OLD MIRAR DE VALLE ROAD

POINT OF COMPLIANCE

VALLEY CENTER ROAD

LEGEND:

DMA1	DMA BASIN IDENTIFICATION
BA1	BIOFILTRATION AREA
	PERVIOUS AREA
	BIOFILTRATION BASIN
	IMPERVIOUS AREA
	BASIN BOUNDARY
	DRAINAGE MANAGEMENT AREA BOUNDARY (DMA)
INLET	STORM WATER STENCILING "NO DUMPING, DRAINS TO MOOSA CREEK" OR SIMILAR
UG1	UNDERGROUND STORAGE
	FLOW DIRECTION

UNDERLYING HYDROLOGIC SOIL GROUP:
SOIL GROUP 'C'

HISTORIC DEPTH TO GROUND WATER:
10.1 FEET BELOW EXISTING GROUND

CRITICAL COARSE SEDIMENT YIELD AREAS:
NONE ONSITE

OFFSITE COARSE SEDIMENT YIELD AREAS –
NONE TRIBUTARY TO THE PROJECT LIMITS

SOURCE CONTROL BMPS

SC2 STORM DRAIN STENCILING OR SIGNAGE
(STENCILS ON CONCRETE CATCH BASINS AND INLETS PROHIBITING ILLEGAL DUMPING)

SC6A ONSITE STORM DRAIN INLETS
(SHOW LOCATIONS OF INLETS AND CATCH BASINS)

SC6D NEED FOR FUTURE INDOOR & STRUCTURAL PEST CONTROL
(NO LANDSCAPING ADJACENT TO DOORS)

SC6E LANDSCAPE/OUTDOOR PESTICIDE USE
(SHOW STORM WATER TREATMENT FACILITIES)

SC6H REFUSE AREAS
(REFUSE AREA WILL BE COVERED, GRADED AND PAVED TO PREVENT RUN-ON AND PROTECTED WITH DOORS AND BIN COVERS TO PREVENT WIND DISPERSAL)

SC6L VEHICLE/EQUIPMENT REPAIR AND MAINTENANCE
(NO VEHICLE REPAIR OR MAINTENANCE WILL BE DONE OUTDOORS)

SC6Q PLAZAS, SIDEWALKS AND PARKING LOTS
(SWEPT REGULARLY TO PREVENT THE ACCUMULATION OF LITTER & DEBRIS)

SITE DESIGN BMPS

SD3 MINIMIZE IMPERVIOUS AREAS
(MINIMIZE THE USE OF IMPERVIOUS AREAS IN LANDSCAPING DESIGN)

SD4 MINIMIZE SOIL COMPACTION
(MINIMIZE SOIL COMPACTION IN LANDSCAPED AREAS)

SD7 LANDSCAPE WITH NATIVE OR DROUGHT TOLERANT SPECIES

SELF MITIGATING DMAS MUST MEET THE FOLLOWING TO BE ELIGIBLE FOR EXCLUSION:

- VEGETATION IN THE NATURAL OR LANDSCAPED AREA IS NATIVE AND/OR NON-NATIVE/NON-INVASIVE DROUGHT TOLERANT SPECIES THAT DO NOT REQUIRE REGULAR APPLICATION OF FERTILIZERS AND PESTICIDES. SEE LANDSCAPE AND IRRIGATION PLANS (PDS2017-LP-17-096) FOR PROPOSED LANDSCAPING.
- SOILS ARE UNDISTURBED NATIVE TOPSOIL, OR DISTURBED SOILS THAT HAVE BEEN AMENDED AND AERATED TO PROMOTE WATER RETENTION CHARACTERISTICS TO UNDISTURBED NATIVE TOPSOIL. CONTRACTOR SHALL AERATE THE SOIL WITHIN THE LIMITS OF GRADING IN THIS AREA UPON COMPLETION OF GRADING ACTIVITIES AND PRIOR TO INSTALLATION OF LANDSCAPING AND IRRIGATION.
- THE INCIDENTAL IMPERVIOUS AREAS ARE LESS THAN 5 PERCENT OF THE SELF-MITIGATING AREA (INFORMATION ONLY).
- THE IMPERVIOUS AREA WITHIN THE SELF-MITIGATED AREA SHOULD NOT BE HYDRAULICALLY CONNECTED TO OTHER IMPERVIOUS AREAS UNLESS IT IS A STORM DRAIN CONVEYANCE SYSTEM SUCH AS A BROW DITCH (INFORMATION ONLY).
- THE SELF-MITIGATING AREA IS HYDRAULICALLY SEPARATE FROM DMAS THAT CONTAIN PERMANENT STORM WATER POLLUTANT CONTROL BMPS (INFORMATION ONLY).

SOURCE CONTROL BMPS FOR BUILDING:
1. FIRE SPRINKLER TEST WATER
(FIRE SPRINKLER WATER DRAINS TO SEWER)
2. MISCELLANEOUS DRAIN OR WASH WATER
(CONDENSATE LINES DISCHARGE TO L/S AND/OR BIOFILTRATION BASINS)
(ROOF TOP EQUIPMENT DESIGNED WITH SECONDARY CONTAINMENT)

FLOW CONTROL STRUCTURE

16'-4" x 1,037 SF BA1 SC6E SC6A INLET

SCALE: 1" = 10'

0 5 10 20 30

GRAPHICAL SCALE

SOURCE CONTROL EXHIBIT / DRAINAGE MANAGEMENT AREA (DMA) EXHIBIT

AUTOZONE

Alidade ENGINEERING
41743 ENTERPRISE CIRCLE NORTH, SUITE 209
TEMECULA, CA 92590
PH: (951) 587-2020
FAX: (951) 587-2626

16-202DMA.dwg 01-28-19

	DMA AREA (FT^2)	POST-PROJECT SURFACE TYPE	DMA RUNOFF FACTOR	DMA AREA X RUNOFF FACTOR	SOIL TYPE	IMP NAME
DMA1	27,144*	IMPERVIOUS	0.9	24,429.6	C	BA1
	1,294**	IMPERVIOUS	0.9	1,164.6		
	8,694	LANDSCAPING	0.1	869.4	IMP SIZING FACTOR	MINIMUM AREA OR VOLUME
			TOTAL	26,463.6	0.03	794

PROPOSED AREA OR VOLUME (FT^2) | IMP AREA
1,037

*INCLUDES 55 SF OF IMPERVIOUS AREA FROM TRACTOR SUPPLY SITE
**ONSITE IMPERVIOUS AREA THAT WAS CONSTRUCTED WITH THE ADJACENT TRACTOR SUPPLY PROJECT.

	DMA AREA (FT^2)	POST-PROJECT SURFACE TYPE	DMA RUNOFF FACTOR	DMA AREA X RUNOFF FACTOR	SOIL TYPE	
DMA2	50	IMPERVIOUS	0.9	45.0	C	
			TOTAL	45.0	DE MINIMIS AREA	

SHEET FLOWS TO ADJACENT PROPERTY TO THE SOUTH (TRACTOR SUPPLY SITE)

	DMA AREA (FT^2)	POST-PROJECT SURFACE TYPE	DMA RUNOFF FACTOR	DMA AREA X RUNOFF FACTOR	SOIL TYPE	
DMA3	3	IMPERVIOUS	0.9	2.7	C	
			TOTAL	2.7	DE MINIMIS AREA	

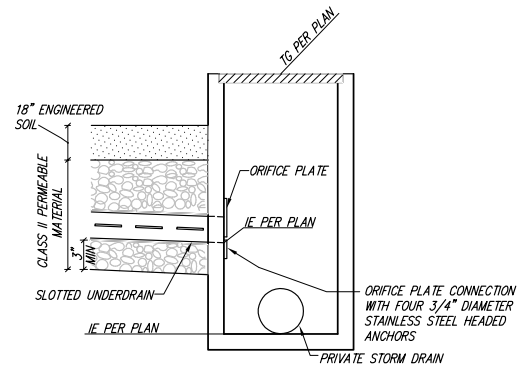
SHEET FLOWS TO ADJACENT PROPERTY TO THE SOUTH (TRACTOR SUPPLY SITE)

	DMA AREA (FT^2)	POST-PROJECT SURFACE TYPE	DMA RUNOFF FACTOR	DMA AREA X RUNOFF FACTOR	SOIL TYPE	
DMA4	172	IMPERVIOUS	0.9	154.8	C	
	3,431	LANDSCAPING	0.1	343.1		
			TOTAL	497.9	SELF-MITIGATING	

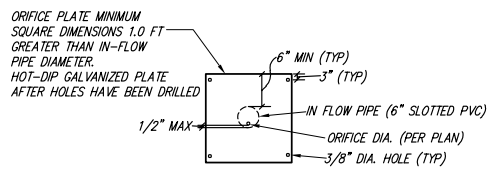
SHEET FLOWS TO VALLEY CENTER ROAD

	DMA AREA (FT^2)	POST-PROJECT SURFACE TYPE	DMA RUNOFF FACTOR	DMA AREA X RUNOFF FACTOR	SOIL TYPE	
DMA5	151	IMPERVIOUS	0.9	135.9	C	
	3,901	LANDSCAPING	0.1	390.1		
			TOTAL	526.0	SELF-MITIGATING	

SHEET FLOWS TO OLD MIRAR DE VALLE ROAD

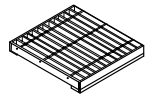


SECTION BIORETENTION CATCH BASIN
NOT TO SCALE

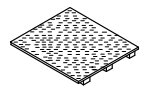


DETAIL: FLOW CONTROL ORIFICE PLATE (PVT)
NOT TO SCALE

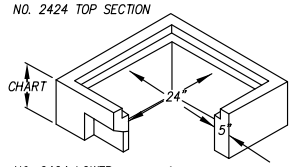
NO. 2424 STEEL TRAFFIC GRATE MAX. 1/2" SPACING WT 103# TRAF.



NO. 2424 STEEL TRAFFIC COVER WT. 114#

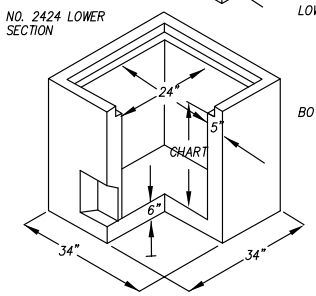


NO. 2424 TOP SECTION



TOP SECTION	HT.	LBS	AVAILABLE
2424 T6	6"	270	NO K.O.
2424 T12	12"	495	(4) 6" X 11"
2424 T18	18"	745	(4) 9" X 12"
2424 T24	24"	870	(4) 14" X 14"

NO. 2424 LOWER SECTION



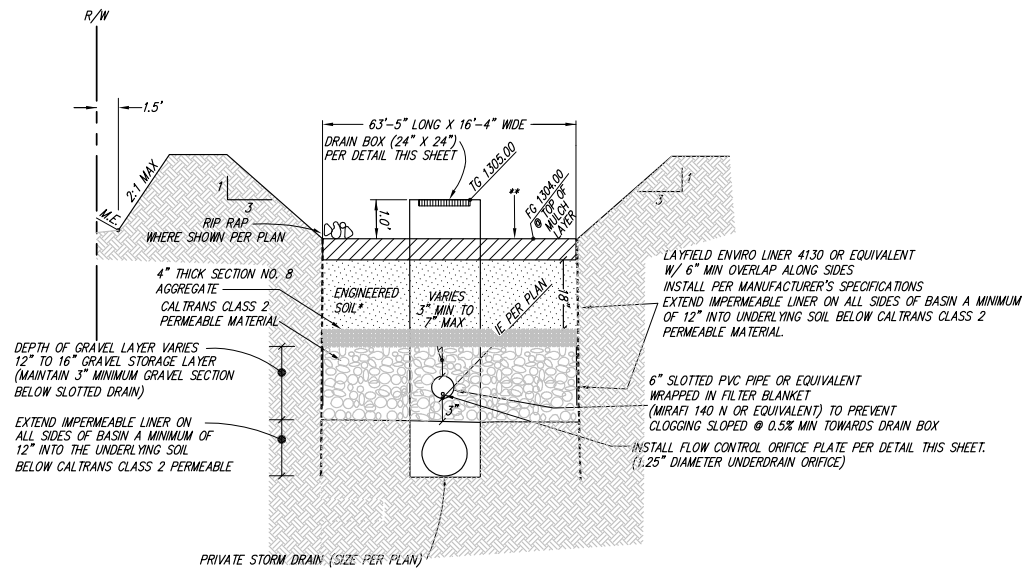
LOWER	HT.	LBS	AVAILABLE
2424 L12	12"	495	(4) 6" X 11"
2424 L18	18"	745	(4) 9" X 12"
2424 L24	24"	870	(4) 14" X 14"

BOTTOM	HT.	LBS	AVAILABLE
2424 B30	30"	1595	(4) 18" X 18"
2424 B36	36"	1905	(4) 18" X 18"

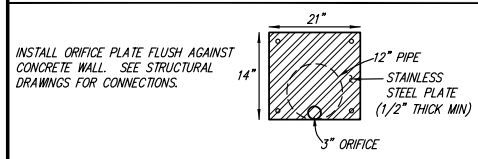
24" X 24" BROOKS BOX CATCH BASIN (PVT)
(OR EQUIVALENT)
NOT TO SCALE

*ENGINEERED SOIL: 75% - 80% SAND, 10% MAX SILT, 3% MAX CLAY AND 9% MAX COMPOST. COMPOST SHOULD COMPOSE NO MORE THAN 5% BULK WEIGHT OF THE ENGINEERED SOIL PER THE COUNTY OF SAN DIEGO LOW IMPACT DESIGN MANUAL, APPENDIX G.

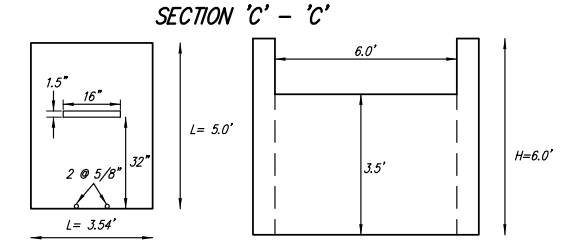
**INSTALL 3" DEPTH SECTION OF WELL-AGED, SHREDDED HARDWOOD MULCH THAT HAS BEEN STOCKPILED OR STORED FOR AT LEAST 12 MONTHS.



DETAIL: BIOFILTRATION BASIN (PVT) W/ NUTRIENT SENSITIVE MEDIA DESIGN
NOT TO SCALE

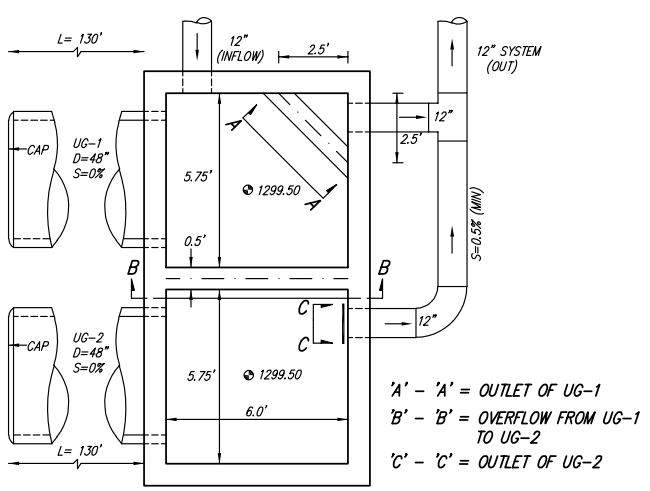


SECTION 'C' - 'C'



SECTION 'A' - 'A'

SECTION 'B' - 'B'



PLAN VIEW
6' X 12' JUNCTION STRUCTURE (PVT)
NOT TO SCALE

BIO CLEAN SCREENING FILTER WITH TROUGH SYSTEM
FOR USE IN CURB INLETS WITH WINGS

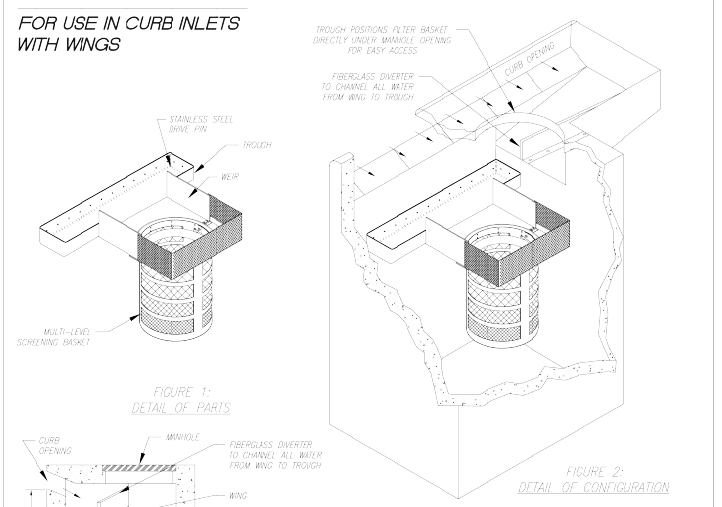


FIGURE 1: DETAIL OF PARTS
FIGURE 2: DETAIL OF CONFIGURATION
FIGURE 3: DETAIL OF MOUNTING
FIGURE 4: DETAIL OF PROFILE

NOTES:
1. TROUGH SYSTEM PROVIDES FOR ENTIRE COVERAGE OF INLET OPENING SO TO DIVERT ALL FLOW TO FILTER.
2. TROUGH SYSTEM MANUFACTURED FROM MARINE GRADE FIBERGLASS, GEL COATED FOR UV PROTECTION.
3. SYSTEM ATTACHED TO THE CATCH BASIN WITH NON-CORROSIVE HARDWARE.
4. FILTER MANUFACTURED OF 100% STAINLESS STEEL.
5. FILTER MADE OF NON-CLOGGING SCREEN WITH 4.7 MM OPENINGS AND MEETS FULL CAPTURE REQUIREMENTS.
6. FILTER CAN BE FITTED WITH HYDROCARBON ABSORBENT BOOM
7. FILTER IS LOCATED DIRECTLY UNDER THE MANHOLE FOR EASY REMOVAL AND MAINTENANCE.
8. LENGTH OF TROUGH CAN VARY FROM 2' TO 30'.
9. OTHER STANDARD AND CUSTOM MODEL SIZES AVAILABLE - CONTACT BIO CLEAN FOR MORE INFORMATION.
10. CONSIDERS A SAFETY FACTOR OF 2.0.
11. BYPASS IS FACILITATED VIA OVERFLOW OF THE TROUGH SYSTEM AND IS EQUAL TO THE CAPACITY OF THE CURB OPENING
12. STORAGE CAPACITY BASED ON THE BASKET HALF FULL.
13. ADDITIONAL TREATMENT AND STORAGE CAPACITY CAN BE ACHIEVED BY UTILIZING MULTIPLE FILTER BASKETS.

DRAWING: BIO CLEAN CURB INLET FILTER DETAILS	MULTI-LEVEL SCREENING
TREATMENT FLOW RATE: 2.85 cfs	MODEL #: BC-CURB-MLS
WARRANTY: 5 YEAR MANUFACTURERS	PROJECT:
BIO CLEAN ENVIRONMENTAL SERVICES, INC. 338 VIA EL CENTRO, OCEANSIDE CA 92058 PHONE: 760-433-7640 FAX: 760-433-3176	REVISIONS: DATE:
DATE: 10/12/2017 SCALE: NTS	REVISIONS: DATE:
DRAFTER: M.C.P. UNITS = INCHES	REVISIONS: DATE:

Bio Clean
A Forterra Company

PAGE 3

BIO CLEAN SCREENING FILTER WITH TROUGH SYSTEM
FOR USE IN CURB INLETS

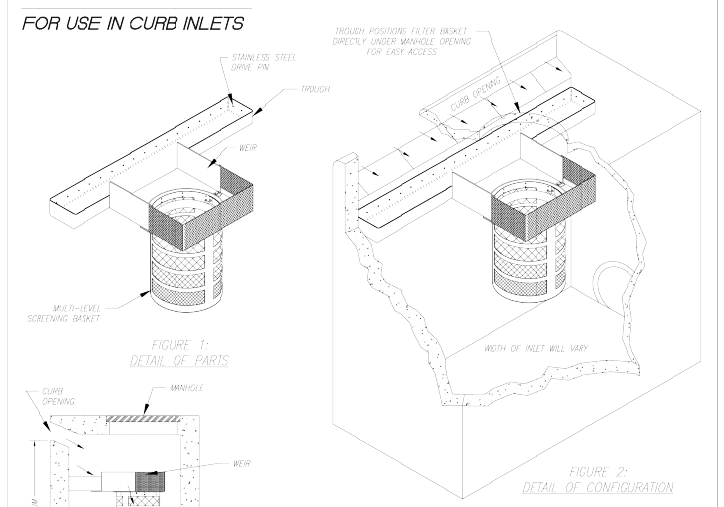


FIGURE 1: DETAIL OF PARTS
FIGURE 2: DETAIL OF CONFIGURATION
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NOTES:
1. TROUGH SYSTEM PROVIDES FOR ENTIRE COVERAGE OF INLET OPENING SO TO DIVERT ALL FLOW TO FILTER.
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10. CONSIDERS A SAFETY FACTOR OF 2.0.
11. BYPASS IS FACILITATED VIA OVERFLOW OF THE TROUGH SYSTEM AND IS EQUAL TO THE CAPACITY OF THE CURB OPENING
12. STORAGE CAPACITY BASED ON THE BASKET HALF FULL.
13. ADDITIONAL TREATMENT AND STORAGE CAPACITY CAN BE ACHIEVED BY UTILIZING MULTIPLE FILTER BASKETS.

MODEL NUMBER	TREATMENT FLOW (cfs)*	SOLIDS STORAGE CAPACITY (cu ft)
BC-CURB-30	2.85	2.21
BC-CURB-24	2.85	1.77
BC-CURB-18	2.85	1.33
BC-CURB-12	2.81	0.88

*SEE PAGE 2 FOR EXPLANATION OF FLOW RATES

DRAWING: BIO CLEAN CURB INLET FILTER DETAILS	MULTI-LEVEL SCREENING
TREATMENT FLOW RATE: 2.85 cfs	MODEL #: BC-CURB-MLS
WARRANTY: 5 YEAR MANUFACTURERS	PROJECT:
BIO CLEAN ENVIRONMENTAL SERVICES, INC. 338 VIA EL CENTRO, OCEANSIDE CA 92058 PHONE: 760-433-7640 FAX: 760-433-3176	REVISIONS: DATE:
DATE: 10/12/2017 SCALE: NTS	REVISIONS: DATE:
DRAFTER: M.C.P. UNITS = INCHES	REVISIONS: DATE:

Bio Clean
A Forterra Company

PAGE 1

DRAINAGE MANAGEMENT AREA (DMA) EXHIBIT
AUTOZONE

Alidade ENGINEERING
41743 ENTERPRISE CIRCLE NORTH, SUITE 209
TEMECULA, CA 92590
PH: (951) 587-2020
FAX: (951) 587-2626

16-2022MA2.dwg 1-29-19

ATTACHMENT 7

Copy of Project's Geotechnical and Groundwater Investigation Report

This is the cover sheet for Attachment 7.

If hardcopy or CD is not attached, the following information should be provided:

Title:
Prepared By:
Date:

This page was left intentionally blank.

SDVOSB . DVBE

SCST, Inc.
Corporate Headquarters
6280 Riverdale Street
San Diego, CA 92120
T 877.215.4321
P 619.280.4321
F 619.280.4717
W www.scst.com

March 30, 2018

SCST No. 170111N
Report No. 2

Mr. Mitch Bramlitt
Autozone Development Corporation
123 South Front Street
Memphis, Tennessee 98103

Subject: UPDATE INFILTRATION RECOMMENDATIONS
PROPOSED AUTOZONE
VALLEY CENTER ROAD
VALLEY CENTER, CALIFORNIA

References: 1) SCST, Inc., 2017, *Infiltration Feasibility Study, Proposed Autozone, Valley Center Road, Valley Center California*, SCST Report No. 170111N, dated February 1.

2) Terradyne LAX,Inc., 2016, *Geotechnical Investigation Report, Proposed Autozone Store 6173, Near Valley Center Road and Mirar De Valle Road, Valley Center, San Diego County, California, Terradyne Project No.: L161055*, dated October 10.

Dear Mr. Bramlitt:

SCST, Inc. (SCST) is pleased to submit this letter updating our recommendations for the proposed biofiltration basin at the subject project. We understand that the original design of the biofiltration basin has changed since our original report.

The new biofiltration basin will base elevation of approximately 1,304 feet above mean sea level (msl) and a maximum ponding depth of approximately 1.306 msl. The biofiltration basin will be surrounded on the north by 2:1 (horizontal:vertical) fill slope less than 5 feet in height, on the south by hardscape surfaces, on the west by the proposed building, and on the east by a retaining wall and a fill slope descending toward Valley Center Road.

UPDATED RECOMMENDATIONS

In order to minimize potential impacts to the adjacent improvements, we recommend that an impermeable barrier be installed on the north side (between the basin and the fill slope), and the west side (between the basin and the proposed building) of the biofiltration basin. The impermeable barrier should extend to a depth of at least 1 foot below the footing depth of the east side of the proposed building.

We appreciate the opportunity to be of service to you on this project. If you have any questions, comments, or require additional information, please call our office at (619) 280-4321.

Respectfully submitted,
SCST, INC.

Douglas A. Skinner, CEG 2472
Senior Geologist

DAS:aw

(1) Addressee via e-mail: mitch.bramlitt@autozone.com
(1) Mr. Brent Moore via e-mail: brent-alidade@att.net

Worksheet C.4-1: Categorization of Infiltration Feasibility Condition

Categorization of Infiltration Feasibility Condition	Worksheet C.4-1

Part 1 - Full Infiltration Feasibility Screening Criteria

Would infiltration of the full design volume be feasible from a physical perspective without any undesirable consequences that cannot be reasonably mitigated?

Note that it is not necessary to investigate each and every criterion in the worksheet if infiltration is precluded. Instead a letter of justification from a geotechnical professional familiar with the local conditions substantiating any geotechnical issues will be required.

Criteria	Screening Question	Yes	No
1	**Is the estimated reliable infiltration rate below proposed facility locations greater than 0.5 inches per hour?** The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	☐	☑

Provide basis:

Results of falling head borehole percolation testing were 0.2 and 0.6-inches per hour. The tested material is believed to be generally representative of the material that will be encountered below the proposed BMP location. The average of the tested infiltration rates do not support allowing infiltration greater than 0.5-inches per hour

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.

2	**Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level?** The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.2.	☐	☑

Provide basis:

The tested infiltration rate at the site does not support allowing infiltration greater than 0.5-inched per hour. Doing so will increase the risk of adverse settlement of the proposed structure to be located adjacent to the planned BMP. Additionally, due to the close proximity of the proposed structure to the proposed BMP location, we recommend the BMP be lined with an impermeable membrane to minimize this risk. We recommend that an impermeable barrier be installed on the north side , and the west side of the biofiltration basin. The impermeable barrier should extend to a depth of at least 1 foot below the footing depth of the east side of the proposed building.SCST does not recommend allowing infiltration greater than 0.5 incher per hour

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.

Worksheet C.4-1 Page 2 of 4			
Criteria	Screening Question	**Yes**	**No**
3	**Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of groundwater contamination (shallow water table, storm water pollutants or other factors) that cannot be mitigated to an acceptable level?** The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☐	☑
Provide basis: Based on our experience with other sites in the site vicinity, historic high groundwater elevations are above the base elevation of the proposed infiltration basin and, if unlined, may increase the risk of groundwater contamination. Allowing infiltration at the site may increase the risk of adverse settlement of the proposed structure to be located adjacent to the planned BMP. SCST does not recommend allowing infiltration greater than 0.5 inches per hour. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.			
4	**Can infiltration greater than 0.5 inches per hour be allowed without causing potential water balance issues such as change of seasonality of ephemeral streams or increased discharge of contaminated groundwater to surface waters?** The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☐	☑
Provide basis: There are no known contaminants occurring at the proposed site, but our tested infiltration rate falls below 0.5 inches per hour so SCST does not recommend allowing infiltration greater than the tested amount. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.			
Part 1 Result*	If all answers to rows 1 - 4 are "**Yes**" a full infiltration design is potentially feasible. The feasibility screening category is **Full Infiltration** If any answer from row 1-4 is "**No**", infiltration may be possible to some extent but would not generally be feasible or desirable to achieve a "full infiltration" design. Proceed to Part 2		

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by County staff to substantiate findings.

Worksheet C.4-1 Page 3 of 4

Part 2 – Partial Infiltration vs. No Infiltration Feasibility Screening Criteria

Would infiltration of water in any appreciable amount be physically feasible without any negative consequences that cannot be reasonably mitigated?

Criteria	Screening Question	Yes	No
5	**Do soil and geologic conditions allow for infiltration in any appreciable rate or volume?** The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	☑	☐

Provide basis:

There are no known contaminants occurring at the proposed site, but our tested infiltration rate falls below 0.5 inches per hour so SCST does not recommend allowing infiltration greater than the tested amount.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

6	**Can Infiltration in any appreciable quantity be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level**? The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.2.	☑	☐

Provide basis:

There tested infiltration rates average 0.4 inches per hour. Boring logs show poorly graded sand at the bottom of our excavation which is believed to be generally representative of the material that will be encountered below proposed BMP locations. The observed infiltration rates support allowing partial infiltration. In order to minimize potential impacts to the adjacent improvements, we recommend that an impermeable barrier be installed on the north side , and the west side of the biofiltration basin. The impermeable barrier should extend to a depth of at least 1 foot below the footing depth of the east side of the proposed building.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

Worksheet C.4-1 Page 4 of 4			
Criteria	Screening Question	**Yes**	**No**
7	**Can Infiltration in any appreciable quantity be allowed without posing significant risk for groundwater related concerns (shallow water table, storm water pollutants or other factors)?** The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☑	☐
Provide basis: Partial infiltration can be allowed at the site provided the recommendations presented in the previous criteria are followed. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.			
8	**Can infiltration be allowed without violating downstream water rights**? The response to this Screening Question must be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☑	☐
Provide basis: This Criteria Should be addressed by the Project Civil Engineer. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.			
Part 2 Result*	If all answers from row 5-8 are yes then partial infiltration design is potentially feasible. The feasibility screening category is **Partial Infiltration.** If any answer from row 5-8 is no, then infiltration of any volume is considered to be **infeasible** within the drainage area. The feasibility screening category is **No Infiltration.**		

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by Agency/Jurisdictions to substantiate findings

SDVOSB . DVBE

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February 9, 2017

SCST No. 170111N
Report No. 1

Mr. Mitch Bramlitt
Autozone Development Corporation
123 South Front Street
Memphis, Tennessee 98103

Subject: INFILTRATION FEASIBILITY STUDY
PROPOSED AUTOZONE
VALLEY CENTER ROAD
VALLEY CENTER, CALIFORNIA

Reference: 1.) Terradyne LAX,Inc., 2016, *Geotehnical Investigation Report, Proposed Autozone Store 6173, Near Valley Center Road and Mirar De Valle Road, Valley Center, San Diego County, California, Terradyne Project No.: L161055*, dated October 10.

Dear Mr. Bramlitt:

SCST, Inc. (SCST) is pleased to submit this report for the infiltration feasibility study performed at the subject project. The project is located at the intersection of Mirar de Valle Road and Valley Center Road in the community of Valley Center, California (Figure 1). We understand the project will consist of the design and construction of an Autozone development. Based on preliminary plans provided to us by the client, we understand that a single BMP location is proposed at the northeast corner of the project site. The purpose of our study was to assess the infiltration characteristics of the subsurface soils in the area of the planned BMP location in accordance with the County of San Diego BMP Design Manual.

SITE DESCRIPTION

The site consists of an empty lot located at the intersection of Mirar de Valle Road and Valley Center Road in the City of Valley Center, San Diego, California (Figure 1). The project site is surrounded by fields, farmland, and rural business developments. The lot is relatively flat, with general sheet flow from south to north and west to east, and bound by Mirar de Valle road to the north and Vallley Center Road to the east. There are no existing improvements. The proposed BMP is located at northeast corner of the lot. Site elevations range from about 1308 feet at the northeast corner to about 1316 feet in the southwest corner of the lot.

PREVIOUS INVESTIGATION

Terradyne LAX, Inc. (Terradyne) performed a geotechnical investigation for the project in September of 2016 (Reference 1). As part of Terradyne's investigation, a total of 8 geotechnical borings were performed at the site. The depths of the borings rang from about 11½ feet to 16½ feet

below existing grade. Two of the borings, borings B-7 and B-9 are located within 50 feet of the proposed BMP (Reference 1).

FIELD EXPLORATION

SCST conducted an infiltration feasibility study at the project site on January 18 and 19, 2017. Our field work consisted of drilling two percolation borings to depths between approximately 4½ feet and 5 feet below the existing ground surface using a 6-inch diameter hand auger. Auger refusal was encountered in one boring at a depth of about 4½ feet. An SCST engineer logged the borings and collected representative soil samples of the materials encountered. Bulk samples were obtained from the borings and transported to our in-house geotechnical laboratory for testing.

Logs of the borings are presented in Appendix I. Soils are classified according to the Unified Soil Classification System (USCS) present on Figure I-1. Figure 2 presents a Subsurface Exploration Map showing the approximate locations of our percolation test borings and the borings drilled by Terradyne in 2016.

SUBSURFACE CONDITIONS

The materials encountered in the percolation borings consist of artificial fill underlain by alluvium. The alluvium consists of moist medium dense clayey sand and poorly graded sand. Groundwater was not encountered in the borings. Additionally, groundwater was not reported in the borings drilled at the aite by Terradyne. However, groundwater levels may fluctuate in the future due to rainfall, irrigation, broken pipes, or changes in site drainage. Because groundwater rise or seepage is difficult to predict, such conditions are typically mitigated if and when they occur.

LABORATORY TESTING

Selected samples obtained from the borings were tested to evaluate pertinent classification and engineering properties and enable development of geotechnical conclusions. Laboratory testing of representative soil samples included sieve analyses. Brief descriptions of the laboratory test procedures and the test results are presented in Appendix II.

PERCOLATION TESTING

In situ borehole percolation tests were performed at two locations at depths of approximately 4½ and 5 feet below the existing ground surface. The test locations were situated so that a minimum of two tests were within 50 feet of the proposed infiltration basin. The testing was performed by an SCST engineer in general accordance with the City of San Diego BMP Design Manual. Table 1 presents the tested infiltration rates. The data and results of the percolation testing are presented in Appendix III.

Table 1: Infiltration Rate Test Results

Test Location	Test Depth (feet)	Material Type at Test Depth (USCS Classification)	Infiltration Rate (inches/hour)
I-1	4½	Poorly Graded Sand (SP)	0.2
I-2	5	Poorly Graded Sand (SP)	0.6

CONCLUSIONS AND RECOMMENDATIONS

Evaluation of storm water infiltration feasibility was performed in general accordance with Appendix I of the County of San Diego BMP Design Manual. Form I-8 from the manual is provided in Appendix IV. In our opinion, alluvial deposits tested during our study are generally representative of the materials that may be encountered between 2 and 15 feet below the existing ground surface.

The tested infiltration rates are greater than 0.1 inch per hour. The tested infiltration rates support partial infiltration based on the County of San Diego definition (greater than 0.01 inches per hour).

We appreciate the opportunity to be of service to you on this project. If you have any questions, comments, or require additional information, please call our office at (619) 280-4321.

Respectfully submitted,
SCST, INC.

REGISTERED PROFESSIONAL GEOLOGIST
DOUGLAS A. SKINNER
No. 2472
CERTIFIED ENGINEERING GEOLOGIST
STATE OF CALIFORNIA

Douglas A. Skinner, CEG 2472
Senior Geologist

Chelsea L Feeney
Staff Engineer

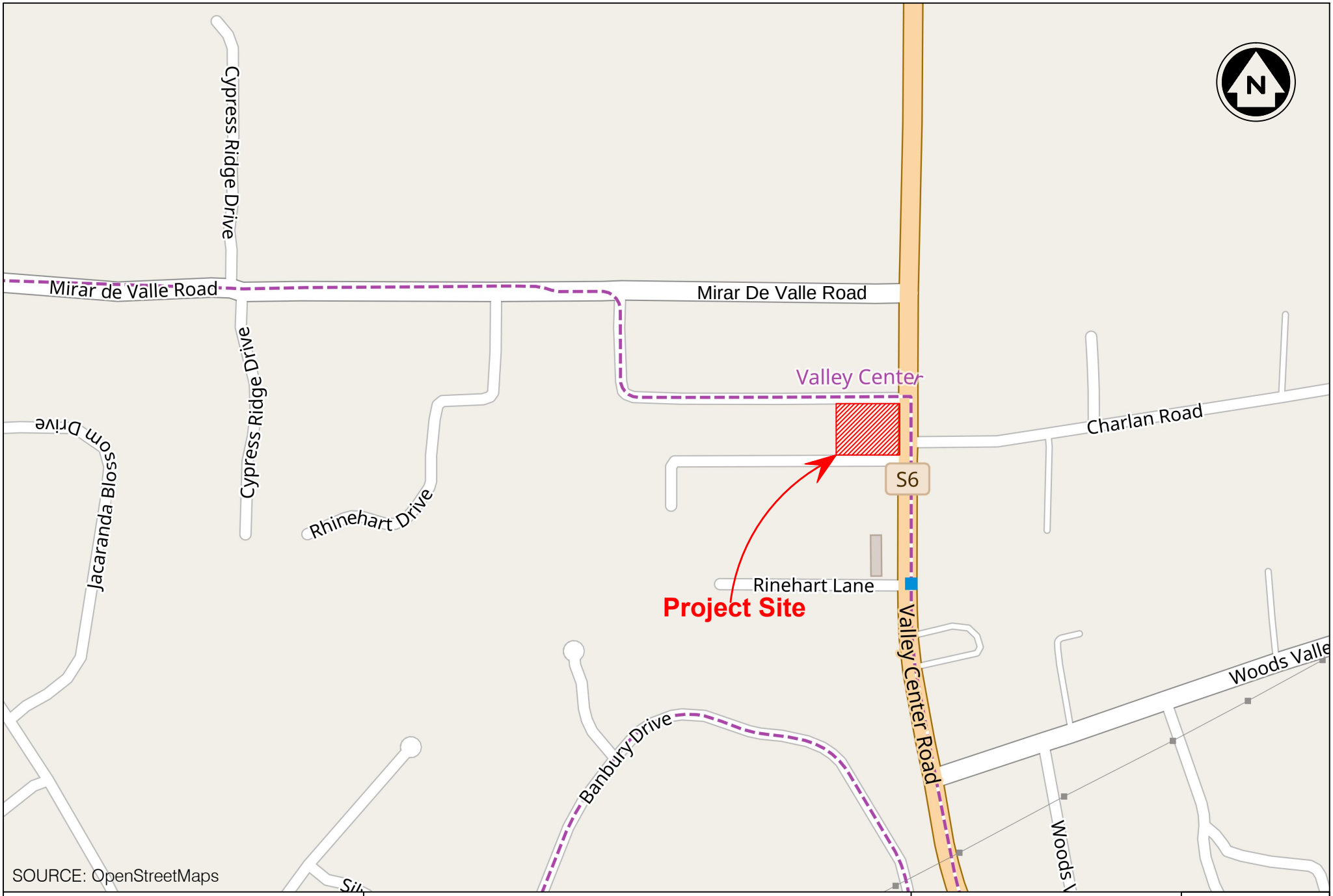
DAS:clf:aw

Attachments:

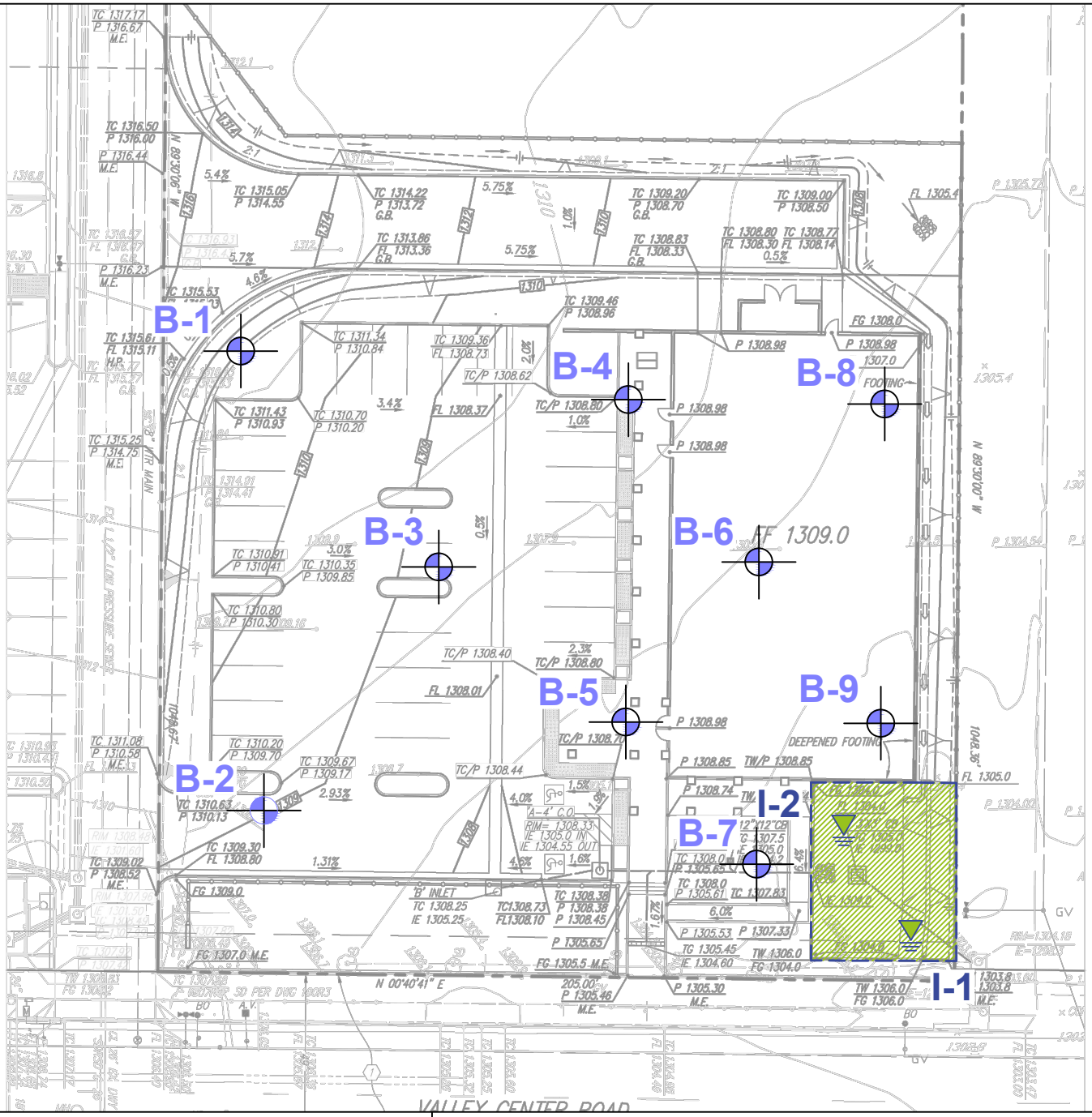
Figures
Figure 1 - Site Vicinity Map
Figure 2 - Subsurface Exploration Map

Appendices
Appendix I - Field Investigation
Appendix II - Laboratory Testing
Appendix III - Infiltration Testing Results
Appendix IV - Form I-8 Categorization of Infiltration Feasibility Condition

(1) Addressee via e-mail: mitch.bramlitt@autozone.com
(1) Mr. Larry Dutton via e-mail: larry-alidade@att.net



SCST, Inc.	SITE VICINITY MAP Proposed Autozone Valley Center Road Valley Center, California	Date: February, 2017 By: MAW Job No.: 170111N-1	Figure: 1



SCST LEGEND:

B-9 — Location of Previous Boring (Terradyne, 2016)

I-2 — Location of Infiltration Test

Location of Infiltration Basin

0 40′ 80′

Scale

NOTE: All Locations are Approximate

SCST, Inc.

SUBSURFACE EXPLORATION MAP
Proposed Autozone
Valley Center Road
Valley Center, California

Date: February, 2017
By: MAW
Job No.: 170111N-1

Figure: 2

APPENDIX I
FIELD INVESTIGATION

Our infiltration feasibility study was conducted on January 18 and 19, 2017 and consisted of drilling two percolation borings to depths of between approximately 4½ feet and 5 feet below the existing surface using a 6-inch diameter hand auger. Auger refusal was encountered in one of the percolation borings at a depth of about 4½ feet. An SCST engineer logged and sampled the materials encountered.

The soils are classified in accordance with the Unified Soil Classification System as illustrated on Figure I-1. The logs of the percolation borings are presented on Figures I-2 and I-3.

SUBSURFACE EXPLORATION LEGEND

UNIFIED SOIL CLASSIFICATION CHART

SOIL DESCRIPTION		GROUP SYMBOL	TYPICAL NAMES
I. COARSE GRAINED, more than 50% of material is larger than No. 200 sieve size.			
GRAVELS More than half of coarse fraction is larger than No. 4 sieve size but smaller than 3".	CLEAN GRAVELS	GW	Well graded gravels, gravel-sand mixtures, little or no fines
		GP	Poorly graded gravels, gravel sand mixtures, little or no fines.
	GRAVELS WITH FINES (Appreciable amount of fines)	GM	Silty gravels, poorly graded gravel-sand-silt mixtures.
		GC	Clayey gravels, poorly graded gravel-sand, clay mixtures.
SANDS More than half of coarse fraction is smaller than No. 4 sieve size.	CLEAN SANDS	SW	Well graded sand, gravelly sands, little or no fines.
		SP	Poorly graded sands, gravelly sands, little or no fines.
		SM	Silty sands, poorly graded sand and silty mixtures.
		SC	Clayey sands, poorly graded sand and clay mixtures.
II. FINE GRAINED, more than 50% of material is smaller than No. 200 sieve size.			
	SILTS AND CLAYS (Liquid Limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, sandy silt or clayey-silt-sand mixtures with slight plasticity.
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
		OL	Organic silts and organic silty clays or low plasticity.
	SILTS AND CLAYS (Liquid Limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.
		CH	Inorganic clays of high plasticity, fat clays.
		OH	Organic clays of medium to high plasticity.
III. HIGHLY ORGANIC SOILS		PT	Peat and other highly organic soils.

SAMPLE SYMBOLS

- ☒ - Bulk Sample
- CAL - Modified California sampler
- CK - Undisturbed Chunk sample
- MS - Maximum Size of Particle
- ST - Shelby Tube
- SPT - Standard Penetration Test sampler

GROUNDWATER SYMBOLS

- ▽ - Water level at time of excavation or as indicated
- ⁂ - Water seepage at time of excavation or as indicated

LABORATORY TEST SYMBOLS

- AL - Atterberg Limits
- CON - Consolidation
- COR - Corrosivity Tests (Resistivity, pH, Chloride, Sulfate)
- DS - Direct Shear
- EI - Expansion Index
- MAX - Maximum Density
- RV - R-Value
- SA - Sieve Analysis
- UC - Unconfined Compression

SCST, Inc.

Autozone Valley Center Infiltration
Valley Center, California

By:	CLF	Date:	February, 2017
Job Number:	170111N-1	Figure:	I-1

LOG OF BORING I-1

Date Drilled: 1/19/2017 | Logged by: JD

Equipment: 6-inch Hand Auger | Project Manager: DAS

Elevation (ft): 1307 | Groundwater Depth (ft): Not Encountered

DEPTH (ft)	USCS	SUMMARY OF SUBSURFACE CONDITIONS	SAMPLES: DRIVEN	SAMPLES: BULK	MOISTURE CONTENT (%)	DRY UNIT WEIGHT (pcf)	LABORATORY TESTS
1	SM	**FILL (Qf):** SILTY SAND, dark brown, moist, organics observed.		X			
2	SC	**ALLUVIUM (Qal):** CLAYEY SAND, very dark gray, moist.		X			SA
3				X			
4	SP	POORLY GRADED SAND, orange brown, moist.		X			
5		**AUGER REFUSAL AT 4.5 FEET.**					
6							
7							
8							
9							
10							

SCST, Inc.

Autozone Valley Center Infiltration

Valley Center, California

By:	CLF	Date:	February, 2017
Job Number:	170111N-1	Figure:	I-2

LOG OF BORING I-2

Date Drilled: 1/19/2017 | Logged by: JD

Equipment: 6-inch Hand Auger | Project Manager: DAS

Elevation (ft): 1308 | Groundwater Depth (ft): Not Encountered

DEPTH (ft)	USCS	SUMMARY OF SUBSURFACE CONDITIONS	SAMPLES: DRIVEN	SAMPLES: BULK	MOISTURE CONTENT (%)	DRY UNIT WEIGHT (pcf)	LABORATORY TESTS
0	SM	**FILL (Qf):** SILTY SAND, dark brown, moist, organic.		X			
1	SC	**ALLUVIUM (Qal):** CLAYEY SAND, black, moist.		X			
2				X			SA
3		Orange brown.		X			
4	SP	POORLY GRADED SAND, yellowish brown, moist, small amount of clay.		X			
5		**BORING TERMINATED AT 5 FEET.**					
6							
7							
8							
9							
10							

SCST, Inc.

AutoZone Valley Center Infiltration

Valley Center, California

By:	CLF	Date:	February, 2017
Job Number:	170111N-1	Figure:	I-3

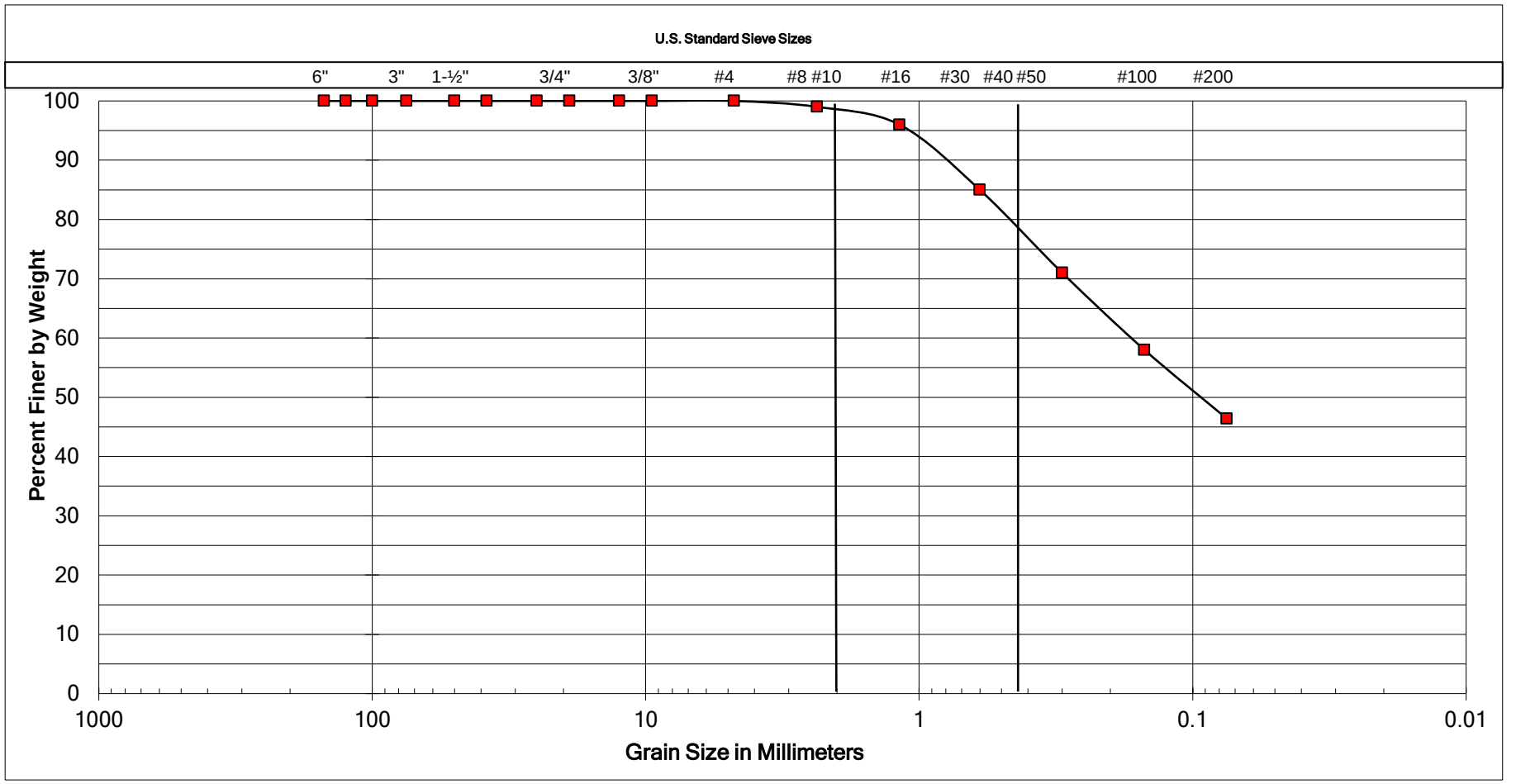
APPENDIX II
LABORATORY TESTING

Laboratory tests were performed to provide geotechnical parameters for engineering analyses. The following tests were performed:

- **CLASSIFICATION:** Field classifications were verified in the laboratory by visual examination. The final soil classifications are in accordance with the Unified Soil Classification System.

- **GRAIN SIZE DISTRIBUTION:** The grain size distribution was determined on one sample in accordance with ASTM D422. Figure II-1 present the test results.

Soil samples not tested are now stored in our laboratory for future reference and analysis, if needed. Unless notified to the contrary, all samples will be disposed of 30 days from the date of this report.



Cobbles	Gravel		Sand			Silt or Clay
	Coarse	Fine	Coarse	Medium	Fine	

SAMPLE LOCATION
B1: 0' to 4.5'

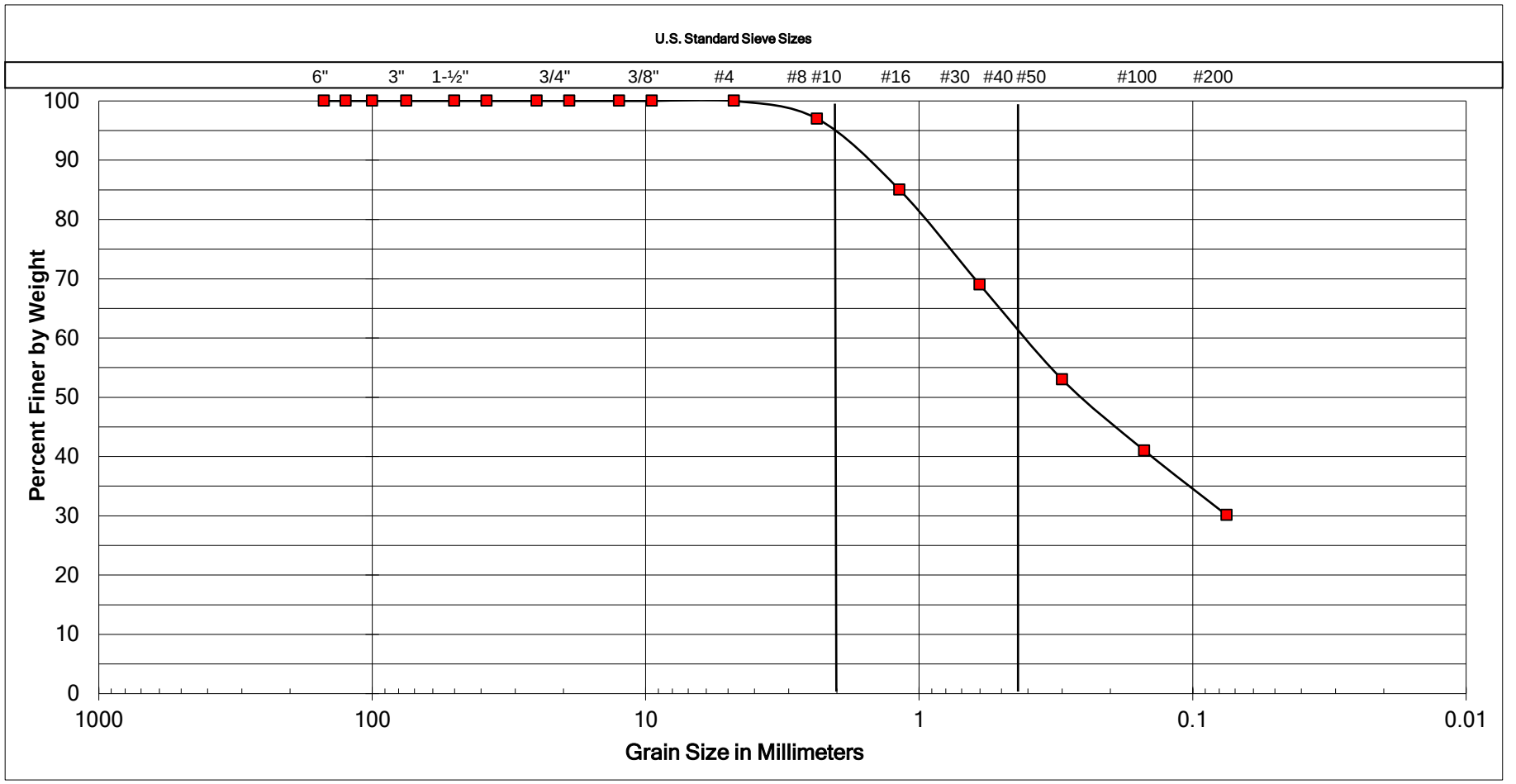
UNIFIED SOIL CLASSIFICATION:	SC
DESCRIPTION	CLAYEY SAND

ATTERBERG LIMITS	
LIQUID LIMIT	--
PLASTIC LIMIT	--
PLASTICITY INDEX	--

SCST, Inc.

Autozone Valley Center Infiltration
Valley Center, California

By:	CLF	Date:	February, 2017
Job Number:	170111N-1	Figure:	II-1



Cobbles	Gravel		Sand			Silt or Clay
	Coarse	Fine	Coarse	Medium	Fine	

SAMPLE LOCATION
B2: 0' to 5'

UNIFIED SOIL CLASSIFICATION:	SC
DESCRIPTION	CLAYEY SAND

ATTERBERG LIMITS	
LIQUID LIMIT	
PLASTIC LIMIT	
PLASTICITY INDEX	

SCST, Inc.

Autozone Valley Center Infiltration
Valley Center, California

By:	CLF	Date:	February, 2017
Job Number:	170111N-1	Figure:	II-2

APPENDIX III
INFILTRATION RATE TEST RESULTS

We performed falling head borehole infiltration tests at two locations (I-1 and I-2) in general conformance with Appendix C of the BMP Design Manual for the San Diego Region. The percolation test holes were prepared by placing about 2 inches of gravel in the bottom of the test hole and installing a 4-inch diameter perforated PVC pipe from the top of the pea gravel to the ground surface or higher. Prior to starting the percolation tests, the holes were filled with water to presoak the soil materials. The percolation tests were performed approximately 24 hours after presoaking by placing clean potable water in the PVC pipe and measuring the drop in the water level. Figures III-1 and III-2 present the results of the testing.

Report of Borehole Percolation Testing

Storm Water Infiltration

Project Name:	Valley Center Infiltration Testing	Test Number:	**I-1**
Job Number:	170111N	Tested By:	JRD
Date Drilled:	1/18/2017	Date Tested:	1/19/2017
Drilling Method:	Hand Tools	Presoak Time:	24 hours
Drilled Depth (feet):	4.5		
Test Hole Diameter (inches):	6		
Gravel Pack:	Yes		
Pipe Diameter (inches):	4		

Trial No.	Time	Time Interval, ΔT (min)	Initial Water Height, H_o (ft)	Final Water Height, H_f (ft)	Change in Water Height, ΔH (in)	Percolation Rate (min/in)
1	14:30 15:00	0:30	0.52	0.44	1.0	30
2	15:00 15:30	0:30	0.52	0.4375	1.0	30
3	15:30 16:00	0:30	0.52	0.43	1.1	27
4	16:00 16:30	0:30	0.5	0.4	0.9	34
5	16:30 17:00	0:30	0.52	0.34	2.1	14
6	17:00 17:30	0:30	0.52	0.48	0.5	60
7	17:30 18:00	0:30	0.52	0.42	1.3	24
8	18:00 18:30	0:30	0.52	0.43	1.1	27
			Observed Percolation Rate:			60 min/in 1.0 in/hr
			Gravel Correction Factor:			1.50
			Corrected Percolation Rate:			90 min/in 0.7 in/hr
			***Tested Infiltation Rate, I_t:**			**0.2 in/hr**

*Tested infiltration rate using the Porchet Method:

$$I_t = \frac{\Delta H(60r)}{\Delta T(r + 2H_{avg})}$$

ΔH = Change in water head height over the time interval [in] = 0.5

r = Test hole radius [in] = 3

ΔT = Time interval [min] = 30

H_{avg} = Average water height over time interval = $12(H_o + H_f)/2$ [in] = 5.7

SCST, Inc.	Autozone Valley Center Infiltration Valley Center, California		
	By: JRD	Date:	February, 2017
	Job No: 170111N-1	Figure:	III-1

Report of Borehole Percolation Testing

Storm Water Infiltration

Project Name:	Valley Center Infiltration Testing	Test Number:	**I-2**
Job Number:	170111N	Tested By:	JRD
Date Drilled:	1/18/2017	Date Tested:	1/19/2017
Drilling Method:	Hand Tools	Presoak Time:	24 hours
Drilled Depth (feet):	5.0		
Test Hole Diameter (inches):	6		
Gravel Pack:	Yes		
Pipe Diameter (inches):	4		

Trial No.	Time	Time Interval, ΔT (min)	Initial Water Height, H_o (ft)	Final Water Height, H_f (ft)	Change in Water Height, ΔH (in)	Percolation Rate (min/in)
1	14:30 15:00	0:30	1.08	0.84	2.9	10
2	15:00 15:30	0:30	1.08	0.93	1.9	16
3	15:30 16:00	0:30	1.08	0.86	2.6	11
4	16:00 16:30	0:30	1.1	0.9	2.5	12
5	16:30 17:00	0:30	1.08	0.90	2.3	13
6	17:00 17:30	0:30	1.08	0.85	2.8	11
7	17:30 18:00	0:30	1.08	0.94	1.8	17
8	18:00 18:30	0:30	1.08	0.92	2.0	15
				Observed Percolation Rate:		11 min/in 5.5 in/hr
				Gravel Correction Factor:		1.50
				Corrected Percolation Rate:		16 min/in 3.7 in/hr
				***Tested Infiltation Rate, I_t:**		**0.6 in/hr**

*Tested infiltration rate using the Porchet Method:

$$I_t = \frac{\Delta H(60r)}{\Delta T(r + 2H_{avg})}$$

ΔH = Change in water head height over the time interval [in] = 2.8

r = Test hole radius [in] = 3

ΔT = Time interval [min] = 30

H_{avg} = Average water height over time interval = $12(H_o + H_f)/2$ [in] = 11.8

APPENDIX IV
FORM I-8 CATEGORIZATION OF INFILTRATION FEASIBILITY CONDITION

Categorization of Infiltration Feasibility Condition		Form I-8	
Part 1 - Full Infiltration Feasibility Screening Criteria Would infiltration of the full design volume be feasible from a physical perspective without any undesirable consequences that cannot be reasonably mitigated?			
Criteria	Screening Question	Yes	No
1	Is the estimated reliable infiltration rate below proposed facility locations greater than 0.5 inches per hour? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	☐	☒
Provide basis: Results of falling head borehole percolation testing were 0.2 and 0.6-inches per hour. The tested material is believed to be generally representative of the material that will be encountered below the proposed BMP location. The average of the tested infiltration rates do not support allowing infiltration greater than 0.5-inches per hour. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide			
2	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2.	☐	☒
Provide basis: The tested infiltration rate at the site does not support allowing infiltration greater than 0.5-inched per hour. Doing so will increase the risk of adverse settlement of the proposed structure to be located adjacent to the planned BMP. Additionally, due to the close proximity of the proposed structure to the proposed BMP location, we recommend the BMP be lined with an impermeable membrane to minimize this risk. SCST does not recommend allowing infiltration greater than 0.5 incher per hour. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.			

Form I-8 Page 2 of 4			
Criteria	Screening Question	Yes	No
3	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of groundwater contamination (shallow water table, storm water pollutants or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☐	☒
Provide basis: Based on our experience with other sites in the site vicinity, historic high groundwater elevations are above the base elevation of the proposed infiltration basin and, if unlined, may increase the risk of groundwater contamination. Allowing infiltration at the site may increase the risk of adverse settlement of the proposed structure to be located adjacent to the planned BMP. SCST does not recommend allowing infiltration greater than 0.5 inches per hour.			
4	Can infiltration greater than 0.5 inches per hour be allowed without causing potential water balance issues such as change of seasonality of ephemeral streams or increased discharge of contaminated groundwater to surface waters? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☐	☒
Provide basis: There are no known contaminants occurring at the proposed site, but our tested infiltration rate falls below 0.5 inches per hour so SCST does not recommend allowing infiltration greater than the tested amount. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.			
Part 1 Result*	If all answers to rows 1 - 4 are "Yes" a full infiltration design is potentially feasible. The feasibility screening category is Full Infiltration If any answer from row 1-4 is "No", infiltration may be possible to some extent but would not generally be feasible or desirable to achieve a "full infiltration" design. Proceed to Part 2		

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by the City Engineer to substantiate findings

Form I-8 Page 3 of 4			
Part 2 – Partial Infiltration vs. No Infiltration Feasibility Screening Criteria Would infiltration of water in any appreciable amount be physically feasible without any negative consequences that cannot be reasonably mitigated?			
Criteria	Screening Question	Yes	No
5	Do soil and geologic conditions allow for infiltration in any appreciable rate or volume? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	☒	☐
Provide basis: There tested infiltration rates average 0.4 inches per hour. Boring logs show poorly graded sand at the bottom of our excavation which is believed to be generally representative of the material that will be encountered below proposed BMP locations. The tested infiltration rated support allowing partial infiltration design based on the City if San Diego's definition of partial infiltration being between 0.01 and 0.5 inches per hour. However, due to the close proximity of the proposed structure to the proposed BMP location, we recommend the BMP be lined with an impermeable membrane to minimize the risk of adverse settlement to the proposed structure.			
6	Can Infiltration in any appreciable quantity be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2.	☐	☒
Provide basis: See responses to Criteria 2, 3, and 5. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.			

Form I-8 Page 4 of 4			
Criteria	Screening Question	Yes	No
7	Can Infiltration in any appreciable quantity be allowed without posing significant risk for groundwater related concerns (shallow water table, storm water pollutants or other factors)? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☒	☐
Provide basis: See response to Criteria 3. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low			
8	Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	☒	☐
Provide basis: This Criteria Should be addressed by the Project Civil Engineer. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.			
Part 2 Result*	If all answers from row 1-4 are yes then partial infiltration design is potentially feasible. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be infeasible within the drainage area. The feasibility screening category is No Infiltration.		

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by the City Engineer to substantiate findings