

**DRAINAGE STUDY  
FOR  
RANCHO GUEJITO TASTING FACILITY AND  
EVENT CENTER**

**Job Number 14557**

**December 3, 2019  
Revised: October 20, 2020**

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**DECLARATION OF RESPONSIBLE CHARGE**

I hereby declare that I am the engineer of work for drainage report for this project, that I have exercised responsible charge over this drainage report as defined in Section 6703 of the Business and Professionals Code, and opine that the drainage report is consistent with current standards.

I understand that the check of the drainage report by the County of San Diego is confined to a review only and does not relieve me, as the engineer of work, of any responsibilities for the drainage report.

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**Jayne Janda-Timba, PE**  
**RCE # 70649, Exp. 06/21**

# 1.0 INTRODUCTION

## 1.1 Project Description

This drainage study presents the hydrologic and hydraulic (H&H) analyses for the development of the Rancho Guejito Tasting Facility and Event Center project (herein referred to as “the project”) and accompanying site improvements. The project is located in the unincorporated area of San Diego County, at 17224 San Pasqual Valley Road (California Highway 78, CA-78), approximately 7.5 miles east of the City of Escondido, California. A vicinity map of the project location is provided in Figure 1-1. This study analyzes the hydrologic characteristics of the project site and contributing drainage areas for two separate levels of detail: (1) a watershed-scale analysis for Guejito Creek (also named Rockwood Canyon Creek), which runs along the western portion of the project site, and (2) a project local watershed analysis to determine the impacts associated with the proposed development. The results from the Guejito Creek Watershed hydrologic analyses were utilized in the hydraulic analysis of Guejito Creek.

## 1.2 Drainage Characteristics

### *Guejito Creek Watershed*

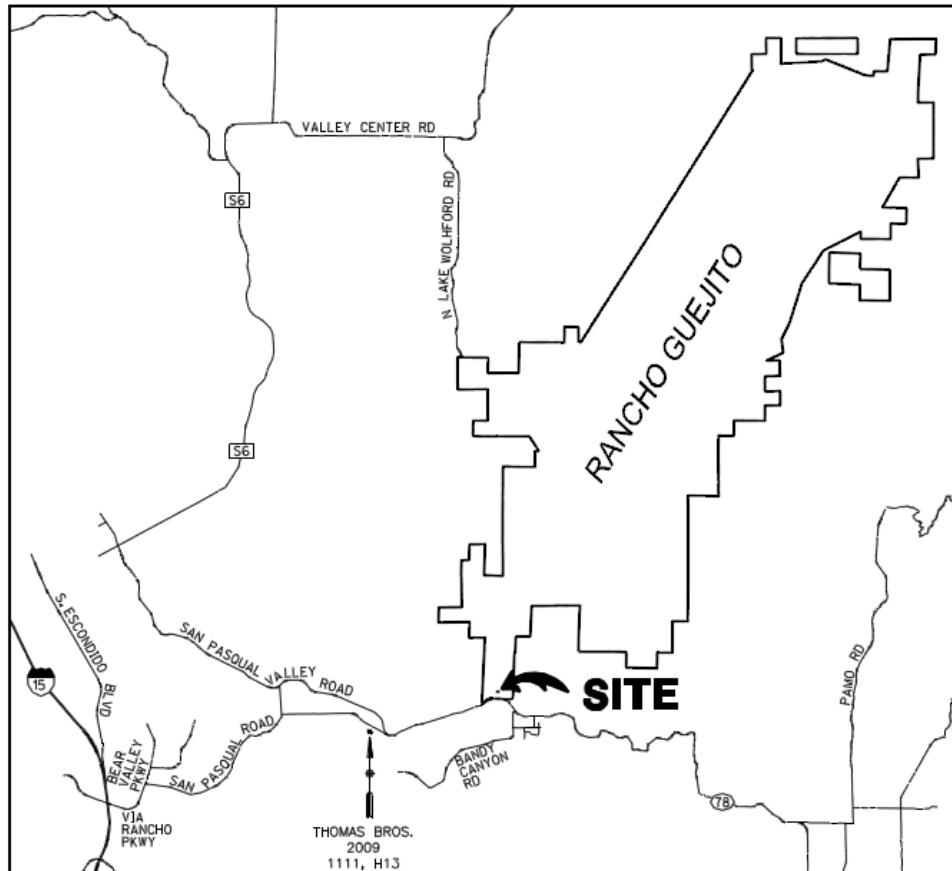
The Guejito Creek watershed covers approximately 27.7 square miles, from Rodriguez Mountain at the northern extent of the watershed to the confluence with Santa Ysabel Creek, south of CA-78. Drainage flows generally from northeast to southwest through largely natural and agricultural areas. The watershed is bordered by steep hillsides to the north, east, and west. Drainage follows relatively well-defined flowpaths, especially in the southern portions of the watershed. Several watercourses confluence upstream of the existing vineyards, and are eventually all collected within Guejito Creek, which runs along the western edge of the project site. The overall longitudinal slope for the watershed is relatively mild, averaging approximately 2% along the 10-mile length of the flowpath within the valley. Guejito Creek passes beneath a bridge structure along CA-78, and then continues southerly for approximately 1,500 feet prior to its confluence with Santa Ysabel Creek. Soil within the watershed consists of a combination of Hydrologic Soil Groups (HSG) A, B, C and D soils (see Appendix A).

### *Project Local Drainage Basin*

The project local drainage basin for this study is the area upstream of the project site that is not collected within Guejito Creek prior to CA-78. The drainage basin contributes to the same discharge location in both existing and post-project conditions. The local drainage basin covers approximately 264 acres, extending from the ridge of the mountains located directly east of the project to a ridge in the agricultural fields area located west of the project. The drainage area consists largely of agricultural areas with few developments, including single-family residences and additional accessory structures. Concentrated drainage flows from the adjacent hillsides onto the existing vineyards, where the flow transitions into a sheet flow condition. Runoff then sheet

flows southerly through the existing vineyards at a mild slope (approximately 0.6%) prior to being collected within an existing depression that is located adjacent to the existing site access driveway along the north side of CA-78. From this collection point, an existing culvert conveys runoff beneath CA-78 and into an existing drainage ditch along the southern roadway shoulder. Runoff in the ditch then flows westerly and confluences with Guejito Creek.

**Figure 1-1: Vicinity Map**



**VICINITY MAP**  
NOT TO SCALE

### **1.3 Flood Hazard Mapping**

The project area is not located within a Federal Emergency Management Agency (FEMA) mapped Special Flood Hazard Area (SFHA). Two creeks run within the vicinity of the project area, Guejito Creek to the west and Santa Ysabel Creek to the south. Santa Ysabel Creek is FEMA mapped Zone AE; however, the 0.2 percent annual chance flood (500-year flood) for Santa Ysabel Creek does not reach the project area. Guejito Creek is not FEMA mapped within the vicinity of project area. The County of San Diego floodplain mapping does not differ from FEMA in this area.

The project is within a Dam Inundation Zone flood hazard from the Lake Sutherland dam approximately 13 miles upstream. In the event that this dam had a major, rapid dam failure while full, the project area would be inundated with floodwater from Santa Ysabel Creek to depth of several feet.

## 2.0 HYDROLOGIC ANALYSES

The proposed development is located within the overall Guejito Creek watershed. Two hydrologic analyses were performed: one for the overall Guejito Creek watershed and one for the project local drainage basin. The hydrologic analysis performed for the drainage to Guejito Creek, upstream of the confluence with Santa Ysabel Creek determined anticipated peak flow rates within the creek and subsequently, through hydraulic analyses, determine water surface elevations expected during the 100-year storm event. The project local basin analysis was performed so that a comparison between the existing and proposed conditions flow rates could be made in order to assess the potential increase in peak flow rates resulting from the proposed project.

### 2.1 Hydrologic Analysis Methodology – Guejito Creek Watershed

The watershed to Guejito Creek is greater than one square mile, and thus in accordance with the June 2003 *San Diego County Hydrology Manual* (Hydrology Manual) (San Diego County, 2003), the Natural Resources Conservation Service (NRCS) hydrologic method was used to determine existing condition peak flow rates for the 100-year frequency storm event.

The current Hydrology Manual requires that 24-hour “nested storm” with the 2/3, 1/3 distributions supersede the old Type B rainfall distribution (The rainfall distribution adopted the nested storm pattern, based on the United States Army Corps of Engineers (USACE), Hydrologic Engineering Center (HEC) Training Document Number 15). The NRCS’s rainfall distribution is based on the Rational Method’s 6-hour intensity-duration relationship and as a result, the NRCS method now closely mimics the results of the Rational Method. There can be significant differences in the peak flow rate calculated by the “Type B” and “nested storm” rainfall distributions, and in some cases the “nested storm” rainfall distribution may result in over three times the calculated peak flow rate in comparison to the “Type B” rainfall distribution calculations for the same watershed. Therefore, it is appropriate to use the NRCS Hydrologic method to determine the peak flow rates.

The US Army Corps of Engineers HEC-1 computer program was used to determine the 100-year peak discharge flow rates. The HEC-1 program allows the engineer to simulate both natural and improved or developed watersheds. Program parameter include basin area, lag time, rainfall distribution, and NRCS curve number (related to infiltration rates). These parameters were determined from soil and vegetative cover/land use maps, and topographic data. A watershed soil group map exhibit is provided in Appendix A. Runoff was estimated using NRCS curve numbers for the watershed. Estimated curve numbers are a function of land use or vegetative cover and soil type. The land uses were based on the County of San Diego land use (SANGIS), aerial imagery, and applicable data gathered during previous visits to the project site. Consistent with the Hydrology Manual guidance, a composite curve number was determined for the watershed

using an area-weighted average of the individual land use areas and corresponding curve numbers. A summary of the watershed parameters used in analysis are provided in Table 2-1. A watershed map is provided in Map Pocket 1.

**Table 2-1: NRCS Method Key Hydrologic Parameters Summary**

| Hydrologic Parameter                                                                                             | Value                                                             |
|------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------|
| Basin Area                                                                                                       | 27.7 square miles (17,700 acres)                                  |
| Topography                                                                                                       | Min. 400 feet, Max. 1,800 feet (NAVD88)                           |
| 100-year Design Storm Precipitation                                                                              | 6-hour, P=3.6 – 3.9 inches; 24-hour, P=6 - 9 inches               |
| Unadjusted Precipitation Zone Number (PZN)<br>(Figure C-1)                                                       | 2.5 (Antecedent Moisture Condition = II)                          |
| Natural Open Channel Roughness<br>(Per <i>San Diego County Hydraulic Design Manual</i> )(San Diego County, 2014) | Mountain Streams: Bottom, Cobbles with Large Boulders., n = 0.060 |

Additional HEC-1 input parameters were calculated. Rainfall rates and distributions were developed using the criteria, as outlined in the Hydrology Manual. Lag time for the watershed was calculated using criteria presented in the Hydrology Manual. The PZN adjustment was performed using the methodologies outlined in the Hydrology Manual. Additional information is provided on Worksheet 4-3, contained in Appendix B, related to the calculation of the input parameters. The HEC-1 model output results are also provided in Appendix B.

The NRCS synthetic unit hydrograph with curvilinear transformation was used to develop runoff hydrographs for the watershed. This unit hydrograph is dimensionless and is a function of the watershed area and lag time.

## 2.2 Hydrologic Analysis Results – Guejito Creek Watershed

The overall 27.7 square-mile Guejito Creek watershed was delineated into four (4) basins for the hydrologic analysis. Please refer to the Drainage Study Map for the Guejito Creek watershed provided in Map Pocket 1. The improvements associated with this project are located within Basin 400. It should be noted that although the project will increase the amount of impervious area on-site, the increase peak discharge for the Guejito Creek watershed will be negligible because of the relative size to the overall watershed and the vastly different time to peak for the proposed improvements compared to the entire watershed. Thus, the existing and proposed conditions are assumed equivalent for the Guejito Creek watershed analysis. Increases in peak flow rates associated with the proposed development are calculated in the section for the local drainage analysis (Section 2.3). The peak flow rate from the Guejito Creek watershed was calculated to be 16,000 cubic feet per second. The summary of input parameters for the Guejito

Creek watershed hydrologic analysis is provided in Table 2-2. Please refer to Appendix B for the HEC-1 inputs and output.

**Table 2-2: Summary of HEC-1 Parameters**

| <b>Basin</b> | <b>Area<br/>(square<br/>miles)</b> | <b>Watershed<br/>Length<br/>(miles)</b> | <b>NRCS<br/>Lag<br/>(hours)</b> | <b>Weighted<br/>CN<sup>1</sup></b> | <b>Basin n<sup>2</sup></b> |
|--------------|------------------------------------|-----------------------------------------|---------------------------------|------------------------------------|----------------------------|
| 100          | 22.5                               | 10.74                                   | 1.956                           | 93                                 | 0.06                       |
| 200          | 3.03                               | 3.36                                    | 0.683                           | 92                                 | 0.06                       |
| 300          | 1.06                               | 2.47                                    | 0.517                           | 92                                 | 0.06                       |
| 400          | 1.10                               | 1.45                                    | 0.639                           | 92                                 | 0.06                       |

Note 1: The PZN for the project site is 2.5. The PZN adjustment factor for 100-year event is 3. Therefore CN<sub>3</sub> values used.

Note 2: Basin n value from Table A-5 of the Hydraulic Design Manual for “Mountain Streams: Bottom, Cobbles with Large Boulders.”

## **2.3 Hydrologic Analysis Methodology – Project Local Watershed**

To determine the peak flow rates expected to reach the project’s point of collection (depression location along the north side of CA-78), the Rational Method was used. In accordance with the Hydrology Manual, the Rational Method should be used for watersheds less than one (1) square mile (local drainage basin is 264 acres). To perform the rational method calculations, Advance Engineering Software (AES) Rational Method computer software was used. The hydrologic model was developed by first dividing the local drainage basin into several subareas. The delineation of each subarea was determined so that area within each subarea is comprised of similar hydrologic features, including topography, land use, and storm drain conveyance system (e.g., urban open channel, pipe, natural open channel, etc.). Nodes were identified at the upstream and downstream extents of each subarea, and subarea hydrologic data was determined, such as the land use(s) and drainage facility geometry, elevations, and lengths. Hydrologic backup information is included in Appendix C. Next, the hydrologic data describing each subarea were incorporated into the AES software in order to create a node-link model for each watershed. For each subarea the AES software performs calculations for the specific hydrologic process occurring in the subarea. There are 15 different hydrologic processes programed into the software, and each process is assigned a code number that is presented on the model results. The AES Rational Method computer software hydrologic processes code numbers are described in Table 2-3.



**Table 2-3: Summary of AES Processes**

| Code    | Subarea Type                                          |
|---------|-------------------------------------------------------|
| Code 1  | Confluence analysis at a node                         |
| Code 2  | Initial subarea analysis                              |
| Code 3  | Pipe flow travel time (computer-estimated pipe sizes) |
| Code 4  | Pipe flow travel time (user-specified pipe size)      |
| Code 5  | Trapezoidal channel travel time                       |
| Code 6  | Street flow analysis through a subarea                |
| Code 7  | User-specified information at a node                  |
| Code 8  | Addition of the subarea runoff to mainline            |
| Code 9  | V-Gutter flow through subarea                         |
| Code 10 | Copy mainstream data onto a memory bank               |
| Code 11 | Confluence a memory bank with the mainstream memory   |
| Code 12 | Clear a memory bank                                   |
| Code 13 | Clear the mainstream memory                           |
| Code 14 | Copy a memory bank onto the mainstream memory         |
| Code 15 | Hydrologic data bank storage functions                |

Key hydrologic parameters utilized in the analysis in accordance with the County of San Diego's design criteria are summarized in Table 2-4.

**Table 2-4. Rational Method Key Hydrologic Parameters Summary**

| Hydrologic Parameter                                                                                                  | Value                                                                                                                            |
|-----------------------------------------------------------------------------------------------------------------------|----------------------------------------------------------------------------------------------------------------------------------|
| Design Storm Precipitation <sup>1</sup>                                                                               | 100-year, 6-hour, P=3.5 inches                                                                                                   |
| Runoff Coefficients <sup>2</sup><br>(Per 2003 County Hydrology Manual)<br>(San Diego County, 2003)                    | Soil A, Open Space = 0.20<br>Soil B, Open Space = 0.25<br>Soil C, Open Space = 0.30                                              |
| Natural Open Channel Roughness <sup>2</sup><br>(Per San Diego County Hydraulic Design Manual)(San Diego County, 2014) | Vineyards, n = 0.040<br><i>Sheet flow through vineyards modeled as Mature Row Crops per Table A-5 of Hydraulic Design Manual</i> |
| Rainfall Intensity <sup>2</sup>                                                                                       | Per Section 3.0 of the County Hydrology Manual<br>(San Diego County, 2003)                                                       |

Note 1: Isopluvial maps provided in Appendix C.

Note 2: Additional hydrologic parameters are shown on the AES software output data (Rational Method Analyses) provided in Appendix C.

## 2.4 Hydrologic Analysis Results – Project Local Watershed

The local drainage basin is defined in this study as the upstream areas draining to the depression along the north side of CA-78, adjacent to the existing access driveway where storm water runoff leaves the site through an existing culvert. In order to appropriately compare the pre-project to post-project conditions, the analysis focuses on the local drainage area and does not consider the runoff from Guejito Creek. All runoff in pre-project and post-project conditions is routed to the local depression and culvert, allowing for a direct comparison between pre-project and post-project peak flow rates. The drainage basin was delineated into four (4) basins and several subbasins for the hydrologic analysis, with all improvements (impervious areas) located within the subbasins associated with Nodes 203 to 110 and Nodes 104 to 110. Therefore, the other subbasins do not change from pre-project to post-project conditions. Please refer to the Drainage Study Maps for the pre-project and post-project conditions which are located in Map Pockets 2 and 3, respectively. In order to appropriately incorporate the increases in impervious cover into modeling, the runoff coefficient for the subbasins with impervious cover, for both the pre-project and post-project conditions, were calculated following the formula from Section 3.1.2 of the Hydrology Manual. The results are summarized below in Table 2-5.

**Table 2-5: Calculated Runoff Coefficient 'C'**

| Nodes                         | Impervious Area (ac) | Total Subbasin Area (ac) | Percent Impervious | Subbasin Natural 'C' | Calculated 'C' <sup>2</sup> |
|-------------------------------|----------------------|--------------------------|--------------------|----------------------|-----------------------------|
| <b>Pre-Project Condition</b>  |                      |                          |                    |                      |                             |
| 104 to 110                    | 0.11                 | 17.0                     | 0.6%               | 0.225                | 0.229                       |
| 203 to 110                    | 0.37                 | 6.0                      | 6.1%               | 0.205                | 0.247                       |
| 402 to 110                    | 0.37                 | 5.7                      | 6.5%               | 0.218                | 0.262                       |
| <b>Post-Project Condition</b> |                      |                          |                    |                      |                             |
| 104 to 105                    | 0.10                 | 5.7                      | 1.8%               | 0.250                | 0.262                       |
| 105 to 106                    | 0.30                 | 2.9                      | 10.4%              | 0.250                | 0.318                       |
| 106 to 107                    | 0                    | 2.3                      | 0.0%               | 0.250                | 0.250                       |
| 107 to 110                    | 0.81                 | 2.9                      | 28.1%              | 0.200                | 0.397                       |
| 203 to 110                    | 1.48                 | 9.3                      | 15.9%              | 0.205                | 0.315                       |
| 402 to 110                    | 0.37                 | 5.7                      | 6.46%              | 0.218                | 0.262                       |

<sup>1</sup> Please refer to Soils Group Map in Appendix A

<sup>2</sup> Runoff coefficient is calculated by  $C = 0.90 \times (\% \text{ Imp}) + C_p \times (1 - \% \text{ Imp})$ ,  $C_p$  = Natural 'C'

The summary of the 100-year, peak flow rates resulting from the rational method analysis at key nodes, included the project area subbasin, is provided in Table 2-6. Please refer to Appendix C for the AES output file of the rational method results.

**Table 2-6: Summary of Local Drainage Basin Rational Method Calculations**

| Condition    | Modified Rational Method Node | Area (acres) | Time of Concentration (Minutes) | Intensity (inches/hour) | Peak Flow Velocity (feet/second) | Peak Flow without Mitigation (cfs) | Peak Flow with Mitigation (cfs) |
|--------------|-------------------------------|--------------|---------------------------------|-------------------------|----------------------------------|------------------------------------|---------------------------------|
| Pre-Project  | 110                           | 262.8        | 40.0                            | 2.4                     | 1.5                              | 194.8                              | n/a                             |
| Post-Project | 110                           | 262.9        | 45.5                            | 2.2                     | 2.1                              | 191.2                              | 191.2                           |

The rational method calculations determine that peak flow rates from the 100-year storm event draining to the existing depression and culvert are expected to decrease by 3.7 cubic feet per second in post-project conditions compared to pre-project as a result of the proposed development. This is a result of a slightly longer time of concentration (Tc) in the post-project condition, which in turn is a result of the proposed project slightly altering the on-site drainage patterns. More specifically, in the pre-project flow from Nodes 104 and Node 203 to Node 110 is in a near straight line across the existing agriculture field. In the post-project condition, flow between the same nodes will be in swales that will direct flows around the proposed project location in the agriculture field where flow currently travels straight. The slight increase in flow path length in post-project conditions results in a slightly longer Tc in the confluence calculations of the rational method and a reduced overall post-project condition peak flow rate.

## **2.5 Caltrans Hydrologic Analysis Methodology – Project Local Watershed**

Because modifications are proposed to Caltrans drainage infrastructure, a hydrologic analysis of the local watershed was performed in accordance with the Caltrans *Highway Design Manual, Sixth Edition* (2018). The project includes widen to Highway 78 to accommodate a turning, deceleration, and acceleration lanes, which in turn will require that the existing dual-cell 5 feet (width) by 2 feet (height) reinforced concrete box (RCB) culvert be extended. The Caltrans hydrologic analysis utilized the same topographic watershed delineations as the County hydrologic analysis. The Hydrologic Engineering Center-Hydrologic Modeling System (HEC-HMS) software was utilized to perform the analysis. Each subbasin in the watershed was represented in the model using the Initial and Constant loss method and the Kinematic Wave transformation method. Rainfall data were incorporated into the model from NOAA Atlas 14 data. In order to prepare an appropriate hyetograph, the guidance provided in Chapter 6 of the FHWA *Hydraulic Design Series No. 2, Second Edition* (2002) was followed.

The Kinematic Wave transformation method was utilized to represent the internal flow routing for each subbasin. This method considers that runoff begins on plane, flows into subcollectors, collectors, and finally a channel. For this project, the modeling used Plane 1 to represent impervious areas and Plane 2 to represent pervious areas. Table 2-7 provides a summary of the data used in the modeling.

**Table 2-7. Kinematic Wave Transformation Method Parameters**

| <b>Feature</b>  | <b>Length<br/>(feet)</b> | <b>Slope<br/>(feet/feet)</b> | <b>Roughness</b> | <b>Width<br/>(feet)</b> | <b>Side Slope<br/>(H:V)</b> |
|-----------------|--------------------------|------------------------------|------------------|-------------------------|-----------------------------|
| <b>Hillside</b> |                          |                              |                  |                         |                             |
| Plane 2         | 25                       | 0.4                          | 0.045            | n/a                     | n/a                         |
| Subcollector    | 75                       | .4                           | 0.045            | 0.5                     | 3                           |
| Collector       | 75                       | .4                           | 0.045            | 2                       | 3                           |
| Channel         | Varies                   | Varies                       | 0.045            | 5                       | 3                           |
| <b>Vineyard</b> |                          |                              |                  |                         |                             |
| Plane 1         | 25                       | 0.01                         | .015             | n/a                     | n/a                         |
| Plane 2         | 25                       | 0.006                        | 0.040            | n/a                     | n/a                         |
| Subcollector    | 75                       | 0.006                        | 0.045            | 0.5                     | 3                           |
| Collector       | 75                       | 0.006                        | 0.045            | 2                       | 3                           |
| Channel         | Varies                   | Varies                       | 0.045            | 5                       | 3                           |
| <b>Street</b>   |                          |                              |                  |                         |                             |
| Plane 1         | 30                       | 0.06                         | 0.015            |                         |                             |
| Plane 2         | 25                       | 0.25                         | 0.040            |                         |                             |
| Subcollector    | 75                       | 0.01                         | 0.015            | 0.5                     | 3                           |
| Collector       | 75                       | 0.25                         | 0.015            | 1                       | 3                           |
| Channel         | Varies                   | Varies                       | 0.040            | 5                       | 4                           |

For the Initial and Constant loss method constant loss rates were based on hydraulic conductivity data obtain from the USGS Web Soil Survey database. The database list a range of values, and for this project the lower end of the range values were used as summarized on Table 2-8. Where applicable, area-weighted values were used. A copy of the Web Soil Survey is provided in Appendix D. Please refer to the Caltrans Drainage Study Maps for the pre-project and post-project conditions which are located in Map Pockets 4 and 5, respectively.

**Table 2-8. Soil Parameters Summary**

| <b>Soil Name</b>       | <b>Hydrologic<br/>Soil Group</b> | <b>Hydraulic<br/>Conductivity (in/hr)</b> |
|------------------------|----------------------------------|-------------------------------------------|
| CnG2-Cieneba Fallbrook | D                                | 0.92                                      |
| RaD2-Ramona            | C                                | 0.2                                       |
| TuB-Tujunga            | A                                | 5.95                                      |
| VaA-Visalia            | A                                | 1.98                                      |

The guidance provided in Chapter 6 of the FHWA *Hydraulic Design Series No. 2, Second Edition* (2002) was followed to prepare a hyetograph for the 100-year, 24-hour and 10-year, 24-hour storm events. This guidance specifies that a ½ - ½ nested distribution be utilized. Tables showing the resulting hyetograph are provided in Appendix D.

## 2.6 Caltrans Hydrologic Analysis Results – Project Local Watershed

The results of the hydrologic modeling are provided in Table 2-9.

**Table 2-9. Caltrans Hydrologic Analysis Results Summary**

| Location                             | 100-Year Storm Event             |                                   | 10-Year Storm Event              |                                   |
|--------------------------------------|----------------------------------|-----------------------------------|----------------------------------|-----------------------------------|
|                                      | Pre-Project Peak Flow Rate (cfs) | Post-Project Peak Flow Rate (cfs) | Pre-Project Peak Flow Rate (cfs) | Post-Project Peak Flow Rate (cfs) |
| J-110 (Upstream of Culvert)          | 121.0                            | 105.7                             | 20.1                             | 20.4                              |
| J-111 (Downstream of Culvert)        | 121.3                            | 106.2                             | 20.6                             | 21.0                              |
| J-112 (at Downstream Project Limits) | 121.5                            | 106.4                             | 20.8                             | 21.3                              |

The results indicate that the project will result in a decrease in peak flows reaching the Caltrans culvert in the post-project condition in comparison to the pre-project condition. More detailed output tables are provided in Appendix D that include the runoff from each subbasin along with the time of the peak flow. In the post-project condition, the majority of flow is a result of runoff from the hillsides and vineyard upstream of the project development area, and the project improvements result in an increase of 350 feet (flow path of 620 feet becomes 970 feet) in the distance that a majority of the runoff must travel to reach the culvert. This in turn results in the peak flow occurring 5 minutes later in the storm in the post-project condition when compared to the pre-project condition. Hydraulic modeling, discussed in Section 3.0, were prepared to estimate the hydraulic performance of the existing and modified (extended) culvert and downstream system during the 100-year peak flow rate for the existing and proposed condition, respectively.

The results indicate that during the 10-year storm event, the project improvements will result in slight increases in peak flow rates at the Caltrans culvert and downstream of the culvert. Hydraulic modeling, discussed in Section 3.0, were prepared to verify that the extended culvert will satisfy the 10-year peak flow rate hydraulic criteria.

### 3.0 LOCAL HYDRAULIC ANALYSES

Local hydraulic analyses were performed to assess the storm water conveyance capacity of the double 5 feet (wide) by 2 feet (high) RCB culvert, spanning CA-78, and the existing dual 24-inch corrugated metal pipe (CMP), spanning the residential driveway, down to Guejito Creek. As described in Section 1.2, runoff sheet flows southerly through project site prior to being collected within an existing depression that is located adjacent to the existing site access driveway along the north side of CA-78. From this collection point, the existing double 5 feet (wide) by 2 feet (high) RCB culvert conveys runoff beneath CA-78 and into an existing drainage ditch along the southern roadway shoulder. Runoff in the ditch then flows westerly, through the existing dual 24-inch CMP, and confluences with Guejito Creek approximately 500 feet downstream. Hydraulic modeling was performed for the pre- and post-project condition in accordance with the Caltrans *Highway Design Manual* for the 10-year and 100-year storm events.

#### 3.1 Local Hydraulic Methodology

Hydraulic calculations were performed for this study using the Water Surface Pressure Gradient for Windows (WSPGW) modeling software. The WSPGW software was utilized to compute and plot the uniform and non-uniform steady flow water surface profiles and pressure gradients in open channels and closed conduits associated with the project. The WSPGW software can perform hydraulic calculations in a system where flow alternates between supercritical, sub-critical, or pressure flow in any sequence. The program uses mathematical and hydraulic principles to calculate key data such as cross-sectional area, wetted perimeter, normal depth, critical depth, pressure, momentum, and velocity. An existing condition WSPGW model was prepared to reflect the existing culvert and downstream, and this model was prepared based on topographic ground survey performed on September 25, 2020. A proposed condition WSPGW model was prepared using the existing condition model as base and incorporation the proposed culvert extension and upstream geometry based on the geometry and elevations shown on the improvement plans.

The WSPGW hydraulic model is a one-dimensional hydraulic model that does not capture the effects of ponding and potential split flow conditions along open channel banks and roadways. To account for split flow conditions (i.e., runoff that bypasses the existing storm drain conveyance system), broad-crested weir calculations in conjunction with the WSPGW results were used to determine the appropriate water surface elevation that would result in the total design peak flow rate for the existing and proposed conditions. Based on topographic survey data, the southern bank of the existing drainage ditch south of CA-78 has a lower elevation than the northern bank adjacent to the road. It is anticipated that runoff will overtop the southern bank and exit the system as weir flow during peak storm events while the remaining flow passes through the existing dual 24-inch CMP. The combined WSPGW modeling and weir flow (split

flow) analysis was an iterative process of varying the flow rate conveyed through the culvert until the resulting water surface elevation produced a flow rate through broad-crested weir flow calculations that, when combined with the 24-inch CMP WSPGW model flow rate, totaled the design peak flow rate.

### 3.2 Local Hydraulic Results

Table 3-1 provides a summary of water surface elevations and peak flow rates through the storm drain infrastructure for existing and proposed conditions.

**Table 3-1. Summary of Local Hydraulic Results**

| Condition                                                                    | Dual 24-inch CMP              |                              |                                 | Double 5 feet (W) by 2 feet (H) RCB |                                                  |
|------------------------------------------------------------------------------|-------------------------------|------------------------------|---------------------------------|-------------------------------------|--------------------------------------------------|
|                                                                              | WSE (ft) - U/S of 24-inch CMP | Flow Rate (cfs) as Weir Flow | Flow Rate (cfs) through Culvert | Flow Rate (cfs) through Culvert     | WSE (ft) - U/S of Double 5-ft (W) x 2-ft (H) RCB |
| <b>Existing Condition<sup>1</sup><br/>(Pre-Project 100-yr Storm Event)</b>   | 422.76                        | 75.1                         | 46.2                            | 121.0                               | 424.03                                           |
| <b>Proposed Condition<sup>1</sup><br/>(Post-Project 100-yr Storm Event)</b>  | 422.72                        | 60.4                         | 45.8                            | 105.7                               | 424.25                                           |
| <b>Existing Condition<sup>2,3</sup><br/>(Pre-Project 10-yr Storm Event)</b>  | 421.13                        | N/A <sup>5</sup>             | 20.1                            | 20.1                                | 421.34                                           |
| <b>Proposed Condition<sup>2,4</sup><br/>(Post-Project 10-yr Storm Event)</b> | 421.18                        | N/A <sup>5</sup>             | 21.3                            | 21.3                                | 422.74                                           |

1. Top of road, edge of pavement at double RCB= 424.62 f and 425.3 for the existing and post-project conditions, respectively.
2. Dual 24-inch CMP: Upstream Soffit = 420.64 ft. Downstream Soffit = 420. 46 ft.
3. Existing Double 5-ft (W) by 2-ft (H) RCB: Upstream Soffit = 422. 65 ft. Downstream Soffit = 422. 37 ft.
4. Proposed Double 5-ft (W) by 2-ft (H) RCB: Upstream Soffit = 424. 09 ft. Downstream Soffit = 422. 37 ft.
5. Weir flow begins along southern bank at elevation 422.17 ft.

Appendix E provides the input and output WSPGW files. Table 3-2 provides a summary of WSPGW file names and descriptions for reference. Broad-crested weir calculations are provided in Appendix E.

**Table 3-2. Summary of WSPGW File Input and Outputs**

| WSPGW File Name | Description                                                                                                                                     |
|-----------------|-------------------------------------------------------------------------------------------------------------------------------------------------|
| <b>rgex100a</b> | Existing Condition - 100-yr Storm Event : Modeling double 5-ft (W) by 2-ft (H) RCB to dual 24-inch CMP, using starting WSE from rgex100b output |
| <b>rgex100b</b> | Existing Condition - 100-yr Storm Event: Modeling dual 24-inch CMP to Guejito Creek                                                             |

| <b>WSPGW File Name</b> | <b>Description</b>                                                                                                                                    |
|------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------|
| <b>rgpr100a</b>        | Proposed Condition - 100-yr Storm Event: Modeling from double 5-ft (W) by 2-ft (H) RCB to dual 24-inch CMP, using starting WSE from rgpr100b upstream |
| <b>rgpr100b</b>        | Proposed Condition - 100-yr Storm Event: Modeling dual 24-inch CMP to Guejito Creek                                                                   |
| <b>rgex10</b>          | Existing Condition - 10-yr Storm Event: Complete Model from double 5-ft (W) by 2-ft (H) RCB to Guejito Creek                                          |
| <b>rgpr10</b>          | Proposed Condition - 10-yr Storm Event: Complete Model from double 5-ft (W) by 2-ft (H) RCB to Guejito Creek                                          |
| <b>rgex100</b>         | Existing Condition - 100-yr Storm Event: Complete Model from double 5-ft (W) by 2-ft (H) RCB to Guejito Creek                                         |
| <b>rgpr100</b>         | Proposed Condition - 100-yr Storm Event: Complete Model from double 5-ft (W) by 2-ft (H) RCB to Guejito Creek                                         |

Based on the results presented in Table 3-1 the project complies with the Highway Design Manual guidance related to the proposed culvert having the hydraulic capacity to convey the 10-year peak flow rate without causing the headwater elevation to rise above the inlet top of the culvert and to convey the 100-year peak flow rate without headwaters rising above an elevation that would cause objectionable backwater depths or outlet velocities. This study acknowledges that due to the flat grades within the project property upstream of the culvert, the analysis indicates that there will be shallow ponding, less than 1 foot depth in general, within the project property for the both existing and post-project conditions during the analyzed 100-year peak flow rates. Due to the flat topography combined with the limited capacity of the channel and dual 24-inch RCPs downstream of the culvert, the ponding within the project property is unavoidable. In the existing condition, the ponding within the project property upstream from the CA-78 culvert is limited to areas occupied by vineyard. In the proposed condition, the ponding within the project property upstream from the CA-78 culvert is generally limited to areas occupied by vineyard with minor areas of drive isles and park inundated with depths generally less than 6 inches. Because the ponding within the project property is an existing condition and in the post-project condition, the shallow ponding is generally the same as in the existing condition, the project does not result in objectionable backwater depths.



## 4.0 GUEJITO CREEK HYDRAULIC ANALYSIS

Hydraulic analyses were also performed for Guejito Creek as part of this study. Guejito Creek runs along the western portion of the project site and drains an approximately 27.7 square mile area upstream of the project site, which has been defined as the Guejito Creek watershed in this study. Existing Condition and Proposed Condition Models were created to analyze the anticipated water surface elevations resulting from the peak flows associated with the 100-year storm event and the hydraulic effects, if any, from the project on Guejito Creek. The peak flow rate was calculated previously in this study as 16,000 cubic feet per second.

The U.S. Army Corps of Engineers' Hydrologic Engineering Center's River Analysis System (HEC-RAS) v.4.1.0 was used to model the hydraulic characteristics of Guejito Creek and determine the water surface elevations associated with the peak flows from the 100-year storm event. HEC-RAS software is designed to perform one-dimensional hydraulic calculations for steady gradually-varied flow in natural and constructed channels.

### 4.1 Existing Condition Model Methodology

An Existing Condition Model was created to determine the anticipated water surface elevations from the 100-year storm event peak flows, and for comparison to the Proposed Condition Model in determining the hydraulic effects of the project. The topographic data utilized to define the Existing Condition Model channel geometry was obtained from SanGIS. The SanGIS obtained data is from a 2015 data set consisting of 5-foot contours, raw Light Detection and Ranging (LiDAR), hydro-flattened digital elevation model (DEM) and is relative to the North American Vertical Datum of 1988 (NAVD88). The DEM was adjusted to -2.19 feet to be consistent with the grading plans datum of National Geodetic Vertical Datum of 1929 (NGVD29). The downstream boundary control utilized for these analyses is based on the FEMA Flood Insurance Study (FIS) one percent annual chance Base Flood Elevation at the Guejito Creek confluence of 415.31 feet in reference to NGVD29 (417.5 NAVD 88). The hydraulic analysis was performed in accordance with guidelines from the San Diego County *Hydraulic Design Manual*.

#### ***Manning's Roughness Coefficient***

The Manning's Roughness Coefficients (Manning's  $n$ ) for the existing conditions model were estimated based on available data including aerial imagery, photos taken, and information gathered during previous visits to the project site. The estimated Manning's  $n$  values were correlated to the values provided in Appendix A of the Hydraulic Design Manual. The applicable excerpts from the Hydraulic Design Manual can be found in Appendix E. A summary of the values is provided in Table 4-1.

**Table 4-1: Manning's Roughness Coefficient, n, Summary for HEC-RAS Analysis**

| Channel Section <sup>1</sup> | Manning's n | Description <sup>2</sup>                        |
|------------------------------|-------------|-------------------------------------------------|
| Left Overbank - Vineyard     | 0.040       | Mature Row Crops                                |
| Channel                      | 0.150       | Maximum lifetime channel roughness <sup>3</sup> |
| Right Overbank - Orchard     | 0.060       | Light Brush and Trees                           |

Note 1: Channel section defined by looking downstream

Note 2: Description from Table A-5 of Hydraulic Design Manual

Note 3: Maximum lifetime channel roughness per Hydraulic Design Manual, freeboard design.

## 4.2 Proposed Condition Model Methodology

From the Existing Condition Model, the Proposed Condition Model was created by changing the applicable cross sections to reflect the proposed project improvements. The project transects cross sections 600 through 1000 of the Proposed Condition Model. Buildings were modeled as ineffective flow areas in the model. Additionally, contraction and expansion areas were modeled upstream and downstream of the buildings at 2:1 and 4:1 (in the direction of flow to the direction perpendicular to flow) ratios, respectively. The n-values of the Proposed Condition Model were kept the same as the Existing Condition Model. See the Hydraulic Workmap in Map Pocket 6 for the proposed improvements and the ineffective areas modeled.

## 4.3 Hydraulic Analysis Results

Comparisons between the Existing Condition Model and Proposed Condition Model water surface elevations are provided in Table 4-2 below.

**Table 4-2: Water Surface Elevations for Guejito Creek**

| Station <sup>1</sup> | Existing WSE <sup>2,3</sup> | Proposed WSE <sup>2,3</sup> | Difference <sup>4</sup> |
|----------------------|-----------------------------|-----------------------------|-------------------------|
| 50                   | 415.31                      | 415.31                      | 0                       |
| 150                  | 417.25                      | 417.25                      | 0                       |
| 250                  | 418.78                      | 418.79                      | 0.01                    |
| 300                  | 419.62                      | 419.62                      | 0                       |
| 349.21               | 421.33                      | 421.32                      | -0.01                   |
| 398.94               | 427.2                       | 427.2                       | 0                       |
| 450                  | 427.43                      | 427.43                      | 0                       |
| 500                  | 427.58                      | 427.58                      | 0                       |
| 550                  | 427.65                      | 427.65                      | 0                       |
| 600                  | 427.75                      | 427.75                      | 0                       |
| 650                  | 427.86                      | 427.85                      | -0.01                   |
| 700                  | 427.96                      | 427.96                      | 0                       |

| Station <sup>1</sup> | Existing WSE <sup>2,3</sup> | Proposed WSE <sup>2,3</sup> | Difference <sup>4</sup> |
|----------------------|-----------------------------|-----------------------------|-------------------------|
| 750                  | 428.08                      | 428.11                      | 0.03                    |
| 800                  | 428.2                       | 428.28                      | 0.08                    |
| 850                  | 428.36                      | 428.54                      | 0.18                    |
| 900                  | 428.52                      | 428.85                      | 0.33                    |
| 950                  | 428.71                      | 429.08                      | 0.37                    |
| 1000                 | 428.91                      | 429.28                      | 0.37                    |
| 1200                 | 429.88                      | 430.03                      | 0.15                    |
| 1400                 | 431.02                      | 431.03                      | 0.01                    |
| 1600                 | 432.26                      | 432.26                      | 0                       |
| 1800                 | 433.35                      | 433.35                      | 0                       |
| 2000                 | 434.48                      | 434.48                      | 0                       |

Note 1: Channel stations are listed in order from downstream to upstream

Note 2: WSE = water surface elevation

Note 3: Elevations are in reference to NGVD 29

Note 4: Difference in water surface elevations are proposed to existing (proposed – existing)

The hydraulic models prepared for this study indicates that flows are not confined within the channel banks of Guejito Creek during the 100-year storm event flow rate of 16,000 cubic feet per second. During this event, the valley provides active conveyance through the existing vineyards and agricultural areas. Based on the hydraulic analysis, the proposed tasting facility falls within the 100-year storm event limits of inundation. Additionally, the comparison of the floodplain water surface elevations indicates that the project will result in a minor rise to the water surface elevations along the upstream portion of the project area, however no rises in water surface elevations are experienced downstream of station 750.

The proposed tasting facility structure begins at hydraulic analysis station 10+00 along Guejito Creek (as defined by this analysis), as shown on the HEC-RAS workmap provided in Map Pocket 4. The anticipated 100-year water surface elevation at this station is 429.28 feet (NGVD29), which is approximately 2.5 feet above the existing ground at this location. The proposed tasting facility structure finished floor surface is 430.0 feet (NGVD29). The proposed event center structure begins at hydraulic analysis station 8+50 along Guejito Creek. The anticipated 100-year water surface elevation at station 8+50 is 428.5 feet (NGVD29), which is approximately 3.5 feet above the existing ground at this location. The proposed event center structure finished floor surface is 428.50 (NGVD29). A summary of the HEC-RAS hydraulic model results is provided in Appendix E.

## 5.0 CEQA DETERMINATION OF SIGNIFICANCE

In accordance with the County of San Diego's *Guidelines for Determining Significance*, dated July 2007, this study evaluated the California Environmental Quality Act (CEQA) level potential for adverse environmental impacts by providing responses to following the questions:

Would the project:

- Substantially alter the existing drainage pattern of the site or area, including through the alteration of the course of a stream or river, in a manner which would result in substantial erosion or siltation on- or off-site?
  - **The Rancho Guejito Tasting Facility and Event Center project will not significantly alter the existing drainage pattern.**
- Substantially alter the existing drainage pattern of the site or area, including through the alteration of the course of a stream or river, or substantially increase the rate or amount of surface runoff in a manner that would result in flooding on or off-site?
  - **The Rancho Guejito Tasting Facility and Event Center project will not substantially increase the amount or rate of surface runoff or the existing drainage pattern.**
- Create or contribute runoff water, which would exceed the capacity of existing or planned stormwater drainage systems or provide substantial additional sources of polluted runoff?
  - **The project proposes water quality measures designed in accordance with County of San Diego guidelines to treat runoff from proposed impervious areas. The proposed development will not significantly increase the amount or rate of runoff.**
- Place housing within a 100-year flood hazard area as mapped on a federal Flood Hazard Boundary, a Flood Insurance Rate Map or another flood hazard delineation map?
  - **The project area is not mapped on a federal Flood Hazard Boundary, a Flood Insurance Rate Map or another flood hazard delineation map. The project does not propose any housing.**
- Place within a 100-year flood hazard area structures, which would impede or redirect flood flows?
  - **The project does not propose structures that would impede or redirect flows significantly.**

- Expose people or structures to a significant risk of loss, injury or death involving flooding, including flooding as a result of the failure of a levee or dam?
  - **The resulting 100-year flow rate calculated using the conservative methodologies outlined in the County of San Diego Hydrology Manual. The hydraulic analysis utilized the conservative peak flow rate and determined the area where the development is proposed may be inundated by approximately 2.5 to 3.5 feet of water above existing elevations should this unlikely event occur. Based on the results of these analyses, the design of the proposed structures has been elevated at or above the 100-year calculated water surface elevation. The Rancho Guejito property currently has procedures in place that result in the closure of the site and the evacuation of personnel in the event that a significant rainfall is forecast. The property owner is aware of the inundation risk and associated maintenance that may be required should inundation occur. The risk of flood inundation to the farmhouse east of the project site (owned by the project applicant) will be increased as a result of project (shallow depth, less than 1 foot). The project application is aware of the slight increase in risk and accepts that increase in risk.**
- Inundation by seiche, tsunami, or mudflow?
  - **The project site is not subject to inundation by seiche, tsunami, or mudflow.**

## 6.0 SUMMARY AND CONCLUSION

The analyses performed in this study determined both the estimated 100-year peak flow rate for the Guejito Creek watershed as well as the pre-project and post-project conditions peak flow rates in the local drainage area associated with the proposed tasting facility and event center development. The analyses following the methodologies outlined in the Hydrology Manual and determined that there will be a slight decrease in the 100-year stormwater peak flow rate (of 3.7 cubic feet per second) in the proposed development condition when compared to pre-project condition. The project will not substantially alter the existing drainage pattern of the site or area. The project will not alter the course of a stream or river in a manner that would result in substantial erosion or siltation on-site or off-site. No significant impacts to either downstream receiving waters or existing storm water conveyance systems are anticipated due to the proposed development.

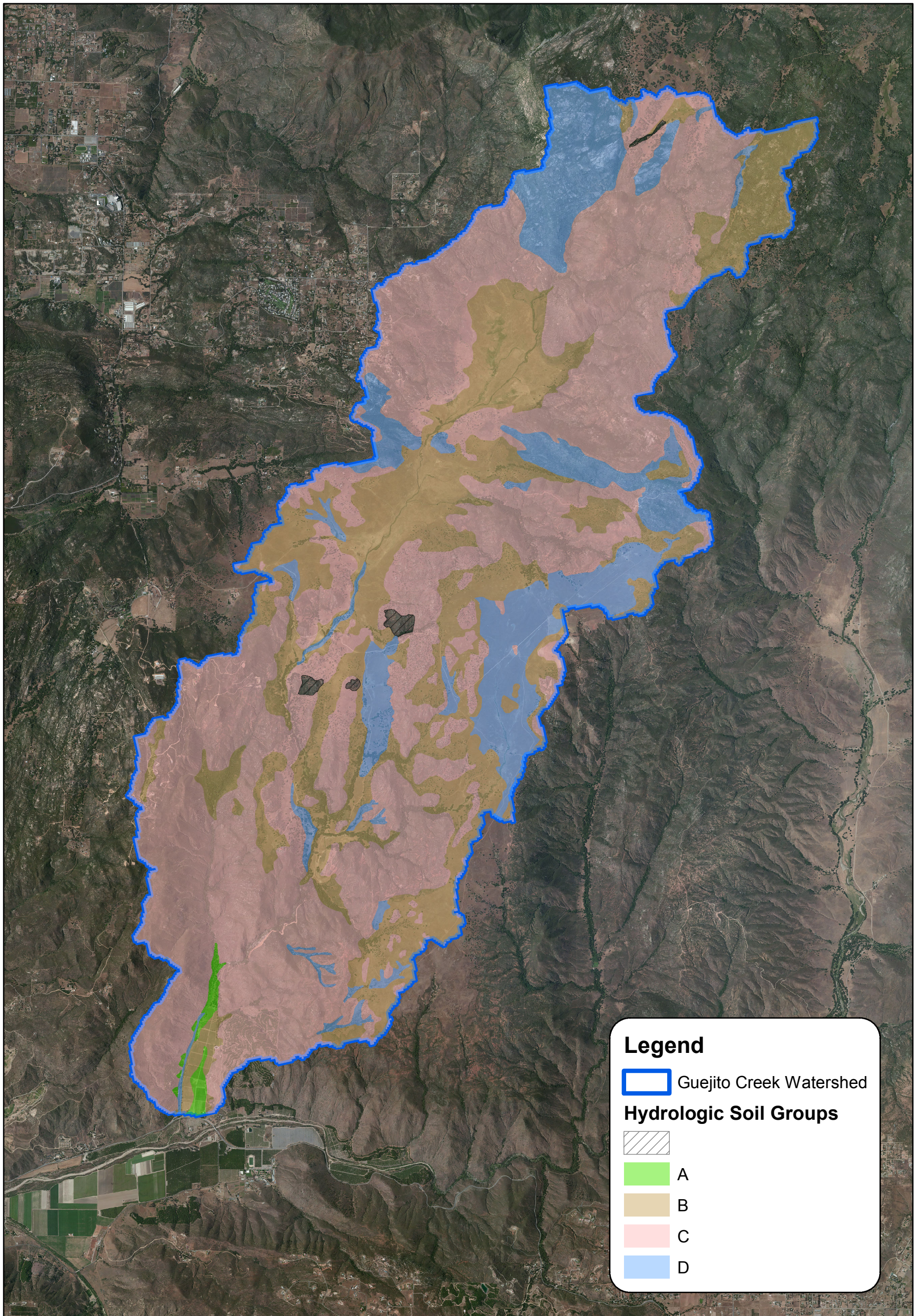
Analyses were performed to evaluate the Guejito Creek hydrologic response for the design storm. The results indicate that the 100-year storm event results in a peak flow rate of approximately 16,000 cubic feet per second within Guejito Creek. HEC-RAS hydraulic modeling was performed using the calculated peak flow rate for the existing and proposed conditions. The hydraulic modeling indicates that the valley in the vicinity of the proposed improvements will provide active conveyance for flows during the 100-year storm event in both the existing and proposed conditions and that the proposed project will result in slight increases to water surface elevations of the creek along the project improvements with the limits of inundation contained to the project property.

The maximum resulting 100-year water surface elevation at the location where the tasting facility is proposed was calculated to be approximately 429.4 feet (NGVD29), which is approximately 2.5 feet above the existing ground at this location. The proposed tasting facility structure finished floor surface is 430.00 feet (NGVD29). The maximum resulting 100-year water surface elevation at the location where the proposed event center was calculated to be approximately 428.5 feet (NGVD29), which is approximately 3.5 feet above the existing ground surface at this location. The proposed event center structure finished floor surface is 428.50 (NGVD29).

## **Appendix A**

### Hydrologic Soil Group Maps





**Legend**

 Guejito Creek Watershed

**Hydrologic Soil Groups**

  A

 B

 C

 D

**GUEJITO CREEK WATERSHED  
HYDROLOGIC SOIL GROUPS MAP  
RANCHO GUEJITO  
TASTING FACILITY AND  
EVENT CENTER**

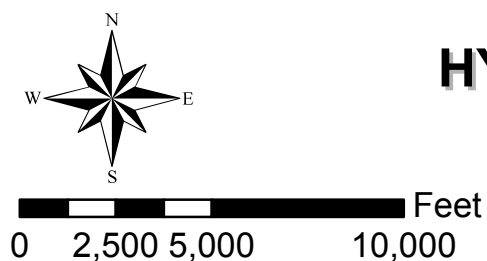
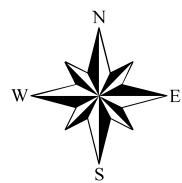
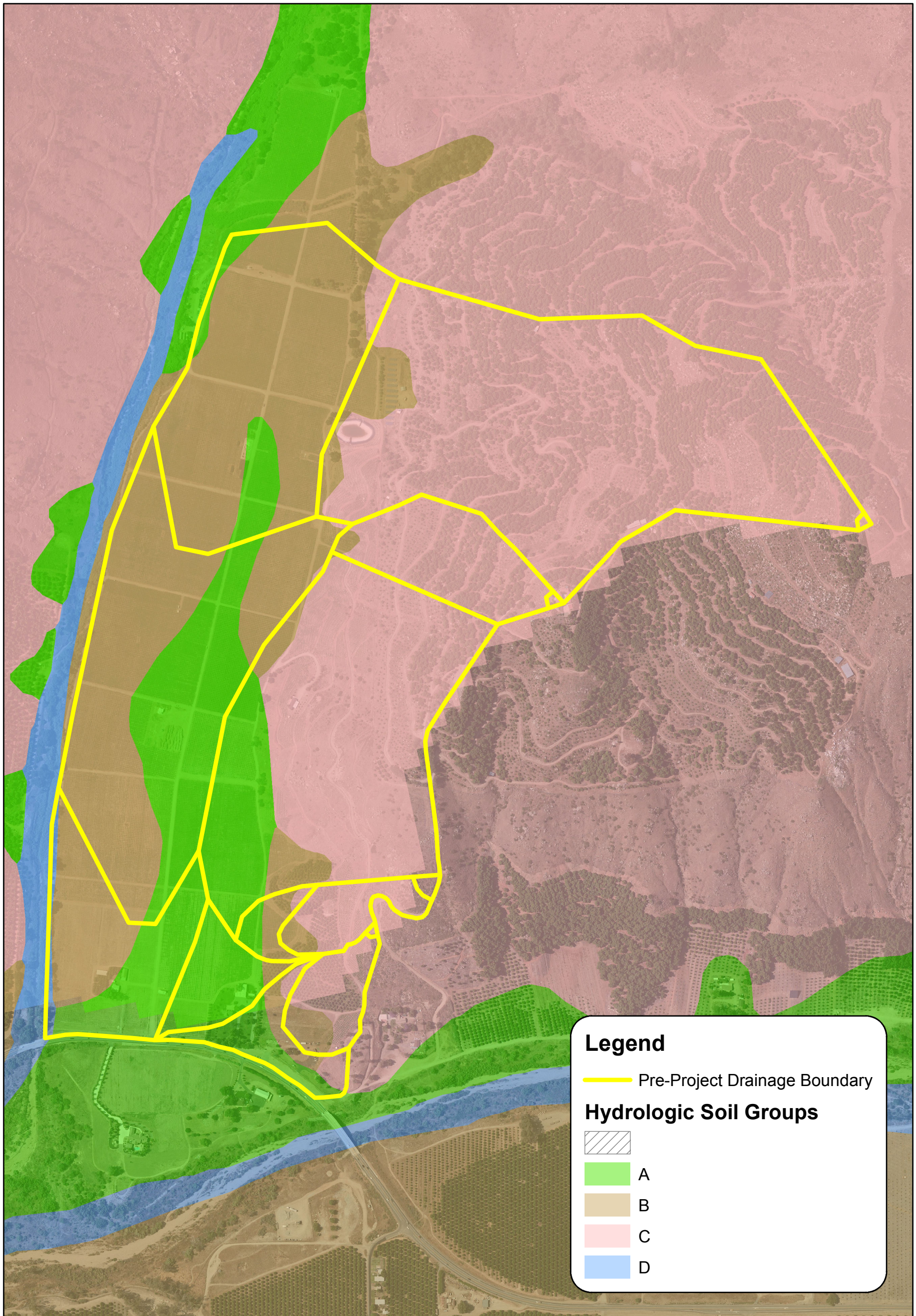
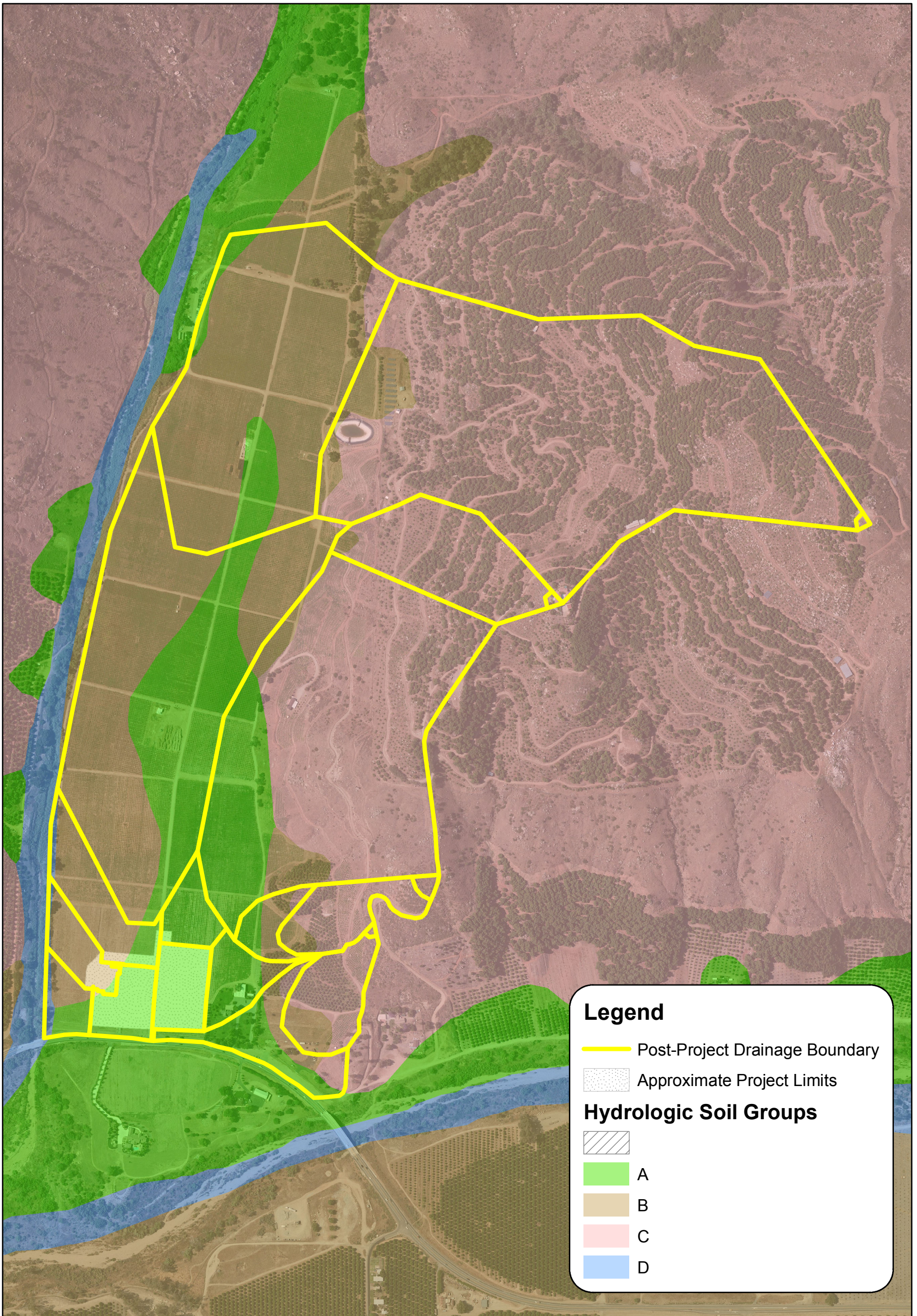


EXHIBIT DATE: 10/24/2019 RICK JN: 14557  
\\cp.rickeng.com\projects\C\_SD\_U\RanchoGuejito\Projects\14557W\_Winery2\GIS\14557\_HSG\_Map1\_GCW.mxd









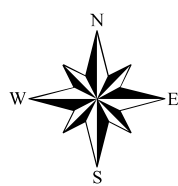
**Legend**

— Post-Project Drainage Boundary

□ Approximate Project Limits

**Hydrologic Soil Groups**

|   |   |
|---|---|
| □ | A |
| □ | B |
| □ | C |
| □ | D |



0 290 580 1,160 Feet

EXHIBIT DATE: 10/24/2019 RICK JN: 14557

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**POST-PROJECT  
HYDROLOGIC SOIL GROUPS MAP  
RANCHO GUEJITO  
TASTING FACILITY AND  
EVENT CENTER**



## **Appendix B**

HEC-1 Program Output and Support Material – Guejito Creek Watershed

NRCS Hydrologic Method  
(Worksheet 4-3)

## NRCS HYDROLOGIC METHOD

## WORKSHEET 4-3

## Peak Discharge Computation

|                                                                                                                                       |                                                                       |                              |                                                 |                    |
|---------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------------------|------------------------------|-------------------------------------------------|--------------------|
| <b>Project Name:</b>                                                                                                                  | <b>Rancho Guejito Tasting Room</b>                                    |                              | <b>RICK JN:</b>                                 | 14557              |
| <b>Computed by:</b>                                                                                                                   | AMC                                                                   |                              | <b>Date:</b>                                    | 6/4/2018           |
| <b>Project Identification (Drainage Area Name):</b>                                                                                   | Rancho Guejito Creek - Basin 100 (upper)                              |                              |                                                 |                    |
| <b>Geographic location of center of drainage area:</b>                                                                                | Long: _____                                                           |                              |                                                 |                    |
|                                                                                                                                       | Drainage Area: <u>22.5</u> square miles                               |                              |                                                 |                    |
|                                                                                                                                       | Storm Frequency (Section 2.3): <u>100</u> -year                       |                              |                                                 |                    |
|                                                                                                                                       | 6-Hour Storm Duration Precipitation (Appendix B): <u>3.90</u> inches  |                              |                                                 |                    |
|                                                                                                                                       | 24-Hour Storm Duration Precipitation (Appendix B): <u>8.75</u> inches |                              |                                                 |                    |
| <b>Precipitation Zone Number (PZN):</b><br>(Section 4.1.2.4 and Appendix C)                                                           | PZN= <u>2.5</u>                                                       |                              |                                                 |                    |
| <b>PZN Adjustment Factor for 5-year to 35-year storm frequency (interpolate):</b><br>(Section 4.1.2.4 and Appendix C)                 | PZN <sub>AF</sub> = N/A                                               |                              |                                                 |                    |
| <b>PZN Adjustment Factor for 35-year to 150-year storm frequency (interpolate):</b><br>(Section 4.1.2.4 and Table 4-6)                | PZN <sub>AF</sub> = 3                                                 |                              |                                                 |                    |
| <b>Composite CN - PZN 2 (Calculated Below)</b>                                                                                        | 83                                                                    | <b>PZN1</b>                  | 67                                              | <b>PZN3</b> 93     |
| <b>PZN Adjusted Runoff Curve Number (interpolate between nearest whole number PZN conditions):</b><br>(Section 4.1.2.4 and Table 4-6) | CN3                                                                   | 93                           | CN3                                             | 93                 |
|                                                                                                                                       | <b>CN3 = 93</b>                                                       |                              | <b>Final CN Used in Analysis - Interpolated</b> |                    |
| <b>Watershed Information:</b>                                                                                                         |                                                                       |                              |                                                 |                    |
| Watershed Length (L) (Section 4.3.1):                                                                                                 | <u>10.74</u>                                                          | miles                        |                                                 |                    |
| Length to Centroid (Lc) (Section 4.3.1):                                                                                              | <u>5.37</u>                                                           | miles                        |                                                 |                    |
| U/S Elev:                                                                                                                             | <u>3370</u>                                                           | feet                         | D/S Elev:                                       | <u>450</u> feet    |
| Slope (s) (Section 4.3.1):                                                                                                            | <u>272</u>                                                            | ft/mile                      |                                                 |                    |
| Basin n Factor (Section 4.3.5):                                                                                                       | <u>0.06</u>                                                           |                              |                                                 |                    |
| <b>Lag Time Calculations:</b>                                                                                                         |                                                                       |                              |                                                 |                    |
| Corps lag (T <sub>L</sub> ) = 24 n ((L x Lc)/s <sup>0.578</sup> ) <sup>0.78</sup> (Section 4.3.1.1):                                  | <u>2.317</u>                                                          | hours                        |                                                 |                    |
| or                                                                                                                                    |                                                                       |                              |                                                 |                    |
| Corps lag (T <sub>L</sub> ) = 0.8 T <sub>c</sub> (Section 4.3.1.2):                                                                   |                                                                       |                              |                                                 |                    |
| Time to Peak = 0.862 x Corps lag (Section 4.1.5.5):                                                                                   | <u>1.998</u>                                                          | hours                        |                                                 |                    |
| Time Step:                                                                                                                            | <u>5</u>                                                              | minutes                      |                                                 |                    |
| NRCS lag (T <sub>L</sub> ):                                                                                                           | <b>1.956</b>                                                          | hours                        |                                                 |                    |
| <b>Basin CN2 Calculation - PZN 2 Table 4-2</b>                                                                                        |                                                                       |                              |                                                 |                    |
| <b>Land Use Type-Cover-Hydrologic Condition (As Applicable)</b>                                                                       | <b>Land Use Area (sq mi)</b>                                          | <b>Percent of Total Area</b> | <b>CN</b>                                       | <b>Weighted CN</b> |
| Open Brush, Poor (NRCS A Soil)                                                                                                        | 0.000                                                                 |                              | 62                                              | 0                  |
| Open Brush, Poor (NRCS B Soil)                                                                                                        | 5.630                                                                 | 25%                          | 76                                              | 19                 |
| Open Brush, Poor (NRCS C Soil)                                                                                                        | 12.380                                                                | 55%                          | 84                                              | 46.2               |
| Open Brush, Poor (NRCS D Soil)                                                                                                        | 4.500                                                                 | 20%                          | 88                                              | 17.6               |
| <b>Total</b>                                                                                                                          | <b>22.50</b>                                                          | <b>100%</b>                  | <b>Composite</b>                                | <b>83</b>          |

| NRCS HYDROLOGIC METHOD                                                                                                                |                                                                 |                                                 |                  |                    |
|---------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------------|-------------------------------------------------|------------------|--------------------|
| <b>WORKSHEET 4-3</b>                                                                                                                  |                                                                 | <b>Peak Discharge Computation</b>               |                  |                    |
| <b>Project Name:</b>                                                                                                                  | <u>Rancho Guejito Tasting Room</u>                              | <b>RICK JN:</b>                                 | <u>14557</u>     |                    |
| <b>Computed by:</b>                                                                                                                   | <u>AMC</u>                                                      | <b>Date:</b>                                    | <u>6/4/2018</u>  |                    |
| <b>Project Identification (Drainage Area Name):</b>                                                                                   | <u>Rancho Guejito Creek - Basin 200 (Rockwell Canyon Creek)</u> |                                                 |                  |                    |
| <b>Geographic location of center of drainage area:</b>                                                                                | <u>Long: _____</u>                                              |                                                 |                  |                    |
|                                                                                                                                       | Drainage Area:                                                  | <u>3.03</u>                                     | square miles     |                    |
|                                                                                                                                       | Storm Frequency (Section 2.3):                                  | <u>100</u>                                      | -year            |                    |
|                                                                                                                                       | 6-Hour Storm Duration Precipitation (Appendix B):               | <u>3.65</u>                                     | inches           |                    |
|                                                                                                                                       | 24-Hour Storm Duration Precipitation (Appendix B):              | <u>7.5</u>                                      | inches           |                    |
| <b>Precipitation Zone Number (PZN):</b><br>(Section 4.1.2.4 and Appendix C)                                                           | PZN=                                                            | <u>2.5</u>                                      |                  |                    |
| <b>PZN Adjustment Factor for 5-year to 35-year storm frequency (interpolate):</b><br>(Section 4.1.2.4 and Appendix C)                 | PZN <sub>AF</sub> =                                             | N/A                                             |                  |                    |
| <b>PZN Adjustment Factor for 35-year to 150-year storm frequency (interpolate):</b><br>(Section 4.1.2.4 and Table 4-6)                | PZN <sub>AF</sub> =                                             | 3                                               |                  |                    |
| <b>Composite CN - PZN 2 (Calculated Below)</b>                                                                                        | 82                                                              | <b>PZN1</b>                                     | 66               | <b>PZN3</b> 92     |
| <b>PZN Adjusted Runoff Curve Number (interpolate between nearest whole number PZN conditions):</b><br>(Section 4.1.2.4 and Table 4-6) | CN3 92                                                          | CN3 92                                          |                  |                    |
|                                                                                                                                       | <b>CN3 = 92</b>                                                 | <b>Final CN Used in Analysis - Interpolated</b> |                  |                    |
| <b>Watershed Information:</b>                                                                                                         |                                                                 |                                                 |                  |                    |
| Watershed Length (L) (Section 4.3.1):                                                                                                 | <u>3.36</u>                                                     | miles                                           |                  |                    |
| Length to Centroid (Lc) (Section 4.3.1):                                                                                              | <u>1.68</u>                                                     | miles                                           |                  |                    |
| U/S Elev:                                                                                                                             | <u>2265</u>                                                     | feet                                            |                  |                    |
| Slope (s) (Section 4.3.1):                                                                                                            | <u>540</u>                                                      | ft/mile                                         |                  |                    |
| Basin n Factor (Section 4.3.5):                                                                                                       | <u>0.06</u>                                                     |                                                 |                  |                    |
| <b>Lag Time Calculations:</b>                                                                                                         |                                                                 |                                                 |                  |                    |
| Corps lag (T <sub>L</sub> ) = 24 n ((L x Lc)/s <sup>0.57</sup> ) <sup>0.38</sup> (Section 4.3.1.1):                                   | 0.841                                                           | hours                                           |                  |                    |
| or                                                                                                                                    |                                                                 |                                                 |                  |                    |
| Corps lag (T <sub>L</sub> ) = 0.8 T <sub>c</sub> (Section 4.3.1.2):                                                                   |                                                                 |                                                 |                  |                    |
| Time to Peak = 0.862 x Corps lag (Section 4.1.5.5):                                                                                   | 0.725                                                           | hours                                           |                  |                    |
| Time Step:                                                                                                                            | 5                                                               | minutes                                         |                  |                    |
| NRCS lag (T <sub>L</sub> ):                                                                                                           | <b>0.683</b>                                                    | hours                                           |                  |                    |
| <b>Basin CN2 Calculation - PZN 2 Table 4-2</b>                                                                                        |                                                                 |                                                 |                  |                    |
| <b>Land Use Type-Cover-Hydrologic Condition (As Applicable)</b>                                                                       | <b>Land Use Area (sq mi)</b>                                    | <b>Percent of Total Area</b>                    | <b>CN</b>        | <b>Weighted CN</b> |
| Open Brush, Poor (NRCS A Soil)                                                                                                        | 0.000                                                           |                                                 | 62               | 0                  |
| Open Brush, Poor (NRCS B Soil)                                                                                                        | 0.300                                                           | 10%                                             | 76               | 7.6                |
| Open Brush, Poor (NRCS C Soil)                                                                                                        | 2.730                                                           | 90%                                             | 84               | 75.6               |
| Open Brush, Poor (NRCS D Soil)                                                                                                        | 0.000                                                           |                                                 | 88               | 0                  |
| <b>Total</b>                                                                                                                          | 3.03                                                            | <b>100%</b>                                     | <b>Composite</b> | <b>83</b>          |

| NRCS HYDROLOGIC METHOD                                                                                                         |                                                      |                          |                                          |             |
|--------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------|--------------------------|------------------------------------------|-------------|
| WORKSHEET 4-3                                                                                                                  |                                                      |                          | Peak Discharge Computation               |             |
| Project Name:                                                                                                                  | Rancho Guejito Tasting Room                          | RICK JN:                 | 14557                                    |             |
| Computed by:                                                                                                                   | AMC                                                  | Date:                    | 6/4/2018                                 |             |
| Project Identification (Drainage Area Name):                                                                                   | Rancho Guejito Creek - Basin 300 (unnamed watershed) |                          |                                          |             |
| Geographic location of center of drainage area:                                                                                | 3<br>Long: _____                                     |                          |                                          |             |
|                                                                                                                                | Drainage Area:                                       | 1.06                     | square miles                             |             |
|                                                                                                                                | Storm Frequency (Section 2.3):                       | 100                      | -year                                    |             |
|                                                                                                                                | 6-Hour Storm Duration Precipitation (Appendix B):    | 3.6                      | inches                                   |             |
|                                                                                                                                | 24-Hour Storm Duration Precipitation (Appendix B):   | 6                        | inches                                   |             |
| Precipitation Zone Number (PZN):<br>(Section 4.1.2.4 and Appendix C)                                                           | PZN=                                                 | 2.5                      |                                          |             |
| PZN Adjustment Factor for 5-year to 35-year storm frequency (interpolate):<br>(Section 4.1.2.4 and Appendix C)                 | PZN <sub>AF</sub> =                                  | N/A                      |                                          |             |
| PZN Adjustment Factor for 35-year to 150-year storm frequency (interpolate):<br>(Section 4.1.2.4 and Table 4-6)                | PZN <sub>AF</sub> =                                  | 3                        |                                          |             |
| Composite CN - PZN 2 (Calculated Below)                                                                                        | 82                                                   | PZN1                     | 66                                       | PZN3 92     |
| PZN Adjusted Runoff Curve Number (interpolate between nearest whole number PZN conditions):<br>(Section 4.1.2.4 and Table 4-6) | CN3                                                  | 92                       | CN3                                      | 92          |
|                                                                                                                                | CN3 =                                                | 92                       | Final CN Used in Analysis - Interpolated |             |
| <b>Watershed Information:</b>                                                                                                  |                                                      |                          |                                          |             |
| Watershed Length (L) (Section 4.3.1):                                                                                          | 2.47                                                 | miles                    |                                          |             |
| Length to Centroid (Lc) (Section 4.3.1):                                                                                       | 1.235                                                | miles                    |                                          |             |
| U/S Elev:                                                                                                                      | 1990                                                 | feet                     | D/S Elev:                                | 450         |
| Slope (s) (Section 4.3.1):                                                                                                     | 623                                                  | ft/mile                  |                                          |             |
| Basin n Factor (Section 4.3.5):                                                                                                | 0.06                                                 |                          |                                          |             |
| <b>Lag Time Calculations:</b>                                                                                                  |                                                      |                          |                                          |             |
| Corps lag (T <sub>L</sub> ) = 24 n ((L x Lc)/s <sup>0.57</sup> ) <sup>0.0008</sup> (Section 4.3.1.1):                          | 0.648                                                | hours                    |                                          |             |
| or                                                                                                                             |                                                      |                          |                                          |             |
| Corps lag (T <sub>L</sub> ) = 0.8 T <sub>c</sub> (Section 4.3.1.2):                                                            |                                                      |                          |                                          |             |
| Time to Peak = 0.862 x Corps lag (Section 4.1.5.5):                                                                            | 0.558                                                | hours                    |                                          |             |
| Time Step:                                                                                                                     | 5                                                    | minutes                  |                                          |             |
| NRCS lag (T <sub>L</sub> ):                                                                                                    | 0.517                                                | hours                    |                                          |             |
| <b>Basin CN2 Calculation - PZN 2 Table 4-2</b>                                                                                 |                                                      |                          |                                          |             |
| Land Use Type-Cover-Hydrologic Condition (As Applicable)                                                                       | Land Use Area<br>(sq mi)                             | Percent of Total<br>Area | CN                                       | Weighted CN |
| Open Brush, Poor (NRCS A Soil)                                                                                                 | 0.000                                                |                          | 62                                       | 0           |
| Open Brush, Poor (NRCS B Soil)                                                                                                 | 0.320                                                | 30%                      | 76                                       | 22.8        |
| Open Brush, Poor (NRCS C Soil)                                                                                                 | 0.690                                                | 65%                      | 84                                       | 54.6        |
| Open Brush, Poor (NRCS D Soil)                                                                                                 | 0.050                                                | 5%                       | 88                                       | 4.4         |
| <b>Total</b>                                                                                                                   | 1.06                                                 | 100%                     | <b>Composite</b>                         | <b>82</b>   |

| NRCS HYDROLOGIC METHOD                                                                                                         |                                                                                                                                                                                                                                        |                          |                                          |             |
|--------------------------------------------------------------------------------------------------------------------------------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------|------------------------------------------|-------------|
| WORKSHEET 4-3                                                                                                                  |                                                                                                                                                                                                                                        |                          | Peak Discharge Computation               |             |
| Project Name:                                                                                                                  | Rancho Guejito Tasting Room                                                                                                                                                                                                            |                          | RICK JN:                                 | 14557       |
| Computed by:                                                                                                                   | AMC                                                                                                                                                                                                                                    |                          | Date:                                    | 6/4/2018    |
| Project Identification (Drainage Area Name):                                                                                   | Rancho Guejito Creek - Basin 400 (Project Site / Active Ag Area)                                                                                                                                                                       |                          |                                          |             |
| Geographic location of center of drainage area:                                                                                | Long: _____                                                                                                                                                                                                                            |                          |                                          |             |
|                                                                                                                                | Drainage Area: <u>1.1</u> square miles<br>Storm Frequency (Section 2.3): <u>100</u> -year<br>6-Hour Storm Duration Precipitation (Appendix B): <u>3.6</u> inches<br>24-Hour Storm Duration Precipitation (Appendix B): <u>6</u> inches |                          |                                          |             |
| Precipitation Zone Number (PZN):<br>(Section 4.1.2.4 and Appendix C)                                                           | PZN = <u>2.5</u>                                                                                                                                                                                                                       |                          |                                          |             |
| PZN Adjustment Factor for 5-year to 35-year storm frequency (interpolate):<br>(Section 4.1.2.4 and Appendix C)                 | PZN <sub>AF</sub> = <u>N/A</u>                                                                                                                                                                                                         |                          |                                          |             |
| PZN Adjustment Factor for 35-year to 150-year storm frequency (interpolate):<br>(Section 4.1.2.4 and Table 4-6)                | PZN <sub>AF</sub> = <u>3</u>                                                                                                                                                                                                           |                          |                                          |             |
| Composite CN - PZN 2 (Calculated Below)                                                                                        | 81                                                                                                                                                                                                                                     | PZN1                     | 64                                       | PZN3        |
|                                                                                                                                |                                                                                                                                                                                                                                        |                          |                                          | 92          |
| PZN Adjusted Runoff Curve Number (interpolate between nearest whole number PZN conditions):<br>(Section 4.1.2.4 and Table 4-6) | CN3                                                                                                                                                                                                                                    | 92                       | CN3                                      | 92          |
|                                                                                                                                | CN3 = <u>92</u>                                                                                                                                                                                                                        |                          | Final CN Used in Analysis - Interpolated |             |
| <b>Watershed Information:</b>                                                                                                  |                                                                                                                                                                                                                                        |                          |                                          |             |
| Watershed Length (L) (Section 4.3.1):                                                                                          | 1.45                                                                                                                                                                                                                                   | miles                    |                                          |             |
| Length to Centroid (Lc) (Section 4.3.1):                                                                                       | 0.725                                                                                                                                                                                                                                  | miles                    |                                          |             |
| U/S Elev:                                                                                                                      | 450                                                                                                                                                                                                                                    | feet                     |                                          |             |
| Slope (s) (Section 4.3.1):                                                                                                     | 26                                                                                                                                                                                                                                     | ft/mile                  |                                          |             |
| Basin n Factor (Section 4.3.5):                                                                                                | 0.06                                                                                                                                                                                                                                   |                          |                                          |             |
| <b>Lag Time Calculations:</b>                                                                                                  |                                                                                                                                                                                                                                        |                          |                                          |             |
| Corps lag (T <sub>L</sub> ) = 24 n ((L x Lc)/s <sup>0.57</sup> ) <sup>0.38</sup> (Section 4.3.1.1):                            | 0.789                                                                                                                                                                                                                                  | hours                    |                                          |             |
| or                                                                                                                             |                                                                                                                                                                                                                                        |                          |                                          |             |
| Corps lag (T <sub>L</sub> ) = 0.8 T <sub>c</sub> (Section 4.3.1.2):                                                            |                                                                                                                                                                                                                                        |                          |                                          |             |
| Time to Peak = 0.862 x Corps lag (Section 4.1.5.5):                                                                            | 0.680                                                                                                                                                                                                                                  | hours                    |                                          |             |
| Time Step:                                                                                                                     | 5                                                                                                                                                                                                                                      | minutes                  |                                          |             |
| NRCS lag (T <sub>L</sub> ):                                                                                                    | 0.639                                                                                                                                                                                                                                  | hours                    |                                          |             |
| Basin CN2 Calculation - PZN 2 Table 4-2                                                                                        |                                                                                                                                                                                                                                        |                          |                                          |             |
| Land Use Type-Cover-Hydrologic Condition (As Applicable)                                                                       | Land Use Area<br>(sq mi)                                                                                                                                                                                                               | Percent of Total<br>Area | CN                                       | Weighted CN |
| Open Brush, Poor (NRCS A Soil)                                                                                                 | 0.110                                                                                                                                                                                                                                  | 10%                      | 62                                       | 6.2         |
| Open Brush, Poor (NRCS B Soil)                                                                                                 | 0.170                                                                                                                                                                                                                                  | 15%                      | 76                                       | 11.4        |
| Open Brush, Poor (NRCS C Soil)                                                                                                 | 0.830                                                                                                                                                                                                                                  | 75%                      | 84                                       | 63          |
| Open Brush, Poor (NRCS D Soil)                                                                                                 | 0.000                                                                                                                                                                                                                                  |                          | 88                                       | 0           |
| <b>Total</b>                                                                                                                   | 1.10                                                                                                                                                                                                                                   | 100%                     | Composite                                | 81          |



## HEC-1 Output

RGCrk.OUT

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 13JUN18 TIME 16:23:50
*
*****
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

```

X   X   XXXXXXX   XXXXX   X
X   X   X   X   X   X   XX
X   X   X   X   X   X   X
XXXXXXX XXXX   X   XXXXX   X
X   X   X   X   X   X   X
X   X   X   X   X   X   X
X   X   XXXXXXX   XXXXX   XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.  
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*** FREE ***
1 ID RANCHO GUEJITO CREEK BASINS 100 THROUGH 400 - 100-YEAR HEC-1 RUN
2 IT 5 01JAN90 1200 300
3 IO 1 2
4 KKBASIN100RUNOFF
5 KM NESTED STORM PER COUNTY OF SAN DIEGO HYDROLOGY MANUAL
6 KM COPYRIGHT 2003 RICK ENGINEERING COMPANY
7 KM FOR THIS DATA TO BE USED PROPERLYIN HEC-1,
8 KM THE 1DATE1 VARIABLE MUST BE 01JAN90 AND THE 1TIME1
9 KM VARIABLE MUST BE 1200 ON THE IT CARD.
10 KM THE FOLLOWING IT CARD IS SUGGESTED TO BE USED IN
11 KM YOUR DATA SET:
12 KM IT 5 01JAN90 1200 300
13 KM 6HR RAINFALL IS 3.9 INCHES
14 KM 24HR RAINFALL IS 8.75 INCHES
15 KM DAR30 = .783246
16 KM DAR60 = .867702
17 KM DAR180 = .93662
18 KM DAR360 = .953003
19 KM DAR1440 = .966541
20 KM BASIN AREA IS 27.69 SQUARE MILES
21 IN 5 01JAN90 1200 300
22 PI .01741 .01746 .01748 .01753 .01756 .01761 .01764 .01769 .01772 .0177
23 PI .0178 .01785 .01788 .01793 .01796 .01801 .01804 .0181 .01813 .01818
24 PI .01821 .01827 .0183 .01836 .01839 .01845 .01848 .01854 .01857 .01864
25 PI .01867 .01873 .01876 .01883 .01886 .01893 .01896 .01903 .01906 .0191
26 PI .01916 .01923 .01927 .01934 .01937 .01945 .01948 .01956 .01959 .0196
27 PI .01971 .01978 .01982 .0199 .01994 .02002 .02006 .02014 .02018 .02027
28 PI .02031 .02039 .02044 .02052 .02057 .02066 .0207 .02079 .02084 .02093
29 PI .02098 .02107 .02112 .02122 .02127 .02137 .02142 .02152 .02157 .0216
30 PI .02173 .02184 .02189 .022 .02206 .02217 .02223 .02234 .0224 .02252
31 PI .02258 .02271 .02277 .02289 .02296 .02309 .02316 .02329 .02336 .0235
32 PI .02357 .02371 .02378 .02393 .024 .02415 .02423 .02439 .02447 .02463
33 PI .02471 .02488 .02497 .02514 .02523 .02541 .0255 .02569 .02579 .02598
34 PI .02608 .02628 .02639 .0266 .0267 .02692 .02704 .02727 .02738 .02763
35 PI .02775 .028 .02813 .02839 .02853 .0288 .02895 .02924 .02939 .0297
36 PI .02985 .03018 .03034 .02022 .02037 .02068 .02084 .02118 .02135 .0217
37 PI .02191 .02231 .02251 .02295 .02317 .02365 .02389 .02442 .02469 .0252
38 PI .02557 .02622 .02656 .02728 .02767 .02848 .02892 .03717 .03752 .0382
39 PI .03871 .03963 .04014 .04126 .04188 .04328 .04406 .04584 .04685 .0491
40 PI .05055 .05376 .05567 .08284 .08426 .08836 .09135 .07941 .09044 .1328
41 PI .18711 .67069 .10651 .09531 .08606 .05785 .05206 .04797 .04491 .0425
42 PI .04068 .03915 .03789 .02938 .02807 .02691 .02589 .02497 .02415 .0234
43 PI .02273 .0221 .02153 .02101 .02052 .03052 .03001 .02954 .02909 .02866
44 PI .02826 .02787 .0275 .02715 .02681 .02649 .02618 .02588 .0256 .02532
45 PI .02505 .0248 .02455 .02431 .02408 .02385 .02364 .02343 .02322 .02302
46 PI .02283 .02264 .02246 .02229 .02211 .02195 .02178 .02162 .02147 .0213
47 PI .02117 .02102 .02088 .02075 .02061 .02048 .02035 .02022 .0201 .01998
48 PI .01986 .01974 .01963 .01952 .01941 .0193 .0192 .01909 .01899 .01889
49 PI .0188 .0187 .0186 .01851 .01842 .01833 .01824 .01816 .01807 .01799
50 PI .0179 .01782 .01774 .01766 .01758 .01751 .01743 0 0 0
51 PI 0 0 0 0 0 0 0 0 0 0
52 PI 0 0 0 0 0 0 0 0 0 0
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1 HEC-1 INPUT PAGE 2

```
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
53 BA 22.50
54 LS 0 93
55 UD 1.956
56 KKBASIN200RUNOFF
57 KM NESTED STORM PER COUNTY OF SAN DIEGO HYDROLOGY MANUAL
58 KM COPYRIGHT 2003 RICK ENGINEERING COMPANY
59 KM FOR THIS DATA TO BE USED PROPERLYIN HEC-1,
60 KM THE 1DATE1 VARIABLE MUST BE 01JAN90 AND THE 1TIME1
61 KM VARIABLE MUST BE 1200 ON THE IT CARD.
62 KM THE FOLLOWING IT CARD IS SUGGESTED TO BE USED IN
63 KM YOUR DATA SET:
64 KM IT 5 01JAN90 1200 300
65 KM 6HR RAINFALL IS 3.65 INCHES
66 KM 24HR RAINFALL IS 7.5 INCHES
67 KM DAR30 = .783246
68 KM DAR60 = .867702
69 KM DAR180 = .93662
70 KM DAR360 = .953003
71 KM DAR1440 = .966541
72 KM BASIN AREA IS 27.69 SQUARE MILES
```



RGCrk.OUT

|     |    |        |        |        |        |        |        |        |        |        |        |
|-----|----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 181 | PI | .00847 | .00852 | .00854 | .00859 | .00861 | .00865 | .00868 | .00872 | .00875 | .0088  |
| 182 | PI | .00882 | .00887 | .00889 | .00894 | .00897 | .00902 | .00905 | .0091  | .00912 | .00918 |
| 183 | PI | .0092  | .00926 | .00929 | .00934 | .00937 | .00943 | .00946 | .00952 | .00955 | .00961 |
| 184 | PI | .00964 | .0097  | .00973 | .00979 | .00982 | .00989 | .00992 | .00999 | .01002 | .01009 |
| 185 | PI | .01012 | .0102  | .01023 | .0103  | .01034 | .01041 | .01045 | .01053 | .01057 | .01064 |
| 186 | PI | .01068 | .01076 | .01081 | .01089 | .01093 | .01102 | .01106 | .01115 | .01119 | .0112  |
| 187 | PI | .01133 | .01143 | .01147 | .01157 | .01162 | .01172 | .01177 | .01188 | .01193 | .0120  |
| 188 | PI | .01209 | .0122  | .01226 | .01238 | .01243 | .01255 | .01262 | .01274 | .0128  | .01293 |
| 189 | PI | .013   | .01314 | .0132  | .01334 | .01342 | .01356 | .01364 | .01379 | .01387 | .01403 |
| 190 | PI | .01411 | .01428 | .01437 | .01454 | .01463 | .01482 | .01491 | .0151  | .0152  | .01541 |
| 191 | PI | .01551 | .01573 | .01584 | .01606 | .01618 | .01642 | .01654 | .0168  | .01693 | .0172  |
| 192 | PI | .01734 | .01763 | .01778 | .01866 | .0188  | .01909 | .01924 | .01955 | .01971 | .02005 |
| 193 | PI | .02022 | .02059 | .02078 | .02118 | .02139 | .02183 | .02206 | .02254 | .02279 | .0233  |
| 194 | PI | .02361 | .0242  | .02452 | .02518 | .02554 | .02629 | .0267  | .03431 | .03463 | .03534 |
| 195 | PI | .03573 | .03658 | .03705 | .03809 | .03866 | .03995 | .04067 | .04232 | .04325 | .0454  |
| 196 | PI | .04666 | .04962 | .05139 | .07647 | .07778 | .08157 | .08432 | .0733  | .08349 | .12258 |
| 197 | PI | .17272 | .61909 | .09832 | .08798 | .07944 | .0534  | .04806 | .04428 | .04146 | .03928 |
| 198 | PI | .03755 | .03614 | .03498 | .02712 | .02591 | .02484 | .0239  | .02305 | .02229 | .02161 |
| 199 | PI | .02098 | .0204  | .01988 | .01939 | .01894 | .01793 | .01748 | .01707 | .01667 | .0163  |
| 200 | PI | .01595 | .01562 | .0153  | .01501 | .01472 | .01445 | .0142  | .01395 | .01371 | .01349 |
| 201 | PI | .01327 | .01307 | .01287 | .01268 | .01249 | .01232 | .01215 | .01198 | .01182 | .0116  |
| 202 | PI | .01152 | .01138 | .01124 | .0111  | .01097 | .01085 | .01072 | .0106  | .01049 | .01038 |
| 203 | PI | .01027 | .01016 | .01006 | .00995 | .00986 | .00976 | .00967 | .00958 | .00949 | .0094  |
| 204 | PI | .00931 | .00923 | .00915 | .00907 | .00899 | .00892 | .00884 | .00877 | .0087  | .00863 |
| 205 | PI | .00856 | .0085  | .00843 | .00837 | .0083  | .00824 | .00818 | .00812 | .00807 | .00801 |
| 206 | PI | .00795 | .0079  | .00784 | .00779 | .00774 | .00769 | .00764 | 0      | 0      | 0      |
| 207 | PI | 0      | 0      | 0      | 0      | 0      | 0      | 0      | 0      | 0      | 0      |
| 208 | PI | 0      | 0      |        |        |        |        |        |        |        |        |
| 209 | BA | 1.1    |        |        |        |        |        |        |        |        |        |
| 210 | LS | 0      | 92     |        |        |        |        |        |        |        |        |
| 211 | UD | 0.639  |        |        |        |        |        |        |        |        |        |

1 HEC-1 INPUT PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

212 KKCONFLUENCE\_100\_THROUGH\_400  
213 HC 4  
214 ZZ

1 SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW

NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

4 BASIN100
.
.
56 BASIN200
.
.
108 BASIN300
.
.
160 BASIN400
.
.
212 CONFLUENCE

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

1 \*\*\*\*\*

\* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*

\* JUN 1998 \*

\* VERSION 4.1 \*

\* RUN DATE 13JUN18 TIME 16:23:50 \*

\* \*\*\*\*\*

\*\*\*\*\*

\* U.S. ARMY CORPS OF ENGINEERS \*

\* HYDROLOGIC ENGINEERING CENTER \*

\* 609 SECOND STREET \*

\* DAVIS, CALIFORNIA 95616 \*

\* (916) 756-1104 \*

\* \*\*\*\*\*

RANCHO GUEJITO CREEK BASINS 100 THROUGH 400 - 100-YEAR HEC-1 RUN

3 IO OUTPUT CONTROL VARIABLES

IPRNT 1 PRINT CONTROL

IPLOT 2 PLOT CONTROL

QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL

IDATE 1JAN90 STARTING DATE

ITIME 1200 STARTING TIME

NQ 300 NUMBER OF HYDROGRAPH ORDINATES

NDDATE 2JAN90 ENDING DATE

NDTIME 1255 ENDING TIME

ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS

TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES

PRECIPITATION DEPTH INCHES

LENGTH, ELEVATION FEET

FLOW CUBIC FEET PER SECOND

STORAGE VOLUME ACRE-Feet

SURFACE AREA ACRES

TEMPERATURE DEGREES FAHRENHEIT

\*\*\* \*\*

4 KK BASIN100 RUNOFF

\*\*\*\*\*

NESTED STORM PER COUNTY OF SAN DIEGO HYDROLOGY MANUAL

COPYRIGHT 2003 RICK ENGINEERING COMPANY

FOR THIS DATA TO BE USED PROPERLY IN HEC-1,

THE 1DATE1 VARIABLE MUST BE 01JAN90 AND THE 1TIME1

VARIABLE MUST BE 1200 ON THE IT CARD.

BASIN AREA IS 27.69 SQUARE MILES

Page 4



|   |     |      |     |     |     |     |       |   |   |           |      |     |     |     |     |        |
|---|-----|------|-----|-----|-----|-----|-------|---|---|-----------|------|-----|-----|-----|-----|--------|
|   |     |      |     |     |     |     |       |   |   | RGCrk.OUT |      |     |     |     |     |        |
| 1 | JAN | 1420 | 29  | .02 | .01 | .01 | 159.  | * | 2 | JAN       | 0250 | 179 | .05 | .00 | .04 | 4305.  |
| 1 | JAN | 1425 | 30  | .02 | .01 | .01 | 188.  | * | 2 | JAN       | 0255 | 180 | .05 | .00 | .05 | 4346.  |
| 1 | JAN | 1430 | 31  | .02 | .01 | .01 | 219.  | * | 2 | JAN       | 0300 | 181 | .05 | .00 | .05 | 4396.  |
| 1 | JAN | 1435 | 32  | .02 | .01 | .01 | 253.  | * | 2 | JAN       | 0305 | 182 | .05 | .00 | .05 | 4454.  |
| 1 | JAN | 1440 | 33  | .02 | .01 | .01 | 289.  | * | 2 | JAN       | 0310 | 183 | .05 | .00 | .05 | 4521.  |
| 1 | JAN | 1445 | 34  | .02 | .01 | .01 | 328.  | * | 2 | JAN       | 0315 | 184 | .06 | .00 | .05 | 4596.  |
| 1 | JAN | 1450 | 35  | .02 | .01 | .01 | 369.  | * | 2 | JAN       | 0320 | 185 | .08 | .00 | .08 | 4680.  |
| 1 | JAN | 1455 | 36  | .02 | .01 | .01 | 413.  | * | 2 | JAN       | 0325 | 186 | .08 | .00 | .08 | 4774.  |
| 1 | JAN | 1500 | 37  | .02 | .01 | .01 | 458.  | * | 2 | JAN       | 0330 | 187 | .09 | .00 | .09 | 4878.  |
| 1 | JAN | 1505 | 38  | .02 | .01 | .01 | 505.  | * | 2 | JAN       | 0335 | 188 | .09 | .00 | .09 | 4994.  |
| 1 | JAN | 1510 | 39  | .02 | .01 | .01 | 554.  | * | 2 | JAN       | 0340 | 189 | .08 | .00 | .08 | 5121.  |
| 1 | JAN | 1515 | 40  | .02 | .01 | .01 | 604.  | * | 2 | JAN       | 0345 | 190 | .09 | .00 | .09 | 5261.  |
| 1 | JAN | 1520 | 41  | .02 | .01 | .01 | 656.  | * | 2 | JAN       | 0350 | 191 | .13 | .00 | .13 | 5415.  |
| 1 | JAN | 1525 | 42  | .02 | .01 | .01 | 709.  | * | 2 | JAN       | 0355 | 192 | .19 | .00 | .18 | 5588.  |
| 1 | JAN | 1530 | 43  | .02 | .01 | .01 | 763.  | * | 2 | JAN       | 0400 | 193 | .67 | .01 | .66 | 5817.  |
| 1 | JAN | 1535 | 44  | .02 | .01 | .01 | 817.  | * | 2 | JAN       | 0405 | 194 | .11 | .00 | .11 | 6068.  |
| 1 | JAN | 1540 | 45  | .02 | .01 | .01 | 872.  | * | 2 | JAN       | 0410 | 195 | .10 | .00 | .09 | 6369.  |
| 1 | JAN | 1545 | 46  | .02 | .01 | .01 | 927.  | * | 2 | JAN       | 0415 | 196 | .09 | .00 | .08 | 6709.  |
| 1 | JAN | 1550 | 47  | .02 | .01 | .01 | 983.  | * | 2 | JAN       | 0420 | 197 | .06 | .00 | .06 | 7072.  |
| 1 | JAN | 1555 | 48  | .02 | .01 | .01 | 1038. | * | 2 | JAN       | 0425 | 198 | .05 | .00 | .05 | 7471.  |
| 1 | JAN | 1600 | 49  | .02 | .01 | .01 | 1093. | * | 2 | JAN       | 0430 | 199 | .05 | .00 | .05 | 7885.  |
| 1 | JAN | 1605 | 50  | .02 | .00 | .01 | 1148. | * | 2 | JAN       | 0435 | 200 | .04 | .00 | .04 | 8340.  |
| 1 | JAN | 1610 | 51  | .02 | .00 | .01 | 1203. | * | 2 | JAN       | 0440 | 201 | .04 | .00 | .04 | 8812.  |
| 1 | JAN | 1615 | 52  | .02 | .00 | .01 | 1256. | * | 2 | JAN       | 0445 | 202 | .04 | .00 | .04 | 9309.  |
| 1 | JAN | 1620 | 53  | .02 | .00 | .02 | 1310. | * | 2 | JAN       | 0450 | 203 | .04 | .00 | .04 | 9839.  |
| 1 | JAN | 1625 | 54  | .02 | .00 | .02 | 1362. | * | 2 | JAN       | 0455 | 204 | .04 | .00 | .04 | 10367. |
| 1 | JAN | 1630 | 55  | .02 | .00 | .02 | 1414. | * | 2 | JAN       | 0500 | 205 | .03 | .00 | .03 | 10921. |
| 1 | JAN | 1635 | 56  | .02 | .00 | .02 | 1465. | * | 2 | JAN       | 0505 | 206 | .03 | .00 | .03 | 11457. |
| 1 | JAN | 1640 | 57  | .02 | .00 | .02 | 1515. | * | 2 | JAN       | 0510 | 207 | .03 | .00 | .03 | 11944. |
| 1 | JAN | 1645 | 58  | .02 | .00 | .02 | 1565. | * | 2 | JAN       | 0515 | 208 | .03 | .00 | .03 | 12388. |
| 1 | JAN | 1650 | 59  | .02 | .00 | .02 | 1613. | * | 2 | JAN       | 0520 | 209 | .02 | .00 | .02 | 12781. |
| 1 | JAN | 1655 | 60  | .02 | .00 | .02 | 1661. | * | 2 | JAN       | 0525 | 210 | .02 | .00 | .02 | 13092. |
| 1 | JAN | 1700 | 61  | .02 | .00 | .02 | 1708. | * | 2 | JAN       | 0530 | 211 | .02 | .00 | .02 | 13362. |
| 1 | JAN | 1705 | 62  | .02 | .00 | .02 | 1754. | * | 2 | JAN       | 0535 | 212 | .02 | .00 | .02 | 13544. |
| 1 | JAN | 1710 | 63  | .02 | .00 | .02 | 1800. | * | 2 | JAN       | 0540 | 213 | .02 | .00 | .02 | 13674. |
| 1 | JAN | 1715 | 64  | .02 | .00 | .02 | 1844. | * | 2 | JAN       | 0545 | 214 | .02 | .00 | .02 | 13740. |
| 1 | JAN | 1720 | 65  | .02 | .00 | .02 | 1888. | * | 2 | JAN       | 0550 | 215 | .02 | .00 | .02 | 13737. |
| 1 | JAN | 1725 | 66  | .02 | .00 | .02 | 1931. | * | 2 | JAN       | 0555 | 216 | .02 | .00 | .02 | 13702. |
| 1 | JAN | 1730 | 67  | .02 | .00 | .02 | 1973. | * | 2 | JAN       | 0600 | 217 | .03 | .00 | .03 | 13612. |
| 1 | JAN | 1735 | 68  | .02 | .00 | .02 | 2014. | * | 2 | JAN       | 0605 | 218 | .03 | .00 | .03 | 13490. |
| 1 | JAN | 1740 | 69  | .02 | .00 | .02 | 2055. | * | 2 | JAN       | 0610 | 219 | .03 | .00 | .03 | 13300. |
| 1 | JAN | 1745 | 70  | .02 | .00 | .02 | 2094. | * | 2 | JAN       | 0615 | 220 | .03 | .00 | .03 | 13065. |
| 1 | JAN | 1750 | 71  | .02 | .00 | .02 | 2133. | * | 2 | JAN       | 0620 | 221 | .03 | .00 | .03 | 12805. |
| 1 | JAN | 1755 | 72  | .02 | .00 | .02 | 2172. | * | 2 | JAN       | 0625 | 222 | .03 | .00 | .03 | 12519. |
| 1 | JAN | 1800 | 73  | .02 | .00 | .02 | 2209. | * | 2 | JAN       | 0630 | 223 | .03 | .00 | .03 | 12217. |
| 1 | JAN | 1805 | 74  | .02 | .00 | .02 | 2246. | * | 2 | JAN       | 0635 | 224 | .03 | .00 | .03 | 11895. |
| 1 | JAN | 1810 | 75  | .02 | .00 | .02 | 2282. | * | 2 | JAN       | 0640 | 225 | .03 | .00 | .03 | 11561. |
| 1 | JAN | 1815 | 76  | .02 | .00 | .02 | 2317. | * | 2 | JAN       | 0645 | 226 | .03 | .00 | .03 | 11211. |
| 1 | JAN | 1820 | 77  | .02 | .00 | .02 | 2352. | * | 2 | JAN       | 0650 | 227 | .03 | .00 | .03 | 10842. |
| 1 | JAN | 1825 | 78  | .02 | .00 | .02 | 2386. | * | 2 | JAN       | 0655 | 228 | .03 | .00 | .03 | 10469. |
| 1 | JAN | 1830 | 79  | .02 | .00 | .02 | 2420. | * | 2 | JAN       | 0700 | 229 | .03 | .00 | .03 | 10076. |
| 1 | JAN | 1835 | 80  | .02 | .00 | .02 | 2453. | * | 2 | JAN       | 0705 | 230 | .03 | .00 | .03 | 9690.  |
| 1 | JAN | 1840 | 81  | .02 | .00 | .02 | 2485. | * | 2 | JAN       | 0710 | 231 | .03 | .00 | .03 | 9329.  |
| 1 | JAN | 1845 | 82  | .02 | .00 | .02 | 2517. | * | 2 | JAN       | 0715 | 232 | .03 | .00 | .02 | 8991.  |
| 1 | JAN | 1850 | 83  | .02 | .00 | .02 | 2549. | * | 2 | JAN       | 0720 | 233 | .02 | .00 | .02 | 8677.  |
| 1 | JAN | 1855 | 84  | .02 | .00 | .02 | 2579. | * | 2 | JAN       | 0725 | 234 | .02 | .00 | .02 | 8405.  |
| 1 | JAN | 1900 | 85  | .02 | .00 | .02 | 2610. | * | 2 | JAN       | 0730 | 235 | .02 | .00 | .02 | 8146.  |
| 1 | JAN | 1905 | 86  | .02 | .00 | .02 | 2640. | * | 2 | JAN       | 0735 | 236 | .02 | .00 | .02 | 7908.  |
| 1 | JAN | 1910 | 87  | .02 | .00 | .02 | 2669. | * | 2 | JAN       | 0740 | 237 | .02 | .00 | .02 | 7683.  |
| 1 | JAN | 1915 | 88  | .02 | .00 | .02 | 2698. | * | 2 | JAN       | 0745 | 238 | .02 | .00 | .02 | 7473.  |
| 1 | JAN | 1920 | 89  | .02 | .00 | .02 | 2727. | * | 2 | JAN       | 0750 | 239 | .02 | .00 | .02 | 7279.  |
| 1 | JAN | 1925 | 90  | .02 | .00 | .02 | 2755. | * | 2 | JAN       | 0755 | 240 | .02 | .00 | .02 | 7096.  |
| 1 | JAN | 1930 | 91  | .02 | .00 | .02 | 2782. | * | 2 | JAN       | 0800 | 241 | .02 | .00 | .02 | 6936.  |
| 1 | JAN | 1935 | 92  | .02 | .00 | .02 | 2810. | * | 2 | JAN       | 0805 | 242 | .02 | .00 | .02 | 6782.  |
| 1 | JAN | 1940 | 93  | .02 | .00 | .02 | 2837. | * | 2 | JAN       | 0810 | 243 | .02 | .00 | .02 | 6634.  |
| 1 | JAN | 1945 | 94  | .02 | .00 | .02 | 2864. | * | 2 | JAN       | 0815 | 244 | .02 | .00 | .02 | 6492.  |
| 1 | JAN | 1950 | 95  | .02 | .00 | .02 | 2890. | * | 2 | JAN       | 0820 | 245 | .02 | .00 | .02 | 6357.  |
| 1 | JAN | 1955 | 96  | .02 | .00 | .02 | 2916. | * | 2 | JAN       | 0825 | 246 | .02 | .00 | .02 | 6231.  |
| 1 | JAN | 2000 | 97  | .02 | .00 | .02 | 2942. | * | 2 | JAN       | 0830 | 247 | .02 | .00 | .02 | 6108.  |
| 1 | JAN | 2005 | 98  | .02 | .00 | .02 | 2968. | * | 2 | JAN       | 0835 | 248 | .02 | .00 | .02 | 5989.  |
| 1 | JAN | 2010 | 99  | .02 | .00 | .02 | 2993. | * | 2 | JAN       | 0840 | 249 | .02 | .00 | .02 | 5875.  |
| 1 | JAN | 2015 | 100 | .02 | .00 | .02 | 3018. | * | 2 | JAN       | 0845 | 250 | .02 | .00 | .02 | 5769.  |
| 1 | JAN | 2020 | 101 | .02 | .00 | .02 | 3043. | * | 2 | JAN       | 0850 | 251 | .02 | .00 | .02 | 5673.  |
| 1 | JAN | 2025 | 102 | .02 | .00 | .02 | 3068. | * | 2 | JAN       | 0855 | 252 | .02 | .00 | .02 | 5578.  |
| 1 | JAN | 2030 | 103 | .02 | .00 | .02 | 3092. | * | 2 | JAN       | 0900 | 253 | .02 | .00 | .02 | 5485.  |
| 1 | JAN | 2035 | 104 | .02 | .00 | .02 | 3117. | * | 2 | JAN       | 0905 | 254 | .02 | .00 | .02 | 5394.  |
| 1 | JAN | 2040 | 105 | .02 | .00 | .02 | 3141. | * | 2 | JAN       | 0910 | 255 | .02 | .00 | .02 | 5309.  |
| 1 | JAN | 2045 | 106 | .02 | .00 | .02 | 3165. | * | 2 | JAN       | 0915 | 256 | .02 | .00 | .02 | 5226.  |
| 1 | JAN | 2050 | 107 | .02 | .00 | .02 | 3189. | * | 2 | JAN       | 0920 | 257 | .02 | .00 | .02 | 5145.  |
| 1 | JAN | 2055 | 108 | .02 | .00 | .02 | 3213. | * | 2 | JAN       | 0925 | 258 | .02 | .00 | .02 | 5066.  |
| 1 | JAN | 2100 | 109 | .02 | .00 | .02 | 3236. | * | 2 | JAN       | 0930 | 259 | .02 | .00 | .02 | 4989.  |
| 1 | JAN | 2105 | 110 | .02 | .00 | .02 | 3260. | * | 2 | JAN       | 0935 | 260 | .02 | .00 | .02 | 4918.  |
| 1 | JAN | 2110 | 111 | .02 | .00 | .02 | 3283. | * | 2 | JAN       | 0940 | 261 | .02 | .00 | .02 | 4849.  |
| 1 | JAN | 2115 | 112 | .02 | .00 | .02 | 3307. | * | 2 | JAN       | 0945 | 262 | .02 | .00 | .02 | 4781.  |
| 1 | JAN | 2120 | 113 | .02 | .00 | .02 | 3330. | * | 2 | JAN       | 0950 | 263 | .02 | .00 | .02 | 4716.  |
| 1 | JAN | 2125 | 114 | .02 | .00 | .02 | 3354. | * | 2 | JAN       | 0955 | 264 | .02 | .00 | .02 | 4652.  |
| 1 | JAN | 2130 | 115 | .03 | .00 | .02 | 3377. | * | 2 | JAN       | 1000 | 265 | .02 | .00 | .02 | 4594.  |
| 1 | JAN | 2135 | 116 | .03 | .00 | .02 | 3400. | * | 2 | JAN       | 1005 | 266 | .02 | .00 | .02 | 4538.  |
| 1 | JAN | 2140 | 117 | .03 | .00 | .02 | 3424. | * | 2 | JAN       | 1010 | 267 | .02 | .00 | .02 | 4483.  |
| 1 | JAN | 2145 | 118 | .03 | .00 | .02 | 3447. | * | 2 | JAN       | 1015 | 268 | .02 | .00 | .02 | 4429.  |
| 1 | JAN | 2150 | 119 | .03 | .00 | .02 | 3470. | * | 2 | JAN       | 1020 | 269 | .02 | .00 | .02 | 4378.  |
| 1 | JAN | 2155 | 120 | .03 | .00 | .02 | 3494. | * | 2 | JAN       | 1025 | 270 | .02 | .00 | .02 | 4329.  |
| 1 | JAN | 2200 | 121 | .03 | .00 | .02 | 3517. | * | 2 | JAN       | 1030 | 271 | .02 | .00 | .02 | 4282.  |
| 1 | JAN | 2205 | 122 | .03 | .00 | .02 | 3541. | * | 2 | JAN       | 1035 | 272 | .02 | .00 | .02 | 4235.  |
| 1 | JAN | 2210 | 123 | .03 | .00 | .02 | 3564. | * | 2 | JAN       | 1040 | 273 | .02 | .00 | .02 | 4190.  |
| 1 | JAN | 2215 | 124 | .03 | .00 | .02 | 3588. | * | 2 | JAN       | 1045 | 274 | .02 | .00 | .02 | 4146.  |
| 1 | JAN | 2220 | 125 | .03 | .00 | .03 | 3611. | * | 2 | JAN       | 1050 | 275 | .02 | .00 | .02 | 4105.  |
| 1 | JAN | 2225 | 126 | .03 | .00 | .03 | 3635. | * | 2 |           |      |     |     |     |     |        |

|            |     |     |     |     |       |   |           |            |     |     |     |     |       |
|------------|-----|-----|-----|-----|-------|---|-----------|------------|-----|-----|-----|-----|-------|
| 2 JAN 0010 | 147 | .02 | .00 | .02 | 4174. | * | RGCrk.OUT | 2 JAN 1240 | 297 | .00 | .00 | .00 | 3332. |
| 2 JAN 0015 | 148 | .02 | .00 | .02 | 4199. | * |           | 2 JAN 1245 | 298 | .00 | .00 | .00 | 3278. |
| 2 JAN 0020 | 149 | .02 | .00 | .02 | 4223. | * |           | 2 JAN 1250 | 299 | .00 | .00 | .00 | 3218. |
| 2 JAN 0025 | 150 | .02 | .00 | .02 | 4245. | * |           | 2 JAN 1255 | 300 | .00 | .00 | .00 | 3152. |

\*\*\*\*\*

TOTAL RAINFALL = 8.44, TOTAL LOSS = .84, TOTAL EXCESS = 7.60

| PEAK FLOW | TIME   |       | 6-HR     | MAXIMUM AVERAGE FLOW |       |
|-----------|--------|-------|----------|----------------------|-------|
| +         | (CFS)  | (HR)  | (CFS)    | 24-HR                | 72-HR |
| +         | 13740. | 17.75 | 8799.    | 4383.                | 4222. |
|           |        |       | (INCHES) | 7.245                | 7.245 |
|           |        |       | (AC-FT)  | 8694.                | 8694. |

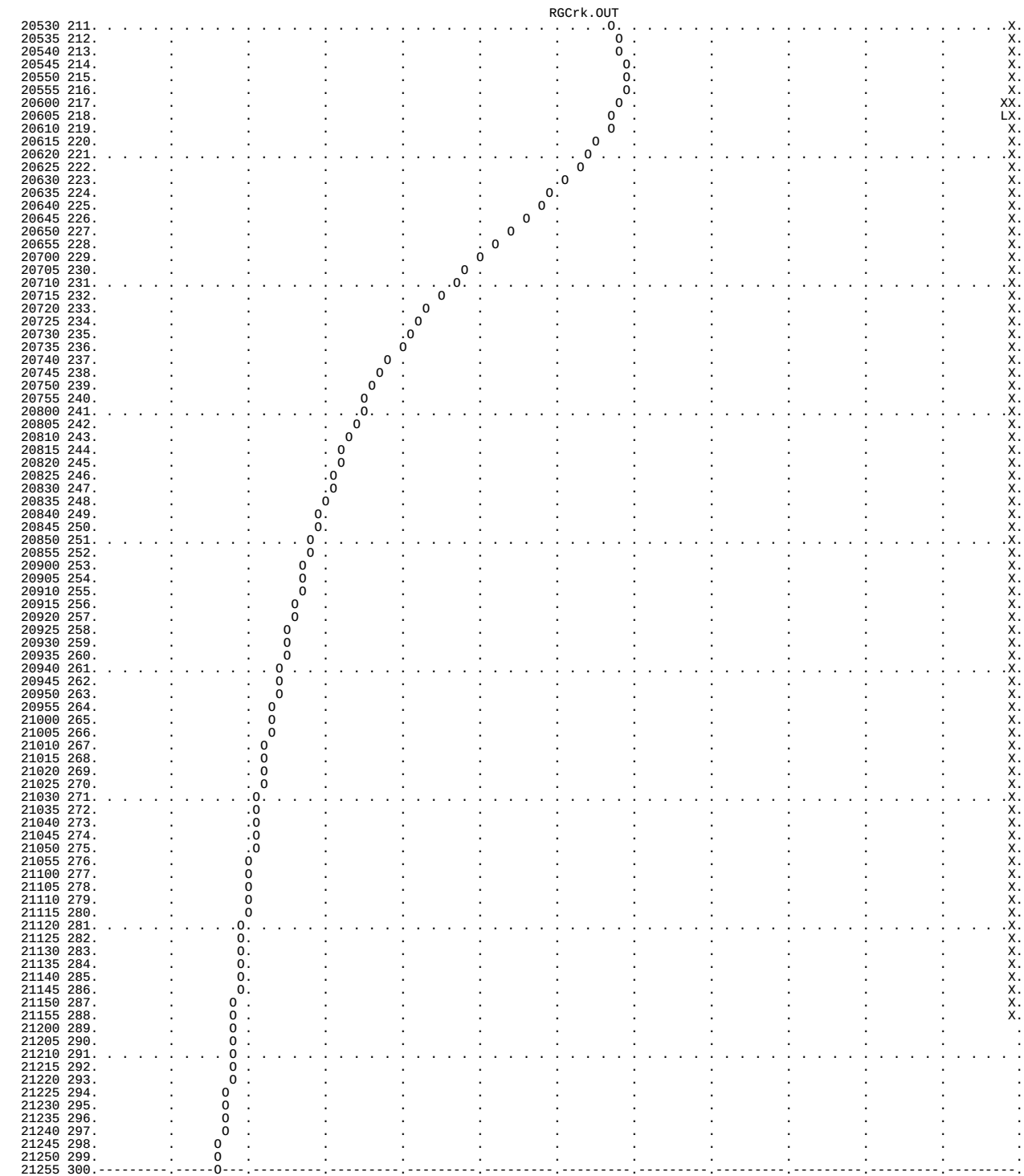
CUMULATIVE AREA = 22.50 SQ MI

1

STATION BASIN100

|        |      | (O) OUTFLOW |       |       |       |       |        |        |        |    |    |             |            |    |    |
|--------|------|-------------|-------|-------|-------|-------|--------|--------|--------|----|----|-------------|------------|----|----|
|        |      | 0.          | 2000. | 4000. | 6000. | 8000. | 10000. | 12000. | 14000. | 0. | 0. | (L) PRECIP, | (X) EXCESS | 0. | 0. |
| DAHRMN | PER  | .0          | .0    | .0    | .0    | .0    | .0     | .0     | .0     | .8 | .6 | .4          | .2         | .0 | .0 |
| 11200  | 10   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11205  | 20   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11210  | 30   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11215  | 40   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11220  | 50   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11225  | 60   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11230  | 70   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11235  | 80   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11240  | 90   | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11245  | 100  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11250  | 110  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11255  | 120  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11300  | 130  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11305  | 140  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11310  | 150  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11315  | 160  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11320  | 170  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11325  | 180  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11330  | 190  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11335  | 200  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11340  | 210  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11345  | 220  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11350  | 230  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11355  | 240  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11400  | 250  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11405  | 260  | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11410  | 27.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11415  | 28.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11420  | 29.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | L. |
| 11425  | 30.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11430  | 31.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11435  | 32.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11440  | 33.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11445  | 34.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11450  | 35.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11455  | 36.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11500  | 37.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11505  | 38.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11510  | 39.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11515  | 40.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11520  | 41.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11525  | 42.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11530  | 43.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11535  | 44.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11540  | 45.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11545  | 46.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11550  | 47.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11555  | 48.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11600  | 49.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11605  | 50.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11610  | 51.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11615  | 52.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11620  | 53.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11625  | 54.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11630  | 55.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11635  | 56.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11640  | 57.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11645  | 58.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11650  | 59.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11655  | 60.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11700  | 61.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11705  | 62.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11710  | 63.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11715  | 64.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11720  | 65.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11725  | 66.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11730  | 67.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11735  | 68.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11740  | 69.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11745  | 70.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11750  | 71.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11755  | 72.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11800  | 73.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11805  | 74.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11810  | 75.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11815  | 76.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11820  | 77.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11825  | 78.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11830  | 79.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11835  | 80.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11840  | 81.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11845  | 82.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11850  | 83.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11855  | 84.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11900  | 85.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11905  | 86.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11910  | 87.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11915  | 88.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11920  | 89.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11925  | 90.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11930  | 91.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |
| 11935  | 92.0 | .           | .     | .     | .     | .     | .      | .      | .      | .  | .  | .           | .          | .  | X. |

[illegible]



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56 KK \*\*\*\*\*  
 \* BASIN200 \* RUNOFF  
 \*

NESTED STORM PER COUNTY OF SAN DIEGO HYDROLOGY MANUAL  
 COPYRIGHT 2003 RICK ENGINEERING COMPANY  
 FOR THIS DATA TO BE USED PROPERLY IN HEC-1,  
 THE 1DATE1 VARIABLE MUST BE 01JAN90 AND THE 1TIME1  
 VARIABLE MUST BE 1200 ON THE IT CARD.  
 THE FOLLOWING IT CARD IS SUGGESTED TO BE USED IN  
 YOUR DATA SET:  
 IT 5 01JAN90 1200 300  
 6HR RAINFALL IS 3.65 INCHES  
 24HR RAINFALL IS 7.5 INCHES  
 DAR30 = .783246  
 DAR60 = .867702  
 DAR180 = .93662  
 DAR360 = .953003  
 DAR1440 = .966541  
 BASIN AREA IS 27.69 SQUARE MILES

|        |                          |                    |
|--------|--------------------------|--------------------|
| 105 BA | SUBBASIN CHARACTERISTICS |                    |
|        | TAREA                    | 3.03 SUBBASIN AREA |

|       |       |      |                           |
|-------|-------|------|---------------------------|
| 74 PB | STORM | 8.44 | BASIN TOTAL PRECIPITATION |
|-------|-------|------|---------------------------|

[illegible]

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107 UD          SCS DIMENSIONLESS UNITGRAPH
                  TLAG          .68  LAG

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UNIT HYDROGRAPH  
43 END-OF-PERIOD ORDINATES

|       |       |       |       |       |       |       |       |       |       |
|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 82.   | 256.  | 493.  | 820.  | 1236. | 1623. | 1884. | 2003. | 2012. | 1938. |
| 1786. | 1607. | 1383. | 1110. | 893.  | 739.  | 611.  | 514.  | 429.  | 357.  |
| 291.  | 244.  | 202.  | 168.  | 139.  | 113.  | 95.   | 79.   | 66.   | 55.   |
| 45.   | 38.   | 31.   | 26.   | 22.   | 19.   | 16.   | 13.   | 10.   | 8.    |
| 6.    | 3.    | 1.    |       |       |       |       |       |       |       |

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| DA | MON | HRMN | ORD | RAIN | LOSS | EXCESS | COMP Q | * | DA | MON | HRMN | ORD | RAIN | LOSS | EXCESS | COMP Q |
|----|-----|------|-----|------|------|--------|--------|---|----|-----|------|-----|------|------|--------|--------|
| 1  | JAN | 1200 | 1   | .00  | .00  | .00    | 0.     | * | 2  | JAN | 0030 | 151 | .02  | .00  | .02    | 586.   |
| 1  | JAN | 1205 | 2   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0035 | 152 | .02  | .00  | .02    | 578.   |
| 1  | JAN | 1210 | 3   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0040 | 153 | .02  | .00  | .02    | 569.   |
| 1  | JAN | 1215 | 4   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0045 | 154 | .02  | .00  | .02    | 561.   |
| 1  | JAN | 1220 | 5   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0050 | 155 | .03  | .00  | .02    | 554.   |
| 1  | JAN | 1225 | 6   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0055 | 156 | .03  | .00  | .02    | 545.   |
| 1  | JAN | 1230 | 7   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0100 | 157 | .03  | .00  | .02    | 545.   |
| 1  | JAN | 1235 | 8   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0105 | 158 | .03  | .00  | .02    | 544.   |
| 1  | JAN | 1240 | 9   | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0110 | 159 | .03  | .00  | .03    | 544.   |
| 1  | JAN | 1245 | 10  | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0115 | 160 | .03  | .00  | .03    | 546.   |
| 1  | JAN | 1250 | 11  | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0120 | 161 | .03  | .00  | .03    | 549.   |
| 1  | JAN | 1255 | 12  | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0125 | 162 | .03  | .00  | .03    | 553.   |
| 1  | JAN | 1300 | 13  | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0130 | 163 | .03  | .00  | .03    | 558.   |
| 1  | JAN | 1305 | 14  | .02  | .02  | .00    | 0.     | * | 2  | JAN | 0135 | 164 | .03  | .00  | .03    | 563.   |
| 1  | JAN | 1310 | 15  | .02  | .01  | .00    | 0.     | * | 2  | JAN | 0140 | 165 | .03  | .00  | .03    | 570.   |
| 1  | JAN | 1315 | 16  | .02  | .01  | .00    | 1.     | * | 2  | JAN | 0145 | 166 | .03  | .00  | .03    | 577.   |
| 1  | JAN | 1320 | 17  | .02  | .01  | .00    | 2.     | * | 2  | JAN | 0150 | 167 | .03  | .00  | .03    | 585.   |
| 1  | JAN | 1325 | 18  | .02  | .01  | .00    | 4.     | * | 2  | JAN | 0155 | 168 | .03  | .00  | .03    | 594.   |
| 1  | JAN | 1330 | 19  | .02  | .01  | .00    | 7.     | * | 2  | JAN | 0200 | 169 | .04  | .00  | .04    | 604.   |
| 1  | JAN | 1335 | 20  | .02  | .01  | .00    | 11.    | * | 2  | JAN | 0205 | 170 | .04  | .00  | .04    | 616.   |
| 1  | JAN | 1340 | 21  | .02  | .01  | .00    | 16.    | * | 2  | JAN | 0210 | 171 | .04  | .00  | .04    | 630.   |
| 1  | JAN | 1345 | 22  | .02  | .01  | .00    | 22.    | * | 2  | JAN | 0215 | 172 | .04  | .00  | .04    | 648.   |
| 1  | JAN | 1350 | 23  | .02  | .01  | .00    | 28.    | * | 2  | JAN | 0220 | 173 | .04  | .00  | .04    | 670.   |
| 1  | JAN | 1355 | 24  | .02  | .01  | .01    | 36.    | * | 2  | JAN | 0225 | 174 | .04  | .00  | .04    | 695.   |
| 1  | JAN | 1400 | 25  | .02  | .01  | .01    | 43.    | * | 2  | JAN | 0230 | 175 | .05  | .00  | .04    | 722.   |
| 1  | JAN | 1405 | 26  | .02  | .01  | .01    | 52.    | * | 2  | JAN | 0235 | 176 | .05  | .00  | .04    | 752.   |
| 1  | JAN | 1410 | 27  | .02  | .01  | .01    | 60.    | * | 2  | JAN | 0240 | 177 | .05  | .00  | .05    | 781.   |
| 1  | JAN | 1415 | 28  | .02  | .01  | .01    | 69.    | * | 2  | JAN | 0245 | 178 | .05  | .00  | .05    | 811.   |
| 1  | JAN | 1420 | 29  | .02  | .01  | .01    | 78.    | * | 2  | JAN | 0250 | 179 | .05  | .00  | .05    | 841.   |
| 1  | JAN | 1425 | 30  | .02  | .01  | .01    | 86.    | * | 2  | JAN | 0255 | 180 | .05  | .00  | .05    | 870.   |
| 1  | JAN | 1430 | 31  | .05  | .02  | .01    | .01    | * | 2  | JAN | 0300 | 181 | .06  | .00  | .05    | 899.   |
| 1  | JAN | 1435 | 32  | .04  | .02  | .01    | .01    | * | 2  | JAN | 0305 | 182 | .06  | .00  | .05    | 927.   |
| 1  | JAN | 1440 | 33  | .02  | .01  | .01    | 112.   | * | 2  | JAN | 0310 | 183 | .06  | .00  | .06    | 956.   |
| 1  | JAN | 1445 | 34  | .02  | .01  | .01    | 120.   | * | 2  | JAN | 0315 | 184 | .06  | .00  | .06    | 985.   |
| 1  | JAN | 1450 | 35  | .02  | .01  | .01    | 128.   | * | 2  | JAN | 0320 | 185 | .09  | .00  | .09    | 1019.  |
| 1  | JAN | 1455 | 36  | .02  | .01  | .01    | 136.   | * | 2  | JAN | 0325 | 186 | .09  | .00  | .09    | 1059.  |
| 1  | JAN | 1500 | 37  | .02  | .01  | .01    | 143.   | * | 2  | JAN | 0330 | 187 | .10  | .00  | .09    | 1107.  |
| 1  | JAN | 1505 | 38  | .02  | .01  | .01    | 151.   | * | 2  | JAN | 0335 | 188 | .10  | .00  | .10    | 1167.  |
| 1  | JAN | 1510 | 39  | .02  | .01  | .01    | 158.   | * | 2  | JAN | 0340 | 189 | .09  | .00  | .08    | 1240.  |
| 1  | JAN | 1515 | 40  | .02  | .01  | .01    | 165.   | * | 2  | JAN | 0345 | 190 | .10  | .00  | .10    | 1324.  |
| 1  | JAN | 1520 | 41  | .02  | .01  | .01    | 172.   | * | 2  | JAN | 0350 | 191 | .14  | .00  | .14    | 1419.  |
| 1  | JAN | 1525 | 42  | .02  | .01  | .01    | 179.   | * | 2  | JAN | 0355 | 192 | .20  | .00  | .20    | 1530.  |
| 1  | JAN | 1530 | 43  | .02  | .01  | .01    | 185.   | * | 2  | JAN | 0400 | 193 | .73  | .01  | .72    | 1702.  |
| 1  | JAN | 1535 | 44  | .02  | .01  | .01    | 192.   | * | 2  | JAN | 0405 | 194 | .12  | .00  | .11    | 1940.  |
| 1  | JAN | 1540 | 45  | .02  | .01  | .01    | 198.   | * | 2  | JAN | 0410 | 195 | .10  | .00  | .10    | 2226.  |
| 1  | JAN | 1545 | 46  | .02  | .01  | .01    | 204.   | * | 2  | JAN | 0415 | 196 | .09  | .00  | .09    | 2571.  |



| RGCrk.OUT  |     |     |     |     |      |   |            |     |     |     |     |       |
|------------|-----|-----|-----|-----|------|---|------------|-----|-----|-----|-----|-------|
| 1 JAN 1550 | 47  | .02 | .01 | .01 | 210. | * | 2 JAN 0420 | 197 | .06 | .00 | .06 | 2952. |
| 1 JAN 1555 | 48  | .02 | .01 | .01 | 216. | * | 2 JAN 0425 | 198 | .06 | .00 | .06 | 3281. |
| 1 JAN 1600 | 49  | .02 | .01 | .01 | 221. | * | 2 JAN 0430 | 199 | .05 | .00 | .05 | 3492. |
| 1 JAN 1605 | 50  | .02 | .01 | .01 | 227. | * | 2 JAN 0435 | 200 | .05 | .00 | .05 | 3577. |
| 1 JAN 1610 | 51  | .02 | .01 | .01 | 232. | * | 2 JAN 0440 | 201 | .05 | .00 | .05 | 3553. |
| 1 JAN 1615 | 52  | .02 | .01 | .01 | 237. | * | 2 JAN 0445 | 202 | .04 | .00 | .04 | 3443. |
| 1 JAN 1620 | 53  | .02 | .01 | .01 | 243. | * | 2 JAN 0450 | 203 | .04 | .00 | .04 | 3258. |
| 1 JAN 1625 | 54  | .02 | .01 | .01 | 248. | * | 2 JAN 0455 | 204 | .04 | .00 | .04 | 3033. |
| 1 JAN 1630 | 55  | .02 | .01 | .01 | 252. | * | 2 JAN 0500 | 205 | .03 | .00 | .03 | 2767. |
| 1 JAN 1635 | 56  | .02 | .01 | .01 | 257. | * | 2 JAN 0505 | 206 | .03 | .00 | .03 | 2472. |
| 1 JAN 1640 | 57  | .02 | .01 | .01 | 262. | * | 2 JAN 0510 | 207 | .03 | .00 | .03 | 2216. |
| 1 JAN 1645 | 58  | .02 | .01 | .01 | 266. | * | 2 JAN 0515 | 208 | .03 | .00 | .03 | 2005. |
| 1 JAN 1650 | 59  | .02 | .01 | .01 | 271. | * | 2 JAN 0520 | 209 | .03 | .00 | .03 | 1817. |
| 1 JAN 1655 | 60  | .02 | .00 | .01 | 275. | * | 2 JAN 0525 | 210 | .03 | .00 | .03 | 1655. |
| 1 JAN 1700 | 61  | .02 | .00 | .01 | 280. | * | 2 JAN 0530 | 211 | .03 | .00 | .03 | 1510. |
| 1 JAN 1705 | 62  | .02 | .00 | .01 | 284. | * | 2 JAN 0535 | 212 | .02 | .00 | .02 | 1379. |
| 1 JAN 1710 | 63  | .02 | .00 | .01 | 288. | * | 2 JAN 0540 | 213 | .02 | .00 | .02 | 1260. |
| 1 JAN 1715 | 64  | .02 | .00 | .01 | 292. | * | 2 JAN 0545 | 214 | .02 | .00 | .02 | 1160. |
| 1 JAN 1720 | 65  | .02 | .00 | .01 | 296. | * | 2 JAN 0550 | 215 | .02 | .00 | .02 | 1069. |
| 1 JAN 1725 | 66  | .02 | .00 | .01 | 300. | * | 2 JAN 0555 | 216 | .02 | .00 | .02 | 990.  |
| 1 JAN 1730 | 67  | .02 | .00 | .01 | 304. | * | 2 JAN 0600 | 217 | .03 | .00 | .03 | 921.  |
| 1 JAN 1735 | 68  | .02 | .00 | .01 | 308. | * | 2 JAN 0605 | 218 | .03 | .00 | .03 | 862.  |
| 1 JAN 1740 | 69  | .02 | .00 | .01 | 312. | * | 2 JAN 0610 | 219 | .03 | .00 | .03 | 814.  |
| 1 JAN 1745 | 70  | .02 | .00 | .01 | 315. | * | 2 JAN 0615 | 220 | .03 | .00 | .03 | 774.  |
| 1 JAN 1750 | 71  | .02 | .00 | .02 | 319. | * | 2 JAN 0620 | 221 | .03 | .00 | .03 | 744.  |
| 1 JAN 1755 | 72  | .02 | .00 | .02 | 323. | * | 2 JAN 0625 | 222 | .03 | .00 | .03 | 721.  |
| 1 JAN 1800 | 73  | .02 | .00 | .02 | 326. | * | 2 JAN 0630 | 223 | .03 | .00 | .03 | 704.  |
| 1 JAN 1805 | 74  | .02 | .00 | .02 | 330. | * | 2 JAN 0635 | 224 | .03 | .00 | .03 | 691.  |
| 1 JAN 1810 | 75  | .02 | .00 | .02 | 334. | * | 2 JAN 0640 | 225 | .03 | .00 | .03 | 680.  |
| 1 JAN 1815 | 76  | .02 | .00 | .02 | 337. | * | 2 JAN 0645 | 226 | .03 | .00 | .03 | 673.  |
| 1 JAN 1820 | 77  | .02 | .00 | .02 | 341. | * | 2 JAN 0650 | 227 | .03 | .00 | .02 | 665.  |
| 1 JAN 1825 | 78  | .02 | .00 | .02 | 344. | * | 2 JAN 0655 | 228 | .02 | .00 | .02 | 659.  |
| 1 JAN 1830 | 79  | .02 | .00 | .02 | 347. | * | 2 JAN 0700 | 229 | .02 | .00 | .02 | 652.  |
| 1 JAN 1835 | 80  | .02 | .00 | .02 | 351. | * | 2 JAN 0705 | 230 | .02 | .00 | .02 | 644.  |
| 1 JAN 1840 | 81  | .02 | .00 | .02 | 354. | * | 2 JAN 0710 | 231 | .02 | .00 | .02 | 636.  |
| 1 JAN 1845 | 82  | .02 | .00 | .02 | 358. | * | 2 JAN 0715 | 232 | .02 | .00 | .02 | 627.  |
| 1 JAN 1850 | 83  | .02 | .00 | .02 | 361. | * | 2 JAN 0720 | 233 | .02 | .00 | .02 | 618.  |
| 1 JAN 1855 | 84  | .02 | .00 | .02 | 364. | * | 2 JAN 0725 | 234 | .02 | .00 | .02 | 609.  |
| 1 JAN 1900 | 85  | .02 | .00 | .02 | 367. | * | 2 JAN 0730 | 235 | .02 | .00 | .02 | 600.  |
| 1 JAN 1905 | 86  | .02 | .00 | .02 | 371. | * | 2 JAN 0735 | 236 | .02 | .00 | .02 | 592.  |
| 1 JAN 1910 | 87  | .02 | .00 | .02 | 374. | * | 2 JAN 0740 | 237 | .02 | .00 | .02 | 585.  |
| 1 JAN 1915 | 88  | .02 | .00 | .02 | 377. | * | 2 JAN 0745 | 238 | .02 | .00 | .02 | 578.  |
| 1 JAN 1920 | 89  | .02 | .00 | .02 | 380. | * | 2 JAN 0750 | 239 | .02 | .00 | .02 | 571.  |
| 1 JAN 1925 | 90  | .02 | .00 | .02 | 384. | * | 2 JAN 0755 | 240 | .02 | .00 | .02 | 565.  |
| 1 JAN 1930 | 91  | .02 | .00 | .02 | 387. | * | 2 JAN 0800 | 241 | .02 | .00 | .02 | 558.  |
| 1 JAN 1935 | 92  | .02 | .00 | .02 | 390. | * | 2 JAN 0805 | 242 | .02 | .00 | .02 | 552.  |
| 1 JAN 1940 | 93  | .02 | .00 | .02 | 393. | * | 2 JAN 0810 | 243 | .02 | .00 | .02 | 546.  |
| 1 JAN 1945 | 94  | .02 | .00 | .02 | 397. | * | 2 JAN 0815 | 244 | .02 | .00 | .02 | 540.  |
| 1 JAN 1950 | 95  | .02 | .00 | .02 | 400. | * | 2 JAN 0820 | 245 | .02 | .00 | .02 | 534.  |
| 1 JAN 1955 | 96  | .02 | .00 | .02 | 403. | * | 2 JAN 0825 | 246 | .02 | .00 | .02 | 528.  |
| 1 JAN 2000 | 97  | .02 | .00 | .02 | 406. | * | 2 JAN 0830 | 247 | .02 | .00 | .02 | 523.  |
| 1 JAN 2005 | 98  | .02 | .00 | .02 | 409. | * | 2 JAN 0835 | 248 | .02 | .00 | .02 | 517.  |
| 1 JAN 2010 | 99  | .02 | .00 | .02 | 413. | * | 2 JAN 0840 | 249 | .02 | .00 | .02 | 512.  |
| 1 JAN 2015 | 100 | .02 | .00 | .02 | 416. | * | 2 JAN 0845 | 250 | .02 | .00 | .02 | 507.  |
| 1 JAN 2020 | 101 | .02 | .00 | .02 | 419. | * | 2 JAN 0850 | 251 | .02 | .00 | .02 | 502.  |
| 1 JAN 2025 | 102 | .02 | .00 | .02 | 422. | * | 2 JAN 0855 | 252 | .02 | .00 | .02 | 497.  |
| 1 JAN 2030 | 103 | .02 | .00 | .02 | 426. | * | 2 JAN 0900 | 253 | .02 | .00 | .02 | 492.  |
| 1 JAN 2035 | 104 | .02 | .00 | .02 | 429. | * | 2 JAN 0905 | 254 | .02 | .00 | .02 | 488.  |
| 1 JAN 2040 | 105 | .02 | .00 | .02 | 432. | * | 2 JAN 0910 | 255 | .02 | .00 | .02 | 483.  |
| 1 JAN 2045 | 106 | .02 | .00 | .02 | 436. | * | 2 JAN 0915 | 256 | .02 | .00 | .02 | 479.  |
| 1 JAN 2050 | 107 | .02 | .00 | .02 | 439. | * | 2 JAN 0920 | 257 | .02 | .00 | .02 | 475.  |
| 1 JAN 2055 | 108 | .02 | .00 | .02 | 442. | * | 2 JAN 0925 | 258 | .02 | .00 | .02 | 470.  |
| 1 JAN 2100 | 109 | .02 | .00 | .02 | 446. | * | 2 JAN 0930 | 259 | .02 | .00 | .02 | 466.  |
| 1 JAN 2105 | 110 | .02 | .00 | .02 | 449. | * | 2 JAN 0935 | 260 | .02 | .00 | .02 | 462.  |
| 1 JAN 2110 | 111 | .02 | .00 | .02 | 453. | * | 2 JAN 0940 | 261 | .02 | .00 | .02 | 459.  |
| 1 JAN 2115 | 112 | .02 | .00 | .02 | 456. | * | 2 JAN 0945 | 262 | .02 | .00 | .02 | 455.  |
| 1 JAN 2120 | 113 | .02 | .00 | .02 | 460. | * | 2 JAN 0950 | 263 | .02 | .00 | .02 | 451.  |
| 1 JAN 2125 | 114 | .02 | .00 | .02 | 463. | * | 2 JAN 0955 | 264 | .02 | .00 | .02 | 448.  |
| 1 JAN 2130 | 115 | .02 | .00 | .02 | 467. | * | 2 JAN 1000 | 265 | .02 | .00 | .02 | 444.  |
| 1 JAN 2135 | 116 | .02 | .00 | .02 | 470. | * | 2 JAN 1005 | 266 | .02 | .00 | .02 | 441.  |
| 1 JAN 2140 | 117 | .02 | .00 | .02 | 474. | * | 2 JAN 1010 | 267 | .02 | .00 | .02 | 438.  |
| 1 JAN 2145 | 118 | .02 | .00 | .02 | 477. | * | 2 JAN 1015 | 268 | .02 | .00 | .02 | 434.  |
| 1 JAN 2150 | 119 | .02 | .00 | .02 | 481. | * | 2 JAN 1020 | 269 | .02 | .00 | .02 | 431.  |
| 1 JAN 2155 | 120 | .02 | .00 | .02 | 485. | * | 2 JAN 1025 | 270 | .02 | .00 | .02 | 428.  |
| 1 JAN 2200 | 121 | .02 | .00 | .02 | 489. | * | 2 JAN 1030 | 271 | .02 | .00 | .02 | 425.  |
| 1 JAN 2205 | 122 | .02 | .00 | .02 | 492. | * | 2 JAN 1035 | 272 | .02 | .00 | .02 | 422.  |
| 1 JAN 2210 | 123 | .03 | .00 | .02 | 496. | * | 2 JAN 1040 | 273 | .02 | .00 | .02 | 419.  |
| 1 JAN 2215 | 124 | .03 | .00 | .02 | 500. | * | 2 JAN 1045 | 274 | .02 | .00 | .02 | 417.  |
| 1 JAN 2220 | 125 | .03 | .00 | .02 | 504. | * | 2 JAN 1050 | 275 | .02 | .00 | .02 | 414.  |
| 1 JAN 2225 | 126 | .03 | .00 | .02 | 508. | * | 2 JAN 1055 | 276 | .02 | .00 | .02 | 411.  |
| 1 JAN 2230 | 127 | .03 | .00 | .02 | 512. | * | 2 JAN 1100 | 277 | .02 | .00 | .02 | 408.  |
| 1 JAN 2235 | 128 | .03 | .00 | .02 | 516. | * | 2 JAN 1105 | 278 | .02 | .00 | .02 | 406.  |
| 1 JAN 2240 | 129 | .03 | .00 | .02 | 520. | * | 2 JAN 1110 | 279 | .02 | .00 | .02 | 403.  |
| 1 JAN 2245 | 130 | .03 | .00 | .02 | 525. | * | 2 JAN 1115 | 280 | .02 | .00 | .02 | 401.  |
| 1 JAN 2250 | 131 | .03 | .00 | .02 | 529. | * | 2 JAN 1120 | 281 | .02 | .00 | .02 | 398.  |
| 1 JAN 2255 | 132 | .03 | .00 | .02 | 533. | * | 2 JAN 1125 | 282 | .02 | .00 | .02 | 396.  |
| 1 JAN 2300 | 133 | .03 | .00 | .03 | 538. | * | 2 JAN 1130 | 283 | .02 | .00 | .02 | 394.  |
| 1 JAN 2305 | 134 | .03 | .00 | .03 | 542. | * | 2 JAN 1135 | 284 | .02 | .00 | .02 | 391.  |
| 1 JAN 2310 | 135 | .03 | .00 | .03 | 547. | * | 2 JAN 1140 | 285 | .02 | .00 | .02 | 389.  |
| 1 JAN 2315 | 136 | .03 | .00 | .03 | 551. | * | 2 JAN 1145 | 286 | .02 | .00 | .02 | 387.  |
| 1 JAN 2320 | 137 | .03 | .00 | .03 | 556. | * | 2 JAN 1150 | 287 | .02 | .00 | .02 | 385.  |
| 1 JAN 2325 | 138 | .03 | .00 | .03 | 561. | * | 2 JAN 1155 | 288 | .02 | .00 | .02 | 382.  |
| 1 JAN 2330 | 139 | .03 | .00 | .03 | 566. | * | 2 JAN 1200 | 289 | .00 | .00 | .00 | 379.  |
| 1 JAN 2335 | 140 | .03 | .00 | .03 | 571. | * | 2 JAN 1205 | 290 | .00 | .00 | .00 | 373.  |
| 1 JAN 2340 | 141 | .03 | .00 | .03 | 576. | * | 2 JAN 1210 | 291 | .00 | .00 | .00 | 363.  |
| 1 JAN 2345 | 142 | .03 | .00 | .03 | 581. | * | 2 JAN 1215 | 292 | .00 | .00 | .00 | 349.  |
| 1 JAN 2350 | 143 | .03 | .00 | .03 | 587. | * | 2 JAN 1220 | 293 | .00 | .00 | .00 | 328.  |
| 1 JAN 2355 | 144 | .03 | .00 | .03 | 592. | * | 2 JAN 1225 | 294 | .00 | .00 | .00 | 302.  |
| 2 JAN 0000 | 145 | .02 | .00 | .02 | 597. | * | 2 JAN 1230 | 295 | .00 | .00 | .00 | 271.  |
| 2 JAN 0005 | 146 | .02 | .00 | .02 | 601. | * | 2 JAN 1235 | 296 | .00 | .00 | .00 | 239.  |
| 2 JAN 0010 | 147 | .02 | .00 | .02 | 603. | * | 2 JAN 1240 | 297 | .00 | .00 | .00 | 207.  |
| 2 JAN 0015 | 148 | .02 | .00 | .02 | 603. | * | 2 JAN 1245 | 298 | .00 | .00 | .00 | 176.  |
| 2 JAN 0020 | 149 | .02 | .00 | .02 | 600. | * | 2 JAN 1250 | 299 | .00 | .00 | .00 | 148.  |
| 2 JAN 0025 | 150 | .02 | .00 | .02 | 594. | * | 2 JAN 1255 | 300 | .00 | .00 | .00 | 122.  |

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TOTAL RAINFALL = 8.44, TOTAL LOSS = .96, TOTAL EXCESS = 7.48

| PEAK FLOW | TIME  | 6-HR  | MAXIMUM AVERAGE FLOW | 24.92-HR |
|-----------|-------|-------|----------------------|----------|
| +         | (CFS) | (HR)  | (CFS)                |          |
| +         | 3577. | 16.58 | 1303.                | 607.     |
|           |       |       | 3.999                | 7.452    |
|           |       |       | (AC-FT)              | 1204.    |
|           |       |       | 646.                 | 1204.    |

CUMULATIVE AREA = 3.03 SQ MI

## STATION BASIN200

|        |        | (O) OUTFLOW |      |      |       |       |       |       |       |       |       |             |            |
|--------|--------|-------------|------|------|-------|-------|-------|-------|-------|-------|-------|-------------|------------|
|        |        | 0.          | 400. | 800. | 1200. | 1600. | 2000. | 2400. | 2800. | 3200. | 3600. | 0.          | 0.         |
|        |        | PER         |      |      |       |       |       |       |       |       |       | (L) PRECIP, | (X) EXCESS |
|        |        |             | .0   | .0   | .0    | .0    | .0    | .0    | .0    | .8    | .6    | .4          | .2         |
| DAHRMN | PER    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11200  | 10     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11205  | 20     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11210  | 30     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11215  | 40     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11220  | 50     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11225  | 60     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11230  | 70     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11235  | 80     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11240  | 90     |             |      |      |       |       |       |       |       |       |       |             |            |
| 11245  | 100    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11250  | 110    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11255  | 120    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11300  | 130    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11305  | 140    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11310  | 150    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11315  | 160    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11320  | 170    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11325  | 180    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11330  | 190    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11335  | 200    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11340  | 210    |             |      |      |       |       |       |       |       |       |       |             |            |
| 11345  | 22.0   |             |      |      |       |       |       |       |       |       |       |             |            |
| 11350  | 23.0   |             |      |      |       |       |       |       |       |       |       |             |            |
| 11355  | 24.0   |             |      |      |       |       |       |       |       |       |       |             |            |
| 11400  | 25.0   |             |      |      |       |       |       |       |       |       |       |             |            |
| 11405  | 26.0   |             |      |      |       |       |       |       |       |       |       |             |            |
| 11410  | 27. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11415  | 28. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11420  | 29. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11425  | 30. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11430  | 31. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11435  | 32. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11440  | 33. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11445  | 34. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11450  | 35. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11455  | 36. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11500  | 37. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11505  | 38. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11510  | 39. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11515  | 40. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11520  | 41. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11525  | 42. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11530  | 43. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11535  | 44. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11540  | 45. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11545  | 46. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11550  | 47. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11555  | 48. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11600  | 49. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11605  | 50. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11610  | 51. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11615  | 52. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11620  | 53. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11625  | 54. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11630  | 55. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11635  | 56. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11640  | 57. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11645  | 58. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11650  | 59. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11655  | 60. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11700  | 61. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11705  | 62. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11710  | 63. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11715  | 64. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11720  | 65. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11725  | 66. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11730  | 67. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11735  | 68. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11740  | 69. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11745  | 70. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11750  | 71. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11755  | 72. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11800  | 73. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11805  | 74. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11810  | 75. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11815  | 76. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11820  | 77. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11825  | 78. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11830  | 79. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11835  | 80. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11840  | 81. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11845  | 82. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11850  | 83. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11855  | 84. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11900  | 85. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11905  | 86. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11910  | 87. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11915  | 88. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11920  | 89. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11925  | 90. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11930  | 91. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11935  | 92. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11940  | 93. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11945  | 94. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11950  | 95. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 11955  | 96. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 12000  | 97. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 12005  | 98. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 12010  | 99. 0  |             |      |      |       |       |       |       |       |       |       |             |            |
| 12015  | 100. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12020  | 101. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12025  | 102. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12030  | 103. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12035  | 104. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12040  | 105. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12045  | 106. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12050  | 107. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12055  | 108. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12100  | 109. 0 |             |      |      |       |       |       |       |       |       |       |             |            |
| 12105  | 110. 0 |             |      |      |       |       |       |       |       |       |       |             |            |



[illegible]

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*****
*                               *
108 KK  *   BASIN300   *       RUNOFF
*                               *
*****

NESTED STORM PER COUNTY OF SAN DIEGO HYDROLOGY MANUAL
COPYRIGHT 2003 RICK ENGINEERING COMPANY
FOR THIS DATA TO BE USED PROPERLY IN HEC-1,
THE 1DATE1 VARIABLE MUST BE 01JAN90 AND THE 1TIME1
VARIABLE MUST BE 1200 ON THE IT CARD.
THE FOLLOWING IT CARD IS SUGGESTED TO BE USED IN
YOUR DATA SET:
IT 5 01JAN90 1200 300
6HR RAINFALL IS 3.6 INCHES
24HR RAINFALL IS 6 INCHES
DAR30 = .783246
DAR60 = .867702
DAR180 = .93662
DAR360 = .953003
DAR1440 = .966541
BASIN AREA IS 27.69 SQUARE MILES

125 IN      TIME DATA FOR INPUT TIME SERIES
           JXMIN      5      TIME INTERVAL IN MINUTES
           JXDATE     1JAN90  STARTING DATE
           JXTIME     1200   STARTING TIME

SUBBASIN RUNOFF DATA

157 BA      SUBBASIN CHARACTERISTICS
           TAREA      1.06   SUBBASIN AREA

PRECIPITATION DATA

126 PB      STORM      8.44   BASIN TOTAL PRECIPITATION

126 PI      INCREMENTAL PRECIPITATION PATTERN
           .01      .01      .01      .01      .01      .01      .01      .01
           .01      .01      .01      .01      .01      .01      .01      .01

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[illegible]

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159 UD          SCS DIMENSIONLESS UNITGRAPH
                TLAG          .52 LAG

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UNIT HYDROGRAPH  
33 END-OF-PERIOD ORDINATES

|      |      |      |      |      |      |      |      |      |      |
|------|------|------|------|------|------|------|------|------|------|
| 59.  | 173. | 354. | 599. | 798. | 904. | 912. | 856. | 757. | 631. |
| 476. | 364. | 284. | 227. | 179. | 138. | 110. | 86.  | 67.  | 52.  |
| 41.  | 33.  | 25.  | 20.  | 16.  | 12.  | 10.  | 8.   | 6.   | 5.   |
| 3.   | 2.   | 1.   |      |      |      |      |      |      |      |

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[illegible]



[illegible]

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|        |                                  |
|--------|----------------------------------|
| 177 IN | TIME DATA FOR INPUT TIME SERIES  |
|        | JXMIN 5 TIME INTERVAL IN MINUTES |
|        | JXDATE 1JAN90 STARTING DATE      |
|        | JXTIME 1200 STARTING TIME        |

209 BA SUBBASIN CHARACTERISTICS  
TAREA 1.10 SUBBASIN AREA

|        |       |      |                           |
|--------|-------|------|---------------------------|
| 178 PB | STORM | 8.44 | BASIN TOTAL PRECIPITATION |
|--------|-------|------|---------------------------|

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 | .01 |
| .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 |
| .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 |
| .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 |
| .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 | .02 |
| .04 | .04 | .04 | .04 | .04 | .04 | .04 | .04 | .04 | .04 |
| .05 | .05 | .05 | .08 | .08 | .08 | .08 | .07 | .08 | .12 |
| .17 | .62 | .10 | .09 | .08 | .05 | .05 | .04 | .04 | .04 |
| .04 | .04 | .03 | .03 | .03 | .02 | .02 | .02 | .02 | .02 |

[illegible]

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211 UD          SCS DIMENSIONLESS UNITGRAPH
                TLAG          .64  LAG

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UNIT HYDROGRAPH  
40 END-OF-PERIOD ORDINATES

|      |      |      |      |      |      |      |      |      |      |
|------|------|------|------|------|------|------|------|------|------|
| 36.  | 110. | 211. | 354. | 531. | 670. | 753. | 779. | 772. | 713. |
| 642. | 555. | 445. | 352. | 287. | 235. | 195. | 161. | 132. | 107. |
| 88.  | 73.  | 59.  | 48.  | 39.  | 32.  | 27.  | 22.  | 18.  | 15.  |
| 12.  | 10.  | 8.   | 7.   | 6.   | 5.   | 4.   | 3.   | 2.   | 1.   |

HYDROGRAPH AT STATION BASIN400

| DA | MON | HRMN | ORD | RAIN | LOSS | EXCESS | COMP Q | * | DA | MON | HRMN | ORD | RAIN | LOSS | EXCESS | COMP Q |
|----|-----|------|-----|------|------|--------|--------|---|----|-----|------|-----|------|------|--------|--------|
| 1  | JAN | 1200 | 1   | .00  | .00  | .00    | 0.     | * | 2  | JAN | 0030 | 151 | .03  | .00  | .03    | 198.   |
| 1  | JAN | 1205 | 2   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0035 | 152 | .03  | .00  | .03    | 201.   |
| 1  | JAN | 1210 | 3   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0040 | 153 | .03  | .00  | .03    | 204.   |
| 1  | JAN | 1215 | 4   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0045 | 154 | .03  | .00  | .03    | 208.   |
| 1  | JAN | 1220 | 5   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0050 | 155 | .03  | .00  | .03    | 211.   |
| 1  | JAN | 1225 | 6   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0055 | 156 | .03  | .00  | .03    | 215.   |
| 1  | JAN | 1230 | 7   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0100 | 157 | .03  | .00  | .03    | 218.   |
| 1  | JAN | 1235 | 8   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0105 | 158 | .03  | .00  | .03    | 221.   |
| 1  | JAN | 1240 | 9   | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0110 | 159 | .03  | .00  | .03    | 225.   |
| 1  | JAN | 1245 | 10  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0115 | 160 | .03  | .00  | .03    | 228.   |
| 1  | JAN | 1250 | 11  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0120 | 161 | .03  | .00  | .03    | 232.   |
| 1  | JAN | 1255 | 12  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0125 | 162 | .03  | .00  | .03    | 236.   |
| 1  | JAN | 1300 | 13  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0130 | 163 | .04  | .00  | .03    | 239.   |
| 1  | JAN | 1305 | 14  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0135 | 164 | .04  | .00  | .03    | 243.   |
| 1  | JAN | 1310 | 15  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0140 | 165 | .04  | .00  | .03    | 247.   |
| 1  | JAN | 1315 | 16  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0145 | 166 | .04  | .00  | .04    | 252.   |
| 1  | JAN | 1320 | 17  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0150 | 167 | .04  | .00  | .04    | 256.   |
| 1  | JAN | 1325 | 18  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0155 | 168 | .04  | .00  | .04    | 261.   |
| 1  | JAN | 1330 | 19  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0200 | 169 | .05  | .00  | .05    | 266.   |
| 1  | JAN | 1335 | 20  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0205 | 170 | .05  | .00  | .05    | 272.   |
| 1  | JAN | 1340 | 21  | .01  | .01  | .00    | 0.     | * | 2  | JAN | 0210 | 171 | .05  | .00  | .05    | 280.   |
| 1  | JAN | 1345 | 22  | .01  | .01  | .00    | 1.     | * | 2  | JAN | 0215 | 172 | .05  | .00  | .05    | 289.   |
| 1  | JAN | 1350 | 23  | .01  | .01  | .00    | 2.     | * | 2  | JAN | 0220 | 173 | .05  | .00  | .05    | 299.   |
| 1  | JAN | 1355 | 24  | .01  | .01  | .00    | 2.     | * | 2  | JAN | 0225 | 174 | .05  | .00  | .05    | 312.   |
| 1  | JAN | 1400 | 25  | .01  | .01  | .00    | 3.     | * | 2  | JAN | 0230 | 175 | .06  | .00  | .05    | 325.   |
| 1  | JAN | 1405 | 26  | .01  | .01  | .00    | 5.     | * | 2  | JAN | 0235 | 176 | .06  | .00  | .05    | 339.   |
| 1  | JAN | 1410 | 27  | .01  | .01  | .00    | 6.     | * | 2  | JAN | 0240 | 177 | .06  | .00  | .06    | 353.   |
| 1  | JAN | 1415 | 28  | .01  | .01  | .00    | 8.     | * | 2  | JAN | 0245 | 178 | .06  | .00  | .06    | 367.   |
| 1  | JAN | 1420 | 29  | .01  | .01  | .00    | 9.     | * | 2  | JAN | 0250 | 179 | .06  | .00  | .06    | 380.   |
| 1  | JAN | 1425 | 30  | .01  | .01  | .00    | 11.    | * | 2  | JAN | 0255 | 180 | .06  | .00  | .06    | 394.   |
| 1  | JAN | 1430 | 31  | .01  | .01  | .00    | 13.    | * | 2  | JAN | 0300 | 181 | .07  | .00  | .06    | 406.   |
| 1  | JAN | 1435 | 32  | .01  | .01  | .00    | 15.    | * | 2  | JAN | 0305 | 182 | .07  | .00  | .07    | 419.   |
| 1  | JAN | 1440 | 33  | .01  | .01  | .00    | 17.    | * | 2  | JAN | 0310 | 183 | .07  | .00  | .07    | 432.</ |

| RGCrk.OUT |     |      |     |     |     |     |      |   |   |     |      |     |     |     |     |      |
|-----------|-----|------|-----|-----|-----|-----|------|---|---|-----|------|-----|-----|-----|-----|------|
| 1         | JAN | 1900 | 85  | .02 | .00 | .01 | 92.  | * | 2 | JAN | 0730 | 235 | .02 | .00 | .02 | 187. |
| 1         | JAN | 1905 | 86  | .02 | .00 | .01 | 93.  | * | 2 | JAN | 0735 | 236 | .02 | .00 | .02 | 183. |
| 1         | JAN | 1910 | 87  | .02 | .00 | .01 | 94.  | * | 2 | JAN | 0740 | 237 | .02 | .00 | .02 | 179. |
| 1         | JAN | 1915 | 88  | .02 | .00 | .01 | 96.  | * | 2 | JAN | 0745 | 238 | .02 | .00 | .02 | 176. |
| 1         | JAN | 1920 | 89  | .02 | .00 | .01 | 97.  | * | 2 | JAN | 0750 | 239 | .02 | .00 | .02 | 173. |
| 1         | JAN | 1925 | 90  | .02 | .00 | .01 | 98.  | * | 2 | JAN | 0755 | 240 | .02 | .00 | .02 | 170. |
| 1         | JAN | 1930 | 91  | .02 | .00 | .01 | 99.  | * | 2 | JAN | 0800 | 241 | .02 | .00 | .02 | 167. |
| 1         | JAN | 1935 | 92  | .02 | .00 | .01 | 100. | * | 2 | JAN | 0805 | 242 | .02 | .00 | .02 | 164. |
| 1         | JAN | 1940 | 93  | .02 | .00 | .01 | 101. | * | 2 | JAN | 0810 | 243 | .02 | .00 | .02 | 161. |
| 1         | JAN | 1945 | 94  | .02 | .00 | .01 | 103. | * | 2 | JAN | 0815 | 244 | .02 | .00 | .02 | 159. |
| 1         | JAN | 1950 | 95  | .02 | .00 | .01 | 104. | * | 2 | JAN | 0820 | 245 | .02 | .00 | .02 | 156. |
| 1         | JAN | 1955 | 96  | .02 | .00 | .01 | 105. | * | 2 | JAN | 0825 | 246 | .02 | .00 | .02 | 154. |
| 1         | JAN | 2000 | 97  | .02 | .00 | .01 | 106. | * | 2 | JAN | 0830 | 247 | .02 | .00 | .02 | 152. |
| 1         | JAN | 2005 | 98  | .02 | .00 | .01 | 107. | * | 2 | JAN | 0835 | 248 | .02 | .00 | .02 | 150. |
| 1         | JAN | 2010 | 99  | .02 | .00 | .01 | 108. | * | 2 | JAN | 0840 | 249 | .02 | .00 | .02 | 147. |
| 1         | JAN | 2015 | 100 | .02 | .00 | .01 | 110. | * | 2 | JAN | 0845 | 250 | .02 | .00 | .02 | 145. |
| 1         | JAN | 2020 | 101 | .02 | .00 | .01 | 111. | * | 2 | JAN | 0850 | 251 | .02 | .00 | .01 | 144. |
| 1         | JAN | 2025 | 102 | .02 | .00 | .01 | 112. | * | 2 | JAN | 0855 | 252 | .01 | .00 | .01 | 142. |
| 1         | JAN | 2030 | 103 | .02 | .00 | .01 | 113. | * | 2 | JAN | 0900 | 253 | .01 | .00 | .01 | 140. |
| 1         | JAN | 2035 | 104 | .02 | .00 | .01 | 114. | * | 2 | JAN | 0905 | 254 | .01 | .00 | .01 | 138. |
| 1         | JAN | 2040 | 105 | .02 | .00 | .02 | 116. | * | 2 | JAN | 0910 | 255 | .01 | .00 | .01 | 137. |
| 1         | JAN | 2045 | 106 | .02 | .00 | .02 | 117. | * | 2 | JAN | 0915 | 256 | .01 | .00 | .01 | 135. |
| 1         | JAN | 2050 | 107 | .02 | .00 | .02 | 118. | * | 2 | JAN | 0920 | 257 | .01 | .00 | .01 | 133. |
| 1         | JAN | 2055 | 108 | .02 | .00 | .02 | 119. | * | 2 | JAN | 0925 | 258 | .01 | .00 | .01 | 132. |
| 1         | JAN | 2100 | 109 | .02 | .00 | .02 | 121. | * | 2 | JAN | 0930 | 259 | .01 | .00 | .01 | 130. |
| 1         | JAN | 2105 | 110 | .02 | .00 | .02 | 122. | * | 2 | JAN | 0935 | 260 | .01 | .00 | .01 | 129. |
| 1         | JAN | 2110 | 111 | .02 | .00 | .02 | 123. | * | 2 | JAN | 0940 | 261 | .01 | .00 | .01 | 128. |
| 1         | JAN | 2115 | 112 | .02 | .00 | .02 | 125. | * | 2 | JAN | 0945 | 262 | .01 | .00 | .01 | 126. |
| 1         | JAN | 2120 | 113 | .02 | .00 | .02 | 126. | * | 2 | JAN | 0950 | 263 | .01 | .00 | .01 | 125. |
| 1         | JAN | 2125 | 114 | .02 | .00 | .02 | 127. | * | 2 | JAN | 0955 | 264 | .01 | .00 | .01 | 124. |
| 1         | JAN | 2130 | 115 | .02 | .00 | .02 | 129. | * | 2 | JAN | 1000 | 265 | .01 | .00 | .01 | 122. |
| 1         | JAN | 2135 | 116 | .02 | .00 | .02 | 130. | * | 2 | JAN | 1005 | 266 | .01 | .00 | .01 | 121. |
| 1         | JAN | 2140 | 117 | .02 | .00 | .02 | 131. | * | 2 | JAN | 1010 | 267 | .01 | .00 | .01 | 120. |
| 1         | JAN | 2145 | 118 | .02 | .00 | .02 | 133. | * | 2 | JAN | 1015 | 268 | .01 | .00 | .01 | 119. |
| 1         | JAN | 2150 | 119 | .02 | .00 | .02 | 134. | * | 2 | JAN | 1020 | 269 | .01 | .00 | .01 | 118. |
| 1         | JAN | 2155 | 120 | .02 | .00 | .02 | 136. | * | 2 | JAN | 1025 | 270 | .01 | .00 | .01 | 117. |
| 1         | JAN | 2200 | 121 | .02 | .00 | .02 | 137. | * | 2 | JAN | 1030 | 271 | .01 | .00 | .01 | 115. |
| 1         | JAN | 2205 | 122 | .02 | .00 | .02 | 139. | * | 2 | JAN | 1035 | 272 | .01 | .00 | .01 | 114. |
| 1         | JAN | 2210 | 123 | .02 | .00 | .02 | 140. | * | 2 | JAN | 1040 | 273 | .01 | .00 | .01 | 113. |
| 1         | JAN | 2215 | 124 | .02 | .00 | .02 | 142. | * | 2 | JAN | 1045 | 274 | .01 | .00 | .01 | 112. |
| 1         | JAN | 2220 | 125 | .02 | .00 | .02 | 143. | * | 2 | JAN | 1050 | 275 | .01 | .00 | .01 | 111. |
| 1         | JAN | 2225 | 126 | .02 | .00 | .02 | 145. | * | 2 | JAN | 1055 | 276 | .01 | .00 | .01 | 110. |
| 1         | JAN | 2230 | 127 | .02 | .00 | .02 | 146. | * | 2 | JAN | 1100 | 277 | .01 | .00 | .01 | 110. |
| 1         | JAN | 2235 | 128 | .02 | .00 | .02 | 148. | * | 2 | JAN | 1105 | 278 | .01 | .00 | .01 | 109. |
| 1         | JAN | 2240 | 129 | .02 | .00 | .02 | 149. | * | 2 | JAN | 1110 | 279 | .01 | .00 | .01 | 108. |
| 1         | JAN | 2245 | 130 | .02 | .00 | .02 | 151. | * | 2 | JAN | 1115 | 280 | .01 | .00 | .01 | 107. |
| 1         | JAN | 2250 | 131 | .02 | .00 | .02 | 153. | * | 2 | JAN | 1120 | 281 | .01 | .00 | .01 | 106. |
| 1         | JAN | 2255 | 132 | .02 | .00 | .02 | 155. | * | 2 | JAN | 1125 | 282 | .01 | .00 | .01 | 105. |
| 1         | JAN | 2300 | 133 | .02 | .00 | .02 | 156. | * | 2 | JAN | 1130 | 283 | .01 | .00 | .01 | 104. |
| 1         | JAN | 2305 | 134 | .02 | .00 | .02 | 158. | * | 2 | JAN | 1135 | 284 | .01 | .00 | .01 | 104. |
| 1         | JAN | 2310 | 135 | .02 | .00 | .02 | 160. | * | 2 | JAN | 1140 | 285 | .01 | .00 | .01 | 103. |
| 1         | JAN | 2315 | 136 | .02 | .00 | .02 | 162. | * | 2 | JAN | 1145 | 286 | .01 | .00 | .01 | 102. |
| 1         | JAN | 2320 | 137 | .02 | .00 | .02 | 164. | * | 2 | JAN | 1150 | 287 | .01 | .00 | .01 | 101. |
| 1         | JAN | 2325 | 138 | .02 | .00 | .02 | 166. | * | 2 | JAN | 1155 | 288 | .01 | .00 | .01 | 101. |
| 1         | JAN | 2330 | 139 | .02 | .00 | .02 | 168. | * | 2 | JAN | 1200 | 289 | .00 | .00 | .00 | 99.  |
| 1         | JAN | 2335 | 140 | .02 | .00 | .02 | 170. | * | 2 | JAN | 1205 | 290 | .00 | .00 | .00 | 97.  |
| 1         | JAN | 2340 | 141 | .03 | .00 | .02 | 172. | * | 2 | JAN | 1210 | 291 | .00 | .00 | .00 | 94.  |
| 1         | JAN | 2345 | 142 | .03 | .00 | .02 | 174. | * | 2 | JAN | 1215 | 292 | .00 | .00 | .00 | 90.  |
| 1         | JAN | 2350 | 143 | .03 | .00 | .02 | 176. | * | 2 | JAN | 1220 | 293 | .00 | .00 | .00 | 84.  |
| 1         | JAN | 2355 | 144 | .03 | .00 | .02 | 178. | * | 2 | JAN | 1225 | 294 | .00 | .00 | .00 | 76.  |
| 2         | JAN | 0000 | 145 | .03 | .00 | .02 | 181. | * | 2 | JAN | 1230 | 295 | .00 | .00 | .00 | 67.  |
| 2         | JAN | 0005 | 146 | .03 | .00 | .03 | 183. | * | 2 | JAN | 1235 | 296 | .00 | .00 | .00 | 58.  |
| 2         | JAN | 0010 | 147 | .03 | .00 | .03 | 186. | * | 2 | JAN | 1240 | 297 | .00 | .00 | .00 | 49.  |
| 2         | JAN | 0015 | 148 | .03 | .00 | .03 | 189. | * | 2 | JAN | 1245 | 298 | .00 | .00 | .00 | 41.  |
| 2         | JAN | 0020 | 149 | .03 | .00 | .03 | 192. | * | 2 | JAN | 1250 | 299 | .00 | .00 | .00 | 34.  |
| 2         | JAN | 0025 | 150 | .03 | .00 | .03 | 195. | * | 2 | JAN | 1255 | 300 | .00 | .00 | .00 | 27.  |

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TOTAL RAINFALL = 8.44, TOTAL LOSS = .96, TOTAL EXCESS = 7.48

| PEAK FLOW | TIME  | 6-HR  | 24-HR    | 72-HR | 24.92-HR |
|-----------|-------|-------|----------|-------|----------|
| +         | (CFS) | (HR)  | (CFS)    |       |          |
| +         | 1656. | 16.58 | 572.     | 221.  | 213.     |
|           |       |       | (INCHES) | 7.463 | 7.463    |
|           |       |       | (AC-FT)  | 283.  | 438.     |

CUMULATIVE AREA = 1.10 SQ MI

1

STATION BASIN400

| (O) OUTFLOW |      |      |      |      |       |       |       |       |       |    |    |
|-------------|------|------|------|------|-------|-------|-------|-------|-------|----|----|
| 0.          | 200. | 400. | 600. | 800. | 1000. | 1200. | 1400. | 1600. | 1800. | 0. | 0. |
| DAHRMN      | PER  |      |      |      |       |       |       |       |       |    |    |
| 11200       | 10   |      |      |      |       |       |       |       |       |    |    |
| 11205       | 20   |      |      |      |       |       |       |       |       |    |    |
| 11210       | 30   |      |      |      |       |       |       |       |       |    |    |
| 11215       | 40   |      |      |      |       |       |       |       |       |    |    |
| 11220       | 50   |      |      |      |       |       |       |       |       |    |    |
| 11225       | 60   |      |      |      |       |       |       |       |       |    |    |
| 11230       | 70   |      |      |      |       |       |       |       |       |    |    |
| 11235       | 80   |      |      |      |       |       |       |       |       |    |    |
| 11240       | 90   |      |      |      |       |       |       |       |       |    |    |
| 11245       | 100  |      |      |      |       |       |       |       |       |    |    |
| 11250       | 110  |      |      |      |       |       |       |       |       |    |    |
| 11255       | 120  |      |      |      |       |       |       |       |       |    |    |
| 11300       | 130  |      |      |      |       |       |       |       |       |    |    |
| 11305       | 140  |      |      |      |       |       |       |       |       |    |    |
| 11310       | 150  |      |      |      |       |       |       |       |       |    |    |
| 11315       | 160  |      |      |      |       |       |       |       |       |    |    |
| 11320       | 170  |      |      |      |       |       |       |       |       |    |    |
| 11325       | 180  |      |      |      |       |       |       |       |       |    |    |
| 11330       | 190  |      |      |      |       |       |       |       |       |    |    |
| 11335       | 200  |      |      |      |       |       |       |       |       |    |    |
| 11340       | 210  |      |      |      |       |       |       |       |       |    |    |
| 11345       | 220  |      |      |      |       |       |       |       |       |    |    |
| 11350       | 230  |      |      |      |       |       |       |       |       |    |    |
| 11355       | 240  |      |      |      |       |       |       |       |       |    |    |
| 11400       | 250  |      |      |      |       |       |       |       |       |    |    |
| 11405       | 260  |      |      |      |       |       |       |       |       |    |    |
| 11410       | 270  |      |      |      |       |       |       |       |       |    |    |
| 11415       | 280  |      |      |      |       |       |       |       |       |    |    |
| 11420       | 290  |      |      |      |       |       |       |       |       |    |    |
| 11425       | 30.0 |      |      |      |       |       |       |       |       |    |    |

(L) PRECIP, (X) EXCESS

| RGCTK.001   |
|-------------|
| 11430 31.0  |
| 11435 32.0  |
| 11440 33.0  |
| 11445 34.0  |
| 11450 35.0  |
| 11455 36.0  |
| 11500 37.0  |
| 11505 38.0  |
| 11510 39.0  |
| 11515 40.0  |
| 11520 41.0  |
| 11525 42.0  |
| 11530 43.0  |
| 11535 44.0  |
| 11540 45.0  |
| 11545 46.0  |
| 11550 47.0  |
| 11555 48.0  |
| 11600 49.0  |
| 11605 50.0  |
| 11610 51.0  |
| 11615 52.0  |
| 11620 53.0  |
| 11625 54.0  |
| 11630 55.0  |
| 11635 56.0  |
| 11640 57.0  |
| 11645 58.0  |
| 11650 59.0  |
| 11655 60.0  |
| 11700 61.0  |
| 11705 62.0  |
| 11710 63.0  |
| 11715 64.0  |
| 11720 65.0  |
| 11725 66.0  |
| 11730 67.0  |
| 11735 68.0  |
| 11740 69.0  |
| 11745 70.0  |
| 11750 71.0  |
| 11755 72.0  |
| 11800 73.0  |
| 11805 74.0  |
| 11810 75.0  |
| 11815 76.0  |
| 11820 77.0  |
| 11825 78.0  |
| 11830 79.0  |
| 11835 80.0  |
| 11840 81.0  |
| 11845 82.0  |
| 11850 83.0  |
| 11855 84.0  |
| 11900 85.0  |
| 11905 86.0  |
| 11910 87.0  |
| 11915 88.0  |
| 11920 89.0  |
| 11925 90.0  |
| 11930 91.0  |
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| 12055 108.0 |
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| 12130 115.0 |
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| 12145 118.0 |
| 12150 119.0 |
| 12155 120.0 |
| 12200 121.0 |
| 12205 122.0 |
| 12210 123.0 |
| 12215 124.0 |
| 12220 125.0 |
| 12225 126.0 |
| 12230 127.0 |
| 12235 128.0 |
| 12240 129.0 |
| 12245 130.0 |
| 12250 131.0 |
| 12255 132.0 |
| 12300 133.0 |
| 12305 134.0 |
| 12310 135.0 |
| 12315 136.0 |
| 12320 137.0 |
| 12325 138.0 |
| 12330 139.0 |
| 12335 140.0 |
| 12340 141.0 |
| 12345 142.0 |
| 12350 143.0 |
| 12355 144.0 |
| 20000 145.0 |
| 20005 146.0 |
| 20010 147.0 |
| 20015 148.0 |





```

21010 267. 0
21015 268. 0
21020 269. 0
21025 270. 0
21030 271. 0
21035 272. 0
21040 273. 0
21045 274. 0
21050 275. 0
21055 276. 0
21100 277. 0
21105 278. 0
21110 279. 0
21115 280. 0
21120 281. 0
21125 282. 0
21130 283. 0
21135 284. 0
21140 285. 0
21145 286. 0
21150 287. 0
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21200 289. 0
21205 290. 0
21210 291. 0
21215 292. 0
21220 293. 0
21225 294. 0
21230 295. 0
21235 296. 0
21240 297. 0
21245 298. 0
21250 299. 0
21255 300.0

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1

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*                               *
212 KK * CONFLUEN * CE_100_THROUGH_400
*                               *
*****

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213 HC HYDROGRAPH COMBINATION  
ICOMP 4 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION CONFLUEN  
SUM OF 4 HYDROGRAPHS

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| DA | MON | HRMN | ORD | FLOW  | DA | MON | HRMN | ORD | FLOW  | DA | MON | HRMN | ORD | FLOW   | DA | MON | HRMN | ORD | FLOW   |
|----|-----|------|-----|-------|----|-----|------|-----|-------|----|-----|------|-----|--------|----|-----|------|-----|--------|
| 1  | JAN | 1200 | 1   | 0.    | 1  | JAN | 1815 | 76  | 2817. | 2  | JAN | 0030 | 151 | 5246.  | 2  | JAN | 0645 | 226 | 12340. |
| 1  | JAN | 1205 | 2   | 0.    | 1  | JAN | 1820 | 77  | 2858. | 2  | JAN | 0035 | 152 | 5263.  | 2  | JAN | 0650 | 227 | 11949. |
| 1  | JAN | 1210 | 3   | 0.    | 1  | JAN | 1825 | 78  | 2898. | 2  | JAN | 0040 | 153 | 5277.  | 2  | JAN | 0655 | 228 | 11557. |
| 1  | JAN | 1215 | 4   | 0.    | 1  | JAN | 1830 | 79  | 2937. | 2  | JAN | 0045 | 154 | 5289.  | 2  | JAN | 0700 | 229 | 11144. |
| 1  | JAN | 1220 | 5   | 0.    | 1  | JAN | 1835 | 80  | 2976. | 2  | JAN | 0050 | 155 | 5299.  | 2  | JAN | 0705 | 230 | 10738. |
| 1  | JAN | 1225 | 6   | 0.    | 1  | JAN | 1840 | 81  | 3014. | 2  | JAN | 0055 | 156 | 5308.  | 2  | JAN | 0710 | 231 | 10359. |
| 1  | JAN | 1230 | 7   | 0.    | 1  | JAN | 1845 | 82  | 3051. | 2  | JAN | 0100 | 157 | 5315.  | 2  | JAN | 0715 | 232 | 10001. |
| 1  | JAN | 1235 | 8   | 0.    | 1  | JAN | 1850 | 83  | 3088. | 2  | JAN | 0105 | 158 | 5320.  | 2  | JAN | 0720 | 233 | 9670.  |
| 1  | JAN | 1240 | 9   | 0.    | 1  | JAN | 1855 | 84  | 3124. | 2  | JAN | 0110 | 159 | 5323.  | 2  | JAN | 0725 | 234 | 9380.  |
| 1  | JAN | 1245 | 10  | 0.    | 1  | JAN | 1900 | 85  | 3160. | 2  | JAN | 0115 | 160 | 5325.  | 2  | JAN | 0730 | 235 | 9105.  |
| 1  | JAN | 1250 | 11  | 0.    | 1  | JAN | 1905 | 86  | 3196. | 2  | JAN | 0120 | 161 | 5326.  | 2  | JAN | 0735 | 236 | 8852.  |
| 1  | JAN | 1255 | 12  | 0.    | 1  | JAN | 1910 | 87  | 3230. | 2  | JAN | 0125 | 162 | 5326.  | 2  | JAN | 0740 | 237 | 8614.  |
| 1  | JAN | 1300 | 13  | 1.    | 1  | JAN | 1915 | 88  | 3265. | 2  | JAN | 0130 | 163 | 5325.  | 2  | JAN | 0745 | 238 | 8390.  |
| 1  | JAN | 1305 | 14  | 1.    | 1  | JAN | 1920 | 89  | 3299. | 2  | JAN | 0135 | 164 | 5325.  | 2  | JAN | 0750 | 239 | 8184.  |
| 1  | JAN | 1310 | 15  | 3.    | 1  | JAN | 1925 | 90  | 3333. | 2  | JAN | 0140 | 165 | 5325.  | 2  | JAN | 0755 | 240 | 7988.  |
| 1  | JAN | 1315 | 16  | 5.    | 1  | JAN | 1930 | 91  | 3366. | 2  | JAN | 0145 | 166 | 5326.  | 2  | JAN | 0800 | 241 | 7816.  |
| 1  | JAN | 1320 | 17  | 9.    | 1  | JAN | 1935 | 92  | 3399. | 2  | JAN | 0150 | 167 | 5329.  | 2  | JAN | 0805 | 242 | 7650.  |
| 1  | JAN | 1325 | 18  | 14.   | 1  | JAN | 1940 | 93  | 3431. | 2  | JAN | 0155 | 168 | 5334.  | 2  | JAN | 0810 | 243 | 7491.  |
| 1  | JAN | 1330 | 19  | 21.   | 1  | JAN | 1945 | 94  | 3464. | 2  | JAN | 0200 | 169 | 5344.  | 2  | JAN | 0815 | 244 | 7338.  |
| 1  | JAN | 1335 | 20  | 31.   | 1  | JAN | 1950 | 95  | 3496. | 2  | JAN | 0205 | 170 | 5359.  | 2  | JAN | 0820 | 245 | 7193.  |
| 1  | JAN | 1340 | 21  | 43.   | 1  | JAN | 1955 | 96  | 3527. | 2  | JAN | 0210 | 171 | 5383.  | 2  | JAN | 0825 | 246 | 7057.  |
| 1  | JAN | 1345 | 22  | 59.   | 1  | JAN | 2000 | 97  | 3559. | 2  | JAN | 0215 | 172 | 5419.  | 2  | JAN | 0830 | 247 | 6924.  |
| 1  | JAN | 1350 | 23  | 77.   | 1  | JAN | 2005 | 98  | 3590. | 2  | JAN | 0220 | 173 | 5466.  | 2  | JAN | 0835 | 248 | 6796.  |
| 1  | JAN | 1355 | 24  | 99.   | 1  | JAN | 2010 | 99  | 3621. | 2  | JAN | 0225 | 174 | 5524.  | 2  | JAN | 0840 | 249 | 6672.  |
| 1  | JAN | 1400 | 25  | 125.  | 1  | JAN | 2015 | 100 | 3651. | 2  | JAN | 0230 | 175 | 5590.  | 2  | JAN | 0845 | 250 | 6558.  |
| 1  | JAN | 1405 | 26  | 153.  | 1  | JAN | 2020 | 101 | 3682. | 2  | JAN | 0235 | 176 | 5663.  | 2  | JAN | 0850 | 251 | 6453.  |
| 1  | JAN | 1410 | 27  | 185.  | 1  | JAN | 2025 | 102 | 3712. | 2  | JAN | 0240 | 177 | 5743.  | 2  | JAN | 0855 | 252 | 6350.  |
| 1  | JAN | 1415 | 28  | 220.  | 1  | JAN | 2030 | 103 | 3743. | 2  | JAN | 0245 | 178 | 5827.  | 2  | JAN | 0900 | 253 | 6249.  |
| 1  | JAN | 1420 | 29  | 258.  | 1  | JAN | 2035 | 104 | 3773. | 2  | JAN | 0250 | 179 | 5917.  | 2  | JAN | 0905 | 254 | 6150.  |
| 1  | JAN | 1425 | 30  | 299.  | 1  | JAN | 2040 | 105 | 3803. | 2  | JAN | 0255 | 180 | 6013.  | 2  | JAN | 0910 | 255 | 6057.  |
| 1  | JAN | 1430 | 31  | 342.  | 1  | JAN | 2045 | 106 | 3832. | 2  | JAN | 0300 | 181 | 6116.  | 2  | JAN | 0915 | 256 | 5967.  |
| 1  | JAN | 1435 | 32  | 388.  | 1  | JAN | 2050 | 107 | 3862. | 2  | JAN | 0305 | 182 | 6228.  | 2  | JAN | 0920 | 257 | 5879.  |
| 1  | JAN | 1440 | 33  | 437.  | 1  | JAN | 2055 | 108 | 3892. | 2  | JAN | 0310 | 183 | 6348.  | 2  | JAN | 0925 | 258 | 5792.  |
| 1  | JAN | 1445 | 34  | 488.  | 1  | JAN | 2100 | 109 | 3921. | 2  | JAN | 0315 | 184 | 6480.  | 2  | JAN | 0930 | 259 | 5709.  |
| 1  | JAN | 1450 | 35  | 541.  | 1  | JAN | 2105 | 110 | 3951. | 2  | JAN | 0320 | 185 | 6630.  | 2  | JAN | 0935 | 260 | 5631.  |
| 1  | JAN | 1455 | 36  | 596.  | 1  | JAN | 2110 | 111 | 3980. | 2  | JAN | 0325 | 186 | 6805.  | 2  | JAN | 0940 | 261 | 5555.  |
| 1  | JAN | 1500 | 37  | 653.  | 1  | JAN | 2115 | 112 | 4010. | 2  | JAN | 0330 | 187 | 7011.  | 2  | JAN | 0945 | 262 | 5482.  |
| 1  | JAN | 1505 | 38  | 711.  | 1  | JAN | 2120 | 113 | 4040. | 2  | JAN | 0335 | 188 | 7255.  | 2  | JAN | 0950 | 263 | 5410.  |
| 1  | JAN | 1510 | 39  | 771.  | 1  | JAN | 2125 | 114 | 4069. | 2  | JAN | 0340 | 189 | 7536.  | 2  | JAN | 0955 | 264 | 5340.  |
| 1  | JAN | 1515 | 40  | 832.  | 1  | JAN | 2130 | 115 | 4099. | 2  | JAN | 0345 | 190 | 7850.  | 2  | JAN | 1000 | 265 | 5277.  |
| 1  | JAN | 1520 | 41  | 894.  | 1  | JAN | 2135 | 116 | 4128. | 2  | JAN | 0350 | 191 | 8197.  | 2  | JAN | 1005 | 266 | 5214.  |
| 1  | JAN | 1525 | 42  | 957.  | 1  | JAN | 2140 | 117 | 4158. | 2  | JAN | 0355 | 192 | 8593.  | 2  | JAN | 1010 | 267 | 5154.  |
| 1  | JAN | 1530 | 43  | 1021. | 1  | JAN | 2145 | 118 | 4188. | 2  | JAN | 0400 | 193 | 9189.  | 2  | JAN | 1015 | 268 | 5095.  |
| 1  | JAN | 1535 | 44  | 1085. | 1  | JAN | 2150 | 119 | 4218. | 2  | JAN | 0405 | 194 | 9956.  | 2  | JAN | 1020 | 269 | 5038.  |
| 1  | JAN | 1540 | 45  | 1150. | 1  | JAN | 2155 | 120 | 4247. | 2  | JAN | 0410 | 195 | 10901. | 2  | JAN | 1025 | 270 | 4984.  |
| 1  | JAN | 1545 | 46  | 1214. | 1  | JAN | 2200 | 121 | 4278. | 2  | JAN | 0415 | 196 | 12008. | 2  | JAN | 1030 | 271 | 4931.  |
| 1  | JAN | 1550 | 47  | 1279. | 1  | JAN | 2205 | 122 | 4308. | 2  | JAN | 0420 | 197 | 13129. | 2  | JAN | 1035 | 272 | 4880.  |
| 1  | JAN | 1555 | 48  | 1343. | 1  | JAN | 2210 | 123 | 4338. | 2  | JAN | 0425 | 198 | 14091. | 2  | JAN | 1040 | 273 | 4830.  |
| 1  | JAN | 1600 | 49  | 1407. | 1  | JAN | 2215 | 124 | 4369. | 2  | JAN | 0430 | 199 | 14790. | 2  | JAN | 1045 | 274 | 4781.  |
| 1  | JAN | 1605 | 50  | 1470. | 1  | JAN | 2220 | 125 | 4399. | 2  | JAN | 0435 | 200 | 15264. | 2  | JAN | 1050 | 275 | 4735.  |
| 1  | JAN | 1610 | 51  | 1533. | 1  | JAN | 2225 | 126 | 4430. | 2  | JAN | 0440 | 201 | 15549. | 2  | JAN | 1055 | 276 | 4690.  |
| 1  | JAN | 1615 | 52  | 1595. | 1  | JAN | 2230 | 127 | 4461. | 2  | JAN | 0445 | 202 | 15685. | 2  | JAN | 1100 | 277 | 4646.  |
| 1  | JAN | 1620 | 53  | 1656. | 1  | JAN | 2235 | 128 | 4492. | 2  | JAN | 0450 | 203 | 15734. | 2  | JAN | 1105 | 278 | 4603.  |
| 1  | JAN | 1625 | 54  | 1717. | 1  | JAN | 2240 | 129 | 4524. | 2  | JAN | 0455 | 204 | 15758. | 2  | JAN | 1110 | 279 | 4562.  |
| 1  | JAN | 1630 | 55  | 1776. | 1  | JAN | 2245 | 130 | 4556. | 2  | JAN | 0500 | 205 | 15778. | 2  | JAN | 1115 | 280 | 4522.  |
| 1  | JAN | 1635 | 56  | 1835. | 1  | JAN | 2250 | 131 | 4588. | 2  | JAN | 0505 | 206 | 15788. | 2  | JAN | 1120 | 281 | 4483.  |
| 1  | JAN | 1640 | 57  | 1893. | 1  | JAN | 2255 | 132 | 4620. | 2  | JAN | 0510 | 207 | 15828. | 2  | JAN | 1125 | 282 | 4445.  |

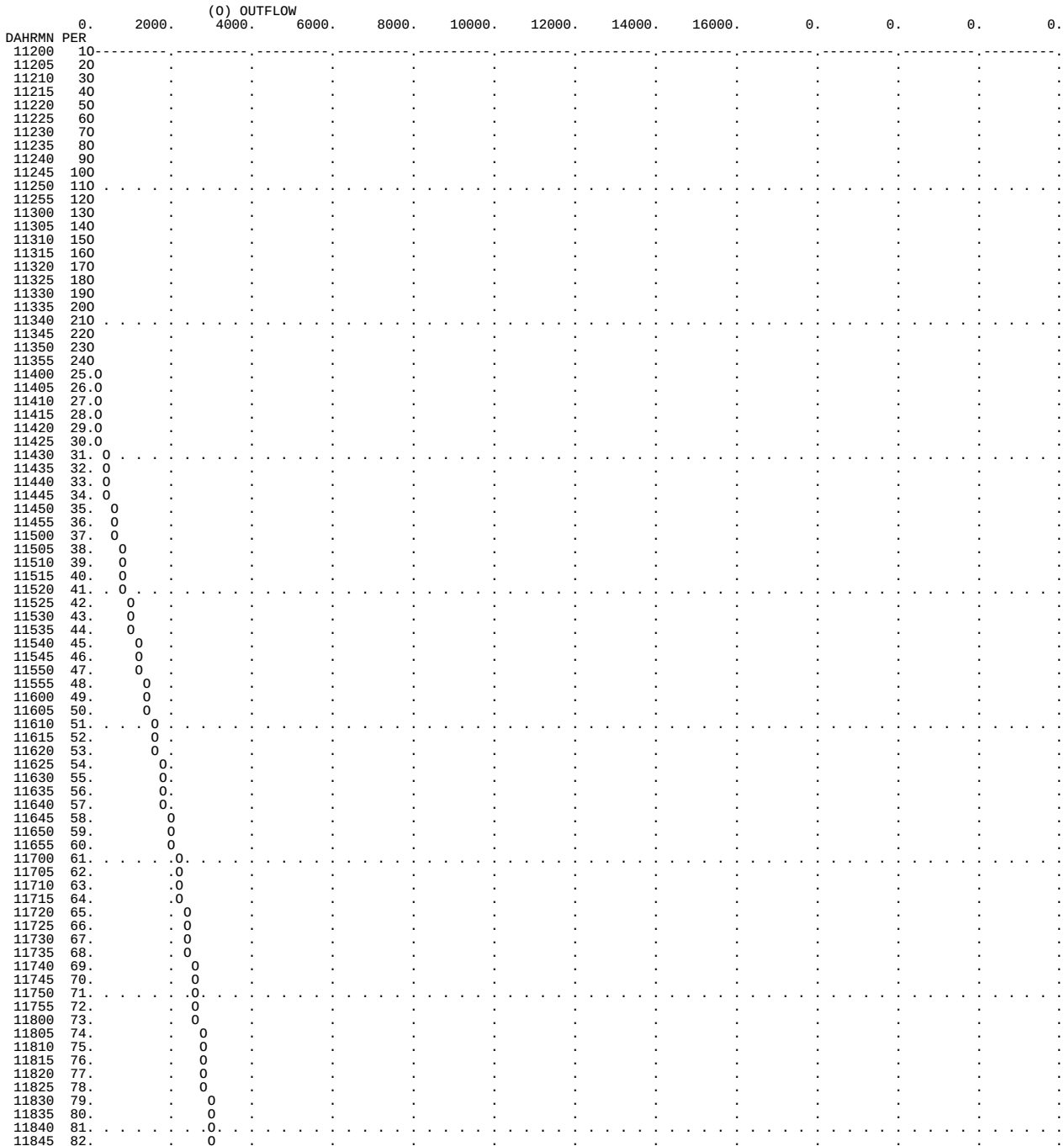
|            |    |       |   | RGCrk.OUT  |     |       |   |            |     |        |   |            |     |       |
|------------|----|-------|---|------------|-----|-------|---|------------|-----|--------|---|------------|-----|-------|
| 1 JAN 1645 | 58 | 1949. | * | 1 JAN 2300 | 133 | 4653. | * | 2 JAN 0515 | 208 | 15890. | * | 2 JAN 1130 | 283 | 4407. |
| 1 JAN 1650 | 59 | 2005. | * | 1 JAN 2305 | 134 | 4686. | * | 2 JAN 0520 | 209 | 15954. | * | 2 JAN 1135 | 284 | 4372. |
| 1 JAN 1655 | 60 | 2060. | * | 1 JAN 2310 | 135 | 4719. | * | 2 JAN 0525 | 210 | 15976. | * | 2 JAN 1140 | 285 | 4338. |
| 1 JAN 1700 | 61 | 2114. | * | 1 JAN 2315 | 136 | 4753. | * | 2 JAN 0530 | 211 | 15988. | * | 2 JAN 1145 | 286 | 4304. |
| 1 JAN 1705 | 62 | 2167. | * | 1 JAN 2320 | 137 | 4787. | * | 2 JAN 0535 | 212 | 15940. | * | 2 JAN 1150 | 287 | 4271. |
| 1 JAN 1710 | 63 | 2219. | * | 1 JAN 2325 | 138 | 4822. | * | 2 JAN 0540 | 213 | 15867. | * | 2 JAN 1155 | 288 | 4239. |
| 1 JAN 1715 | 64 | 2270. | * | 1 JAN 2330 | 139 | 4857. | * | 2 JAN 0545 | 214 | 15760. | * | 2 JAN 1200 | 289 | 4205. |
| 1 JAN 1720 | 65 | 2320. | * | 1 JAN 2335 | 140 | 4893. | * | 2 JAN 0550 | 215 | 15603. | * | 2 JAN 1205 | 290 | 4166. |
| 1 JAN 1725 | 66 | 2370. | * | 1 JAN 2340 | 141 | 4929. | * | 2 JAN 0555 | 216 | 15433. | * | 2 JAN 1210 | 291 | 4119. |
| 1 JAN 1730 | 67 | 2418. | * | 1 JAN 2345 | 142 | 4965. | * | 2 JAN 0600 | 217 | 15228. | * | 2 JAN 1215 | 292 | 4060. |
| 1 JAN 1735 | 68 | 2466. | * | 1 JAN 2350 | 143 | 5002. | * | 2 JAN 0605 | 218 | 15006. | * | 2 JAN 1220 | 293 | 3988. |
| 1 JAN 1740 | 69 | 2512. | * | 1 JAN 2355 | 144 | 5040. | * | 2 JAN 0610 | 219 | 14734. | * | 2 JAN 1225 | 294 | 3905. |
| 1 JAN 1745 | 70 | 2558. | * | 2 JAN 0000 | 145 | 5077. | * | 2 JAN 0615 | 220 | 14428. | * | 2 JAN 1230 | 295 | 3814. |
| 1 JAN 1750 | 71 | 2603. | * | 2 JAN 0005 | 146 | 5113. | * | 2 JAN 0620 | 221 | 14111. | * | 2 JAN 1235 | 296 | 3718. |
| 1 JAN 1755 | 72 | 2648. | * | 2 JAN 0010 | 147 | 5147. | * | 2 JAN 0625 | 222 | 13775. | * | 2 JAN 1240 | 297 | 3619. |
| 1 JAN 1800 | 73 | 2691. | * | 2 JAN 0015 | 148 | 5177. | * | 2 JAN 0630 | 223 | 13434. | * | 2 JAN 1245 | 298 | 3519. |
| 1 JAN 1805 | 74 | 2734. | * | 2 JAN 0020 | 149 | 5204. | * | 2 JAN 0635 | 224 | 13078. | * | 2 JAN 1250 | 299 | 3418. |
| 1 JAN 1810 | 75 | 2776. | * | 2 JAN 0025 | 150 | 5227. | * | 2 JAN 0640 | 225 | 12715. | * | 2 JAN 1255 | 300 | 3317. |

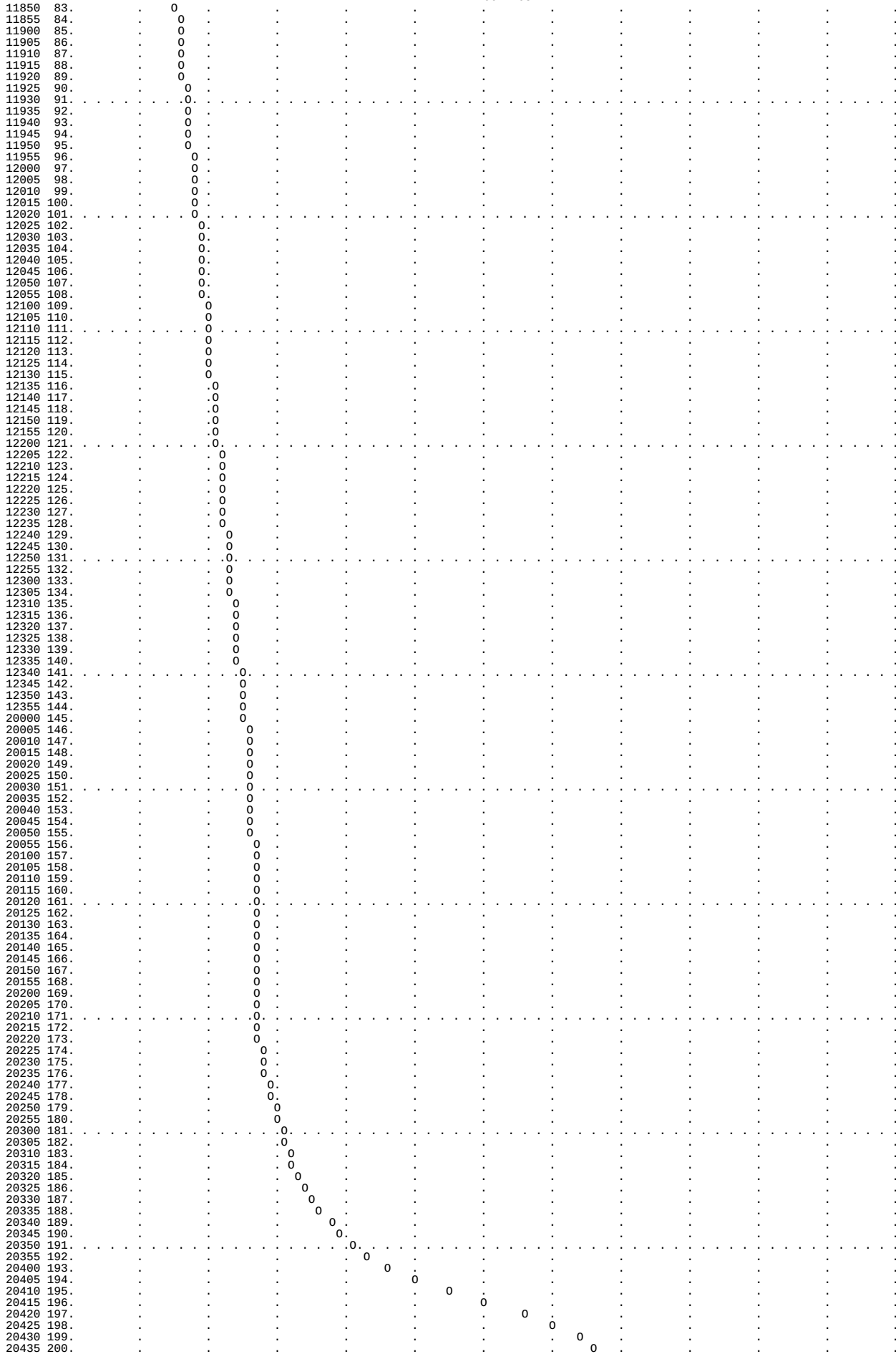
\*\*\*\*\*

| PEAK FLOW         | TIME  | MAXIMUM AVERAGE FLOW |        |        |          |
|-------------------|-------|----------------------|--------|--------|----------|
| (CFS)             | (HR)  | 6-HR                 | 24-HR  | 72-HR  | 24.92-HR |
| 15988.            | 17.50 | 11057.               | 5424.  | 5225.  | 5225.    |
|                   |       | 3.713                | 7.285  | 7.285  | 7.285    |
|                   |       | 5483.                | 10759. | 10759. | 10759.   |
| CUMULATIVE AREA = |       | 27.69 SQ MI          |        |        |          |

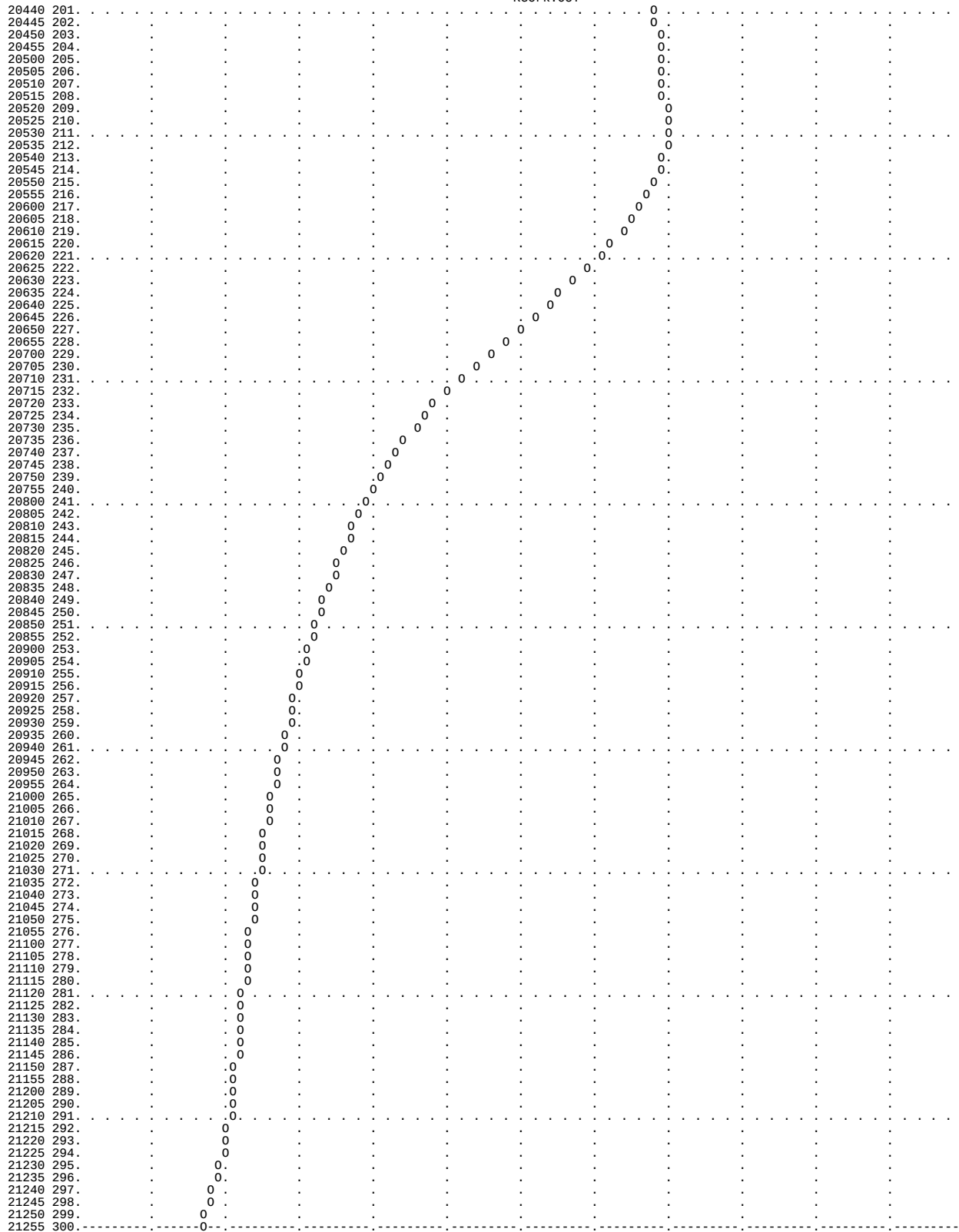
1

STATION CONFLUEN





RGCrk.OUT



1  
1

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

|   | OPERATION     | STATION  | PEAK<br>FLOW | TIME OF<br>PEAK | AVERAGE FLOW FOR MAXIMUM PERIOD |         |         | BASIN<br>AREA | MAXIMUM<br>STAGE | TIME OF<br>MAX STAGE |
|---|---------------|----------|--------------|-----------------|---------------------------------|---------|---------|---------------|------------------|----------------------|
| + |               |          |              |                 | 6-HOUR                          | 24-HOUR | 72-HOUR |               |                  |                      |
| + | HYDROGRAPH AT | BASIN100 | 13740.       | 17.75           | 8799.                           | 4383.   | 4222.   | 22.50         |                  |                      |
| + | HYDROGRAPH AT | BASIN200 | 3577.        | 16.58           | 1303.                           | 607.    | 585.    | 3.03          |                  |                      |
| + | HYDROGRAPH AT | BASIN300 | 1780.        | 16.42           | 552.                            | 213.    | 205.    | 1.06          |                  |                      |

|   |               |          |        |       |        |       |       |       |
|---|---------------|----------|--------|-------|--------|-------|-------|-------|
| + | HYDROGRAPH AT | BASIN400 | 1656.  | 16.58 | 572.   | 221.  | 213.  | 1.10  |
| + | 4 COMBINED AT | CONFLUEN | 15988. | 17.50 | 11057. | 5424. | 5225. | 27.69 |

\*\*\* NORMAL END OF HEC-1 \*\*\*

## **Appendix C**

AES Program Output and Support Material – Local Drainage Basin

County of San Diego Isopluvial Maps  
(100-Year, 6-Hour and 24-Hour)



# County of San Diego Hydrology Manual



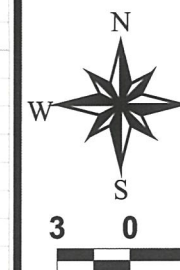
## Rainfall Isopluvials

### 100 Year Rainfall Event - 6 Hours

----- Isopluvial (inches)

P<sub>6</sub> = 3.5 inches

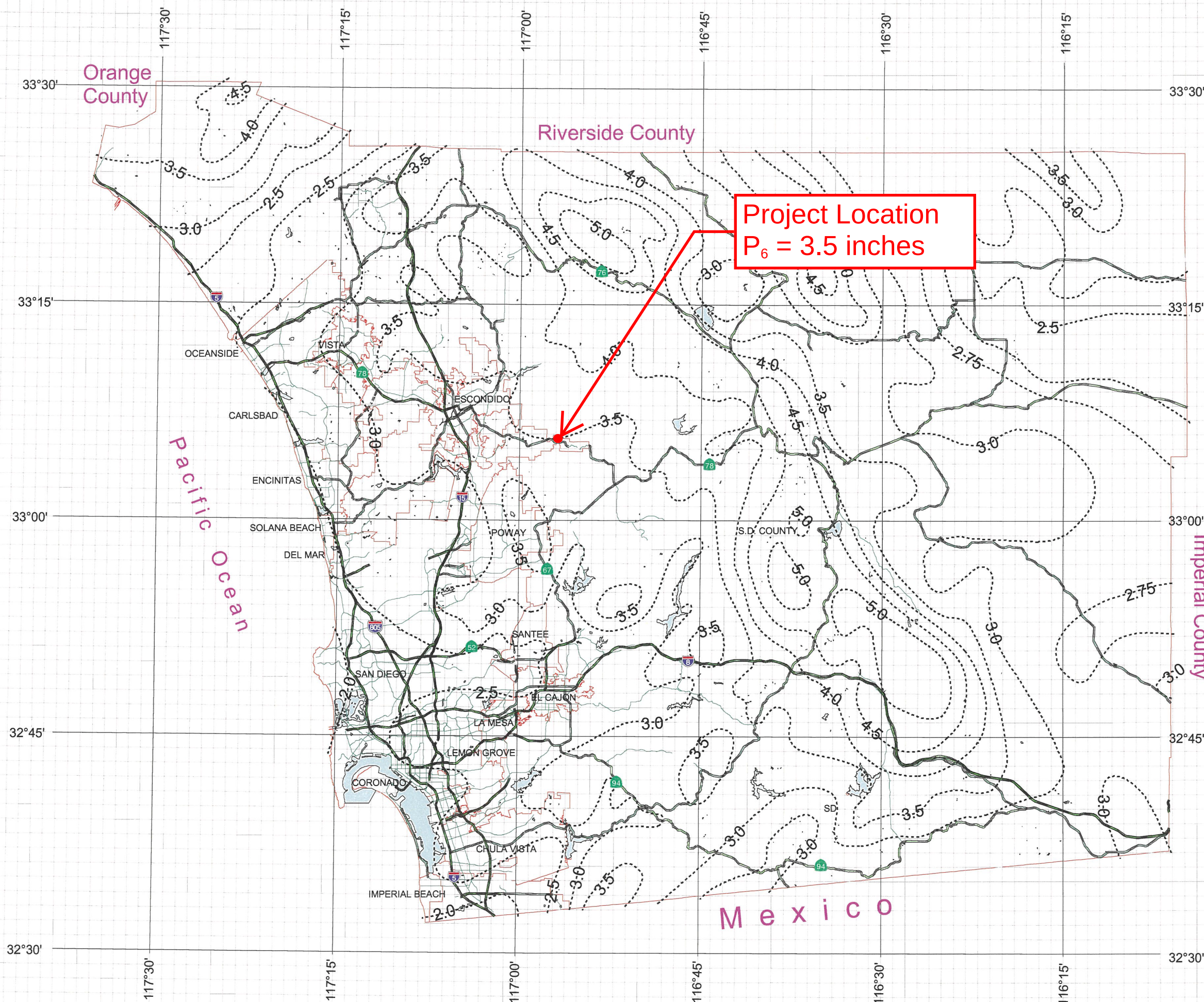
Rancho Guejito Tasting  
Room & Event Center



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# County of San Diego Hydrology Manual



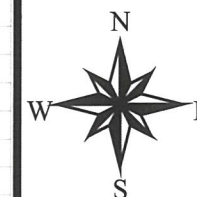
## Rainfall Isopluvials

### 100 Year Rainfall Event - 24 Hours

----- Isopluvial (inches)

$P_{24} = 6$  inches

Rancho Guejito Tasting  
Room & Event Center

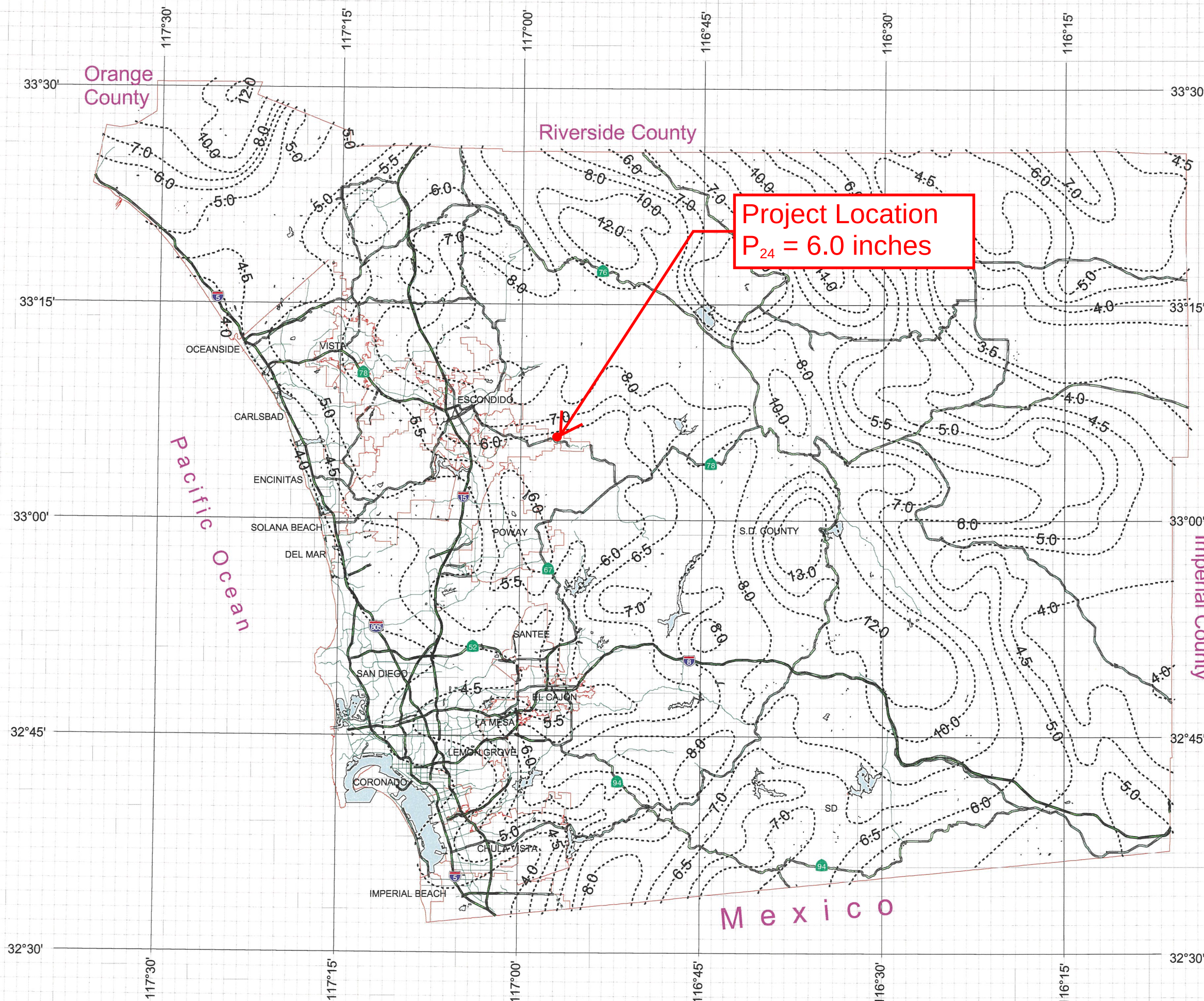


3 0 3 Miles

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## AES Output and Results (Pre-Project Conditions)

\*\*\*\*\*  
 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT  
 2003,1985,1981 HYDROLOGY MANUAL  
 (c) Copyright 1982-2014 Advanced Engineering Software (aes)  
 Ver. 21.0 Release Date: 06/01/2014 License ID 1261

Analysis prepared by:

RICK ENGINEERING COMPANY  
 5620 Friars Road  
 San Diego, California 92110  
 619-291-0707 Fax 619-291-4165

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
 \* RANCHO GUEJITO TASTING FACILITY AND EVENT CENTER - LOCAL HYDROLOGY ANALY \*  
 \* EXISTING CONDITIONS (100-YEAR STORM EVENT) \*  
 \* RICK ENGINEERING COMPANY, 2019 \*  
 \*\*\*\*\*

FILE NAME: RGW\_E00.DAT  
 TIME/DATE OF STUDY: 18:46 10/23/2019

-----  
 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
 -----

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.500  
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00  
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS  
 \*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*  

| NO. | WIDTH<br>(FT) | CROWN TO<br>CROSSFALL<br>(FT) | STREET-CROSSFALL<br>IN- / OUT- / PARK-<br>SIDE / SIDE / WAY | HEIGHT<br>(FT) | CURB GUTTER-GEOMETRIES:<br>WIDTH LIP HIKE<br>(FT) (FT) (FT) | MANING<br>FACTOR<br>(n) |
|-----|---------------|-------------------------------|-------------------------------------------------------------|----------------|-------------------------------------------------------------|-------------------------|
| 1   | 30.0          | 20.0                          | 0.018/0.018/0.020                                           | 0.67           | 2.00 0.0313 0.167                                           | 0.0150                  |

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET  
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
  2. (Depth)\*(Velocity) constraint = 6.0 (FT\*FT/S)
- \*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00  
 UPSTREAM ELEVATION(FEET) = 1520.00  
 DOWNSTREAM ELEVATION(FEET) = 1480.00  
 ELEVATION DIFFERENCE(FEET) = 40.00  
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.979  
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.217  
 SUBAREA RUNOFF(CFS) = 0.25  
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.25

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 51  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1480.00 DOWNSTREAM(FEET) = 450.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 3300.00 CHANNEL SLOPE = 0.3121  
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 3.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 5.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.743  
 SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 65.61  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 12.38  
 AVERAGE FLOW DEPTH(FEET) = 0.74 TRAVEL TIME(MIN.) = 4.44  
 Tc(MIN.) = 10.42  
 SUBAREA AREA(ACRES) = 74.00 SUBAREA RUNOFF(CFS) = 127.48  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.300  
 TOTAL AREA(ACRES) = 74.1 PEAK FLOW RATE(CFS) = 127.66

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 1.05 FLOW VELOCITY(FEET/SEC.) = 14.96  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 3380.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

ELEVATION DATA: UPSTREAM(FEET) = 450.00 DOWNSTREAM(FEET) = 442.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 900.00 CHANNEL SLOPE = 0.0089  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.233  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2500  
 SOIL CLASSIFICATION IS "B"  
 S.C.S. CURVE NUMBER (AMC II) = 85  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 148.05  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.38  
 AVERAGE FLOW DEPTH(FEET) = 1.02 TRAVEL TIME(MIN.) = 6.30  
 Tc(MIN.) = 16.72  
 SUBAREA AREA(ACRES) = 38.40 SUBAREA RUNOFF(CFS) = 40.64  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.283  
 TOTAL AREA(ACRES) = 112.5 PEAK FLOW RATE(CFS) = 134.75

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.98 FLOW VELOCITY(FEET/SEC.) = 2.32  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 4280.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

ELEVATION DATA: UPSTREAM(FEET) = 442.00 DOWNSTREAM(FEET) = 428.80  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 2200.00 CHANNEL SLOPE = 0.0060  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.646  
 \*USER SPECIFIED(SUBAREA):  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2300  
 S.C.S. CURVE NUMBER (AMC II) = 85  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 149.01  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.05  
 AVERAGE FLOW DEPTH(FEET) = 1.11 TRAVEL TIME(MIN.) = 17.92  
 Tc(MIN.) = 34.64  
 SUBAREA AREA(ACRES) = 46.50 SUBAREA RUNOFF(CFS) = 28.30  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.267  
 TOTAL AREA(ACRES) = 159.0 PEAK FLOW RATE(CFS) = 134.75

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 1.07 FLOW VELOCITY(FEET/SEC.) = 2.00  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 6480.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 104.00 TO NODE 110.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

ELEVATION DATA: UPSTREAM(FEET) = 428.80 DOWNSTREAM(FEET) = 425.50  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 620.00 CHANNEL SLOPE = 0.0053  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.411  
 \*USER SPECIFIED(SUBAREA):  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2290  
 S.C.S. CURVE NUMBER (AMC II) = 85  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 139.44  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.92  
 AVERAGE FLOW DEPTH(FEET) = 1.11 TRAVEL TIME(MIN.) = 5.38  
 Tc(MIN.) = 40.02  
 SUBAREA AREA(ACRES) = 17.00 SUBAREA RUNOFF(CFS) = 9.39  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.264  
 TOTAL AREA(ACRES) = 176.0 PEAK FLOW RATE(CFS) = 134.75

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 1.09 FLOW VELOCITY(FEET/SEC.) = 1.90  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 110.00 = 7100.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 10

&gt;&gt;&gt;&gt;MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 &lt;&lt;&lt;&lt;&lt;

```

*****
FLOW PROCESS FROM NODE    200.00 TO NODE    201.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(FEET) =      80.00
UPSTREAM ELEVATION(FEET) =    1010.00
DOWNSTREAM ELEVATION(FEET) =     970.00
ELEVATION DIFFERENCE(FEET) =      40.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      5.979
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  8.217
SUBAREA RUNOFF(CFS) =      0.25
TOTAL AREA(ACRES) =      0.10    TOTAL RUNOFF(CFS) =      0.25

*****
FLOW PROCESS FROM NODE    201.00 TO NODE    202.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =     970.00  DOWNSTREAM(FEET) =    450.00
CHANNEL LENGTH THRU SUBAREA(FEET) =  1500.00  CHANNEL SLOPE =  0.3467
CHANNEL BASE(FEET) =      5.00  "Z" FACTOR =  3.000
MANNING'S FACTOR = 0.045  MAXIMUM DEPTH(FEET) =  5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.144
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      11.24
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  7.34
AVERAGE FLOW DEPTH(FEET) =  0.26  TRAVEL TIME(MIN.) =  3.40
Tc(MIN.) =  9.38
SUBAREA AREA(ACRES) =     11.70    SUBAREA RUNOFF(CFS) =    21.57
AREA-AVERAGE RUNOFF COEFFICIENT =  0.300
TOTAL AREA(ACRES) =     11.8    PEAK FLOW RATE(CFS) =    21.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.39  FLOW VELOCITY(FEET/SEC.) =  9.14
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    202.00 =    1580.00 FEET.

*****
FLOW PROCESS FROM NODE    202.00 TO NODE    203.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    450.00  DOWNSTREAM(FEET) =    429.50
CHANNEL LENGTH THRU SUBAREA(FEET) =  2350.00  CHANNEL SLOPE =  0.0087
CHANNEL BASE(FEET) =     10.00  "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.040  MAXIMUM DEPTH(FEET) =  3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  2.767
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2750
S.C.S. CURVE NUMBER (AMC II) = 63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =     41.88
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  1.71
AVERAGE FLOW DEPTH(FEET) =  0.61  TRAVEL TIME(MIN.) = 22.94
Tc(MIN.) = 32.32
SUBAREA AREA(ACRES) =     50.00    SUBAREA RUNOFF(CFS) =    38.05
AREA-AVERAGE RUNOFF COEFFICIENT =  0.280
TOTAL AREA(ACRES) =     61.8    PEAK FLOW RATE(CFS) =    47.84

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.64  FLOW VELOCITY(FEET/SEC.) =  1.76
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    203.00 =    3930.00 FEET.

*****
FLOW PROCESS FROM NODE    203.00 TO NODE    203.00 IS CODE =   1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) =  32.32
RAINFALL INTENSITY(INCH/HR) =  2.77
TOTAL STREAM AREA(ACRES) =    61.80
PEAK FLOW RATE(CFS) AT CONFLUENCE =    47.84

*****
FLOW PROCESS FROM NODE    300.00 TO NODE    301.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

```

```

                                RGW_E00.RES
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00
UPSTREAM ELEVATION(FEET) = 707.00
DOWNSTREAM ELEVATION(FEET) = 657.00
ELEVATION DIFFERENCE(FEET) = 50.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.979
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.217
SUBAREA RUNOFF(CFS) = 0.74
TOTAL AREA(ACRES) = 0.30 TOTAL RUNOFF(CFS) = 0.74

*****
FLOW PROCESS FROM NODE 301.00 TO NODE 302.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 657.00 DOWNSTREAM(FEET) = 432.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1040.00 CHANNEL SLOPE = 0.2163
CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 3.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.053
*USER SPECIFIED(SUBAREA):
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .2900
S.C.S. CURVE NUMBER (AMC II) = 63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.22
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.78
AVERAGE FLOW DEPTH(FEET) = 0.20 TRAVEL TIME(MIN.) = 3.63
Tc(MIN.) = 9.60
SUBAREA AREA(ACRES) = 5.00 SUBAREA RUNOFF(CFS) = 8.78
AREA-AVERAGE RUNOFF COEFFICIENT = 0.291
TOTAL AREA(ACRES) = 5.3 PEAK FLOW RATE(CFS) = 9.32

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.27 FLOW VELOCITY(FEET/SEC.) = 5.90
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 302.00 = 1120.00 FEET.

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 203.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 432.00 DOWNSTREAM(FEET) = 429.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 250.00 CHANNEL SLOPE = 0.0100
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.003
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2250
S.C.S. CURVE NUMBER (AMC II) = 63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.68
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.26
AVERAGE FLOW DEPTH(FEET) = 0.32 TRAVEL TIME(MIN.) = 3.30
Tc(MIN.) = 12.91
SUBAREA AREA(ACRES) = 2.40 SUBAREA RUNOFF(CFS) = 2.70
AREA-AVERAGE RUNOFF COEFFICIENT = 0.270
TOTAL AREA(ACRES) = 7.7 PEAK FLOW RATE(CFS) = 10.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 FLOW VELOCITY(FEET/SEC.) = 1.27
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 203.00 = 1370.00 FEET.

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.91
RAINFALL INTENSITY(INCH/HR) = 5.00
TOTAL STREAM AREA(ACRES) = 7.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.41

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 47.84 32.32 2.767 61.80
2 10.41 12.91 5.003 7.70

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

```

| STREAM<br>NUMBER | RUNOFF<br>(CFS) | Tc<br>(MIN.) | INTENSITY<br>(INCH/HOUR) |
|------------------|-----------------|--------------|--------------------------|
| 1                | 29.51           | 12.91        | 5.003                    |
| 2                | 53.60           | 32.32        | 2.767                    |

## COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 53.60 Tc(MIN.) = 32.32  
 TOTAL AREA(ACRES) = 69.5  
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 203.00 = 3930.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 203.00 TO NODE 110.00 IS CODE = 51  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 429.50 DOWNSTREAM(FEET) = 425.50  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 760.00 CHANNEL SLOPE = 0.0053  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.386  
 \*USER SPECIFIED(SUBAREA):  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2470  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 55.37  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.52  
 AVERAGE FLOW DEPTH(FEET) = 0.76 TRAVEL TIME(MIN.) = 8.35  
 Tc(MIN.) = 40.67  
 SUBAREA AREA(ACRES) = 6.00 SUBAREA RUNOFF(CFS) = 3.54  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.276  
 TOTAL AREA(ACRES) = 75.5 PEAK FLOW RATE(CFS) = 53.60

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.75 FLOW VELOCITY(FEET/SEC.) = 1.50  
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 110.00 = 4690.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 40.67  
 RAINFALL INTENSITY(INCH/HR) = 2.39  
 TOTAL STREAM AREA(ACRES) = 75.50  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 53.60

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 400.00 TO NODE 401.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00  
 UPSTREAM ELEVATION(FEET) = 575.00  
 DOWNSTREAM ELEVATION(FEET) = 548.00  
 ELEVATION DIFFERENCE(FEET) = 27.00  
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.979  
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.217  
 SUBAREA RUNOFF(CFS) = 0.25  
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.25

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 51  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 548.00 DOWNSTREAM(FEET) = 432.80  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 820.00 CHANNEL SLOPE = 0.1405  
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 3.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 5.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.200  
 \*USER SPECIFIED(SUBAREA):  
 SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .2900  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.31  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.17  
 AVERAGE FLOW DEPTH(FEET) = 0.22 TRAVEL TIME(MIN.) = 3.27  
 Tc(MIN.) = 9.25  
 SUBAREA AREA(ACRES) = 5.50 SUBAREA RUNOFF(CFS) = 9.89  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.290  
 TOTAL AREA(ACRES) = 5.6 PEAK FLOW RATE(CFS) = 10.07



```

                                RGW_E00.RES
END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 FLOW VELOCITY(FEET/SEC.) = 5.22
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 402.00 = 900.00 FEET.
*****
FLOW PROCESS FROM NODE 402.00 TO NODE 110.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 432.80 DOWNSTREAM(FEET) = 425.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 780.00 CHANNEL SLOPE = 0.0094
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.880
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2620
S.C.S. CURVE NUMBER (AMC II) = 63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 13.00
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.32
AVERAGE FLOW DEPTH(FEET) = 0.36 TRAVEL TIME(MIN.) = 9.88
Tc(MIN.) = 19.14
SUBAREA AREA(ACRES) = 5.70 SUBAREA RUNOFF(CFS) = 5.79
AREA-AVERAGE RUNOFF COEFFICIENT = 0.276
TOTAL AREA(ACRES) = 11.3 PEAK FLOW RATE(CFS) = 12.10

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 FLOW VELOCITY(FEET/SEC.) = 1.28
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 110.00 = 1680.00 FEET.
*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 19.14
RAINFALL INTENSITY(INCH/HR) = 3.88
TOTAL STREAM AREA(ACRES) = 11.30
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.10

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 53.60 40.67 2.386 75.50
2 12.10 19.14 3.880 11.30

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 45.06 19.14 3.880
2 61.04 40.67 2.386

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 61.04 Tc(MIN.) = 40.67
TOTAL AREA(ACRES) = 86.8
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 110.00 = 4690.00 FEET.
*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 61.04 40.67 2.386 86.80
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 110.00 = 4690.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 134.75 40.02 2.411 176.00
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 110.00 = 7100.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 194.81 40.02 2.411
2 194.40 40.67 2.386

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

```

RGW\_E00.RES  
PEAK FLOW RATE(CFS) = 194.81 TC(MIN.) = 40.02  
TOTAL AREA(ACRES) = 262.8

=====

END OF STUDY SUMMARY:  
TOTAL AREA(ACRES) = 262.8 TC(MIN.) = 40.02  
PEAK FLOW RATE(CFS) = 194.81

=====

END OF RATIONAL METHOD ANALYSIS

±

## AES Output and Results (Post-Project Conditions)

\*\*\*\*\*  
 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT  
 2003,1985,1981 HYDROLOGY MANUAL  
 (c) Copyright 1982-2014 Advanced Engineering Software (aes)  
 Ver. 21.0 Release Date: 06/01/2014 License ID 1261

Analysis prepared by:

RICK ENGINEERING COMPANY  
 5620 Friars Road  
 San Diego, California 92110  
 619-291-0707 Fax 619-291-4165

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
 \* RANCHO GUEJITO TASTING FACILITY AND EVENT CENTER - LOCAL HYDROLOGY ANALY \*  
 \* PROPOSED CONDITIONS (100-YEAR STROM EVENT) \*  
 \* RICK ENGINEERING COMPANY, 2019 \*  
 \*\*\*\*\*

FILE NAME: RGW\_P00.DAT  
 TIME/DATE OF STUDY: 05:19 10/24/2019

-----  
 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
 -----

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.500  
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00  
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS  
 \*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*  

| NO. | WIDTH<br>(FT) | CROWN TO<br>CROSSFALL<br>(FT) | STREET-CROSSFALL<br>IN- / OUT- / PARK-<br>SIDE / SIDE / WAY | HEIGHT<br>(FT) | WIDTH<br>(FT) | LIP<br>(FT) | HIKE<br>(FT) | FACTOR<br>(n) |
|-----|---------------|-------------------------------|-------------------------------------------------------------|----------------|---------------|-------------|--------------|---------------|
| 1   | 30.0          | 20.0                          | 0.018/0.018/0.020                                           | 0.67           | 2.00          | 0.0313      | 0.167        | 0.0150        |

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
 1. Relative Flow-Depth = 0.00 FEET  
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
 2. (Depth)\*(Velocity) constraint = 6.0 (FT\*FT/S)  
 \*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<  
 =====

SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00  
 UPSTREAM ELEVATION(FEET) = 1520.00  
 DOWNSTREAM ELEVATION(FEET) = 1480.00  
 ELEVATION DIFFERENCE(FEET) = 40.00  
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.979  
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.217  
 SUBAREA RUNOFF(CFS) = 0.25  
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.25

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 51  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<  
 =====

ELEVATION DATA: UPSTREAM(FEET) = 1480.00 DOWNSTREAM(FEET) = 450.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 3300.00 CHANNEL SLOPE = 0.3121  
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 3.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.743  
 SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 65.61  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 12.38  
 AVERAGE FLOW DEPTH(FEET) = 0.74 TRAVEL TIME(MIN.) = 4.44  
 Tc(MIN.) = 10.42  
 SUBAREA AREA(ACRES) = 74.00 SUBAREA RUNOFF(CFS) = 127.48  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.300  
 TOTAL AREA(ACRES) = 74.1 PEAK FLOW RATE(CFS) = 127.66

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 1.05 FLOW VELOCITY(FEET/SEC.) = 14.96  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 3380.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

ELEVATION DATA: UPSTREAM(FEET) = 450.00 DOWNSTREAM(FEET) = 442.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 900.00 CHANNEL SLOPE = 0.0089  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.233  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2500  
 SOIL CLASSIFICATION IS "B"  
 S.C.S. CURVE NUMBER (AMC II) = 85  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 148.05  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.38  
 AVERAGE FLOW DEPTH(FEET) = 1.02 TRAVEL TIME(MIN.) = 6.30  
 Tc(MIN.) = 16.72  
 SUBAREA AREA(ACRES) = 38.40 SUBAREA RUNOFF(CFS) = 40.64  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.283  
 TOTAL AREA(ACRES) = 112.5 PEAK FLOW RATE(CFS) = 134.75

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.98 FLOW VELOCITY(FEET/SEC.) = 2.32  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 4280.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

ELEVATION DATA: UPSTREAM(FEET) = 442.00 DOWNSTREAM(FEET) = 428.80  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 2200.00 CHANNEL SLOPE = 0.0060  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.646  
 \*USER SPECIFIED(SUBAREA):  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2300  
 S.C.S. CURVE NUMBER (AMC II) = 85  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 149.01  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.05  
 AVERAGE FLOW DEPTH(FEET) = 1.11 TRAVEL TIME(MIN.) = 17.92  
 Tc(MIN.) = 34.64  
 SUBAREA AREA(ACRES) = 46.50 SUBAREA RUNOFF(CFS) = 28.30  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.267  
 TOTAL AREA(ACRES) = 159.0 PEAK FLOW RATE(CFS) = 134.75

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 1.07 FLOW VELOCITY(FEET/SEC.) = 2.00  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 6480.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 104.00 TO NODE 105.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

ELEVATION DATA: UPSTREAM(FEET) = 428.80 DOWNSTREAM(FEET) = 427.90  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 330.00 CHANNEL SLOPE = 0.0027  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 5.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.479  
 \*USER SPECIFIED(SUBAREA):  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2620  
 S.C.S. CURVE NUMBER (AMC II) = 85  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 136.60  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.49  
 AVERAGE FLOW DEPTH(FEET) = 1.26 TRAVEL TIME(MIN.) = 3.68  
 Tc(MIN.) = 38.32  
 SUBAREA AREA(ACRES) = 5.70 SUBAREA RUNOFF(CFS) = 3.70  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.267  
 TOTAL AREA(ACRES) = 164.7 PEAK FLOW RATE(CFS) = 134.75

## END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 1.25 FLOW VELOCITY(FEET/SEC.) = 1.49  
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 105.00 = 6810.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 51

&gt;&gt;&gt;&gt;COMPUTE TRAPEZOIDAL CHANNEL FLOW&lt;&lt;&lt;&lt;&lt;

&gt;&gt;&gt;&gt;TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)&lt;&lt;&lt;&lt;&lt;

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=====
ELEVATION DATA: UPSTREAM(FEET) = 427.90 DOWNSTREAM(FEET) = 426.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 225.00 CHANNEL SLOPE = 0.0062
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.429
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .3180
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 135.87
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.04
AVERAGE FLOW DEPTH(FEET) = 2.06 TRAVEL TIME(MIN.) = 1.23
Tc(MIN.) = 39.55
SUBAREA AREA(ACRES) = 2.90 SUBAREA RUNOFF(CFS) = 2.24
AREA-AVERAGE RUNOFF COEFFICIENT = 0.268
TOTAL AREA(ACRES) = 167.6 PEAK FLOW RATE(CFS) = 134.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 2.06 FLOW VELOCITY(FEET/SEC.) = 3.02
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 106.00 = 7035.00 FEET.

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*****
FLOW PROCESS FROM NODE 106.00 TO NODE 107.00 IS CODE = 51
=====

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>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 426.50 DOWNSTREAM(FEET) = 425.80
CHANNEL LENGTH THRU SUBAREA(FEET) = 270.00 CHANNEL SLOPE = 0.0026
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.351
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2500
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 135.42
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.18
AVERAGE FLOW DEPTH(FEET) = 2.44 TRAVEL TIME(MIN.) = 2.07
Tc(MIN.) = 41.62
SUBAREA AREA(ACRES) = 2.30 SUBAREA RUNOFF(CFS) = 1.35
AREA-AVERAGE RUNOFF COEFFICIENT = 0.268
TOTAL AREA(ACRES) = 169.9 PEAK FLOW RATE(CFS) = 134.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 2.44 FLOW VELOCITY(FEET/SEC.) = 2.18
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 107.00 = 7305.00 FEET.

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*****
FLOW PROCESS FROM NODE 107.00 TO NODE 110.00 IS CODE = 51
=====

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```

>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 425.80 DOWNSTREAM(FEET) = 425.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 340.00 CHANNEL SLOPE = 0.0009
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.219
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .3970
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 136.02
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.46
AVERAGE FLOW DEPTH(FEET) = 3.00 TRAVEL TIME(MIN.) = 3.88
Tc(MIN.) = 45.50
SUBAREA AREA(ACRES) = 2.90 SUBAREA RUNOFF(CFS) = 2.56
AREA-AVERAGE RUNOFF COEFFICIENT = 0.270
TOTAL AREA(ACRES) = 172.8 PEAK FLOW RATE(CFS) = 134.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 2.99 FLOW VELOCITY(FEET/SEC.) = 1.45
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 110.00 = 7645.00 FEET.

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*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 10
=====

```

```

>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
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```

*****
FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 21
=====

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```

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

```

```

SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00

```

UPSTREAM ELEVATION(FEET) = 1010.00  
 DOWNSTREAM ELEVATION(FEET) = 970.00  
 ELEVATION DIFFERENCE(FEET) = 40.00  
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.979  
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.217  
 SUBAREA RUNOFF(CFS) = 0.25  
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.25

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 51  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 970.00 DOWNSTREAM(FEET) = 450.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 1500.00 CHANNEL SLOPE = 0.3467  
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 3.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 5.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.144  
 SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.24  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 7.34  
 AVERAGE FLOW DEPTH(FEET) = 0.26 TRAVEL TIME(MIN.) = 3.40  
 Tc(MIN.) = 9.38  
 SUBAREA AREA(ACRES) = 11.70 SUBAREA RUNOFF(CFS) = 21.57  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.300  
 TOTAL AREA(ACRES) = 11.8 PEAK FLOW RATE(CFS) = 21.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.39 FLOW VELOCITY(FEET/SEC.) = 9.14  
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 1580.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 51  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 450.00 DOWNSTREAM(FEET) = 429.50  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 2350.00 CHANNEL SLOPE = 0.0087  
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.767  
 \*USER SPECIFIED(SUBAREA):  
 VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2750  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 41.88  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.71  
 AVERAGE FLOW DEPTH(FEET) = 0.61 TRAVEL TIME(MIN.) = 22.94  
 Tc(MIN.) = 32.32  
 SUBAREA AREA(ACRES) = 50.00 SUBAREA RUNOFF(CFS) = 38.05  
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.280  
 TOTAL AREA(ACRES) = 61.8 PEAK FLOW RATE(CFS) = 47.84

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.64 FLOW VELOCITY(FEET/SEC.) = 1.76  
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 203.00 = 3930.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 32.32  
 RAINFALL INTENSITY(INCH/HR) = 2.77  
 TOTAL STREAM AREA(ACRES) = 61.80  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 47.84

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 300.00 TO NODE 301.00 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000  
 SOIL CLASSIFICATION IS "C"  
 S.C.S. CURVE NUMBER (AMC II) = 63  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 80.00  
 UPSTREAM ELEVATION(FEET) = 707.00  
 DOWNSTREAM ELEVATION(FEET) = 657.00  
 ELEVATION DIFFERENCE(FEET) = 50.00  
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.979  
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.217



```

                                RGW_P00.RES
SUBAREA RUNOFF(CFS) =          0.74
TOTAL AREA(ACRES) =          0.30  TOTAL RUNOFF(CFS) =          0.74
*****
FLOW PROCESS FROM NODE      301.00 TO NODE      302.00 IS CODE =   51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    657.00  DOWNSTREAM(FEET) =    432.00
CHANNEL LENGTH THRU SUBAREA(FEET) =   1040.00  CHANNEL SLOPE =   0.2163
CHANNEL BASE(FEET) =     5.00  "Z" FACTOR =   3.000
MANNING'S FACTOR = 0.045  MAXIMUM DEPTH(FEET) =    5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   6.053
*USER SPECIFIED(SUBAREA):
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .2900
S.C.S. CURVE NUMBER (AMC II) =   63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =    5.22
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =   4.78
AVERAGE FLOW DEPTH(FEET) =   0.20  TRAVEL TIME(MIN.) =   3.63
Tc(MIN.) =    9.60
SUBAREA AREA(ACRES) =     5.00  SUBAREA RUNOFF(CFS) =    8.78
AREA-AVERAGE RUNOFF COEFFICIENT =   0.291
TOTAL AREA(ACRES) =     5.3  PEAK FLOW RATE(CFS) =    9.32

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =   0.27  FLOW VELOCITY(FEET/SEC.) =   5.90
LONGEST FLOWPATH FROM NODE      300.00 TO NODE      302.00 =   1120.00 FEET.
*****
FLOW PROCESS FROM NODE      302.00 TO NODE      203.00 IS CODE =   51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    432.00  DOWNSTREAM(FEET) =    429.50
CHANNEL LENGTH THRU SUBAREA(FEET) =    250.00  CHANNEL SLOPE =   0.0100
CHANNEL BASE(FEET) =    10.00  "Z" FACTOR =  50.000
MANNING'S FACTOR = 0.040  MAXIMUM DEPTH(FEET) =    3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   5.003
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2250
S.C.S. CURVE NUMBER (AMC II) =   63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =   10.68
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    1.26
AVERAGE FLOW DEPTH(FEET) =   0.32  TRAVEL TIME(MIN.) =    3.30
Tc(MIN.) =   12.91
SUBAREA AREA(ACRES) =     2.40  SUBAREA RUNOFF(CFS) =    2.70
AREA-AVERAGE RUNOFF COEFFICIENT =   0.270
TOTAL AREA(ACRES) =     7.7  PEAK FLOW RATE(CFS) =   10.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =   0.32  FLOW VELOCITY(FEET/SEC.) =    1.27
LONGEST FLOWPATH FROM NODE      300.00 TO NODE      203.00 =   1370.00 FEET.
*****
FLOW PROCESS FROM NODE      203.00 TO NODE      203.00 IS CODE =    1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) =   12.91
RAINFALL INTENSITY(INCH/HR) =    5.00
TOTAL STREAM AREA(ACRES) =    7.70
PEAK FLOW RATE(CFS) AT CONFLUENCE =   10.41

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
  1         47.84      32.32      2.767      61.80
  2         10.41      12.91      5.003       7.70

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
  1         29.51      12.91      5.003
  2         53.60      32.32      2.767

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =    53.60  Tc(MIN.) =   32.32
TOTAL AREA(ACRES) =   69.5
LONGEST FLOWPATH FROM NODE      200.00 TO NODE      203.00 =   3930.00 FEET.

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*****RGW_P00.RES*****
FLOW PROCESS FROM NODE    203.00 TO NODE    110.00 IS CODE =   51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    429.50  DOWNSTREAM(FEET) =    425.50
CHANNEL LENGTH THRU SUBAREA(FEET) =    982.00  CHANNEL SLOPE =    0.0041
CHANNEL BASE(FEET) =    1.00  "Z" FACTOR =    10.000
MANNING'S FACTOR =    0.040  MAXIMUM DEPTH(FEET) =    2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    2.405
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .3150
S.C.S. CURVE NUMBER (AMC II) =    63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =    57.12
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    2.08
AVERAGE FLOW DEPTH(FEET) =    1.61  TRAVEL TIME(MIN.) =    7.86
Tc(MIN.) =    40.18
SUBAREA AREA(ACRES) =    9.30      SUBAREA RUNOFF(CFS) =    7.04
AREA-AVERAGE RUNOFF COEFFICIENT =    0.283
TOTAL AREA(ACRES) =    78.8      PEAK FLOW RATE(CFS) =    53.62

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =    1.57  FLOW VELOCITY(FEET/SEC.) =    2.05
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    110.00 =    4912.00 FEET.

*****
FLOW PROCESS FROM NODE    110.00 TO NODE    110.00 IS CODE =    1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS =    2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM    1 ARE:
TIME OF CONCENTRATION(MIN.) =    40.18
RAINFALL INTENSITY(INCH/HR) =    2.40
TOTAL STREAM AREA(ACRES) =    78.80
PEAK FLOW RATE(CFS) AT CONFLUENCE =    53.62

*****
FLOW PROCESS FROM NODE    400.00 TO NODE    401.00 IS CODE =   21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =    63
INITIAL SUBAREA FLOW-LENGTH(FEET) =    80.00
UPSTREAM ELEVATION(FEET) =    575.00
DOWNSTREAM ELEVATION(FEET) =    548.00
ELEVATION DIFFERENCE(FEET) =    27.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =    5.979
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    8.217
SUBAREA RUNOFF(CFS) =    0.25
TOTAL AREA(ACRES) =    0.10  TOTAL RUNOFF(CFS) =    0.25

*****
FLOW PROCESS FROM NODE    401.00 TO NODE    402.00 IS CODE =   51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    548.00  DOWNSTREAM(FEET) =    432.80
CHANNEL LENGTH THRU SUBAREA(FEET) =    820.00  CHANNEL SLOPE =    0.1405
CHANNEL BASE(FEET) =    5.00  "Z" FACTOR =    3.000
MANNING'S FACTOR =    0.045  MAXIMUM DEPTH(FEET) =    5.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    6.200
*USER SPECIFIED(SUBAREA):
SAGEBRUSH FAIR COVER RUNOFF COEFFICIENT = .2900
S.C.S. CURVE NUMBER (AMC II) =    63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =    5.31
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    4.17
AVERAGE FLOW DEPTH(FEET) =    0.22  TRAVEL TIME(MIN.) =    3.27
Tc(MIN.) =    9.25
SUBAREA AREA(ACRES) =    5.50      SUBAREA RUNOFF(CFS) =    9.89
AREA-AVERAGE RUNOFF COEFFICIENT =    0.290
TOTAL AREA(ACRES) =    5.6      PEAK FLOW RATE(CFS) =    10.07

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =    0.32  FLOW VELOCITY(FEET/SEC.) =    5.22
LONGEST FLOWPATH FROM NODE    400.00 TO NODE    402.00 =    900.00 FEET.

*****
FLOW PROCESS FROM NODE    402.00 TO NODE    110.00 IS CODE =   51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

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                                RGW_P00.RES
ELEVATION DATA: UPSTREAM(FEET) = 432.80 DOWNSTREAM(FEET) = 425.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 780.00 CHANNEL SLOPE = 0.0094
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.040 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.880
*USER SPECIFIED(SUBAREA):
VINEYARDS(DISKED) COVER RUNOFF COEFFICIENT = .2620
S.C.S. CURVE NUMBER (AMC II) = 63
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 13.00
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.32
AVERAGE FLOW DEPTH(FEET) = 0.36 TRAVEL TIME(MIN.) = 9.88
Tc(MIN.) = 19.14
SUBAREA AREA(ACRES) = 5.70 SUBAREA RUNOFF(CFS) = 5.79
AREA-AVERAGE RUNOFF COEFFICIENT = 0.276
TOTAL AREA(ACRES) = 11.3 PEAK FLOW RATE(CFS) = 12.10

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 FLOW VELOCITY(FEET/SEC.) = 1.28
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 110.00 = 1680.00 FEET.

*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 19.14
RAINFALL INTENSITY(INCH/HR) = 3.88
TOTAL STREAM AREA(ACRES) = 11.30
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.10

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 53.62 40.18 2.405 78.80
2 12.10 19.14 3.880 11.30

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 45.33 19.14 3.880
2 61.12 40.18 2.405

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 61.12 Tc(MIN.) = 40.18
TOTAL AREA(ACRES) = 90.1
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 110.00 = 4912.00 FEET.

*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 61.12 40.18 2.405 90.10
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 110.00 = 4912.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 134.75 45.50 2.219 172.80
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 110.00 = 7645.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 180.10 40.18 2.405
2 191.15 45.50 2.219

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 191.15 Tc(MIN.) = 45.50
TOTAL AREA(ACRES) = 262.9
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 262.9 TC(MIN.) = 45.50
PEAK FLOW RATE(CFS) = 191.15
=====
END OF RATIONAL METHOD ANALYSIS

```

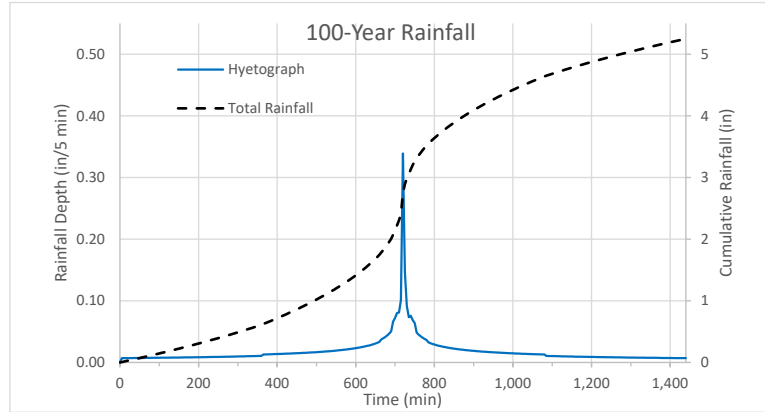


## **Appendix D**

HEC-HMS Hydrologic Analysis Support Material and Results

## 100-Year NOAA Atlas 14 Hyetograph Generation

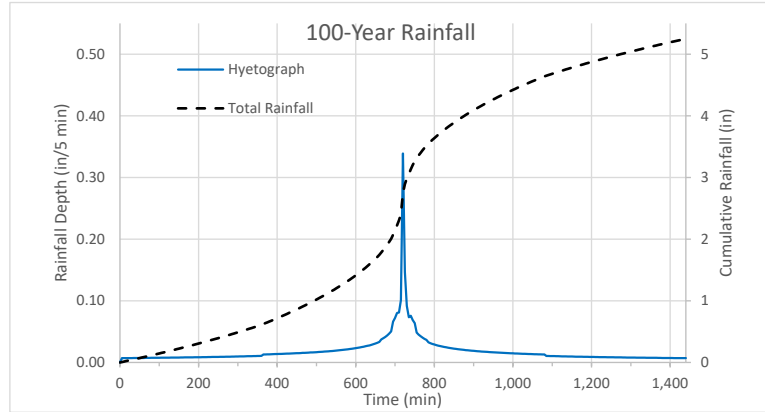
| Duration<br>(min)    | Rain Depth<br>(in) |
|----------------------|--------------------|
| 5                    | 0.339              |
| 10                   | 0.486              |
| 15                   | 0.587              |
| 30                   | 0.833              |
| 60                   | 1.26               |
| 120                  | 1.77               |
| 180                  | 2.13               |
| 360                  | 2.93               |
| 720                  | 4.03               |
| 1440                 | 5.25               |
| Time Step =          | 5 min              |
| Rainfall Sum Check = | 5.250 in           |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| -   | 0           | 0            | 0         | 0          |
| 287 | 0.007       | 0.007        | 0.219     | 5          |
| 285 | 0.014       | 0.007        | 0.220     | 10         |
| 283 | 0.021       | 0.007        | 0.221     | 15         |
| 281 | 0.028       | 0.0071       | 0.222     | 20         |
| 279 | 0.035       | 0.0071       | 0.223     | 25         |
| 277 | 0.042       | 0.0071       | 0.224     | 30         |
| 275 | 0.050       | 0.0072       | 0.225     | 35         |
| 273 | 0.057       | 0.0072       | 0.226     | 40         |
| 271 | 0.064       | 0.0072       | 0.227     | 45         |
| 269 | 0.071       | 0.0073       | 0.228     | 50         |
| 267 | 0.079       | 0.0073       | 0.229     | 55         |
| 265 | 0.086       | 0.0073       | 0.230     | 60         |
| 263 | 0.093       | 0.0074       | 0.231     | 65         |
| 261 | 0.101       | 0.0074       | 0.232     | 70         |
| 259 | 0.108       | 0.0074       | 0.234     | 75         |
| 257 | 0.116       | 0.0075       | 0.235     | 80         |
| 255 | 0.123       | 0.0075       | 0.236     | 85         |
| 253 | 0.131       | 0.0075       | 0.237     | 90         |
| 251 | 0.138       | 0.0076       | 0.238     | 95         |
| 249 | 0.146       | 0.0076       | 0.239     | 100        |
| 247 | 0.153       | 0.0077       | 0.241     | 105        |
| 245 | 0.161       | 0.0077       | 0.242     | 110        |
| 243 | 0.169       | 0.0077       | 0.243     | 115        |
| 241 | 0.177       | 0.0078       | 0.244     | 120        |
| 239 | 0.184       | 0.0078       | 0.245     | 125        |
| 237 | 0.192       | 0.0079       | 0.247     | 130        |
| 235 | 0.200       | 0.0079       | 0.248     | 135        |
| 233 | 0.208       | 0.0079       | 0.249     | 140        |
| 231 | 0.216       | 0.008        | 0.251     | 145        |
| 229 | 0.224       | 0.008        | 0.252     | 150        |
| 227 | 0.232       | 0.0081       | 0.253     | 155        |
| 225 | 0.240       | 0.0081       | 0.255     | 160        |
| 223 | 0.249       | 0.0082       | 0.256     | 165        |
| 221 | 0.257       | 0.0082       | 0.258     | 170        |
| 219 | 0.265       | 0.0083       | 0.259     | 175        |
| 217 | 0.273       | 0.0083       | 0.261     | 180        |
| 215 | 0.282       | 0.0083       | 0.262     | 185        |
| 213 | 0.290       | 0.0084       | 0.26      | 190        |
| 211 | 0.298       | 0.0084       | 0.27      | 195        |
| 209 | 0.307       | 0.0085       | 0.27      | 200        |
| 207 | 0.315       | 0.0085       | 0.27      | 205        |
| 205 | 0.324       | 0.0086       | 0.27      | 210        |
| 203 | 0.333       | 0.0086       | 0.27      | 215        |
| 201 | 0.341       | 0.0087       | 0.27      | 220        |
| 199 | 0.350       | 0.0088       | 0.28      | 225        |
| 197 | 0.359       | 0.0088       | 0.28      | 230        |
| 195 | 0.368       | 0.0089       | 0.28      | 235        |
| 193 | 0.377       | 0.0089       | 0.28      | 240        |
| 191 | 0.386       | 0.009        | 0.28      | 245        |
| 189 | 0.395       | 0.009        | 0.28      | 250        |

## 100-Year NOAA Atlas 14 Hyetograph Generation

| Duration<br>(min)    | Rain Depth<br>(in) |
|----------------------|--------------------|
| 5                    | 0.339              |
| 10                   | 0.486              |
| 15                   | 0.587              |
| 30                   | 0.833              |
| 60                   | 1.26               |
| 120                  | 1.77               |
| 180                  | 2.13               |
| 360                  | 2.93               |
| 720                  | 4.03               |
| 1440                 | 5.25               |
| Time Step =          | 5 min              |
| Rainfall Sum Check = | 5.250 in           |

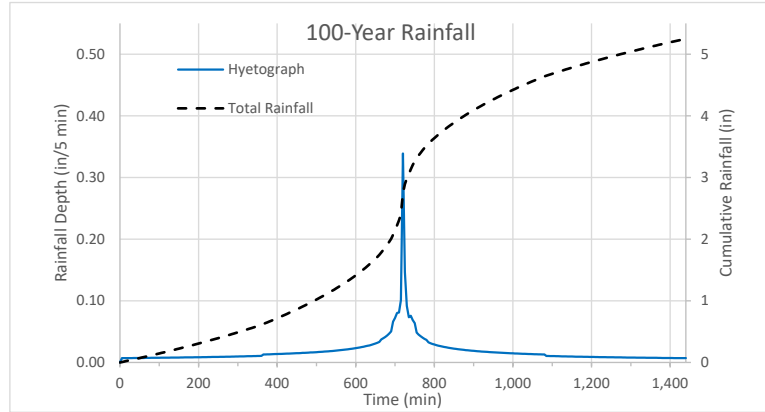


| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 187 | 0.404       | 0.0091       | 0.29      | 255        |
| 185 | 0.413       | 0.0092       | 0.29      | 260        |
| 183 | 0.422       | 0.0092       | 0.29      | 265        |
| 181 | 0.432       | 0.0093       | 0.29      | 270        |
| 179 | 0.441       | 0.0093       | 0.29      | 275        |
| 177 | 0.450       | 0.0094       | 0.30      | 280        |
| 175 | 0.460       | 0.0095       | 0.30      | 285        |
| 173 | 0.469       | 0.0095       | 0.30      | 290        |
| 171 | 0.479       | 0.0096       | 0.30      | 295        |
| 169 | 0.489       | 0.0097       | 0.30      | 300        |
| 167 | 0.498       | 0.0098       | 0.31      | 305        |
| 165 | 0.508       | 0.0098       | 0.31      | 310        |
| 163 | 0.518       | 0.0099       | 0.31      | 315        |
| 161 | 0.528       | 0.01         | 0.31      | 320        |
| 159 | 0.538       | 0.0101       | 0.32      | 325        |
| 157 | 0.548       | 0.0101       | 0.32      | 330        |
| 155 | 0.559       | 0.0102       | 0.32      | 335        |
| 153 | 0.569       | 0.0103       | 0.32      | 340        |
| 151 | 0.579       | 0.0104       | 0.33      | 345        |
| 149 | 0.590       | 0.0105       | 0.33      | 350        |
| 147 | 0.600       | 0.0106       | 0.33      | 355        |
| 145 | 0.611       | 0.0107       | 0.33      | 360        |
| 143 | 0.624       | 0.0129       | 0.34      | 365        |
| 141 | 0.637       | 0.013        | 0.34      | 370        |
| 139 | 0.650       | 0.0131       | 0.34      | 375        |
| 137 | 0.663       | 0.0132       | 0.35      | 380        |
| 135 | 0.677       | 0.0134       | 0.35      | 385        |
| 133 | 0.690       | 0.0135       | 0.35      | 390        |
| 131 | 0.704       | 0.0136       | 0.35      | 395        |
| 129 | 0.717       | 0.0137       | 0.36      | 400        |
| 127 | 0.731       | 0.0138       | 0.36      | 405        |
| 125 | 0.745       | 0.0139       | 0.36      | 410        |
| 123 | 0.759       | 0.014        | 0.37      | 415        |
| 121 | 0.773       | 0.0142       | 0.37      | 420        |
| 119 | 0.788       | 0.0143       | 0.37      | 425        |
| 117 | 0.802       | 0.0144       | 0.38      | 430        |
| 115 | 0.817       | 0.0146       | 0.38      | 435        |
| 113 | 0.831       | 0.0147       | 0.38      | 440        |
| 111 | 0.846       | 0.0148       | 0.39      | 445        |
| 109 | 0.861       | 0.015        | 0.39      | 450        |
| 107 | 0.876       | 0.0151       | 0.39      | 455        |
| 105 | 0.892       | 0.0153       | 0.40      | 460        |
| 103 | 0.907       | 0.0155       | 0.40      | 465        |
| 101 | 0.923       | 0.0156       | 0.41      | 470        |
| 99  | 0.939       | 0.0158       | 0.41      | 475        |
| 97  | 0.955       | 0.016        | 0.42      | 480        |
| 95  | 0.971       | 0.0162       | 0.42      | 485        |
| 93  | 0.987       | 0.0163       | 0.43      | 490        |
| 91  | 1.004       | 0.0165       | 0.43      | 495        |
| 89  | 1.020       | 0.0167       | 0.44      | 500        |
| 87  | 1.037       | 0.0169       | 0.44      | 505        |



## 100-Year NOAA Atlas 14 Hyetograph Generation

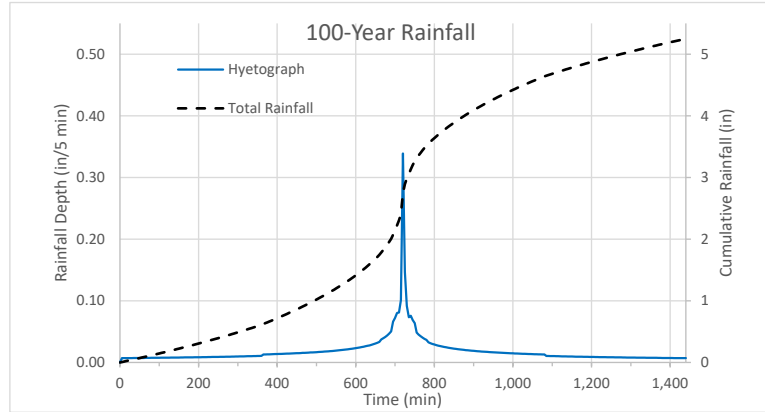
| Duration<br>(min)    | Rain Depth<br>(in) |
|----------------------|--------------------|
| 5                    | 0.339              |
| 10                   | 0.486              |
| 15                   | 0.587              |
| 30                   | 0.833              |
| 60                   | 1.26               |
| 120                  | 1.77               |
| 180                  | 2.13               |
| 360                  | 2.93               |
| 720                  | 4.03               |
| 1440                 | 5.25               |
| Time Step =          | 5 min              |
| Rainfall Sum Check = | 5.250 in           |



| N  | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|----|-------------|--------------|-----------|------------|
| 85 | 1.054       | 0.0172       | 0.45      | 510        |
| 83 | 1.072       | 0.0174       | 0.45      | 515        |
| 81 | 1.089       | 0.0176       | 0.46      | 520        |
| 79 | 1.107       | 0.0179       | 0.46      | 525        |
| 77 | 1.125       | 0.0181       | 0.47      | 530        |
| 75 | 1.144       | 0.0184       | 0.48      | 535        |
| 73 | 1.162       | 0.0186       | 0.49      | 540        |
| 71 | 1.181       | 0.0189       | 0.49      | 545        |
| 69 | 1.201       | 0.0192       | 0.50      | 550        |
| 67 | 1.220       | 0.0195       | 0.51      | 555        |
| 65 | 1.240       | 0.0199       | 0.52      | 560        |
| 63 | 1.260       | 0.0202       | 0.53      | 565        |
| 61 | 1.281       | 0.0206       | 0.53      | 570        |
| 59 | 1.302       | 0.0209       | 0.54      | 575        |
| 57 | 1.323       | 0.0213       | 0.55      | 580        |
| 55 | 1.345       | 0.0218       | 0.57      | 585        |
| 53 | 1.367       | 0.0222       | 0.58      | 590        |
| 51 | 1.390       | 0.0227       | 0.59      | 595        |
| 49 | 1.413       | 0.0232       | 0.60      | 600        |
| 47 | 1.437       | 0.0237       | 0.62      | 605        |
| 45 | 1.461       | 0.0243       | 0.63      | 610        |
| 43 | 1.486       | 0.0249       | 0.65      | 615        |
| 41 | 1.511       | 0.0255       | 0.66      | 620        |
| 39 | 1.538       | 0.0263       | 0.68      | 625        |
| 37 | 1.565       | 0.027        | 0.70      | 630        |
| 35 | 1.592       | 0.0276       | 0.72      | 635        |
| 33 | 1.621       | 0.0286       | 0.74      | 640        |
| 31 | 1.650       | 0.0296       | 0.77      | 645        |
| 29 | 1.681       | 0.0307       | 0.80      | 650        |
| 27 | 1.713       | 0.0319       | 0.83      | 655        |
| 25 | 1.746       | 0.0333       | 0.87      | 660        |
| 23 | 1.784       | 0.0374       | 0.90      | 665        |
| 21 | 1.823       | 0.0392       | 0.95      | 670        |
| 19 | 1.864       | 0.0413       | 1.00      | 675        |
| 17 | 1.908       | 0.0438       | 1.06      | 680        |
| 15 | 1.955       | 0.0468       | 1.13      | 685        |
| 13 | 2.005       | 0.0504       | 1.21      | 690        |
| 11 | 2.071       | 0.0662       | 1.31      | 695        |
| 9  | 2.143       | 0.0721       | 1.42      | 700        |
| 7  | 2.224       | 0.0803       | 1.57      | 705        |
| 5  | 2.305       | 0.081        | 1.82      | 710        |
| 3  | 2.406       | 0.101        | 2.35      | 715        |
| 1  | 2.745       | 0.339        | 4.07      | 720        |
| 2  | 2.892       | 0.147        | 2.92      | 725        |
| 4  | 2.983       | 0.0918       | 2.04      | 730        |
| 6  | 3.057       | 0.0733       | 1.67      | 735        |
| 8  | 3.132       | 0.0758       | 1.48      | 740        |
| 10 | 3.201       | 0.0689       | 1.36      | 745        |
| 12 | 3.265       | 0.0638       | 1.26      | 750        |
| 14 | 3.314       | 0.0485       | 1.17      | 755        |
| 16 | 3.359       | 0.0452       | 1.09      | 760        |

## 100-Year NOAA Atlas 14 Hyetograph Generation

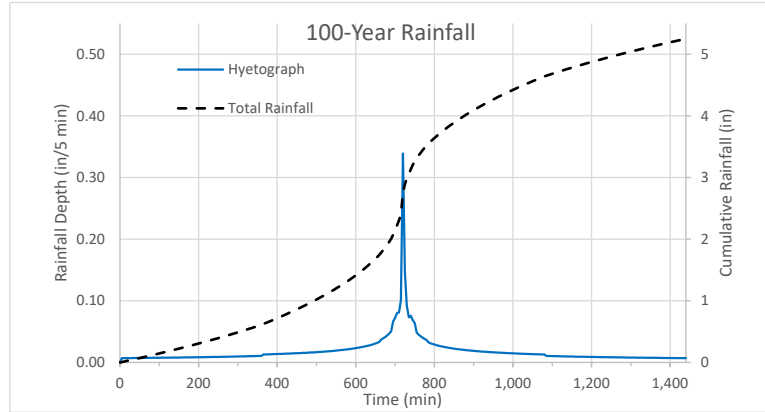
| Duration<br>(min)    | Rain Depth<br>(in) |
|----------------------|--------------------|
| 5                    | 0.339              |
| 10                   | 0.486              |
| 15                   | 0.587              |
| 30                   | 0.833              |
| 60                   | 1.26               |
| 120                  | 1.77               |
| 180                  | 2.13               |
| 360                  | 2.93               |
| 720                  | 4.03               |
| 1440                 | 5.25               |
| Time Step =          | 5 min              |
| Rainfall Sum Check = | 5.250 in           |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 18  | 3.401       | 0.0425       | 1.03      | 765        |
| 20  | 3.441       | 0.0402       | 0.97      | 770        |
| 22  | 3.480       | 0.0382       | 0.93      | 775        |
| 24  | 3.516       | 0.0366       | 0.89      | 780        |
| 26  | 3.549       | 0.0326       | 0.85      | 785        |
| 28  | 3.580       | 0.0313       | 0.81      | 790        |
| 30  | 3.610       | 0.0301       | 0.78      | 795        |
| 32  | 3.639       | 0.0291       | 0.76      | 800        |
| 34  | 3.667       | 0.0281       | 0.73      | 805        |
| 36  | 3.695       | 0.0272       | 0.71      | 810        |
| 38  | 3.721       | 0.0266       | 0.69      | 815        |
| 40  | 3.747       | 0.0259       | 0.67      | 820        |
| 42  | 3.772       | 0.0252       | 0.65      | 825        |
| 44  | 3.797       | 0.0246       | 0.64      | 830        |
| 46  | 3.821       | 0.024        | 0.62      | 835        |
| 48  | 3.844       | 0.0234       | 0.61      | 840        |
| 50  | 3.867       | 0.0229       | 0.60      | 845        |
| 52  | 3.890       | 0.0224       | 0.58      | 850        |
| 54  | 3.912       | 0.022        | 0.57      | 855        |
| 56  | 3.933       | 0.0215       | 0.56      | 860        |
| 58  | 3.954       | 0.0211       | 0.55      | 865        |
| 60  | 3.975       | 0.0208       | 0.54      | 870        |
| 62  | 3.995       | 0.0204       | 0.53      | 875        |
| 64  | 4.015       | 0.02         | 0.52      | 880        |
| 66  | 4.035       | 0.0197       | 0.51      | 885        |
| 68  | 4.055       | 0.0194       | 0.50      | 890        |
| 70  | 4.074       | 0.0191       | 0.50      | 895        |
| 72  | 4.092       | 0.0188       | 0.49      | 900        |
| 74  | 4.111       | 0.0185       | 0.48      | 905        |
| 76  | 4.129       | 0.0182       | 0.47      | 910        |
| 78  | 4.147       | 0.018        | 0.47      | 915        |
| 80  | 4.165       | 0.0177       | 0.46      | 920        |
| 82  | 4.182       | 0.0175       | 0.46      | 925        |
| 84  | 4.200       | 0.0173       | 0.45      | 930        |
| 86  | 4.217       | 0.0171       | 0.44      | 935        |
| 88  | 4.234       | 0.0168       | 0.44      | 940        |
| 90  | 4.250       | 0.0166       | 0.43      | 945        |
| 92  | 4.267       | 0.0164       | 0.43      | 950        |
| 94  | 4.283       | 0.0163       | 0.42      | 955        |
| 96  | 4.299       | 0.0161       | 0.42      | 960        |
| 98  | 4.315       | 0.0159       | 0.41      | 965        |
| 100 | 4.331       | 0.0157       | 0.41      | 970        |
| 102 | 4.346       | 0.0155       | 0.41      | 975        |
| 104 | 4.362       | 0.0154       | 0.40      | 980        |
| 106 | 4.377       | 0.0152       | 0.40      | 985        |
| 108 | 4.392       | 0.0151       | 0.39      | 990        |
| 110 | 4.407       | 0.0149       | 0.39      | 995        |
| 112 | 4.422       | 0.0148       | 0.39      | 1,000      |
| 114 | 4.436       | 0.0146       | 0.38      | 1,005      |
| 116 | 4.451       | 0.0145       | 0.38      | 1,010      |
| 118 | 4.465       | 0.0144       | 0.37      | 1,015      |

## 100-Year NOAA Atlas 14 Hyetograph Generation

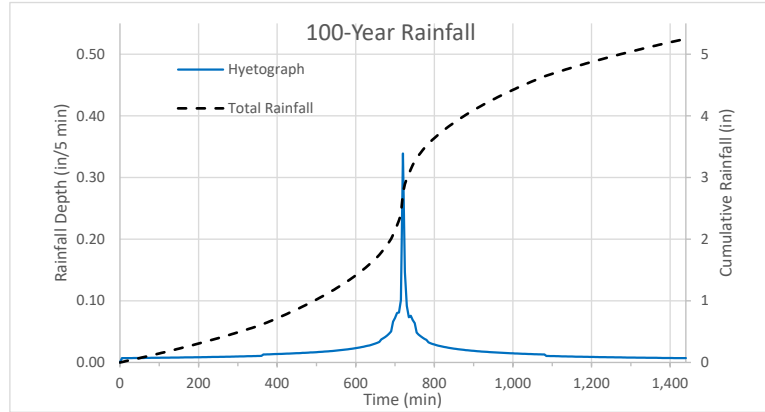
| Duration<br>(min)    | Rain Depth<br>(in) |
|----------------------|--------------------|
| 5                    | 0.339              |
| 10                   | 0.486              |
| 15                   | 0.587              |
| 30                   | 0.833              |
| 60                   | 1.26               |
| 120                  | 1.77               |
| 180                  | 2.13               |
| 360                  | 2.93               |
| 720                  | 4.03               |
| 1440                 | 5.25               |
| Time Step =          | 5 min              |
| Rainfall Sum Check = | 5.250 in           |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 120 | 4.479       | 0.0142       | 0.37      | 1,020      |
| 122 | 4.493       | 0.0141       | 0.37      | 1,025      |
| 124 | 4.507       | 0.014        | 0.36      | 1,030      |
| 126 | 4.521       | 0.0139       | 0.36      | 1,035      |
| 128 | 4.535       | 0.0137       | 0.36      | 1,040      |
| 130 | 4.549       | 0.0136       | 0.36      | 1,045      |
| 132 | 4.562       | 0.0135       | 0.35      | 1,050      |
| 134 | 4.576       | 0.0134       | 0.35      | 1,055      |
| 136 | 4.589       | 0.0133       | 0.35      | 1,060      |
| 138 | 4.602       | 0.0132       | 0.34      | 1,065      |
| 140 | 4.615       | 0.0131       | 0.34      | 1,070      |
| 142 | 4.628       | 0.013        | 0.34      | 1,075      |
| 144 | 4.641       | 0.0129       | 0.34      | 1,080      |
| 146 | 4.652       | 0.0106       | 0.33      | 1,085      |
| 148 | 4.662       | 0.0105       | 0.33      | 1,090      |
| 150 | 4.673       | 0.0104       | 0.33      | 1,095      |
| 152 | 4.683       | 0.0103       | 0.33      | 1,100      |
| 154 | 4.693       | 0.0103       | 0.32      | 1,105      |
| 156 | 4.703       | 0.0102       | 0.32      | 1,110      |
| 158 | 4.713       | 0.0101       | 0.32      | 1,115      |
| 160 | 4.723       | 0.01         | 0.32      | 1,120      |
| 162 | 4.733       | 0.0099       | 0.31      | 1,125      |
| 164 | 4.743       | 0.0099       | 0.31      | 1,130      |
| 166 | 4.753       | 0.0098       | 0.31      | 1,135      |
| 168 | 4.763       | 0.0097       | 0.31      | 1,140      |
| 170 | 4.772       | 0.0097       | 0.30      | 1,145      |
| 172 | 4.782       | 0.0096       | 0.30      | 1,150      |
| 174 | 4.792       | 0.0095       | 0.30      | 1,155      |
| 176 | 4.801       | 0.0094       | 0.30      | 1,160      |
| 178 | 4.810       | 0.0094       | 0.30      | 1,165      |
| 180 | 4.820       | 0.0093       | 0.29      | 1,170      |
| 182 | 4.829       | 0.0093       | 0.29      | 1,175      |
| 184 | 4.838       | 0.0092       | 0.29      | 1,180      |
| 186 | 4.847       | 0.0091       | 0.29      | 1,185      |
| 188 | 4.856       | 0.0091       | 0.29      | 1,190      |
| 190 | 4.865       | 0.009        | 0.28      | 1,195      |
| 192 | 4.874       | 0.009        | 0.28      | 1,200      |
| 194 | 4.883       | 0.0089       | 0.28      | 1,205      |
| 196 | 4.892       | 0.0088       | 0.28      | 1,210      |
| 198 | 4.901       | 0.0088       | 0.28      | 1,215      |
| 200 | 4.910       | 0.0087       | 0.27      | 1,220      |
| 202 | 4.918       | 0.0087       | 0.27      | 1,225      |
| 204 | 4.927       | 0.0086       | 0.27      | 1,230      |
| 206 | 4.935       | 0.0086       | 0.27      | 1,235      |
| 208 | 4.944       | 0.0085       | 0.27      | 1,240      |
| 210 | 4.952       | 0.0085       | 0.27      | 1,245      |
| 212 | 4.961       | 0.0084       | 0.26      | 1,250      |
| 214 | 4.969       | 0.0084       | 0.26      | 1,255      |
| 216 | 4.978       | 0.0083       | 0.26      | 1,260      |
| 218 | 4.986       | 0.0083       | 0.26      | 1,265      |
| 220 | 4.994       | 0.0082       | 0.26      | 1,270      |

## 100-Year NOAA Atlas 14 Hyetograph Generation

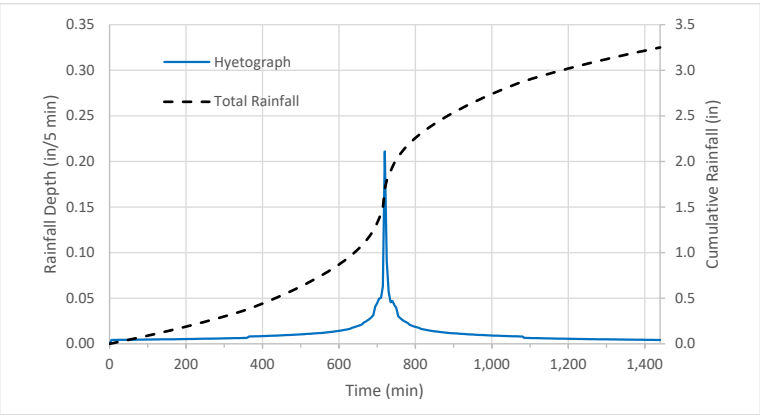
| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.339              |
| 10                            | 0.486              |
| 15                            | 0.587              |
| 30                            | 0.833              |
| 60                            | 1.26               |
| 120                           | 1.77               |
| 180                           | 2.13               |
| 360                           | 2.93               |
| 720                           | 4.03               |
| 1440                          | 5.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 5.250 in |                    |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 222 | 5.002       | 0.0082       | 0.26      | 1,275      |
| 224 | 5.010       | 0.0081       | 0.26      | 1,280      |
| 226 | 5.018       | 0.0081       | 0.25      | 1,285      |
| 228 | 5.026       | 0.008        | 0.25      | 1,290      |
| 230 | 5.034       | 0.008        | 0.25      | 1,295      |
| 232 | 5.042       | 0.008        | 0.25      | 1,300      |
| 234 | 5.050       | 0.0079       | 0.25      | 1,305      |
| 236 | 5.058       | 0.0079       | 0.25      | 1,310      |
| 238 | 5.066       | 0.0078       | 0.25      | 1,315      |
| 240 | 5.074       | 0.0078       | 0.25      | 1,320      |
| 242 | 5.082       | 0.0078       | 0.24      | 1,325      |
| 244 | 5.089       | 0.0077       | 0.24      | 1,330      |
| 246 | 5.097       | 0.0077       | 0.24      | 1,335      |
| 248 | 5.105       | 0.0076       | 0.24      | 1,340      |
| 250 | 5.112       | 0.0076       | 0.24      | 1,345      |
| 252 | 5.120       | 0.0076       | 0.24      | 1,350      |
| 254 | 5.127       | 0.0075       | 0.24      | 1,355      |
| 256 | 5.135       | 0.0075       | 0.24      | 1,360      |
| 258 | 5.142       | 0.0075       | 0.23      | 1,365      |
| 260 | 5.150       | 0.0074       | 0.23      | 1,370      |
| 262 | 5.157       | 0.0074       | 0.23      | 1,375      |
| 264 | 5.164       | 0.0073       | 0.23      | 1,380      |
| 266 | 5.172       | 0.0073       | 0.23      | 1,385      |
| 268 | 5.179       | 0.0073       | 0.23      | 1,390      |
| 270 | 5.186       | 0.0072       | 0.23      | 1,395      |
| 272 | 5.193       | 0.0072       | 0.23      | 1,400      |
| 274 | 5.201       | 0.0072       | 0.23      | 1,405      |
| 276 | 5.208       | 0.0071       | 0.23      | 1,410      |
| 278 | 5.215       | 0.0071       | 0.22      | 1,415      |
| 280 | 5.222       | 0.0071       | 0.22      | 1,420      |
| 282 | 5.229       | 0.0071       | 0.22      | 1,425      |
| 284 | 5.236       | 0.007        | 0.22      | 1,430      |
| 286 | 5.243       | 0.007        | 0.22      | 1,435      |
| 288 | 5.250       | 0.007        | 0.22      | 1,440      |

10-Year NOAA Atlas 14 Hyetograph Generation

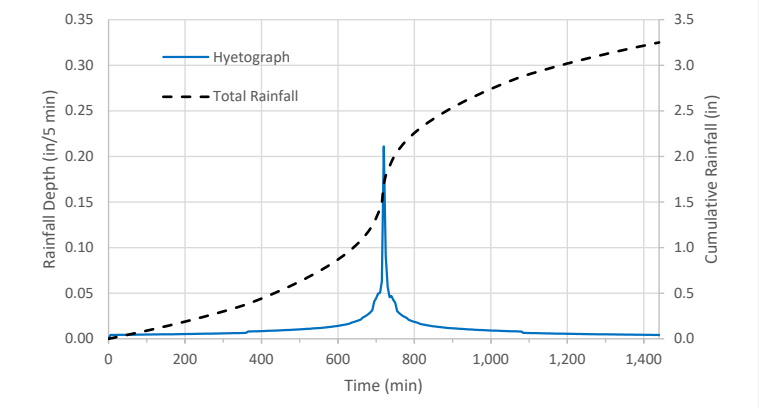
| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.211              |
| 10                            | 0.302              |
| 15                            | 0.365              |
| 30                            | 0.518              |
| 60                            | 0.782              |
| 120                           | 1.1                |
| 180                           | 1.33               |
| 360                           | 1.82               |
| 720                           | 2.5                |
| 1440                          | 3.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 3.251 in |                    |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| -   | 0           | 0            | 0         | 0          |
| 287 | 0.004       | 0.0043       | 0.136     | 5          |
| 285 | 0.009       | 0.0043       | 0.136     | 10         |
| 283 | 0.013       | 0.0043       | 0.137     | 15         |
| 281 | 0.017       | 0.0043       | 0.138     | 20         |
| 279 | 0.022       | 0.0044       | 0.138     | 25         |
| 277 | 0.026       | 0.0044       | 0.139     | 30         |
| 275 | 0.030       | 0.0044       | 0.139     | 35         |
| 273 | 0.035       | 0.0044       | 0.140     | 40         |
| 271 | 0.039       | 0.0044       | 0.141     | 45         |
| 269 | 0.044       | 0.0045       | 0.141     | 50         |
| 267 | 0.048       | 0.0045       | 0.142     | 55         |
| 265 | 0.053       | 0.0045       | 0.143     | 60         |
| 263 | 0.057       | 0.0045       | 0.143     | 65         |
| 261 | 0.062       | 0.0045       | 0.144     | 70         |
| 259 | 0.066       | 0.0046       | 0.145     | 75         |
| 257 | 0.071       | 0.0046       | 0.145     | 80         |
| 255 | 0.076       | 0.0046       | 0.146     | 85         |
| 253 | 0.080       | 0.0046       | 0.147     | 90         |
| 251 | 0.085       | 0.0047       | 0.147     | 95         |
| 249 | 0.090       | 0.0047       | 0.148     | 100        |
| 247 | 0.094       | 0.0047       | 0.149     | 105        |
| 245 | 0.099       | 0.0047       | 0.150     | 110        |
| 243 | 0.104       | 0.0048       | 0.150     | 115        |
| 241 | 0.109       | 0.0048       | 0.151     | 120        |
| 239 | 0.113       | 0.0048       | 0.152     | 125        |
| 237 | 0.118       | 0.0048       | 0.153     | 130        |
| 235 | 0.123       | 0.0049       | 0.154     | 135        |
| 233 | 0.128       | 0.0049       | 0.154     | 140        |
| 231 | 0.133       | 0.0049       | 0.155     | 145        |
| 229 | 0.138       | 0.0049       | 0.156     | 150        |
| 227 | 0.143       | 0.005        | 0.157     | 155        |
| 225 | 0.148       | 0.005        | 0.158     | 160        |
| 223 | 0.153       | 0.005        | 0.159     | 165        |
| 221 | 0.158       | 0.005        | 0.160     | 170        |
| 219 | 0.163       | 0.0051       | 0.161     | 175        |
| 217 | 0.168       | 0.0051       | 0.161     | 180        |
| 215 | 0.173       | 0.0051       | 0.162     | 185        |
| 213 | 0.178       | 0.0052       | 0.16      | 190        |
| 211 | 0.183       | 0.0052       | 0.16      | 195        |
| 209 | 0.189       | 0.0052       | 0.17      | 200        |
| 207 | 0.194       | 0.0053       | 0.17      | 205        |
| 205 | 0.199       | 0.0053       | 0.17      | 210        |
| 203 | 0.204       | 0.0053       | 0.17      | 215        |
| 201 | 0.210       | 0.0053       | 0.17      | 220        |
| 199 | 0.215       | 0.0054       | 0.17      | 225        |
| 197 | 0.221       | 0.0054       | 0.17      | 230        |
| 195 | 0.226       | 0.0055       | 0.17      | 235        |
| 193 | 0.232       | 0.0055       | 0.17      | 240        |
| 191 | 0.237       | 0.0055       | 0.18      | 245        |
| 189 | 0.243       | 0.0056       | 0.18      | 250        |

10-Year NOAA Atlas 14 Hyetograph Generation

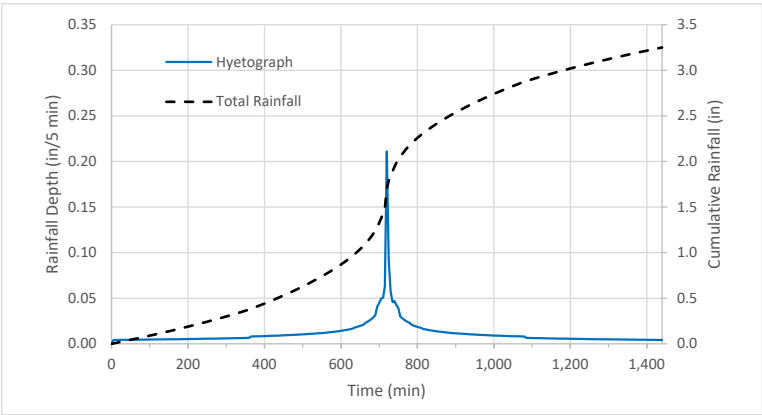
| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.211              |
| 10                            | 0.302              |
| 15                            | 0.365              |
| 30                            | 0.518              |
| 60                            | 0.782              |
| 120                           | 1.1                |
| 180                           | 1.33               |
| 360                           | 1.82               |
| 720                           | 2.5                |
| 1440                          | 3.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 3.251 in |                    |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 187 | 0.248       | 0.0056       | 0.18      | 255        |
| 185 | 0.254       | 0.0056       | 0.18      | 260        |
| 183 | 0.260       | 0.0057       | 0.18      | 265        |
| 181 | 0.265       | 0.0057       | 0.18      | 270        |
| 179 | 0.271       | 0.0058       | 0.18      | 275        |
| 177 | 0.277       | 0.0058       | 0.18      | 280        |
| 175 | 0.283       | 0.0058       | 0.19      | 285        |
| 173 | 0.288       | 0.0059       | 0.19      | 290        |
| 171 | 0.294       | 0.0059       | 0.19      | 295        |
| 169 | 0.300       | 0.006        | 0.19      | 300        |
| 167 | 0.306       | 0.006        | 0.19      | 305        |
| 165 | 0.312       | 0.006        | 0.19      | 310        |
| 163 | 0.319       | 0.0061       | 0.19      | 315        |
| 161 | 0.325       | 0.0061       | 0.19      | 320        |
| 159 | 0.331       | 0.0062       | 0.20      | 325        |
| 157 | 0.337       | 0.0062       | 0.20      | 330        |
| 155 | 0.343       | 0.0063       | 0.20      | 335        |
| 153 | 0.350       | 0.0063       | 0.20      | 340        |
| 151 | 0.356       | 0.0064       | 0.20      | 345        |
| 149 | 0.363       | 0.0064       | 0.20      | 350        |
| 147 | 0.369       | 0.0065       | 0.21      | 355        |
| 145 | 0.376       | 0.0066       | 0.21      | 360        |
| 143 | 0.384       | 0.008        | 0.21      | 365        |
| 141 | 0.392       | 0.0081       | 0.21      | 370        |
| 139 | 0.400       | 0.0081       | 0.21      | 375        |
| 137 | 0.408       | 0.0082       | 0.21      | 380        |
| 135 | 0.416       | 0.0083       | 0.22      | 385        |
| 133 | 0.425       | 0.0083       | 0.22      | 390        |
| 131 | 0.433       | 0.0084       | 0.22      | 395        |
| 129 | 0.441       | 0.0085       | 0.22      | 400        |
| 127 | 0.450       | 0.0085       | 0.22      | 405        |
| 125 | 0.459       | 0.0086       | 0.23      | 410        |
| 123 | 0.467       | 0.0087       | 0.23      | 415        |
| 121 | 0.476       | 0.0088       | 0.23      | 420        |
| 119 | 0.485       | 0.0088       | 0.23      | 425        |
| 117 | 0.494       | 0.0089       | 0.23      | 430        |
| 115 | 0.503       | 0.009        | 0.24      | 435        |
| 113 | 0.512       | 0.0091       | 0.24      | 440        |
| 111 | 0.521       | 0.0092       | 0.24      | 445        |
| 109 | 0.530       | 0.0093       | 0.24      | 450        |
| 107 | 0.540       | 0.0094       | 0.25      | 455        |
| 105 | 0.549       | 0.0095       | 0.25      | 460        |
| 103 | 0.559       | 0.0096       | 0.25      | 465        |
| 101 | 0.568       | 0.0097       | 0.25      | 470        |
| 99  | 0.578       | 0.0098       | 0.26      | 475        |
| 97  | 0.588       | 0.0099       | 0.26      | 480        |
| 95  | 0.598       | 0.01         | 0.26      | 485        |
| 93  | 0.608       | 0.0101       | 0.26      | 490        |
| 91  | 0.618       | 0.0102       | 0.27      | 495        |
| 89  | 0.629       | 0.0104       | 0.27      | 500        |
| 87  | 0.639       | 0.0105       | 0.27      | 505        |

10-Year NOAA Atlas 14 Hyetograph Generation

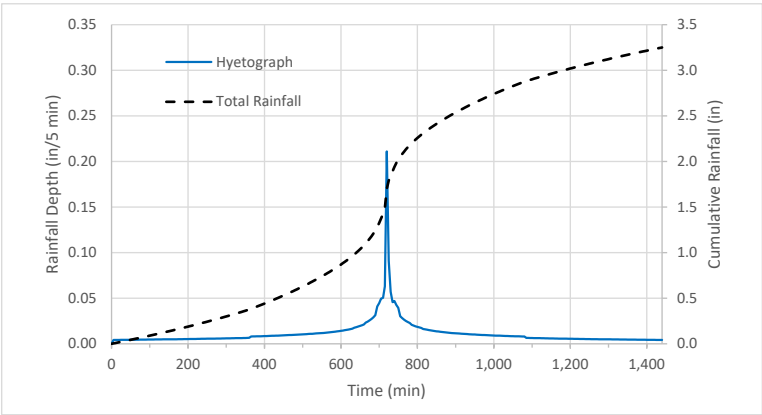
| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.211              |
| 10                            | 0.302              |
| 15                            | 0.365              |
| 30                            | 0.518              |
| 60                            | 0.782              |
| 120                           | 1.1                |
| 180                           | 1.33               |
| 360                           | 1.82               |
| 720                           | 2.5                |
| 1440                          | 3.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 3.251 in |                    |



| N  | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|----|-------------|--------------|-----------|------------|
| 85 | 0.650       | 0.0106       | 0.28      | 510        |
| 83 | 0.660       | 0.0108       | 0.28      | 515        |
| 81 | 0.671       | 0.0109       | 0.29      | 520        |
| 79 | 0.682       | 0.011        | 0.29      | 525        |
| 77 | 0.694       | 0.0112       | 0.29      | 530        |
| 75 | 0.705       | 0.0114       | 0.30      | 535        |
| 73 | 0.717       | 0.0115       | 0.30      | 540        |
| 71 | 0.728       | 0.0116       | 0.31      | 545        |
| 69 | 0.740       | 0.0118       | 0.31      | 550        |
| 67 | 0.752       | 0.0119       | 0.32      | 555        |
| 65 | 0.764       | 0.0121       | 0.32      | 560        |
| 63 | 0.776       | 0.0124       | 0.33      | 565        |
| 61 | 0.789       | 0.0126       | 0.33      | 570        |
| 59 | 0.802       | 0.0128       | 0.34      | 575        |
| 57 | 0.815       | 0.0131       | 0.35      | 580        |
| 55 | 0.828       | 0.0133       | 0.35      | 585        |
| 53 | 0.842       | 0.0136       | 0.36      | 590        |
| 51 | 0.856       | 0.0139       | 0.37      | 595        |
| 49 | 0.870       | 0.0142       | 0.37      | 600        |
| 47 | 0.884       | 0.0145       | 0.38      | 605        |
| 45 | 0.899       | 0.0149       | 0.39      | 610        |
| 43 | 0.914       | 0.0153       | 0.40      | 615        |
| 41 | 0.930       | 0.0157       | 0.41      | 620        |
| 39 | 0.946       | 0.0161       | 0.42      | 625        |
| 37 | 0.963       | 0.0166       | 0.44      | 630        |
| 35 | 0.981       | 0.0177       | 0.45      | 635        |
| 33 | 0.999       | 0.0183       | 0.46      | 640        |
| 31 | 1.018       | 0.0189       | 0.48      | 645        |
| 29 | 1.037       | 0.0196       | 0.50      | 650        |
| 27 | 1.058       | 0.0204       | 0.52      | 655        |
| 25 | 1.079       | 0.0212       | 0.54      | 660        |
| 23 | 1.102       | 0.0233       | 0.56      | 665        |
| 21 | 1.127       | 0.0244       | 0.59      | 670        |
| 19 | 1.152       | 0.0258       | 0.62      | 675        |
| 17 | 1.180       | 0.0273       | 0.66      | 680        |
| 15 | 1.209       | 0.0291       | 0.70      | 685        |
| 13 | 1.240       | 0.0314       | 0.75      | 690        |
| 11 | 1.281       | 0.0409       | 0.81      | 695        |
| 9  | 1.326       | 0.0446       | 0.88      | 700        |
| 7  | 1.375       | 0.0497       | 0.97      | 705        |
| 5  | 1.426       | 0.0504       | 1.13      | 710        |
| 3  | 1.489       | 0.063        | 1.46      | 715        |
| 1  | 1.700       | 0.211        | 2.53      | 720        |
| 2  | 1.791       | 0.091        | 1.81      | 725        |
| 4  | 1.848       | 0.0571       | 1.27      | 730        |
| 6  | 1.893       | 0.0456       | 1.04      | 735        |
| 8  | 1.940       | 0.0469       | 0.92      | 740        |
| 10 | 1.983       | 0.0426       | 0.84      | 745        |
| 12 | 2.022       | 0.0394       | 0.78      | 750        |
| 14 | 2.052       | 0.0302       | 0.72      | 755        |
| 16 | 2.081       | 0.0282       | 0.68      | 760        |

10-Year NOAA Atlas 14 Hyetograph Generation

| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.211              |
| 10                            | 0.302              |
| 15                            | 0.365              |
| 30                            | 0.518              |
| 60                            | 0.782              |
| 120                           | 1.1                |
| 180                           | 1.33               |
| 360                           | 1.82               |
| 720                           | 2.5                |
| 1440                          | 3.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 3.251 in |                    |

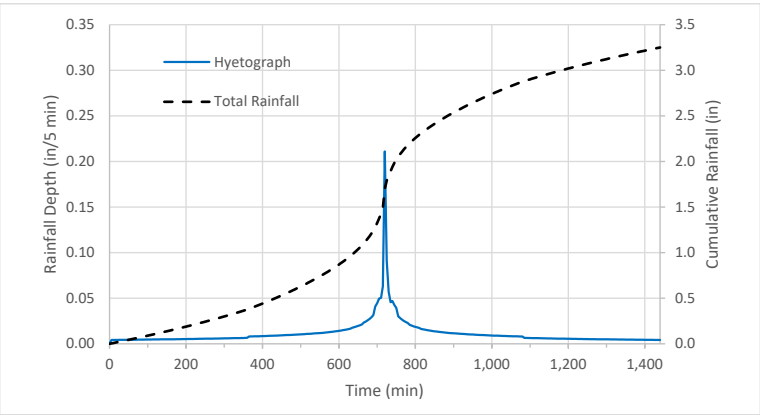


| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 18  | 2.107       | 0.0265       | 0.64      | 765        |
| 20  | 2.132       | 0.0251       | 0.60      | 770        |
| 22  | 2.156       | 0.0239       | 0.58      | 775        |
| 24  | 2.179       | 0.0228       | 0.55      | 780        |
| 26  | 2.200       | 0.0208       | 0.53      | 785        |
| 28  | 2.220       | 0.02         | 0.51      | 790        |
| 30  | 2.239       | 0.0192       | 0.49      | 795        |
| 32  | 2.257       | 0.0186       | 0.47      | 800        |
| 34  | 2.275       | 0.018        | 0.46      | 805        |
| 36  | 2.293       | 0.0174       | 0.44      | 810        |
| 38  | 2.309       | 0.0163       | 0.43      | 815        |
| 40  | 2.325       | 0.0159       | 0.42      | 820        |
| 42  | 2.341       | 0.0155       | 0.41      | 825        |
| 44  | 2.356       | 0.0151       | 0.40      | 830        |
| 46  | 2.370       | 0.0147       | 0.39      | 835        |
| 48  | 2.385       | 0.0144       | 0.38      | 840        |
| 50  | 2.399       | 0.014        | 0.37      | 845        |
| 52  | 2.412       | 0.0137       | 0.36      | 850        |
| 54  | 2.426       | 0.0135       | 0.36      | 855        |
| 56  | 2.439       | 0.0132       | 0.35      | 860        |
| 58  | 2.452       | 0.0129       | 0.34      | 865        |
| 60  | 2.465       | 0.0127       | 0.34      | 870        |
| 62  | 2.477       | 0.0125       | 0.33      | 875        |
| 64  | 2.489       | 0.0123       | 0.32      | 880        |
| 66  | 2.502       | 0.012        | 0.32      | 885        |
| 68  | 2.513       | 0.0118       | 0.31      | 890        |
| 70  | 2.525       | 0.0117       | 0.31      | 895        |
| 72  | 2.537       | 0.0115       | 0.30      | 900        |
| 74  | 2.548       | 0.0114       | 0.30      | 905        |
| 76  | 2.559       | 0.0113       | 0.30      | 910        |
| 78  | 2.570       | 0.0111       | 0.29      | 915        |
| 80  | 2.581       | 0.011        | 0.29      | 920        |
| 82  | 2.592       | 0.0108       | 0.28      | 925        |
| 84  | 2.603       | 0.0107       | 0.28      | 930        |
| 86  | 2.613       | 0.0105       | 0.28      | 935        |
| 88  | 2.624       | 0.0104       | 0.27      | 940        |
| 90  | 2.634       | 0.0103       | 0.27      | 945        |
| 92  | 2.644       | 0.0102       | 0.27      | 950        |
| 94  | 2.654       | 0.01         | 0.26      | 955        |
| 96  | 2.664       | 0.0099       | 0.26      | 960        |
| 98  | 2.674       | 0.0098       | 0.26      | 965        |
| 100 | 2.684       | 0.0097       | 0.25      | 970        |
| 102 | 2.693       | 0.0096       | 0.25      | 975        |
| 104 | 2.703       | 0.0095       | 0.25      | 980        |
| 106 | 2.712       | 0.0094       | 0.25      | 985        |
| 108 | 2.722       | 0.0093       | 0.24      | 990        |
| 110 | 2.731       | 0.0092       | 0.24      | 995        |
| 112 | 2.740       | 0.0091       | 0.24      | 1,000      |
| 114 | 2.749       | 0.009        | 0.24      | 1,005      |
| 116 | 2.758       | 0.009        | 0.23      | 1,010      |
| 118 | 2.767       | 0.0089       | 0.23      | 1,015      |



10-Year NOAA Atlas 14 Hyetograph Generation

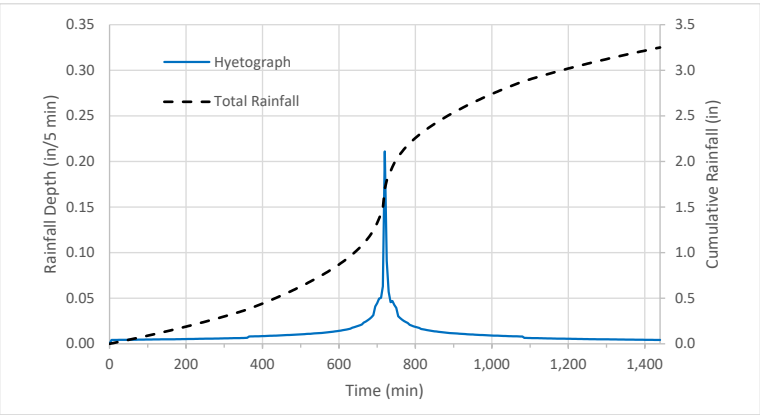
| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.211              |
| 10                            | 0.302              |
| 15                            | 0.365              |
| 30                            | 0.518              |
| 60                            | 0.782              |
| 120                           | 1.1                |
| 180                           | 1.33               |
| 360                           | 1.82               |
| 720                           | 2.5                |
| 1440                          | 3.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 3.251 in |                    |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 120 | 2.776       | 0.0088       | 0.23      | 1,020      |
| 122 | 2.784       | 0.0087       | 0.23      | 1,025      |
| 124 | 2.793       | 0.0086       | 0.23      | 1,030      |
| 126 | 2.802       | 0.0086       | 0.22      | 1,035      |
| 128 | 2.810       | 0.0085       | 0.22      | 1,040      |
| 130 | 2.819       | 0.0084       | 0.22      | 1,045      |
| 132 | 2.827       | 0.0084       | 0.22      | 1,050      |
| 134 | 2.835       | 0.0083       | 0.22      | 1,055      |
| 136 | 2.843       | 0.0082       | 0.22      | 1,060      |
| 138 | 2.852       | 0.0082       | 0.21      | 1,065      |
| 140 | 2.860       | 0.0081       | 0.21      | 1,070      |
| 142 | 2.868       | 0.008        | 0.21      | 1,075      |
| 144 | 2.876       | 0.008        | 0.21      | 1,080      |
| 146 | 2.882       | 0.0065       | 0.21      | 1,085      |
| 148 | 2.889       | 0.0065       | 0.21      | 1,090      |
| 150 | 2.895       | 0.0064       | 0.20      | 1,095      |
| 152 | 2.901       | 0.0064       | 0.20      | 1,100      |
| 154 | 2.908       | 0.0063       | 0.20      | 1,105      |
| 156 | 2.914       | 0.0063       | 0.20      | 1,110      |
| 158 | 2.920       | 0.0062       | 0.20      | 1,115      |
| 160 | 2.926       | 0.0062       | 0.20      | 1,120      |
| 162 | 2.932       | 0.0061       | 0.19      | 1,125      |
| 164 | 2.939       | 0.0061       | 0.19      | 1,130      |
| 166 | 2.945       | 0.006        | 0.19      | 1,135      |
| 168 | 2.951       | 0.006        | 0.19      | 1,140      |
| 170 | 2.957       | 0.0059       | 0.19      | 1,145      |
| 172 | 2.962       | 0.0059       | 0.19      | 1,150      |
| 174 | 2.968       | 0.0059       | 0.19      | 1,155      |
| 176 | 2.974       | 0.0058       | 0.18      | 1,160      |
| 178 | 2.980       | 0.0058       | 0.18      | 1,165      |
| 180 | 2.986       | 0.0057       | 0.18      | 1,170      |
| 182 | 2.991       | 0.0057       | 0.18      | 1,175      |
| 184 | 2.997       | 0.0057       | 0.18      | 1,180      |
| 186 | 3.003       | 0.0056       | 0.18      | 1,185      |
| 188 | 3.008       | 0.0056       | 0.18      | 1,190      |
| 190 | 3.014       | 0.0055       | 0.18      | 1,195      |
| 192 | 3.019       | 0.0055       | 0.17      | 1,200      |
| 194 | 3.025       | 0.0055       | 0.17      | 1,205      |
| 196 | 3.030       | 0.0054       | 0.17      | 1,210      |
| 198 | 3.035       | 0.0054       | 0.17      | 1,215      |
| 200 | 3.041       | 0.0054       | 0.17      | 1,220      |
| 202 | 3.046       | 0.0053       | 0.17      | 1,225      |
| 204 | 3.051       | 0.0053       | 0.17      | 1,230      |
| 206 | 3.057       | 0.0053       | 0.17      | 1,235      |
| 208 | 3.062       | 0.0052       | 0.17      | 1,240      |
| 210 | 3.067       | 0.0052       | 0.17      | 1,245      |
| 212 | 3.072       | 0.0052       | 0.16      | 1,250      |
| 214 | 3.077       | 0.0051       | 0.16      | 1,255      |
| 216 | 3.083       | 0.0051       | 0.16      | 1,260      |
| 218 | 3.088       | 0.0051       | 0.16      | 1,265      |
| 220 | 3.093       | 0.0051       | 0.16      | 1,270      |

10-Year NOAA Atlas 14 Hyetograph Generation

| Duration<br>(min)             | Rain Depth<br>(in) |
|-------------------------------|--------------------|
| 5                             | 0.211              |
| 10                            | 0.302              |
| 15                            | 0.365              |
| 30                            | 0.518              |
| 60                            | 0.782              |
| 120                           | 1.1                |
| 180                           | 1.33               |
| 360                           | 1.82               |
| 720                           | 2.5                |
| 1440                          | 3.25               |
| Time Step = 5 min             |                    |
| Rainfall Sum Check = 3.251 in |                    |



| N   | PN (inches) | Incr. P (in) | I (in/hr) | Time (min) |
|-----|-------------|--------------|-----------|------------|
| 222 | 3.098       | 0.005        | 0.16      | 1,275      |
| 224 | 3.103       | 0.005        | 0.16      | 1,280      |
| 226 | 3.108       | 0.005        | 0.16      | 1,285      |
| 228 | 3.113       | 0.0049       | 0.16      | 1,290      |
| 230 | 3.118       | 0.0049       | 0.16      | 1,295      |
| 232 | 3.123       | 0.0049       | 0.16      | 1,300      |
| 234 | 3.127       | 0.0049       | 0.15      | 1,305      |
| 236 | 3.132       | 0.0048       | 0.15      | 1,310      |
| 238 | 3.137       | 0.0048       | 0.15      | 1,315      |
| 240 | 3.142       | 0.0048       | 0.15      | 1,320      |
| 242 | 3.147       | 0.0048       | 0.15      | 1,325      |
| 244 | 3.151       | 0.0047       | 0.15      | 1,330      |
| 246 | 3.156       | 0.0047       | 0.15      | 1,335      |
| 248 | 3.161       | 0.0047       | 0.15      | 1,340      |
| 250 | 3.165       | 0.0047       | 0.15      | 1,345      |
| 252 | 3.170       | 0.0046       | 0.15      | 1,350      |
| 254 | 3.175       | 0.0046       | 0.15      | 1,355      |
| 256 | 3.179       | 0.0046       | 0.15      | 1,360      |
| 258 | 3.184       | 0.0046       | 0.15      | 1,365      |
| 260 | 3.188       | 0.0046       | 0.14      | 1,370      |
| 262 | 3.193       | 0.0045       | 0.14      | 1,375      |
| 264 | 3.197       | 0.0045       | 0.14      | 1,380      |
| 266 | 3.202       | 0.0045       | 0.14      | 1,385      |
| 268 | 3.206       | 0.0045       | 0.14      | 1,390      |
| 270 | 3.211       | 0.0045       | 0.14      | 1,395      |
| 272 | 3.215       | 0.0044       | 0.14      | 1,400      |
| 274 | 3.220       | 0.0044       | 0.14      | 1,405      |
| 276 | 3.224       | 0.0044       | 0.14      | 1,410      |
| 278 | 3.228       | 0.0044       | 0.14      | 1,415      |
| 280 | 3.233       | 0.0044       | 0.14      | 1,420      |
| 282 | 3.237       | 0.0043       | 0.14      | 1,425      |
| 284 | 3.241       | 0.0043       | 0.14      | 1,430      |
| 286 | 3.246       | 0.0043       | 0.14      | 1,435      |
| 288 | 3.250       | 0.0043       | 0.14      | 1,440      |



United States  
Department of  
Agriculture

NRCS

Natural  
Resources  
Conservation  
Service

A product of the National  
Cooperative Soil Survey,  
a joint effort of the United  
States Department of  
Agriculture and other  
Federal agencies, State  
agencies including the  
Agricultural Experiment  
Stations, and local  
participants

# Custom Soil Resource Report for San Diego County Area, California

Rancho Guejito Winery  
Hydrologic Soils



October 2, 2020

# Preface

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Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist ([http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2\\_053951](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951)).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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# How Soil Surveys Are Made

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Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and



## Custom Soil Resource Report

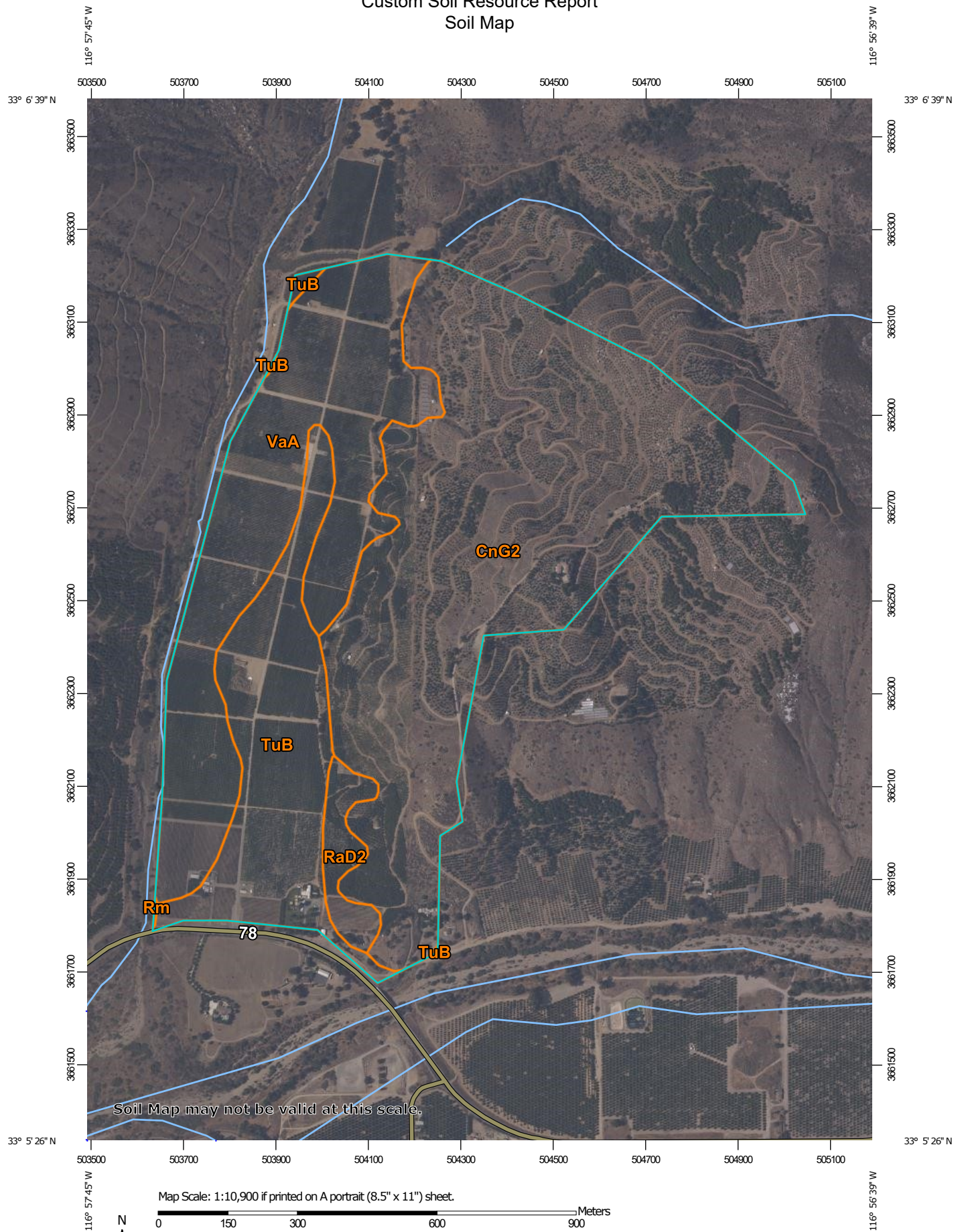
identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

---

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

# Custom Soil Resource Report Soil Map



# Custom Soil Resource Report

## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

### Water Features

 Streams and Canals

### Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

### Background

 Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: San Diego County Area, California  
Survey Area Data: Version 15, May 27, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 18, 2018—Aug 22, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

| Map Unit Symbol                    | Map Unit Name                                                        | Acres in AOI | Percent of AOI |
|------------------------------------|----------------------------------------------------------------------|--------------|----------------|
| CnG2                               | Cieneba-Fallbrook rocky sandy loams, 30 to 65 percent slopes, eroded | 151.8        | 54.0%          |
| RaD2                               | Ramona sandy loam, 9 to 15 percent slopes, eroded                    | 7.2          | 2.6%           |
| Rm                                 | Riverwash                                                            | 0.1          | 0.0%           |
| TuB                                | Tujunga sand, 0 to 5 percent slopes                                  | 46.5         | 16.5%          |
| VaA                                | Visalia sandy loam, 0 to 2 percent slopes                            | 75.5         | 26.9%          |
| <b>Totals for Area of Interest</b> |                                                                      | <b>281.2</b> | <b>100.0%</b>  |

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

## Custom Soil Resource Report

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## San Diego County Area, California

### CnG2—Cieneba-Fallbrook rocky sandy loams, 30 to 65 percent slopes, eroded

#### Map Unit Setting

*National map unit symbol:* hb9x  
*Elevation:* 300 to 4,000 feet  
*Mean annual precipitation:* 12 to 35 inches  
*Mean annual air temperature:* 57 to 64 degrees F  
*Frost-free period:* 200 to 320 days  
*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Cieneba and similar soils:* 40 percent  
*Fallbrook and similar soils:* 35 percent  
*Rock outcrop:* 20 percent  
*Minor components:* 5 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Cieneba

##### Setting

*Landform:* Hills  
*Landform position (two-dimensional):* Backslope  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Parent material:* Residuum weathered from granite and granodiorite

##### Typical profile

*H1 - 0 to 10 inches:* coarse sandy loam  
*H2 - 10 to 14 inches:* weathered bedrock

##### Properties and qualities

*Slope:* 30 to 65 percent  
*Depth to restrictive feature:* 4 to 20 inches to paralithic bedrock  
*Drainage class:* Somewhat excessively drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* High (1.98 to 5.95 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water capacity:* Very low (about 1.0 inches)

##### Interpretive groups

*Land capability classification (irrigated):* 7e  
*Land capability classification (nonirrigated):* 7e  
*Hydrologic Soil Group:* D  
*Ecological site:* R019XD060CA - SHALLOW LOAMY (1975)  
*Hydric soil rating:* No

## Description of Fallbrook

### Setting

*Landform:* Hills  
*Landform position (two-dimensional):* Backslope  
*Landform position (three-dimensional):* Side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Parent material:* Residuum weathered from granodiorite

### Typical profile

*H1 - 0 to 6 inches:* sandy loam  
*H2 - 6 to 24 inches:* sandy clay loam, loam  
*H2 - 6 to 24 inches:* weathered bedrock  
*H3 - 24 to 28 inches:*

### Properties and qualities

*Slope:* 30 to 65 percent  
*Depth to restrictive feature:* 20 to 40 inches to paralithic bedrock  
*Drainage class:* Well drained  
*Runoff class:* High  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high  
(0.57 to 1.98 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water capacity:* Moderate (about 6.5 inches)

### Interpretive groups

*Land capability classification (irrigated):* 6e  
*Land capability classification (nonirrigated):* 6e  
*Hydrologic Soil Group:* C  
*Ecological site:* R019XD029CA  
*Hydric soil rating:* No

## Description of Rock Outcrop

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 8  
*Hydrologic Soil Group:* D  
*Hydric soil rating:* No

## Minor Components

### Vista

*Percent of map unit:* 5 percent  
*Hydric soil rating:* No



## **RaD2—Ramona sandy loam, 9 to 15 percent slopes, eroded**

### **Map Unit Setting**

*National map unit symbol:* hbfv

*Elevation:* 250 to 3,500 feet

*Mean annual precipitation:* 10 to 20 inches

*Mean annual air temperature:* 63 degrees F

*Frost-free period:* 230 to 320 days

*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Ramona and similar soils:* 85 percent

*Minor components:* 15 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Ramona**

#### **Setting**

*Landform:* Alluvial fans

*Landform position (two-dimensional):* Toeslope

*Landform position (three-dimensional):* Base slope, rise

*Down-slope shape:* Linear

*Across-slope shape:* Convex

*Parent material:* Alluvium derived from granite

#### **Typical profile**

*H1 - 0 to 10 inches:* sandy loam

*H2 - 10 to 60 inches:* sandy clay loam, clay loam

*H2 - 10 to 60 inches:* sandy clay loam, sandy loam

*H3 - 60 to 74 inches:*

*H3 - 60 to 74 inches:*

#### **Properties and qualities**

*Slope:* 9 to 15 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* High

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high (0.20 to 0.57 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water capacity:* Very high (about 16.2 inches)

#### **Interpretive groups**

*Land capability classification (irrigated):* 4e

*Land capability classification (nonirrigated):* 4e

## Custom Soil Resource Report

*Hydrologic Soil Group: C*  
*Ecological site: R019XD029CA*  
*Hydric soil rating: No*

### Minor Components

#### Greenfield

*Percent of map unit: 5 percent*  
*Hydric soil rating: No*

#### Fallbrook

*Percent of map unit: 5 percent*  
*Hydric soil rating: No*

#### Placentia

*Percent of map unit: 5 percent*  
*Hydric soil rating: No*

### Rm—Riverwash

#### Map Unit Setting

*National map unit symbol: hbg6*  
*Elevation: 700 to 2,900 feet*  
*Mean annual precipitation: 8 to 15 inches*  
*Mean annual air temperature: 46 to 52 degrees F*  
*Frost-free period: 110 to 180 days*  
*Farmland classification: Not prime farmland*

#### Map Unit Composition

*Riverwash: 100 percent*  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Riverwash

##### Setting

*Landform: Drainageways*  
*Parent material: Sandy, gravelly, or cobbly alluvium derived from mixed sources*

##### Typical profile

*H1 - 0 to 6 inches: gravelly coarse sand*  
*H2 - 6 to 60 inches: stratified extremely gravelly coarse sand to gravelly sand*

##### Properties and qualities

*Slope: 0 to 4 percent*  
*Drainage class: Excessively drained*  
*Runoff class: Negligible*  
*Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)*  
*Depth to water table: About 60 to 72 inches*  
*Frequency of flooding: OccasionalNone*  
*Available water capacity: Very low (about 1.9 inches)*

**Interpretive groups**

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 8

*Hydrologic Soil Group:* D

*Hydric soil rating:* Yes

**TuB—Tujunga sand, 0 to 5 percent slopes**

**Map Unit Setting**

*National map unit symbol:* hbh0

*Elevation:* 10 to 2,500 feet

*Mean annual precipitation:* 10 to 25 inches

*Mean annual air temperature:* 59 to 64 degrees F

*Frost-free period:* 280 to 350 days

*Farmland classification:* Farmland of statewide importance

**Map Unit Composition**

*Tujunga and similar soils:* 85 percent

*Minor components:* 15 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

**Description of Tujunga**

**Setting**

*Landform:* Flood plains

*Landform position (three-dimensional):* Riser, flat

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Alluvium derived from granite

**Typical profile**

*H1 - 0 to 14 inches:* sand

*H2 - 14 to 34 inches:* loamy sand, fine sand, sand

*H2 - 14 to 34 inches:* stratified gravelly sand to gravelly loamy sand

*H2 - 14 to 34 inches:*

*H3 - 34 to 60 inches:*

**Properties and qualities**

*Slope:* 0 to 5 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Somewhat excessively drained

*Runoff class:* Negligible

*Capacity of the most limiting layer to transmit water (Ksat):* High to very high (5.95 to 19.98 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* Rare

*Frequency of ponding:* None

*Available water capacity:* Moderate (about 6.7 inches)

**Interpretive groups**

*Land capability classification (irrigated):* 3s

*Land capability classification (nonirrigated):* 4e

## Custom Soil Resource Report

*Hydrologic Soil Group: A*  
*Ecological site: R019XD035CA*  
*Hydric soil rating: No*

### Minor Components

#### Grangeville

*Percent of map unit: 5 percent*  
*Hydric soil rating: No*

#### Ramona

*Percent of map unit: 5 percent*  
*Hydric soil rating: No*

#### Visalia

*Percent of map unit: 2 percent*  
*Hydric soil rating: No*

#### Riverwash

*Percent of map unit: 2 percent*  
*Landform: Drainageways*  
*Hydric soil rating: Yes*

#### Unnamed

*Percent of map unit: 1 percent*  
*Landform: Flood plains*  
*Hydric soil rating: Yes*

## VaA—Visalia sandy loam, 0 to 2 percent slopes

### Map Unit Setting

*National map unit symbol: hbh2*  
*Elevation: 600 to 1,200 feet*  
*Mean annual precipitation: 15 inches*  
*Mean annual air temperature: 57 degrees F*  
*Frost-free period: 200 to 350 days*  
*Farmland classification: Prime farmland if irrigated and either protected from flooding or not frequently flooded during the growing season*

### Map Unit Composition

*Visalia and similar soils: 85 percent*  
*Minor components: 15 percent*  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Visalia

#### Setting

*Landform: Alluvial fans*  
*Landform position (two-dimensional): Toeslope*  
*Landform position (three-dimensional): Riser, flat*  
*Down-slope shape: Linear*  
*Across-slope shape: Convex*

## Custom Soil Resource Report

*Parent material:* Alluvium derived from granite

### Typical profile

*H1 - 0 to 12 inches:* sandy loam

*H2 - 12 to 40 inches:* sandy loam, fine sandy loam

*H2 - 12 to 40 inches:* very fine sandy loam, loam

*H3 - 40 to 60 inches:*

*H3 - 40 to 60 inches:*

### Properties and qualities

*Slope:* 0 to 2 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* Very low

*Capacity of the most limiting layer to transmit water (Ksat):* High (1.98 to 5.95 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* Rare

*Frequency of ponding:* None

*Available water capacity:* Very high (about 14.1 inches)

### Interpretive groups

*Land capability classification (irrigated):* 1

*Land capability classification (nonirrigated):* 2c

*Hydrologic Soil Group:* A

*Hydric soil rating:* No

### Minor Components

#### Greenfield

*Percent of map unit:* 5 percent

*Hydric soil rating:* No

#### Grangeville

*Percent of map unit:* 5 percent

*Hydric soil rating:* No

#### Tujunga

*Percent of map unit:* 2 percent

*Hydric soil rating:* No

#### Placentia

*Percent of map unit:* 2 percent

*Hydric soil rating:* No

#### Unnamed

*Percent of map unit:* 1 percent

*Landform:* Flood plains

*Hydric soil rating:* Yes

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## HEC-HMS Output Table

### Pre-Project 100-Year

| Subbasin     | Area (mi2) | Q (cfs) | Peak Time        | Vol. (in) |
|--------------|------------|---------|------------------|-----------|
| Subbasin-1A  | 0.0186     | 37.8    | 15Jan2020, 11:59 | 0.38      |
| Subbasin-1B  | 0.115      | 233.4   | 15Jan2020, 12:00 | 0.38      |
| Subbasin-2   | 0.127      | 216.7   | 15Jan2020, 12:08 | 0.38      |
| Subbasin-3   | 0.0478     | 10.1    | 15Jan2020, 12:19 | 0.17      |
| Subbasin-4   | 0.24855    | 146.3   | 15Jan2020, 12:35 | 0.24      |
| Subbasin-5B  | 0.2601125  | 117.6   | 15Jan2020, 12:44 | 0.23      |
| Subbasin-5A  | 0.0139063  | 4.9     | 15Jan2020, 12:11 | 0.22      |
| Subbasin-6   | 0.01859    | 37.8    | 15Jan2020, 11:59 | 0.38      |
| Subbasin-7   | 0.051715   | 85.1    | 15Jan2020, 12:02 | 0.38      |
| Subbasin-8   | 0.054684   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-9   | 0.010625   | 21.6    | 15Jan2020, 11:59 | 0.38      |
| Subbasin-10  | 0.073589   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-11  | 0.0202     | 43.4    | 15Jan2020, 11:59 | 0.52      |
| Subbasin-12  | 0.097689   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-13  | 0.01047    | 22.5    | 15Jan2020, 11:59 | 0.52      |
| Subbasin-14  | 0.117534   | 22.7    | 15Jan2020, 12:12 | 0.08      |
|              |            |         |                  |           |
|              |            |         |                  |           |
| Subbasin-15  | 0.00875    | 19.9    | 15Jan2020, 11:59 | 0.72      |
| Subbasin-16  | 0.0176562  | 24.6    | 15Jan2020, 12:08 | 0.61      |
| J-110        | 0.409209   | 121     | 15Jan2020, 12:44 | 0.2       |
| Subbasin-ST1 | 0.0017188  | 2.7     | 15Jan2020, 12:01 | 3.41      |
| J-111        | 0.4109278  | 121.3   | 15Jan2020, 12:44 | 0.22      |
| Subbasin-ST2 | 0.00125    | 1.2     | 15Jan2020, 12:01 | 2.09      |
| J-112        | 0.4121778  | 121.5   | 15Jan2020, 12:44 | 0.22      |

### Post-Project 100-Year

| Subbasin     | Area (mi2) | Q (cfs) | Peak Time        | Vol. (in) |
|--------------|------------|---------|------------------|-----------|
| Subbasin-1A  | 0.0186     | 37.8    | 15Jan2020, 11:59 | 0.38      |
| Subbasin-1B  | 0.115      | 233.4   | 15Jan2020, 12:00 | 0.38      |
| Subbasin-2   | 0.127      | 216.7   | 15Jan2020, 12:08 | 0.38      |
| Subbasin-3   | 0.0478     | 10.1    | 15Jan2020, 12:19 | 0.17      |
| Subbasin-4   | 0.24855    | 146.3   | 15Jan2020, 12:35 | 0.24      |
| Subbasin-5A  | 0.2618312  | 133.6   | 15Jan2020, 12:39 | 0.25      |
| Subbasin-5B  | 0.2685499  | 102.7   | 15Jan2020, 12:49 | 0.25      |
| Subbasin-6   | 0.01859    | 37.8    | 15Jan2020, 11:59 | 0.38      |
| Subbasin-7   | 0.051715   | 85.1    | 15Jan2020, 12:02 | 0.38      |
| Subbasin-8   | 0.054684   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-9   | 0.010625   | 21.6    | 15Jan2020, 11:59 | 0.38      |
| Subbasin-10  | 0.073589   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-11  | 0.0202     | 43.4    | 15Jan2020, 11:59 | 0.52      |
| Subbasin-12  | 0.097689   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-13  | 0.01047    | 22.5    | 15Jan2020, 11:59 | 0.52      |
| Subbasin-14A | 0.1147215  | 23      | 15Jan2020, 12:03 | 0.07      |
| Swale_1      | 0.1147215  | 22.4    | 15Jan2020, 12:05 | 0.07      |
| Subbasin-14B | 0.1226903  | 23.5    | 15Jan2020, 12:11 | 0.14      |
| Subbasin-15  | 0.00875    | 19.9    | 15Jan2020, 11:59 | 0.72      |
| Subbasin-16  | 0.0175     | 24.4    | 15Jan2020, 12:08 | 0.62      |
| J-110        | 0.4087402  | 105.7   | 15Jan2020, 12:49 | 0.23      |
| Subbasin-ST1 | 0.0021875  | 3.9     | 15Jan2020, 12:00 | 3.67      |
| J-111        | 0.4109277  | 106.2   | 15Jan2020, 12:49 | 0.25      |
| Subbasin-ST2 | 0.00125    | 1.5     | 15Jan2020, 12:01 | 2.62      |
| J-112        | 0.4121777  | 106.4   | 15Jan2020, 12:49 | 0.26      |



## HEC-HMS Output Table

### Pre-Project 10-Year

| Subbasin     | Area (mi2) | Q (cfs) | Peak Time        | Vol. (in) |
|--------------|------------|---------|------------------|-----------|
| Subbasin-1A  | 0.0186     | 19.4    | 15Jan2020, 11:59 | 0.15      |
| Subbasin-1B  | 0.115      | 117.3   | 15Jan2020, 12:00 | 0.15      |
| Subbasin-2   | 0.127      | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-3   | 0.0478     | 1.7     | 15Jan2020, 12:33 | 0.04      |
| Subbasin-4   | 0.24855    | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-5B  | 0.2601125  | 0.1     | 15Jan2020, 12:28 | 0         |
| Subbasin-5A  | 0.0139063  | 0.9     | 15Jan2020, 12:18 | 0.08      |
| Subbasin-6   | 0.01859    | 19.3    | 15Jan2020, 11:59 | 0.15      |
| Subbasin-7   | 0.051715   | 37.3    | 15Jan2020, 12:03 | 0.15      |
| Subbasin-8   | 0.054684   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-9   | 0.010625   | 11.1    | 15Jan2020, 11:58 | 0.15      |
| Subbasin-10  | 0.073589   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-11  | 0.0202     | 23.3    | 15Jan2020, 11:59 | 0.18      |
| Subbasin-12  | 0.097689   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-13  | 0.01047    | 12.1    | 15Jan2020, 12:00 | 0.18      |
| Subbasin-14  | 0.117534   | 12.3    | 15Jan2020, 12:14 | 0.04      |
|              |            |         |                  |           |
|              |            |         |                  |           |
| Subbasin-15  | 0.00875    | 11.2    | 15Jan2020, 11:59 | 0.25      |
| Subbasin-16  | 0.0176562  | 12      | 15Jan2020, 12:11 | 0.27      |
| J-110        | 0.409209   | 20.1    | 15Jan2020, 12:14 | 0.03      |
| Subbasin-ST1 | 0.0017188  | 1.7     | 15Jan2020, 12:01 | 2.11      |
| J-111        | 0.4109278  | 20.6    | 15Jan2020, 12:14 | 0.04      |
| Subbasin-ST2 | 0.00125    | 0.7     | 15Jan2020, 12:01 | 1.3       |
| J-112        | 0.4121778  | 20.8    | 15Jan2020, 12:14 | 0.04      |

### Post-Project 100-Year

| Subbasin     | Area (mi2) | Q (cfs) | Peak Time        | Vol. (in) |
|--------------|------------|---------|------------------|-----------|
| Subbasin-1A  | 0.0186     | 19.4    | 15Jan2020, 11:59 | 0.15      |
| Subbasin-1B  | 0.115      | 117.3   | 15Jan2020, 12:00 | 0.15      |
| Subbasin-2   | 0.127      | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-3   | 0.0478     | 1.7     | 15Jan2020, 12:33 | 0.04      |
| Subbasin-4   | 0.24855    | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-5A  | 0.2618312  | 2.4     | 15Jan2020, 12:07 | 0.01      |
| Subbasin-5B  | 0.2685499  | 2.7     | 15Jan2020, 12:24 | 0.02      |
| Subbasin-6   | 0.01859    | 19.3    | 15Jan2020, 11:59 | 0.15      |
| Subbasin-7   | 0.051715   | 37.3    | 15Jan2020, 12:03 | 0.15      |
| Subbasin-8   | 0.054684   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-9   | 0.010625   | 11.1    | 15Jan2020, 11:58 | 0.15      |
| Subbasin-10  | 0.073589   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-11  | 0.0202     | 23.3    | 15Jan2020, 11:59 | 0.18      |
| Subbasin-12  | 0.097689   | 0       | 15Jan2020, 00:00 | 0         |
| Subbasin-13  | 0.01047    | 12.1    | 15Jan2020, 12:00 | 0.18      |
| Subbasin-14A | 0.1147215  | 12.2    | 15Jan2020, 12:04 | 0.03      |
| Swale_1      | 0.1147215  | 11.7    | 15Jan2020, 12:06 | 0.03      |
| Subbasin-14B | 0.1226903  | 12.1    | 15Jan2020, 12:14 | 0.08      |
| Subbasin-15  | 0.00875    | 11.2    | 15Jan2020, 11:59 | 0.25      |
| Subbasin-16  | 0.0175     | 12.6    | 15Jan2020, 12:10 | 0.28      |
| J-110        | 0.4087402  | 20.4    | 15Jan2020, 12:14 | 0.05      |
| Subbasin-ST1 | 0.0021875  | 2.3     | 15Jan2020, 12:01 | 2.27      |
| J-111        | 0.4109277  | 21      | 15Jan2020, 12:14 | 0.06      |
| Subbasin-ST2 | 0.00125    | 0.9     | 15Jan2020, 12:01 | 1.62      |
| J-112        | 0.4121777  | 21.3    | 15Jan2020, 12:14 | 0.07      |

## **Appendix E**

### HEC-RAS Hydraulic Analysis Results

## HEC-RAS Existing Condition Model Printouts

## HEC-RAS Proposed Condition Model Printouts

## HEC-RAS Results Summary Table

## **Appendix F**

CALTRANS As-Built Plans for Guejito Creek Bridge

# **Appendix H**

## **Project Plan**

## **Map Pocket 1**

Drainage Study Map for Guejito Creek Watershed



## **Map Pocket 2**

Drainage Study Map for Rancho Guejito Tasting Facility and Event Center (Pre-Project)

## **Map Pocket 3**

Drainage Study Map for Rancho Guejito Tasting Facility and Event Center (Post-Project)

## **Map Pocket 4**

Caltrans Drainage Study Map for Rancho Guejito Tasting Facility and Event Center  
(Pre-Project)

## **Map Pocket 5**

Caltrans Drainage Study Map for Rancho Guejito Tasting Facility and Event Center  
(Post-Project)

## **Map Pocket 6**

HEC-RAS Hydraulic Workmap