061

SUBAREA RUNOFF(CFS) = 1.41
TOTAL AREA(ACRES) = 0.57 TOTAL RUNOFF(CFS) = 1.41

*****
FLOW PROCESS FROM NODE 271.00 TO NODE 272.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 650.00 DOWNSTREAM(FEET) = 630.00
FLOW LENGTH(FEET) = 850.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.51
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.41
PIPE TRAVEL TIME(MIN.) = 2.57 Tc(MIN.) = 8.84
LONGEST FLOWPATH FROM NODE 270.00 TO NODE 272.00 = 950.00 FEET.

*****
FLOW PROCESS FROM NODE 271.00 TO NODE 272.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.656
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.81 SUBAREA RUNOFF(CFS) = 3.58
TOTAL AREA(ACRES) = 2.4 TOTAL RUNOFF(CFS) = 4.71
Tc(MIN.) = 8.84

*****
FLOW PROCESS FROM NODE 272.00 TO NODE 273.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 630.00 DOWNSTREAM(FEET) = 608.00
FLOW LENGTH(FEET) = 663.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.80
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.71
PIPE TRAVEL TIME(MIN.) = 1.26 Tc(MIN.) = 10.10
LONGEST FLOWPATH FROM NODE 270.00 TO NODE 273.00 = 1613.00 FEET.

*****
FLOW PROCESS FROM NODE 272.00 TO NODE 273.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.191
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 3.06 SUBAREA RUNOFF(CFS) = 5.56
TOTAL AREA(ACRES) = 5.4 TOTAL RUNOFF(CFS) = 9.88
Tc(MIN.) = 10.10

*****
FLOW PROCESS FROM NODE 273.00 TO NODE 274.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 607.50 DOWNSTREAM(FEET) = 594.00
FLOW LENGTH(FEET) = 70.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.41
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.88
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 10.15
LONGEST FLOWPATH FROM NODE 270.00 TO NODE 274.00 = 1683.00 FEET.

*****
FLOW PROCESS FROM NODE 274.00 TO NODE 258.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 5594.00 DOWNSTREAM(FEET) = 593.00
FLOW LENGTH(FEET) = 29.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 222.54
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.88
PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 10.15
LONGEST FLOWPATH FROM NODE 270.00 TO NODE 258.00 = 1712.00 FEET.

*****
FLOW PROCESS FROM NODE 258.00 TO NODE 258.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA

```

```

NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
  1          9.88      10.15        5.172          5.44
LONGEST FLOWPATH FROM NODE      270.00 TO NODE      258.00 =      1712.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
  1          185.03      13.89        4.226          90.54
LONGEST FLOWPATH FROM NODE      120.00 TO NODE      258.00 =      4237.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
  1          145.16      10.15        5.172
  2          193.11      13.89        4.226

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      193.11      Tc(MIN.) =      13.89
TOTAL AREA(ACRES) =      96.0

*****
FLOW PROCESS FROM NODE      258.00 TO NODE      258.00 IS CODE =      12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      258.00 TO NODE      275.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      294.00      DOWNSTREAM(FEET) =      293.00
FLOW LENGTH(FEET) =      159.00      MANNING'S N =      0.013
DEPTH OF FLOW IN 60.0 INCH PIPE IS 47.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      11.67
ESTIMATED PIPE DIAMETER(INCH) =      60.00      NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      193.11
PIPE TRAVEL TIME(MIN.) =      0.23      Tc(MIN.) =      14.12
LONGEST FLOWPATH FROM NODE      120.00 TO NODE      275.00 =      4396.00 FEET.

*****
FLOW PROCESS FROM NODE      275.00 TO NODE      275.00 IS CODE =      1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =      14.12
RAINFALL INTENSITY(INCH/HR) =      4.18
TOTAL STREAM AREA(ACRES) =      95.98
PEAK FLOW RATE(CFS) AT CONFLUENCE =      193.11

*****
FLOW PROCESS FROM NODE      278.00 TO NODE      279.00 IS CODE =      21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) =      0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      70.00
UPSTREAM ELEVATION(FEET) =      606.00
DOWNSTREAM ELEVATION(FEET) =      604.00
ELEVATION DIFFERENCE(FEET) =      2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      7.323
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      6.386
SUBAREA RUNOFF(CFS) =      0.65
TOTAL AREA(ACRES) =      0.25      TOTAL RUNOFF(CFS) =      0.65

*****
FLOW PROCESS FROM NODE      279.00 TO NODE      280.00 IS CODE =      61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) =      604.00      DOWNSTREAM ELEVATION(FEET) =      596.00
STREET LENGTH(FEET) =      519.00      CURB HEIGHT(INCHES) =      6.0
STREET HALFWIDTH(FEET) =      18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =      9.00
INSIDE STREET CROSSFALL(DECIMAL) =      0.020
OUTSIDE STREET CROSSFALL(DECIMAL) =      0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF =      2
STREET PARKWAY CROSSFALL(DECIMAL) =      0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =      0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section =      0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      1.97
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) =      0.24
HALFSTREET FLOOD WIDTH(FEET) =      5.81
AVERAGE FLOW VELOCITY(FEET/SEC.) =      2.17
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =      0.52
STREET FLOW TRAVEL TIME(MIN.) =      3.99      Tc(MIN.) =      11.32
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      4.822
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) =      0
AREA-AVERAGE RUNOFF COEFFICIENT =      0.410
SUBAREA AREA(ACRES) =      1.32      SUBAREA RUNOFF(CFS) =      2.61
TOTAL AREA(ACRES) =      1.6      PEAK FLOW RATE(CFS) =      3.10

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.27      HALFSTREET FLOOD WIDTH(FEET) =      7.27
FLOW VELOCITY(FEET/SEC.) =      2.40      DEPTH*VELOCITY(FT*FT/SEC.) =      0.65
LONGEST FLOWPATH FROM NODE      278.00 TO NODE      280.00 =      589.00 FEET.

*****
FLOW PROCESS FROM NODE      280.00 TO NODE      275.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      595.70      DOWNSTREAM(FEET) =      595.00
FLOW LENGTH(FEET) =      40.00      MANNING'S N =      0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      6.22
ESTIMATED PIPE DIAMETER(INCH) =      18.00      NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      3.10
PIPE TRAVEL TIME(MIN.) =      0.11      Tc(MIN.) =      11.42
LONGEST FLOWPATH FROM NODE      278.00 TO NODE      275.00 =      629.00 FEET.

*****
FLOW PROCESS FROM NODE      275.00 TO NODE      275.00 IS CODE =      1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) =      11.42
RAINFALL INTENSITY(INCH/HR) =      4.79
TOTAL STREAM AREA(ACRES) =      1.57
PEAK FLOW RATE(CFS) AT CONFLUENCE =      3.10

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
  1          193.11      14.12        4.182          95.98
  2           3.10      11.42        4.793          1.57

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
  1          159.39      11.42        4.793
  2          195.82      14.12        4.182

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      195.82      Tc(MIN.) =      14.12
TOTAL AREA(ACRES) =      97.6
LONGEST FLOWPATH FROM NODE      120.00 TO NODE      275.00 =      4396.00 FEET.

*****
FLOW PROCESS FROM NODE      275.00 TO NODE      281.00 IS CODE =      31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      596.00      DOWNSTREAM(FEET) =      595.00
FLOW LENGTH(FEET) =      45.00      MANNING'S N =      0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 36.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      18.88
ESTIMATED PIPE DIAMETER(INCH) =      48.00      NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      195.82
PIPE TRAVEL TIME(MIN.) =      0.04      Tc(MIN.) =      14.16
LONGEST FLOWPATH FROM NODE      120.00 TO NODE      281.00 =      4441.00 FEET.

*****
FLOW PROCESS FROM NODE      281.00 TO NODE      281.00 IS CODE =      10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      300.00 TO NODE      301.00 IS CODE =      21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) =      0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      0.15
UPSTREAM ELEVATION(FEET) =      698.00
DOWNSTREAM ELEVATION(FEET) =      696.00
ELEVATION DIFFERENCE(FEET) =      2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      0.081
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =      1.04
TOTAL AREA(ACRES) =      0.15      TOTAL RUNOFF(CFS) =      1.04

*****
FLOW PROCESS FROM NODE      301.00 TO NODE      302.00 IS CODE =      61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) =      696.00      DOWNSTREAM ELEVATION(FEET) =      654.00
STREET LENGTH(FEET) =      1090.00      CURB HEIGHT(INCHES) =      6.0

```

```

STREET HALFWIDTH( FEET ) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK( FEET ) = 9.00
INSIDE STREET CROSSFALL( DECIMAL ) = 0.020
OUTSIDE STREET CROSSFALL( DECIMAL ) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL( DECIMAL ) = 0.020
Manning's FRICTION FACTOR for Streetflow Section( curb-to-curb ) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW( CFS ) = 9.79
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH( FEET ) = 0.33
HALFSTREET FLOOD WIDTH( FEET ) = 9.95
AVERAGE FLOW VELOCITY( FEET/SEC. ) = 4.42
PRODUCT OF DEPTH&VELOCITY( FT*FT/SEC. ) = 1.44
STREET FLOW TRAVEL TIME( MIN. ) = 4.11 Tc( MIN. ) = 4.19
100 YEAR RAINFALL INTENSITY( INCH/HOUR ) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED( SUBAREA ):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER ( AMC II ) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA( ACRES ) = 2.52 SUBAREA RUNOFF( CFS ) = 17.50
TOTAL AREA( ACRES ) = 2.7 PEAK FLOW RATE( CFS ) = 18.54

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH( FEET ) = 0.39 HALFSTREET FLOOD WIDTH( FEET ) = 13.04
FLOW VELOCITY( FEET/SEC. ) = 5.09 DEPTH*VELOCITY( FT*FT/SEC. ) = 1.97
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 302.00 = 1090.15 FEET.

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 302.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION( MIN. ) = 4.19
RAINFALL INTENSITY( INCH/HR ) = 8.17
TOTAL STREAM AREA( ACRES ) = 2.67
PEAK FLOW RATE( CFS ) AT CONFLUENCE = 18.54

*****
FLOW PROCESS FROM NODE 305.00 TO NODE 306.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED( SUBAREA ):
RESIDENTIAL ( 1. DU/AC OR LESS ) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER ( AMC II ) = 0
INITIAL SUBAREA FLOW-LENGTH( FEET ) = 85.00
UPSTREAM ELEVATION( FEET ) = 662.00
DOWNSTREAM ELEVATION( FEET ) = 659.00
ELEVATION DIFFERENCE( FEET ) = 3.00
SUBAREA OVERLAND TIME OF FLOW( MIN. ) = 7.521
100 YEAR RAINFALL INTENSITY( INCH/HOUR ) = 6.277
SUBAREA RUNOFF( CFS ) = 0.59
TOTAL AREA( ACRES ) = 0.23 TOTAL RUNOFF( CFS ) = 0.59

*****
FLOW PROCESS FROM NODE 306.00 TO NODE 307.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE ( NON-PRESSURE FLOW )<<<<
-----
ELEVATION DATA: UPSTREAM( FEET ) = 659.00 DOWNSTREAM( FEET ) = 655.00
FLOW LENGTH( FEET ) = 262.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER( INCH ) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.6 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) = 3.66
ESTIMATED PIPE DIAMETER( INCH ) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS ) = 0.59
PIPE TRAVEL TIME( MIN. ) = 1.19 Tc( MIN. ) = 8.71
LONGEST FLOWPATH FROM NODE 305.00 TO NODE 307.00 = 347.00 FEET.

*****
FLOW PROCESS FROM NODE 306.00 TO NODE 307.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
100 YEAR RAINFALL INTENSITY( INCH/HOUR ) = 5.709
*USER SPECIFIED( SUBAREA ):
RESIDENTIAL ( 1. DU/AC OR LESS ) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER ( AMC II ) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3622
SUBAREA AREA( ACRES ) = 0.90 SUBAREA RUNOFF( CFS ) = 1.80
TOTAL AREA( ACRES ) = 1.1 TOTAL RUNOFF( CFS ) = 2.34
Tc( MIN. ) = 8.71

*****
FLOW PROCESS FROM NODE 307.00 TO NODE 302.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE ( NON-PRESSURE FLOW )<<<<
-----
ELEVATION DATA: UPSTREAM( FEET ) = 655.00 DOWNSTREAM( FEET ) = 654.00
FLOW LENGTH( FEET ) = 90.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER( INCH ) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.7 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) = 4.88
ESTIMATED PIPE DIAMETER( INCH ) = 18.00 NUMBER OF PIPES = 1

*****
PIPE-FLOW( CFS ) = 2.34
PIPE TRAVEL TIME( MIN. ) = 0.31 Tc( MIN. ) = 9.02
LONGEST FLOWPATH FROM NODE 305.00 TO NODE 302.00 = 437.00 FEET.

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 302.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION( MIN. ) = 9.02
RAINFALL INTENSITY( INCH/HR ) = 5.58
TOTAL STREAM AREA( ACRES ) = 1.13
PEAK FLOW RATE( CFS ) AT CONFLUENCE = 2.34

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER ( CFS ) ( MIN. ) ( INCH/HOUR ) ( ACRE )
1 18.54 4.19 8.168 2.67
2 2.34 9.02 5.582 1.13

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER ( CFS ) ( MIN. ) ( INCH/HOUR )
1 19.62 4.19 8.168
2 15.01 9.02 5.582

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE( CFS ) = 19.62 Tc( MIN. ) = 4.19
TOTAL AREA( ACRES ) = 3.8
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 302.00 = 1090.15 FEET.

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 309.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE ( NON-PRESSURE FLOW )<<<<
-----
ELEVATION DATA: UPSTREAM( FEET ) = 654.00 DOWNSTREAM( FEET ) = 653.00
FLOW LENGTH( FEET ) = 45.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.9 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) = 10.75
ESTIMATED PIPE DIAMETER( INCH ) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS ) = 19.62
PIPE TRAVEL TIME( MIN. ) = 0.07 Tc( MIN. ) = 4.26
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 309.00 = 1135.15 FEET.

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 309.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<
-----

*****
FLOW PROCESS FROM NODE 311.00 TO NODE 312.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
-----
*USER SPECIFIED( SUBAREA ):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER ( AMC II ) = 0
INITIAL SUBAREA FLOW-LENGTH( FEET ) = 100.00
UPSTREAM ELEVATION( FEET ) = 690.00
DOWNSTREAM ELEVATION( FEET ) = 680.00
ELEVATION DIFFERENCE( FEET ) = 10.00
SUBAREA OVERLAND TIME OF FLOW( MIN. ) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY( INCH/HOUR ) = 7.061
SUBAREA RUNOFF( CFS ) = 0.47
TOTAL AREA( ACRES ) = 0.19 TOTAL RUNOFF( CFS ) = 0.47

*****
FLOW PROCESS FROM NODE 312.00 TO NODE 313.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE ( NON-PRESSURE FLOW )<<<<
-----
ELEVATION DATA: UPSTREAM( FEET ) = 680.00 DOWNSTREAM( FEET ) = 660.00
FLOW LENGTH( FEET ) = 722.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER( INCH ) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.1 INCHES
PIPE-FLOW VELOCITY( FEET/SEC. ) = 4.19
ESTIMATED PIPE DIAMETER( INCH ) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW( CFS ) = 0.47
PIPE TRAVEL TIME( MIN. ) = 2.87 Tc( MIN. ) = 9.14
LONGEST FLOWPATH FROM NODE 311.00 TO NODE 313.00 = 822.00 FEET.

*****
FLOW PROCESS FROM NODE 312.00 TO NODE 313.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
100 YEAR RAINFALL INTENSITY( INCH/HOUR ) = 5.537
*USER SPECIFIED( SUBAREA ):
RESIDENTIAL ( 1. DU/AC OR LESS ) RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER ( AMC II ) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA( ACRES ) = 2.74 SUBAREA RUNOFF( CFS ) = 5.31

```



```

TOTAL AREA(ACRES) =          2.9    TOTAL RUNOFF(CFS) =          5.68
TC(MIN.) =          9.14

*****
FLOW PROCESS FROM NODE    313.00 TO NODE    314.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    660.00  DOWNSTREAM(FEET) =    650.00
FLOW LENGTH(FEET) =    193.00  MANNING'S N =    0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS    6.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    10.88
ESTIMATED PIPE DIAMETER(INCH) =    18.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =          5.68
PIPE TRAVEL TIME(MIN.) =    0.30    Tc(MIN.) =    9.43
LONGEST FLOWPATH FROM NODE    311.00 TO NODE    314.00 =    1015.00 FEET.

*****
FLOW PROCESS FROM NODE    314.00 TO NODE    314.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =    9.43
RAINFALL INTENSITY(INCH/HR) =    5.42
TOTAL STREAM AREA(ACRES) =    2.93
PEAK FLOW RATE(CFS) AT CONFLUENCE =          5.68

*****
FLOW PROCESS FROM NODE    316.00 TO NODE    317.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) =    0
INITIAL SUBAREA FLOW-LENGTH(FEET) =    70.00
UPSTREAM ELEVATION(FEET) =    709.00
DOWNSTREAM ELEVATION(FEET) =    705.00
ELEVATION DIFFERENCE(FEET) =    4.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =    4.886
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =          0.89
TOTAL AREA(ACRES) =    0.21    TOTAL RUNOFF(CFS) =          0.89

*****
FLOW PROCESS FROM NODE    317.00 TO NODE    318.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) =    705.00  DOWNSTREAM ELEVATION(FEET) =    650.00
STREET LENGTH(FEET) =    1054.00  CURB HEIGHT(INCHES) =    6.0
STREET HALFWIDTH(FEET) =    18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =    9.00
INSIDE STREET CROSSFALL(DECIMAL) =    0.020
OUTSIDE STREET CROSSFALL(DECIMAL) =    0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) =    0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) =    0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section =    0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =    10.34
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) =    0.32
HALFSTREET FLOOD WIDTH(FEET) =    9.53
AVERAGE FLOW VELOCITY(FEET/SEC.) =    5.04
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =    1.60
STREET FLOW TRAVEL TIME(MIN.) =    3.49    Tc(MIN.) =    8.37
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    5.858
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) =    0
AREA-AVERAGE RUNOFF COEFFICIENT =    0.520
SUBAREA AREA(ACRES) =    6.11    SUBAREA RUNOFF(CFS) =    18.61
TOTAL AREA(ACRES) =    6.3    PEAK FLOW RATE(CFS) =    19.25

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) =    0.37    HALFSTREET FLOOD WIDTH(FEET) =    12.41
FLOW VELOCITY(FEET/SEC.) =    5.80    DEPTH*VELOCITY(FT*FT/SEC.) =    2.17
LONGEST FLOWPATH FROM NODE    316.00 TO NODE    318.00 =    1124.00 FEET.

*****
FLOW PROCESS FROM NODE    318.00 TO NODE    314.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) =    8.37
RAINFALL INTENSITY(INCH/HR) =    5.86
TOTAL STREAM AREA(ACRES) =    6.32
PEAK FLOW RATE(CFS) AT CONFLUENCE =    19.25

** CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA

NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1          5.68    9.43    5.424    2.93
2         19.25    8.37    5.858    6.32

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1         24.29    8.37    5.858
2         23.50    9.43    5.424

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =    24.29    Tc(MIN.) =    8.37
TOTAL AREA(ACRES) =    9.2
LONGEST FLOWPATH FROM NODE    316.00 TO NODE    314.00 =    1124.00 FEET.

*****
FLOW PROCESS FROM NODE    314.00 TO NODE    319.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    650.50  DOWNSTREAM(FEET) =    649.00
FLOW LENGTH(FEET) =    164.00  MANNING'S N =    0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS    19.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    8.14
ESTIMATED PIPE DIAMETER(INCH) =    27.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    24.29
PIPE TRAVEL TIME(MIN.) =    0.34    Tc(MIN.) =    8.71
LONGEST FLOWPATH FROM NODE    316.00 TO NODE    319.00 =    1288.00 FEET.

*****
FLOW PROCESS FROM NODE    319.00 TO NODE    319.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =    8.71
RAINFALL INTENSITY(INCH/HR) =    5.71
TOTAL STREAM AREA(ACRES) =    9.25
PEAK FLOW RATE(CFS) AT CONFLUENCE =    24.29

*****
FLOW PROCESS FROM NODE    321.00 TO NODE    322.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .7900
S.C.S. CURVE NUMBER (AMC II) =    0
INITIAL SUBAREA FLOW-LENGTH(FEET) =    100.00
UPSTREAM ELEVATION(FEET) =    690.00
DOWNSTREAM ELEVATION(FEET) =    680.00
ELEVATION DIFFERENCE(FEET) =    10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =    2.590
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =          1.03
TOTAL AREA(ACRES) =    0.16    TOTAL RUNOFF(CFS) =          1.03

*****
FLOW PROCESS FROM NODE    322.00 TO NODE    323.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    680.00  DOWNSTREAM(FEET) =    651.00
FLOW LENGTH(FEET) =    1080.00  MANNING'S N =    0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS    3.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    5.27
ESTIMATED PIPE DIAMETER(INCH) =    18.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =          1.03
PIPE TRAVEL TIME(MIN.) =    3.42    Tc(MIN.) =    6.01
LONGEST FLOWPATH FROM NODE    321.00 TO NODE    323.00 =    1180.00 FEET.

*****
FLOW PROCESS FROM NODE    322.00 TO NODE    323.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.257
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .7900
S.C.S. CURVE NUMBER (AMC II) =    0
AREA-AVERAGE RUNOFF COEFFICIENT =    0.7900
SUBAREA AREA(ACRES) =    7.97    SUBAREA RUNOFF(CFS) =    45.69
TOTAL AREA(ACRES) =    8.1    TOTAL RUNOFF(CFS) =    46.61
TC(MIN.) =    6.01

*****
FLOW PROCESS FROM NODE    323.00 TO NODE    319.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    653.00  DOWNSTREAM(FEET) =    649.50
FLOW LENGTH(FEET) =    75.00  MANNING'S N =    0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS    19.2 INCHES

```

```

PIPE-FLOW VELOCITY(FEET/SEC.) = 17.28
ESTIMATED PIPE DIAMETER(INCH) = 24.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 46.61
PIPE TRAVEL TIME(MIN.) = 0.07    Tc(MIN.) = 6.08
LONGEST FLOWPATH FROM NODE 321.00 TO NODE 319.00 = 1255.00 FEET.

*****
FLOW PROCESS FROM NODE 319.00 TO NODE 319.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 6.08
RAINFALL INTENSITY(INCH/HR) = 7.20
TOTAL STREAM AREA(ACRES) = 8.13
PEAK FLOW RATE(CFS) AT CONFLUENCE = 46.61

** CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1          24.29    8.71    5.711        9.25
2          46.61    6.08    7.201        8.13

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)      (MIN.)  (INCH/HOUR)
1          65.87    6.08    7.201
2          61.26    8.71    5.711

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 65.87    Tc(MIN.) = 6.08
TOTAL AREA(ACRES) = 17.4
LONGEST FLOWPATH FROM NODE 316.00 TO NODE 319.00 = 1288.00 FEET.

*****
FLOW PROCESS FROM NODE 319.00 TO NODE 324.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 649.50 DOWNSTREAM(FEET) = 649.00
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 26.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.77
ESTIMATED PIPE DIAMETER(INCH) = 33.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 65.87
PIPE TRAVEL TIME(MIN.) = 0.04    Tc(MIN.) = 6.12
LONGEST FLOWPATH FROM NODE 316.00 TO NODE 324.00 = 1318.00 FEET.

*****
FLOW PROCESS FROM NODE 324.00 TO NODE 324.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 6.12
RAINFALL INTENSITY(INCH/HR) = 7.17
TOTAL STREAM AREA(ACRES) = 17.38
PEAK FLOW RATE(CFS) AT CONFLUENCE = 65.87

*****
FLOW PROCESS FROM NODE 326.00 TO NODE 327.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 651.00
DOWNSTREAM ELEVATION(FEET) = 650.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.940
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.83
TOTAL AREA(ACRES) = 0.12    TOTAL RUNOFF(CFS) = 0.83

*****
FLOW PROCESS FROM NODE 327.00 TO NODE 328.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 651.00 DOWNSTREAM ELEVATION(FEET) = 649.00
STREET LENGTH(FEET) = 121.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.53
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.22
HALFSTREET FLOOD WIDTH(FEET) = 4.87
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.15
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.48
STREET FLOW TRAVEL TIME(MIN.) = 0.94    Tc(MIN.) = 3.88
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 0.20    SUBAREA RUNOFF(CFS) = 1.39
TOTAL AREA(ACRES) = 0.3    PEAK FLOW RATE(CFS) = 2.22

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.25    HALFSTREET FLOOD WIDTH(FEET) = 6.04
FLOW VELOCITY(FEET/SEC.) = 2.30    DEPTH*VELOCITY(FT*FT/SEC.) = 0.57
LONGEST FLOWPATH FROM NODE 326.00 TO NODE 328.00 = 181.00 FEET.

*****
FLOW PROCESS FROM NODE 328.00 TO NODE 324.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 3.88
RAINFALL INTENSITY(INCH/HR) = 8.17
TOTAL STREAM AREA(ACRES) = 0.32
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.22

** CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1          65.87    6.12    7.172        17.38
2          2.22    3.88    8.168        0.32

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)      (MIN.)  (INCH/HOUR)
1          60.06    3.88    8.168
2          67.83    6.12    7.172

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 67.83    Tc(MIN.) = 6.12
TOTAL AREA(ACRES) = 17.7
LONGEST FLOWPATH FROM NODE 316.00 TO NODE 324.00 = 1318.00 FEET.

*****
FLOW PROCESS FROM NODE 324.00 TO NODE 309.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 651.00 DOWNSTREAM(FEET) = 650.00
FLOW LENGTH(FEET) = 102.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 27.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.78
ESTIMATED PIPE DIAMETER(INCH) = 39.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 67.83
PIPE TRAVEL TIME(MIN.) = 0.16    Tc(MIN.) = 6.27
LONGEST FLOWPATH FROM NODE 316.00 TO NODE 309.00 = 1420.00 FEET.

*****
FLOW PROCESS FROM NODE 324.00 TO NODE 309.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<
=====
*****
FLOW PROCESS FROM NODE 330.00 TO NODE 331.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 980.00
DOWNSTREAM ELEVATION(FEET) = 970.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.91
TOTAL AREA(ACRES) = 0.37    TOTAL RUNOFF(CFS) = 0.91

*****
FLOW PROCESS FROM NODE 331.00 TO NODE 332.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 880.00 DOWNSTREAM(FEET) = 850.00
FLOW LENGTH(FEET) = 1000.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.8 INCHES

```

```

PIPE-FLOW VELOCITY(FEET/SEC.) = 5.28
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.91
PIPE TRAVEL TIME(MIN.) = 3.15    Tc(MIN.) = 9.42
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 332.00 = 1100.00 FEET.

*****
FLOW PROCESS FROM NODE 331.00 TO NODE 332.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.428
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4046
SUBAREA AREA(ACRES) = 3.77    SUBAREA RUNOFF(CFS) = 8.39
TOTAL AREA(ACRES) = 4.1    TOTAL RUNOFF(CFS) = 9.09
TC(MIN.) = 9.42

*****
FLOW PROCESS FROM NODE 332.00 TO NODE 333.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 850.00    DOWNSTREAM(FEET) = 760.00
FLOW LENGTH(FEET) = 150.00    MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 29.85
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.09
PIPE TRAVEL TIME(MIN.) = 0.08    Tc(MIN.) = 9.50
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 333.00 = 1250.00 FEET.

*****
FLOW PROCESS FROM NODE 332.00 TO NODE 333.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.397
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4072
SUBAREA AREA(ACRES) = 3.77    SUBAREA RUNOFF(CFS) = 8.34
TOTAL AREA(ACRES) = 7.9    TOTAL RUNOFF(CFS) = 17.38
TC(MIN.) = 9.50

*****
FLOW PROCESS FROM NODE 333.00 TO NODE 334.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 755.00    DOWNSTREAM ELEVATION(FEET) = 648.00
STREET LENGTH(FEET) = 1769.00    CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 26.02
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.40
HALFSTREET FLOOD WIDTH(FEET) = 13.61
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.61
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 2.63
STREET FLOW TRAVEL TIME(MIN.) = 4.46    Tc(MIN.) = 13.97
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.211
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.373
SUBAREA AREA(ACRES) = 11.64    SUBAREA RUNOFF(CFS) = 17.15
TOTAL AREA(ACRES) = 19.5    PEAK FLOW RATE(CFS) = 30.72

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.42    HALFSTREET FLOOD WIDTH(FEET) = 14.59
FLOW VELOCITY(FEET/SEC.) = 6.84    DEPTH*VELOCITY(FT*FT/SEC.) = 2.86
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 334.00 = 3019.00 FEET.

*****
FLOW PROCESS FROM NODE 334.00 TO NODE 309.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 643.00    DOWNSTREAM(FEET) = 641.00
FLOW LENGTH(FEET) = 277.00    MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 22.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.84
ESTIMATED PIPE DIAMETER(INCH) = 30.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 30.72
PIPE TRAVEL TIME(MIN.) = 0.59    Tc(MIN.) = 14.56
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 309.00 = 3296.00 FEET.

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 309.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1    30.72    14.56    4.100    19.55
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 309.00 = 3296.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1    19.62    4.26    8.168    3.80
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 309.00 = 1135.15 FEET.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1    28.62    4.26    8.168
2    40.57    14.56    4.100

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 40.57    Tc(MIN.) = 14.56
TOTAL AREA(ACRES) = 23.3

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 309.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1    40.57    14.56    4.100    23.35
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 309.00 = 3296.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1    67.83    6.27    7.055    17.70
LONGEST FLOWPATH FROM NODE 316.00 TO NODE 309.00 = 1420.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1    85.31    6.27    7.055
2    79.98    14.56    4.100

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 85.31    Tc(MIN.) = 6.27
TOTAL AREA(ACRES) = 41.0

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 309.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 3 <<<<<
=====

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 309.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 2 <<<<<
=====

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 335.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 645.00    DOWNSTREAM(FEET) = 638.00
FLOW LENGTH(FEET) = 552.00    MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 30.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.42
ESTIMATED PIPE DIAMETER(INCH) = 39.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 85.31
PIPE TRAVEL TIME(MIN.) = 0.74    Tc(MIN.) = 7.02
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 335.00 = 3848.00 FEET.

*****
FLOW PROCESS FROM NODE 335.00 TO NODE 335.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 7.02
RAINFALL INTENSITY(INCH/HR) = 6.57
TOTAL STREAM AREA(ACRES) = 41.05
PEAK FLOW RATE(CFS) AT CONFLUENCE = 85.31

*****
FLOW PROCESS FROM NODE 337.00 TO NODE 338.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100

```



```

S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 653.00
DOWNSTREAM ELEVATION(FEET) = 651.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.323
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.386
SUBAREA RUNOFF(CFS) = 0.39
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 0.39

*****
FLOW PROCESS FROM NODE 338.00 TO NODE 339.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 651.00 DOWNSTREAM(FEET) = 643.00
FLOW LENGTH(FEET) = 496.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.33
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.39
PIPE TRAVEL TIME(MIN.) = 2.49 Tc(MIN.) = 9.81
LONGEST FLOWPATH FROM NODE 337.00 TO NODE 339.00 = 566.00 FEET.

*****
FLOW PROCESS FROM NODE 338.00 TO NODE 339.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.289
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 1.21 SUBAREA RUNOFF(CFS) = 2.62
TOTAL AREA(ACRES) = 1.4 TOTAL RUNOFF(CFS) = 2.95
TC(MIN.) = 9.81

*****
FLOW PROCESS FROM NODE 339.00 TO NODE 335.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 638.00 DOWNSTREAM(FEET) = 637.00
FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.42
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.95
PIPE TRAVEL TIME(MIN.) = 0.13 Tc(MIN.) = 9.94
LONGEST FLOWPATH FROM NODE 337.00 TO NODE 335.00 = 616.00 FEET.

*****
FLOW PROCESS FROM NODE 335.00 TO NODE 335.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.94
RAINFALL INTENSITY(INCH/HR) = 5.24
TOTAL STREAM AREA(ACRES) = 1.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.95

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 85.31 7.02 6.565 41.05
2 2.95 9.94 5.244 1.36

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 87.39 7.02 6.565
2 71.09 9.94 5.244

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 87.39 Tc(MIN.) = 7.02
TOTAL AREA(ACRES) = 42.4
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 335.00 = 3848.00 FEET.

*****
FLOW PROCESS FROM NODE 335.00 TO NODE 340.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 638.00 DOWNSTREAM(FEET) = 622.00
FLOW LENGTH(FEET) = 1400.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.19
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 87.39
PIPE TRAVEL TIME(MIN.) = 1.91 Tc(MIN.) = 8.93
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 340.00 = 5248.00 FEET.

*****
FLOW PROCESS FROM NODE 340.00 TO NODE 340.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 8.93
RAINFALL INTENSITY(INCH/HR) = 5.62
TOTAL STREAM AREA(ACRES) = 42.41
PEAK FLOW RATE(CFS) AT CONFLUENCE = 87.39

*****
FLOW PROCESS FROM NODE 342.00 TO NODE 343.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 650.00
DOWNSTREAM ELEVATION(FEET) = 648.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.653
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.69
TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.69

*****
FLOW PROCESS FROM NODE 343.00 TO NODE 344.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 648.00 DOWNSTREAM ELEVATION(FEET) = 627.00
STREET LENGTH(FEET) = 2193.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.30
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.39
HALFSTREET FLOOD WIDTH(FEET) = 13.04
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.56
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.99
STREET FLOW TRAVEL TIME(MIN.) = 14.30 Tc(MIN.) = 16.95
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.716
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 5.01 SUBAREA RUNOFF(CFS) = 15.82
TOTAL AREA(ACRES) = 5.1 PEAK FLOW RATE(CFS) = 16.14

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.45 HALFSTREET FLOOD WIDTH(FEET) = 16.28
FLOW VELOCITY(FEET/SEC.) = 2.92 DEPTH*VELOCITY(FT*FT/SEC.) = 1.32
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 2193.0 FT WITH ELEVATION-DROP = 21.0 FT, IS 34.8 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 344.00
LONGEST FLOWPATH FROM NODE 342.00 TO NODE 344.00 = 2263.00 FEET.

*****
FLOW PROCESS FROM NODE 344.00 TO NODE 340.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 622.50 DOWNSTREAM(FEET) = 622.00
FLOW LENGTH(FEET) = 26.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.78
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.14
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 17.00
LONGEST FLOWPATH FROM NODE 342.00 TO NODE 340.00 = 2289.00 FEET.

*****
FLOW PROCESS FROM NODE 340.00 TO NODE 340.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 17.00
RAINFALL INTENSITY(INCH/HR) = 3.71
TOTAL STREAM AREA(ACRES) = 5.11
PEAK FLOW RATE(CFS) AT CONFLUENCE = 16.14

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)

```

NUMBER	(CFS)	(MIN.)	(INCH/ HOUR)	(ACRE)
1	87.39	8.93	5.619	42.41
2	16.14	17.00	3.710	5.11

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

**** PEAK FLOW RATE TABLE ****

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/ HOUR)
1	95.87	8.93	5.619
2	73.84	17.00	3.710

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 95.87 Tc(MIN.) = 8.93
TOTAL AREA(ACRES) = 47.5
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 340.00 = 5248.00 FEET.

FLOW PROCESS FROM NODE 340.00 TO NODE 345.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 622.50 DOWNSTREAM(FEET) = 602.00
FLOW LENGTH(FEET) = 1115.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 28.4 INCHES
PIPE-FLOW VELOCITY(FEET/ SEC.) = 14.84
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 95.87
PIPE TRAVEL TIME(MIN.) = 1.25 Tc(MIN.) = 10.18
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 345.00 = 6363.00 FEET.

FLOW PROCESS FROM NODE 345.00 TO NODE 345.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.18
RAINFALL INTENSITY(INCH/ HR) = 5.16
TOTAL STREAM AREA(ACRES) = 47.52
PEAK FLOW RATE(CFS) AT CONFLUENCE = 95.87

FLOW PROCESS FROM NODE 351.00 TO NODE 352.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 627.00
DOWNSTREAM ELEVATION(FEET) = 626.50
ELEVATION DIFFERENCE(FEET) = 0.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.709
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 54.29
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/ HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.11
TOTAL AREA(ACRES) = 0.16 TOTAL RUNOFF(CFS) = 1.11

FLOW PROCESS FROM NODE 352.00 TO NODE 353.00 IS CODE = 61

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<

UPSTREAM ELEVATION(FEET) = 626.50 DOWNSTREAM ELEVATION(FEET) = 607.00
STREET LENGTH(FEET) = 785.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.78
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 7.97
AVERAGE FLOW VELOCITY(FEET/ SEC.) = 3.17
PRODUCT OF DEPTH&VELOCITY(FT*FT/ SEC.) = 0.91
STREET FLOW TRAVEL TIME(MIN.) = 4.13 Tc(MIN.) = 7.84
100 YEAR RAINFALL INTENSITY(INCH/ HOUR) = 6.113
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 1.39 SUBAREA RUNOFF(CFS) = 7.22
TOTAL AREA(ACRES) = 1.5 PEAK FLOW RATE(CFS) = 8.05

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.33 HALFSTREET FLOOD WIDTH(FEET) = 10.09

FLOW VELOCITY(FEET/ SEC.) = 3.54 DEPTH*VELOCITY(FT*FT/ SEC.) = 1.16
LONGEST FLOWPATH FROM NODE 351.00 TO NODE 353.00 = 855.00 FEET.

FLOW PROCESS FROM NODE 353.00 TO NODE 345.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 607.00 DOWNSTREAM(FEET) = 606.50
FLOW LENGTH(FEET) = 35.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.7 INCHES
PIPE-FLOW VELOCITY(FEET/ SEC.) = 7.39
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 8.05
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 7.91
LONGEST FLOWPATH FROM NODE 351.00 TO NODE 345.00 = 890.00 FEET.

FLOW PROCESS FROM NODE 345.00 TO NODE 345.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.91
RAINFALL INTENSITY(INCH/ HR) = 6.07
TOTAL STREAM AREA(ACRES) = 1.55
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.05

FLOW PROCESS FROM NODE 347.00 TO NODE 348.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/ AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 670.00
DOWNSTREAM ELEVATION(FEET) = 666.00
ELEVATION DIFFERENCE(FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.813
100 YEAR RAINFALL INTENSITY(INCH/ HOUR) = 7.412
SUBAREA RUNOFF(CFS) = 0.94
TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.94

FLOW PROCESS FROM NODE 348.00 TO NODE 349.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 666.00 DOWNSTREAM(FEET) = 610.00
FLOW LENGTH(FEET) = 1120.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.5 INCHES
PIPE-FLOW VELOCITY(FEET/ SEC.) = 6.36
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.94
PIPE TRAVEL TIME(MIN.) = 2.94 Tc(MIN.) = 8.75
LONGEST FLOWPATH FROM NODE 347.00 TO NODE 349.00 = 1190.00 FEET.

FLOW PROCESS FROM NODE 348.00 TO NODE 349.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

100 YEAR RAINFALL INTENSITY(INCH/ HOUR) = 5.694
*USER SPECIFIED(SUBAREA):
RESIDENTIAL (1. DU/ AC OR LESS) RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4100
SUBAREA AREA(ACRES) = 8.70 SUBAREA RUNOFF(CFS) = 20.31
TOTAL AREA(ACRES) = 9.0 TOTAL RUNOFF(CFS) = 21.03
Tc(MIN.) = 8.75

FLOW PROCESS FROM NODE 349.00 TO NODE 345.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 605.00 DOWNSTREAM(FEET) = 598.00
FLOW LENGTH(FEET) = 130.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/ SEC.) = 15.18
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 21.03
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 8.89
LONGEST FLOWPATH FROM NODE 347.00 TO NODE 345.00 = 1320.00 FEET.

FLOW PROCESS FROM NODE 349.00 TO NODE 345.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 8.89
RAINFALL INTENSITY(INCH/ HR) = 5.63
TOTAL STREAM AREA(ACRES) = 9.01

```

PEAK FLOW RATE(CFS) AT CONFLUENCE = 21.03

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1           95.87      10.18      5.163          47.52
2           8.05       7.91       6.074          1.55
3           21.03      8.89       5.634          9.01

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1           108.27      7.91       6.074
2           116.35      8.89       5.634
3           121.99      10.18      5.163

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 121.99 Tc(MIN.) = 10.18
TOTAL AREA(ACRES) = 58.1
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 345.00 = 6363.00 FEET.

*****
FLOW PROCESS FROM NODE 354.00 TO NODE 354.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.18
RAINFALL INTENSITY(INCH/HR) = 5.16
TOTAL STREAM AREA(ACRES) = 58.08
PEAK FLOW RATE(CFS) AT CONFLUENCE = 121.99

*****
FLOW PROCESS FROM NODE 357.00 TO NODE 358.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 70.00
UPSTREAM ELEVATION(FEET) = 665.00
DOWNSTREAM ELEVATION(FEET) = 656.00
ELEVATION DIFFERENCE(FEET) = 9.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.748
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.60
TOTAL AREA(ACRES) = 0.23 TOTAL RUNOFF(CFS) = 1.60

*****
FLOW PROCESS FROM NODE 358.00 TO NODE 359.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 656.00 DOWNSTREAM ELEVATION(FEET) = 604.50
STREET LENGTH(FEET) = 835.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 9.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.85
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.29
HALFSTREET FLOOD WIDTH(FEET) = 8.15
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.01
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.45
STREET FLOW TRAVEL TIME(MIN.) = 2.78 Tc(MIN.) = 4.52
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 1.80 SUBAREA RUNOFF(CFS) = 12.50
TOTAL AREA(ACRES) = 2.0 PEAK FLOW RATE(CFS) = 14.09

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.51
FLOW VELOCITY(FEET/SEC.) = 5.76 DEPTH*VELOCITY(FT*FT/SEC.) = 1.94
LONGEST FLOWPATH FROM NODE 357.00 TO NODE 359.00 = 905.00 FEET.

*****
FLOW PROCESS FROM NODE 359.00 TO NODE 355.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 599.00 DOWNSTREAM(FEET) = 598.00
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.62
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.09
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 4.57
LONGEST FLOWPATH FROM NODE 357.00 TO NODE 355.00 = 935.00 FEET.

*****
FLOW PROCESS FROM NODE 355.00 TO NODE 354.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 558.00 DOWNSTREAM(FEET) = 557.00
FLOW LENGTH(FEET) = 60.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.99
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.09
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 4.68
LONGEST FLOWPATH FROM NODE 357.00 TO NODE 354.00 = 995.00 FEET.

*****
FLOW PROCESS FROM NODE 354.00 TO NODE 354.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.68
RAINFALL INTENSITY(INCH/HR) = 8.17
TOTAL STREAM AREA(ACRES) = 2.03
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.09

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1           121.99      10.18      5.163          58.08
2           14.09       4.68       8.168          2.03

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1           91.21       4.68      8.168
2           130.90      10.18     5.163

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 130.90 Tc(MIN.) = 10.18
TOTAL AREA(ACRES) = 60.1
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 354.00 = 6363.00 FEET.

*****
FLOW PROCESS FROM NODE 354.00 TO NODE 361.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 600.00 DOWNSTREAM(FEET) = 585.00
FLOW LENGTH(FEET) = 158.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 29.86
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 130.90
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 10.27
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 361.00 = 6521.00 FEET.

*****
FLOW PROCESS FROM NODE 361.00 TO NODE 281.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 580.00 DOWNSTREAM(FEET) = 579.00
FLOW LENGTH(FEET) = 817.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 72.0 INCH PIPE IS 53.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.79
ESTIMATED PIPE DIAMETER(INCH) = 72.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 130.90
PIPE TRAVEL TIME(MIN.) = 2.35 Tc(MIN.) = 12.62
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 281.00 = 7338.00 FEET.

*****
FLOW PROCESS FROM NODE 361.00 TO NODE 281.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.494
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5315
SUBAREA AREA(ACRES) = 3.68 SUBAREA RUNOFF(CFS) = 5.79
TOTAL AREA(ACRES) = 63.8 TOTAL RUNOFF(CFS) = 152.39
Tc(MIN.) = 12.62

*****
FLOW PROCESS FROM NODE 281.00 TO NODE 281.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

```



```

=====
** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1           152.39     12.62      4.494      63.79
LONGEST FLOWPATH FROM NODE 330.00 TO NODE 281.00 = 7338.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1           195.82     14.16      4.174      97.55
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 281.00 = 4441.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
1           327.00     12.62      4.494
2           337.35     14.16      4.174

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 337.35 Tc(MIN.) = 14.16
TOTAL AREA(ACRES) = 161.3

*****
FLOW PROCESS FROM NODE 281.00 TO NODE 281.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 161.3 TC(MIN.) = 14.16
PEAK FLOW RATE(CFS) = 337.35
=====
END OF RATIONAL METHOD ANALYSIS

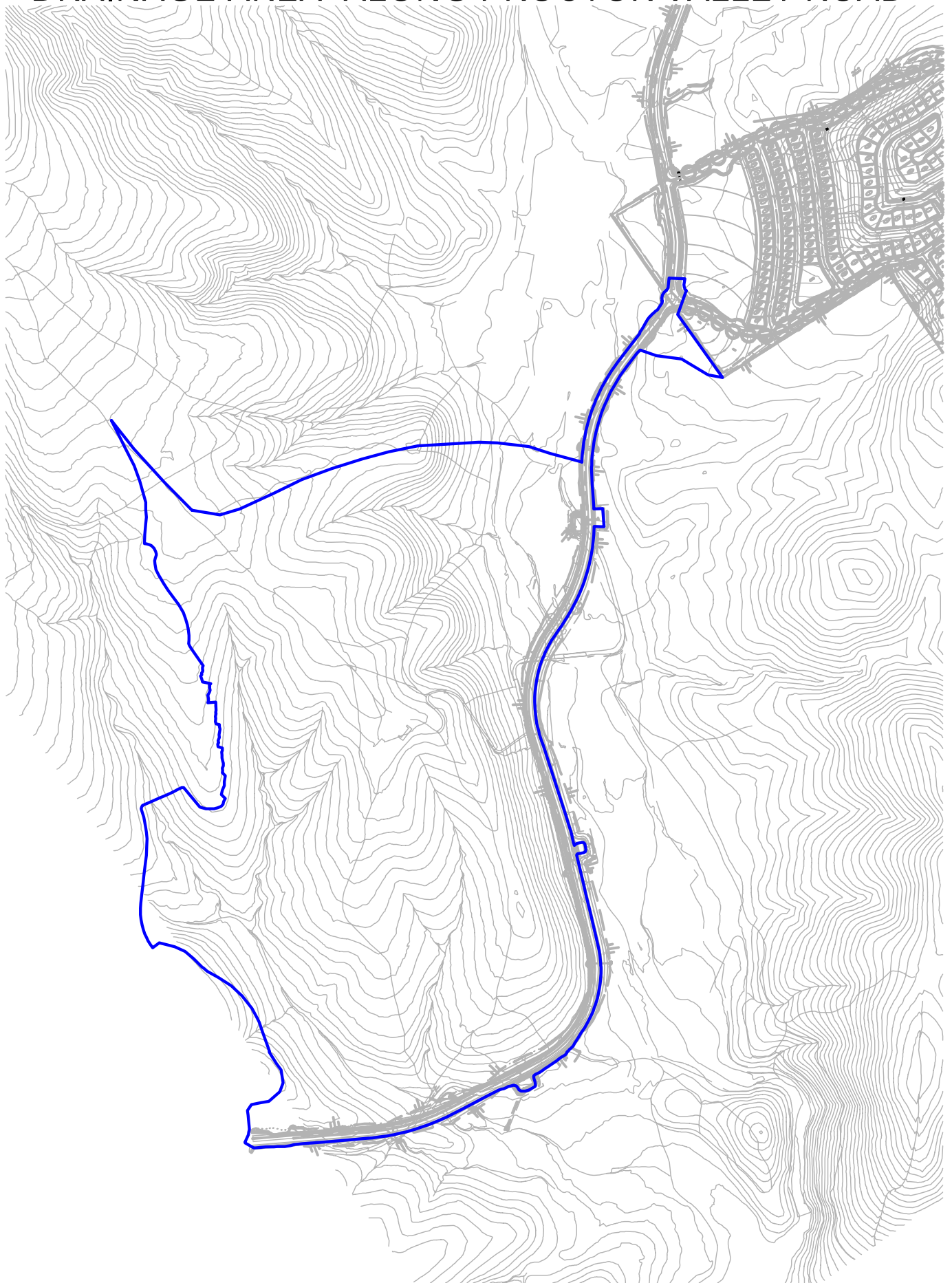
```

CHAPTER 5

5.1.4 – Rational Method Hydrologic Analysis (AES 2015)

Drainage Areas Along Proctor Valley Road (V14 South)

OTAY RANCH VILLAGE 14 & PA 16/19 DRAINAGE AREA ALONG PROCTOR VALLEY ROAD



```

*****
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1239

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* Village 14- Drainage Area along Proctor Valley Road *
* 100- year hydrology model *
*
*****
FILE NAME: R:\1235\HYD\CALCS\AES\SRP\PVR-S.DAT
TIME/DATE OF STUDY: 13:28 11/14/2016
-----
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
-----
2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.100
SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
  HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
  WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
== =====
1 16.0 8.0 0.020/0.020/0.020 0.50 2.00 0.0312 0.125 0.0150
2 12.0 6.0 0.020/0.020/0.020 0.50 1.50 0.0312 0.125 0.0130

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*****
FLOW PROCESS FROM NODE 950.00 TO NODE 951.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 595.00
DOWNSTREAM ELEVATION(FEET) = 593.50
ELEVATION DIFFERENCE(FEET) = 1.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.230
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 67.50
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.04
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 1.04

*****
FLOW PROCESS FROM NODE 951.00 TO NODE 952.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
=====
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 593.50 DOWNSTREAM ELEVATION(FEET) = 587.00
STREET LENGTH(FEET) = 701.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 11.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.26
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.32
HALFSTREET FLOOD WIDTH(FEET) = 9.48
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.09
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.66
STREET FLOW TRAVEL TIME(MIN.) = 5.58 Tc(MIN.) = 8.81
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.669
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850

SUBAREA AREA(ACRES) = 1.30 SUBAREA RUNOFF(CFS) = 6.26
TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) = 6.99

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.36 HALFSTREET FLOOD WIDTH(FEET) = 11.64
FLOW VELOCITY(FEET/SEC.) = 2.37 DEPTH*VELOCITY(FT*FT/SEC.) = 0.85
LONGEST FLOWPATH FROM NODE 950.00 TO NODE 952.00 = 801.00 FEET.

*****
FLOW PROCESS FROM NODE 953.00 TO NODE 957.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 582.00 DOWNSTREAM(FEET) = 577.00
FLOW LENGTH(FEET) = 504.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.22
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.99
PIPE TRAVEL TIME(MIN.) = 1.35 Tc(MIN.) = 10.16
LONGEST FLOWPATH FROM NODE 950.00 TO NODE 957.00 = 1305.00 FEET.

*****
FLOW PROCESS FROM NODE 957.00 TO NODE 957.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.16
RAINFALL INTENSITY(INCH/HR) = 5.17
TOTAL STREAM AREA(ACRES) = 1.45
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.99

*****
FLOW PROCESS FROM NODE 955.00 TO NODE 956.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 586.75
DOWNSTREAM ELEVATION(FEET) = 585.75
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.486
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.11
TOTAL AREA(ACRES) = 0.16 TOTAL RUNOFF(CFS) = 1.11

*****
FLOW PROCESS FROM NODE 956.00 TO NODE 957.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 585.75 DOWNSTREAM ELEVATION(FEET) = 581.75
STREET LENGTH(FEET) = 394.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 11.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.92
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.28
HALFSTREET FLOOD WIDTH(FEET) = 7.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.99
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.56
STREET FLOW TRAVEL TIME(MIN.) = 3.30 Tc(MIN.) = 6.78
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.709
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 0.63 SUBAREA RUNOFF(CFS) = 3.59
TOTAL AREA(ACRES) = 0.8 PEAK FLOW RATE(CFS) = 4.51

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 HALFSTREET FLOOD WIDTH(FEET) = 9.48
FLOW VELOCITY(FEET/SEC.) = 2.22 DEPTH*VELOCITY(FT*FT/SEC.) = 0.70
LONGEST FLOWPATH FROM NODE 955.00 TO NODE 957.00 = 494.00 FEET.

*****
FLOW PROCESS FROM NODE 957.00 TO NODE 957.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2

```



```

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 6.78
RAINFALL INTENSITY(INCH/HR) = 6.71
TOTAL STREAM AREA(ACRES) = 0.79
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.51

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 6.99 10.16 5.171 1.45
2 4.51 6.78 6.709 0.79

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 9.17 6.78 6.709
2 10.46 10.16 5.171

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 10.46 Tc(MIN.) = 10.16
TOTAL AREA(ACRES) = 2.2
LONGEST FLOWPATH FROM NODE 950.00 TO NODE 957.00 = 1305.00 FEET.

*****
FLOW PROCESS FROM NODE 957.00 TO NODE 962.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 577.00 DOWNSTREAM(FEET) = 572.00
FLOW LENGTH(FEET) = 470.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 14.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.81
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.46
PIPE TRAVEL TIME(MIN.) = 1.15 Tc(MIN.) = 11.31
LONGEST FLOWPATH FROM NODE 950.00 TO NODE 962.00 = 1775.00 FEET.

*****
FLOW PROCESS FROM NODE 962.00 TO NODE 962.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
-----
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.31
RAINFALL INTENSITY(INCH/HR) = 4.83
TOTAL STREAM AREA(ACRES) = 2.24
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.46

*****
FLOW PROCESS FROM NODE 960.00 TO NODE 961.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 90.00
UPSTREAM ELEVATION(FEET) = 585.40
DOWNSTREAM ELEVATION(FEET) = 584.60
ELEVATION DIFFERENCE(FEET) = 0.80
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.557
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 57.78
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.11
TOTAL AREA(ACRES) = 0.16 TOTAL RUNOFF(CFS) = 1.11

*****
FLOW PROCESS FROM NODE 961.00 TO NODE 962.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 584.60 DOWNSTREAM ELEVATION(FEET) = 577.10
STREET LENGTH(FEET) = 751.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 11.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.53
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.35
HALFSTREET FLOOD WIDTH(FEET) = 11.21
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.37
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.83
STREET FLOW TRAVEL TIME(MIN.) = 5.27 Tc(MIN.) = 8.83
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.659
*USER SPECIFIED(SUBAREA):

NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 2.19 SUBAREA RUNOFF(CFS) = 10.53
TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 11.30

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.41 HALFSTREET FLOOD WIDTH(FEET) = 13.96
FLOW VELOCITY(FEET/SEC.) = 2.73 DEPTH*VELOCITY(FT*FT/SEC.) = 1.11
LONGEST FLOWPATH FROM NODE 960.00 TO NODE 962.00 = 841.00 FEET.

*****
FLOW PROCESS FROM NODE 962.00 TO NODE 962.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.83
RAINFALL INTENSITY(INCH/HR) = 5.66
TOTAL STREAM AREA(ACRES) = 2.35
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.30

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 10.46 11.31 4.825 2.24
2 11.30 8.83 5.659 2.35

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 20.22 8.83 5.659
2 20.10 11.31 4.825

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 20.22 Tc(MIN.) = 8.83
TOTAL AREA(ACRES) = 4.6
LONGEST FLOWPATH FROM NODE 950.00 TO NODE 962.00 = 1775.00 FEET.

*****
FLOW PROCESS FROM NODE 962.00 TO NODE 963.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 572.00 DOWNSTREAM(FEET) = 570.00
FLOW LENGTH(FEET) = 120.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 14.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.83
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 20.22
PIPE TRAVEL TIME(MIN.) = 0.20 Tc(MIN.) = 9.04
LONGEST FLOWPATH FROM NODE 950.00 TO NODE 963.00 = 1895.00 FEET.

*****
FLOW PROCESS FROM NODE 965.00 TO NODE 966.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00
UPSTREAM ELEVATION(FEET) = 585.40
DOWNSTREAM ELEVATION(FEET) = 584.36
ELEVATION DIFFERENCE(FEET) = 1.04
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.311
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 64.50
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.11
TOTAL AREA(ACRES) = 0.16 TOTAL RUNOFF(CFS) = 1.11

*****
FLOW PROCESS FROM NODE 966.00 TO NODE 967.00 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 584.35 DOWNSTREAM ELEVATION(FEET) = 579.00
STREET LENGTH(FEET) = 558.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 11.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.59
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30

```

```

    HALFSTREET FLOOD WIDTH(FEET) =      8.74
    AVERAGE FLOW VELOCITY(FEET/SEC.) =      2.04
    PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =      0.61
    STREET FLOW TRAVEL TIME(MIN.) =      4.56    Tc(MIN.) =      7.87
    100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      6.095
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) =      0
AREA-AVERAGE RUNOFF COEFFICIENT =      0.850
SUBAREA AREA(ACRES) =      0.94    SUBAREA RUNOFF(CFS) =      4.87
TOTAL AREA(ACRES) =      1.1    PEAK FLOW RATE(CFS) =      5.70

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34    HALFSTREET FLOOD WIDTH(FEET) = 10.67
FLOW VELOCITY(FEET/SEC.) = 2.27    DEPTH*VELOCITY(FT*FT/SEC.) = 0.77
LONGEST FLOWPATH FROM NODE      965.00 TO NODE      967.00 =      638.00 FEET.

*****
FLOW PROCESS FROM NODE      967.00 TO NODE      972.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      575.00    DOWNSTREAM(FEET) =      565.00
FLOW LENGTH(FEET) =      900.00    MANNING'S N =      0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS      9.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      6.19
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      5.70
PIPE TRAVEL TIME(MIN.) =      2.42    Tc(MIN.) =      10.29
LONGEST FLOWPATH FROM NODE      965.00 TO NODE      972.00 =      1538.00 FEET.

*****
FLOW PROCESS FROM NODE      972.00 TO NODE      972.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =      10.29
RAINFALL INTENSITY(INCH/HR) =      5.13
TOTAL STREAM AREA(ACRES) =      1.10
PEAK FLOW RATE(CFS) AT CONFLUENCE =      5.70

*****
FLOW PROCESS FROM NODE      970.00 TO NODE      971.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) =      0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      100.00
UPSTREAM ELEVATION(FEET) =      578.90
DOWNSTREAM ELEVATION(FEET) =      577.90
ELEVATION DIFFERENCE(FEET) =      1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      3.486
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
    THE MAXIMUM OVERLAND FLOW LENGTH =      60.00
    (Reference: Table 3-1B of Hydrology Manual)
    THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
    100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =      1.32
TOTAL AREA(ACRES) =      0.19    TOTAL RUNOFF(CFS) =      1.32

*****
FLOW PROCESS FROM NODE      971.00 TO NODE      972.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) =      577.90    DOWNSTREAM ELEVATION(FEET) =      570.00
STREET LENGTH(FEET) =      782.00    CURB HEIGHT(INCHES) =      6.0
STREET HALFWIDTH(FEET) =      22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =      11.00
INSIDE STREET CROSSFALL(DECIMAL) =      0.020
OUTSIDE STREET CROSSFALL(DECIMAL) =      0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) =      0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) =      0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section =      0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      6.51
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) =      0.35
HALFSTREET FLOOD WIDTH(FEET) =      11.13
AVERAGE FLOW VELOCITY(FEET/SEC.) =      2.40
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =      0.84
STREET FLOW TRAVEL TIME(MIN.) =      5.44    Tc(MIN.) =      8.92
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      5.622
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) =      0
AREA-AVERAGE RUNOFF COEFFICIENT =      0.850
SUBAREA AREA(ACRES) =      2.12    SUBAREA RUNOFF(CFS) =      10.13
TOTAL AREA(ACRES) =      2.3    PEAK FLOW RATE(CFS) =      11.04

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.40    HALFSTREET FLOOD WIDTH(FEET) = 13.88

FLOW VELOCITY(FEET/SEC.) =      2.70    DEPTH*VELOCITY(FT*FT/SEC.) =      1.09
LONGEST FLOWPATH FROM NODE      970.00 TO NODE      972.00 =      882.00 FEET.

*****
FLOW PROCESS FROM NODE      972.00 TO NODE      972.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) =      8.92
RAINFALL INTENSITY(INCH/HR) =      5.62
TOTAL STREAM AREA(ACRES) =      2.31
PEAK FLOW RATE(CFS) AT CONFLUENCE =      11.04

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1            5.70      10.29      5.126      1.10
2           11.04      8.92      5.622      2.31

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           15.98      8.92      5.622
2           15.76      10.29      5.126

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      15.98    Tc(MIN.) =      8.92
TOTAL AREA(ACRES) =      3.4
LONGEST FLOWPATH FROM NODE      965.00 TO NODE      972.00 =      1538.00 FEET.

*****
FLOW PROCESS FROM NODE      972.00 TO NODE      973.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      565.00    DOWNSTREAM(FEET) =      560.00
FLOW LENGTH(FEET) =      195.00    MANNING'S N =      0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS      14.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      10.57
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      15.98
PIPE TRAVEL TIME(MIN.) =      0.31    Tc(MIN.) =      9.23
LONGEST FLOWPATH FROM NODE      965.00 TO NODE      973.00 =      1733.00 FEET.

*****
FLOW PROCESS FROM NODE      975.00 TO NODE      976.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) =      0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      100.00
UPSTREAM ELEVATION(FEET) =      574.80
DOWNSTREAM ELEVATION(FEET) =      573.90
ELEVATION DIFFERENCE(FEET) =      0.90
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      3.550
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
    THE MAXIMUM OVERLAND FLOW LENGTH =      58.00
    (Reference: Table 3-1B of Hydrology Manual)
    THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
    100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =      1.11
TOTAL AREA(ACRES) =      0.16    TOTAL RUNOFF(CFS) =      1.11

*****
FLOW PROCESS FROM NODE      976.00 TO NODE      977.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) =      573.90    DOWNSTREAM ELEVATION(FEET) =      563.40
STREET LENGTH(FEET) =      1089.00    CURB HEIGHT(INCHES) =      6.0
STREET HALFWIDTH(FEET) =      22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =      11.00
INSIDE STREET CROSSFALL(DECIMAL) =      0.020
OUTSIDE STREET CROSSFALL(DECIMAL) =      0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) =      0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) =      0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section =      0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      11.19
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) =      0.41
HALFSTREET FLOOD WIDTH(FEET) =      14.05
AVERAGE FLOW VELOCITY(FEET/SEC.) =      2.67
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =      1.09
STREET FLOW TRAVEL TIME(MIN.) =      6.79    Tc(MIN.) =      10.34
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =      5.112
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) =      0

```

```

AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 4.46 SUBAREA RUNOFF(CFS) = 19.38
TOTAL AREA(ACRES) = 4.6 PEAK FLOW RATE(CFS) = 20.07

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.48 HALFSTREET FLOOD WIDTH(FEET) = 17.75
FLOW VELOCITY(FEET/SEC.) = 3.07 DEPTH*VELOCITY(FT*FT/SEC.) = 1.48
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
      AND L = 1089.0 FT WITH ELEVATION-DROP = 10.5 FT, IS 31.0 CFS,
      WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 977.00
LONGEST FLOWPATH FROM NODE 975.00 TO NODE 977.00 = 1189.00 FEET.

*****
FLOW PROCESS FROM NODE 977.00 TO NODE 977.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.34
RAINFALL INTENSITY(INCH/HR) = 5.11
TOTAL STREAM AREA(ACRES) = 4.62
PEAK FLOW RATE(CFS) AT CONFLUENCE = 20.07

*****
FLOW PROCESS FROM NODE 980.00 TO NODE 981.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 98.00
UPSTREAM ELEVATION(FEET) = 626.00
DOWNSTREAM ELEVATION(FEET) = 624.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.081
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 75.41
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.168
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.25
TOTAL AREA(ACRES) = 0.18 TOTAL RUNOFF(CFS) = 1.25

*****
FLOW PROCESS FROM NODE 981.00 TO NODE 982.00 IS CODE = 61
-----
>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 624.00 DOWNSTREAM ELEVATION(FEET) = 592.00
STREET LENGTH(FEET) = 910.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 22.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 11.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.36
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.30
HALFSTREET FLOOD WIDTH(FEET) = 8.44
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.83
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.13
STREET FLOW TRAVEL TIME(MIN.) = 3.96 Tc(MIN.) = 7.04
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.548
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .8500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.850
SUBAREA AREA(ACRES) = 1.81 SUBAREA RUNOFF(CFS) = 10.07
TOTAL AREA(ACRES) = 2.0 PEAK FLOW RATE(CFS) = 11.08

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.34 HALFSTREET FLOOD WIDTH(FEET) = 10.74
FLOW VELOCITY(FEET/SEC.) = 4.35 DEPTH*VELOCITY(FT*FT/SEC.) = 1.49
LONGEST FLOWPATH FROM NODE 980.00 TO NODE 982.00 = 1008.00 FEET.

*****
FLOW PROCESS FROM NODE 982.00 TO NODE 977.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 585.00 DOWNSTREAM(FEET) = 560.00
FLOW LENGTH(FEET) = 948.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.07
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 11.08
PIPE TRAVEL TIME(MIN.) = 1.57 Tc(MIN.) = 8.61
LONGEST FLOWPATH FROM NODE 980.00 TO NODE 977.00 = 1956.00 FEET.

*****
FLOW PROCESS FROM NODE 977.00 TO NODE 977.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 8.61
RAINFALL INTENSITY(INCH/HR) = 5.75
TOTAL STREAM AREA(ACRES) = 1.99
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.08

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 20.07 10.34 5.112 4.62
2 11.08 8.61 5.751 1.99

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 27.80 8.61 5.751
2 29.92 10.34 5.112

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 29.92 Tc(MIN.) = 10.34
TOTAL AREA(ACRES) = 6.6
LONGEST FLOWPATH FROM NODE 980.00 TO NODE 977.00 = 1956.00 FEET.

*****
FLOW PROCESS FROM NODE 977.00 TO NODE 984.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 560.00 DOWNSTREAM(FEET) = 550.00
FLOW LENGTH(FEET) = 295.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.13
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 29.92
PIPE TRAVEL TIME(MIN.) = 0.35 Tc(MIN.) = 10.69
LONGEST FLOWPATH FROM NODE 980.00 TO NODE 984.00 = 2251.00 FEET.

*****
FLOW PROCESS FROM NODE 985.00 TO NODE 986.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 715.00
DOWNSTREAM ELEVATION(FEET) = 705.00
ELEVATION DIFFERENCE(FEET) = 10.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.061
SUBAREA RUNOFF(CFS) = 0.47
TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.47

*****
FLOW PROCESS FROM NODE 986.00 TO NODE 987.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 700.00 DOWNSTREAM(FEET) = 560.00
FLOW LENGTH(FEET) = 1154.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.00
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.04
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.47
PIPE TRAVEL TIME(MIN.) = 2.73 Tc(MIN.) = 9.00
LONGEST FLOWPATH FROM NODE 985.00 TO NODE 987.00 = 1254.00 FEET.

*****
FLOW PROCESS FROM NODE 986.00 TO NODE 987.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.591
*USER SPECIFIED(SUBAREA):
NEIGHBORHOOD COMMERCIAL RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 14.04 SUBAREA RUNOFF(CFS) = 27.47
TOTAL AREA(ACRES) = 14.2 TOTAL RUNOFF(CFS) = 27.85
Tc(MIN.) = 9.00

*****
FLOW PROCESS FROM NODE 987.00 TO NODE 988.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 560.00 DOWNSTREAM(FEET) = 550.00
FLOW LENGTH(FEET) = 177.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.79

```

```

ESTIMATED PIPE DIAMETER(INCH) = 21.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 27.85
PIPE TRAVEL TIME(MIN.) = 0.18    Tc(MIN.) = 9.17
LONGEST FLOWPATH FROM NODE 985.00 TO NODE 988.00 = 1431.00 FEET.

*****
FLOW PROCESS FROM NODE 990.00 TO NODE 991.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 677.00
DOWNSTREAM ELEVATION(FEET) = 670.00
ELEVATION DIFFERENCE(FEET) = 7.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.058
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.540
SUBAREA RUNOFF(CFS) = 0.16
TOTAL AREA(ACRES) = 0.07    TOTAL RUNOFF(CFS) = 0.16

*****
FLOW PROCESS FROM NODE 991.00 TO NODE 992.00 IS CODE = 53
-----
>>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 670.00    DOWNSTREAM(FEET) = 570.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 513.00    CHANNEL SLOPE = 0.1949
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.16
FLOW VELOCITY(FEET/SEC) = 2.47 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 3.46    Tc(MIN.) = 10.52
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 992.00 = 613.00 FEET.

*****
FLOW PROCESS FROM NODE 991.00 TO NODE 992.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.056
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 7.79    SUBAREA RUNOFF(CFS) = 13.79
TOTAL AREA(ACRES) = 7.9    TOTAL RUNOFF(CFS) = 13.91
Tc(MIN.) = 10.52

*****
FLOW PROCESS FROM NODE 992.00 TO NODE 993.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 570.00    DOWNSTREAM(FEET) = 545.00
FLOW LENGTH(FEET) = 160.00    MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.78
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 13.91
PIPE TRAVEL TIME(MIN.) = 0.13    Tc(MIN.) = 10.64
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 993.00 = 773.00 FEET.

*****
FLOW PROCESS FROM NODE 994.00 TO NODE 994.00 IS CODE = 7
-----
>>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<<
=====
USER-SPECIFIED VALUES ARE AS FOLLOWS:
Tc(MIN) = 22.29    RAIN INTENSITY(INCH/HOUR) = 3.11
TOTAL AREA(ACRES) = 261.03    TOTAL RUNOFF(CFS) = 469.76

*****
FLOW PROCESS FROM NODE 994.00 TO NODE 995.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 605.00    DOWNSTREAM(FEET) = 600.00
FLOW LENGTH(FEET) = 233.00    MANNING'S N = 0.013
DEPTH OF FLOW IN 66.0 INCH PIPE IS 52.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 23.00
ESTIMATED PIPE DIAMETER(INCH) = 66.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 469.76
PIPE TRAVEL TIME(MIN.) = 0.17    Tc(MIN.) = 22.46
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 995.00 = 1006.00 FEET.

*****
FLOW PROCESS FROM NODE 994.00 TO NODE 995.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.099
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5754
SUBAREA AREA(ACRES) = 2.78    SUBAREA RUNOFF(CFS) = 3.02
TOTAL AREA(ACRES) = 263.8    TOTAL RUNOFF(CFS) = 470.50
Tc(MIN.) = 22.46

*****
FLOW PROCESS FROM NODE 995.00 TO NODE 996.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 600.00    DOWNSTREAM(FEET) = 575.00
FLOW LENGTH(FEET) = 771.00    MANNING'S N = 0.013
DEPTH OF FLOW IN 63.0 INCH PIPE IS 46.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 27.23
ESTIMATED PIPE DIAMETER(INCH) = 63.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 470.50
PIPE TRAVEL TIME(MIN.) = 0.47    Tc(MIN.) = 22.93
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 996.00 = 1777.00 FEET.

*****
FLOW PROCESS FROM NODE 997.00 TO NODE 997.00 IS CODE = 7
-----
>>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<<
=====
USER-SPECIFIED VALUES ARE AS FOLLOWS:
Tc(MIN) = 20.04    RAIN INTENSITY(INCH/HOUR) = 3.34
TOTAL AREA(ACRES) = 718.66    TOTAL RUNOFF(CFS) = 1373.90

*****
FLOW PROCESS FROM NODE 997.00 TO NODE 998.00 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 600.00    DOWNSTREAM(FEET) = 559.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1925.00    CHANNEL SLOPE = 0.0213
CHANNEL FLOW THRU SUBAREA(CFS) = 1373.90
FLOW VELOCITY(FEET/SEC) = 15.88 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.02    Tc(MIN.) = 22.06
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 998.00 = 3702.00 FEET.

*****
FLOW PROCESS FROM NODE 997.00 TO NODE 998.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.135
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5634
SUBAREA AREA(ACRES) = 32.59    SUBAREA RUNOFF(CFS) = 35.76
TOTAL AREA(ACRES) = 751.2    TOTAL RUNOFF(CFS) = 1373.90
Tc(MIN.) = 22.06
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****
FLOW PROCESS FROM NODE 998.00 TO NODE 998.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 22.06
RAINFALL INTENSITY(INCH/HR) = 3.14
TOTAL STREAM AREA(ACRES) = 751.25
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1373.90

*****
FLOW PROCESS FROM NODE 998.10 TO NODE 998.20 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 745.00
DOWNSTREAM ELEVATION(FEET) = 742.00
ELEVATION DIFFERENCE(FEET) = 3.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.361
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.451
SUBAREA RUNOFF(CFS) = 0.17
TOTAL AREA(ACRES) = 0.09    TOTAL RUNOFF(CFS) = 0.17

*****
FLOW PROCESS FROM NODE 998.20 TO NODE 998.30 IS CODE = 52
-----
>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 742.00    DOWNSTREAM(FEET) = 650.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1153.00    CHANNEL SLOPE = 0.0798
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.17
FLOW VELOCITY(FEET/SEC) = 4.24 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 4.54    Tc(MIN.) = 13.90
LONGEST FLOWPATH FROM NODE 998.10 TO NODE 998.30 = 1253.00 FEET.

*****
FLOW PROCESS FROM NODE 998.20 TO NODE 998.30 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.224
*USER SPECIFIED(SUBAREA):

```



```

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 10.82 SUBAREA RUNOFF(CFS) = 16.00
TOTAL AREA(ACRES) = 10.9 TOTAL RUNOFF(CFS) = 16.13
TC(MIN.) = 13.90

*****
FLOW PROCESS FROM NODE 998.30 TO NODE 998.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 650.00 DOWNSTREAM(FEET) = 559.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1816.00 CHANNEL SLOPE = 0.0501
CHANNEL FLOW THRU SUBAREA(CFS) = 16.13
FLOW VELOCITY(FEET/SEC) = 6.33 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 4.78 Tc(MIN.) = 18.68
LONGEST FLOWPATH FROM NODE 998.10 TO NODE 998.00 = 3069.00 FEET.

*****
FLOW PROCESS FROM NODE 998.30 TO NODE 998.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.491
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 36.38 SUBAREA RUNOFF(CFS) = 44.45
TOTAL AREA(ACRES) = 47.3 TOTAL RUNOFF(CFS) = 57.78
TC(MIN.) = 18.68

*****
FLOW PROCESS FROM NODE 998.00 TO NODE 998.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 18.68
RAINFALL INTENSITY(INCH/HR) = 3.49
TOTAL STREAM AREA(ACRES) = 47.29
PEAK FLOW RATE(CFS) AT CONFLUENCE = 57.78

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 1373.90 22.06 3.135 751.25
2 57.78 18.68 3.491 47.29

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 1220.83 18.68 3.491
2 1425.80 22.06 3.135

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1425.80 Tc(MIN.) = 22.06
TOTAL AREA(ACRES) = 798.5
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 998.00 = 3702.00 FEET.

*****
FLOW PROCESS FROM NODE 998.00 TO NODE 999.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 559.00 DOWNSTREAM(FEET) = 550.00
FLOW LENGTH(FEET) = 210.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 90.0 INCH PIPE IS 68.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 39.79
ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1425.80
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 22.15
LONGEST FLOWPATH FROM NODE 990.00 TO NODE 999.00 = 3912.00 FEET.
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 798.5 TC(MIN.) = 22.15
PEAK FLOW RATE(CFS) = 1425.80
=====
END OF RATIONAL METHOD ANALYSIS

```

CHAPTER 5

5.1.5 – Rational Method Hydrologic Analysis (AES 2015)

Eastern Slopes Drainage Areas bypassing WQ Basins