

DRAINAGE STUDY

FOR

**GS Valley Expansion
28435 Lizard Rocks Road
Valley Center, California 92082
(in unincorporated area of San Diego County)**

Prepared For:

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A handwritten signature in blue ink, appearing to read "Gregory R. Cooke".

Gregory R. Cooke

RCE 39478

Exp. 12-31-21

Prepared on: May 27, 2021

DRC Project No. 19-132

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SECTION I

Introduction

A drainage study has been prepared for the GS Valley Expansion project in Valley Center, an unincorporated area of San Diego County. The project site is located on 28435 Lizard Rocks Road on the east side of the end of Lizard Rocks Road. The drainage area associated with this project (Study Area) is approximately 17.81 acres and the disturbed area associated with grading on this project is approximately 2.16 acres.

SECTION II

Discussion

The project site consists of the north lot, which is currently occupied by several residential structures and undeveloped areas, and the south lot, which is an existing industrial development currently occupied by eight (8) storage facilities and one (1) office building, paved drive aisles, vegetated slopes along the westerly and easterly property lines, and an above-ground stormwater detention basin in the southwesterly corner of the site. The proposed improvements include the demolition of the residential structures, the construction of a storage facility, paved drive aisles, concrete sidewalk, landscaped areas and slopes, concrete curb and gutter, underground storm drain system, and replacing the existing above-ground stormwater detention basin with an underground system.

In the existing condition, the project site can be divided into five (5) Drainage Management Areas: DMA-A, DMA-B, DMA-C, DMA-D, and DMA-E. Stormwater runoff generated from the north lot and approximately 3.95 acres of offsite area east of the site drains southwesterly overland into Lizard Rocks Road. Stormwater runoff generated from the north lot and the 3.95-acre offsite area is intercepted by the bioretention swale and drainage inlets near the southwest corner of the north lot. In the south lot, the area has been developed with two (2) separate stormwater mitigation features. Run-on from approximately 7.86 acres of offsite area east of the site drains westerly onto the lot into a concrete ditch that diverts run-on from approximately 2.11 acres of offsite area northerly into the site and run-on from the other 5.75-acre area southerly offsite. The offsite run-on, outflow from the existing detention basin, and the on-site sloped area along the southerly property line all flow in an existing concrete channel along the southerly property line and discharge through existing wall opening into Cole Grade Road approximately 250 feet west of the site.

- DMA-A – A 6.03-acre drainage area consisted of 2.08 acres of mostly undeveloped land that covers the entire north lot and a portion of Lizard Rocks Road along project frontage as well as 3.95 acres of mostly vacant land east of the site. DMA-A drains overland from northeast to southwest and is tributary to the bioretention swale and existing drainage inlets on both sides of Lizard Rocks Road near the southwest corner of the north lot. Runoff intercepted by the inlet overflows into Point of Discharge #1 (POC #1), discharges through the existing rip-rap dissipater, and drains into the vacant land across the street from the site per Improvement Plan No. CG 4646. The offsite run-on flows overland with the stormwater runoff generated on-site towards Lizard Rocks Road.

- DMA-B – An area of 0.53 acres of developed land located just south of the north lot that surface drains into existing drainage inlets and is tributary to an existing Storm Chamber system west of the existing buildings. Stormwater drains into existing Storm Chamber system and overflows from an existing discharge basin per Grading Plan No. L-14940. From there, stormwater sheet flows northwesterly into the existing drainage inlet on Lizard Rocks Road per Improvement Plan No. CG 4646. Runoff intercepted by the inlet overflows into POC #1, discharges through the existing rip-rap dissipater, and drains into the vacant land across the street from the site.
- DMA-C – A 5.16-acre drainage area consisted of 2.11 acres of offsite partially vacant land east of the site and 3.05 acres of developed land located in the south lot that surface drains into onsite gutters, onsite drainage inlets, and an existing concrete channel located offsite. All offsite run-ons are intercepted by an existing concrete ditch onsite along the easterly property line that drains the run-on into onsite gutters. Stormwater runoff generated from DMA-C is discharged into an existing above-ground excavated detention basin and then into an existing off-site concrete channel that runs along the southerly property line of the site. Ultimately, the entire DMA drains into POC #2 located in the southwest corner of the site at the offsite concrete channel. All stormwater runoff in the offsite concrete ditch is discharged into Cole Grade Road.
- DMA-D – A 5.97-acre drainage area consisted of 0.22 acres of onsite slope between existing storage facilities and the easterly and southerly property lines as well as 5.75 acres of offsite area east of the site that has a residential structure but is mostly undeveloped. Offsite run-ons are intercepted by an existing concrete ditch onsite along the easterly property line that drains the run-on southerly into the existing concrete channel south of the property and comingles with the stormwater runoff discharged from the DMA-C and onsite portion of DMA-D at POC #2.
- DMA-E – An area of 0.12 acres of pervious slope located mainly along the southwesterly property line that is considered self-mitigating.

In the proposed condition, the project site remains to have five (5) Drainage Management Areas as in the existing condition. A new underground detention system in conjunction with a manhole installed with a weir and orifice for flow control will mitigate stormwater runoff generated from the north lot. The existing Storm Chamber will remain to mitigate stormwater runoff from the area just south of the north lot. In the remaining south lot, an underground detention system is proposed to replace the existing above-ground detention basin. Points of Discharge (POC) remain the same as in the existing condition.

- DMA-A – A 6.10-acre drainage area including the proposed 1.86-acre commercial development located mainly in the north lot that surface drains into proposed drainage inlets connected to the proposed underground storm drain system, 0.27 acres of Lizard Rock Roads along site frontage, and 3.97 acres of offsite area that is mostly vacant. Run-on from the 3.97-acre offsite area will be intercepted by a proposed concrete ditch along the easterly property line that drains into a proposed inlet that connects to the existing inlet on the east side of Lizard Rocks Road via a proposed storm drain pipe. The proposed underground storm drain system discharges the runoff from 1.86-acres of the DMA into the proposed underground detention system sized to address both stormwater quality and hydromodification with a flow-controlled manhole. Drainage pattern within Lizard Rocks remains the same as in the existing condition. Runoff generated from Lizard Rocks Road within the DMA surface drain into the existing bioretention swale and existing drainage inlets on both sides of Lizard Rocks Road near the southwest corner of the north lot.

Runoff intercepted by the inlet overflows into Point of Discharge #1 (POC #1), discharges through the existing rip-rap dissipater, and drains into the vacant land across the street from the site per Improvement Plan No. CG 4646. Within DMA-A, subarea 8A is considered self-mitigating since it's entirely landscaped and drains offsite into Lizard Rocks Road and enters the existing inlets which overflows into POC #1.

A proprietary biofiltration unit is proposed for treatment of the first-flush stormwater runoff from the north lot before discharging into the existing drainage inlet located on the east side of Lizard Rocks Road at the southwest corner of the site. Infiltration is not suitable at the project site due to perched groundwater per the soils study prepared by the project geotechnical engineer.

- DMA-B – An area of 0.40 acres of previously developed commercial land in the south lot located just to the south of DMA-A. This DMA remains undisturbed and the tributary area is reduced in the proposed condition. This area will continue to drain to the existing Storm Chamber system, overflow from the existing discharge basin, sheet flow into the existing drainage inlet on Lizard Rocks Road, and overflows into POC #1.
- DMA-C – A 5.21-acre drainage area consisted of 2.60 acres of previously developed commercial land located in the south lot that remains the same drainage pattern as in the existing condition, 0.50 acres of area that is a combination of previously developed commercial land in the south lot and a sliver of new slope along the easterly property line in the north lot, and 2.11 acres of offsite partially vacant area east of the site. All offsite run-ons are intercepted by an existing concrete ditch onsite along the easterly property line that drains the run-on into onsite gutters. Stormwater runoff generated from DMA-C, except for subarea 9C.1 and 9C.2, are discharged into an underground detention system that replaces the existing excavated detention basin but will continue to discharge into the existing offsite concrete channel along the southerly property line, designated as POC #2. A manhole with a weir structure and an orifice is proposed just downstream of the underground detention system for flow control. Subarea 9C.1 and 9C.2 bypass the underground detention system and drains directly into the storm drain pipe that outlets into the offsite concrete ditch. As in the existing condition, all stormwater runoff in the offsite concrete ditch is discharged into Cole Grade Road.
- DMA-D – A 5.97-acre drainage area that is not disturbed and is consisted of 0.22 acres of onsite slope between existing storage facilities and the easterly and southerly property lines as well as 5.75 acres of offsite area east of the site that has a residential structure but is mostly undeveloped. Offsite run-ons are intercepted by an existing concrete ditch onsite along the easterly property line that drains the run-on southerly into the existing concrete channel south of the property and comingles with the stormwater runoff discharged from the DMA-C and onsite portion of DMA-D at POC #2.
- DMA-E – An area of 0.12 acres of pervious slope located mainly along the southwesterly property line that is not disturbed and is considered self-mitigating.

Existing and proposed drainage patterns for subareas tributary to each POC have been analyzed with Bentley FlowMaster V8i software to determine that the proposed development would not result in substantial erosion or siltation on- or off-site and will be further discussed in Section VI Summary.

The proposed project does not place housing or any other structures within the 100-year flood hazard area as mapped on the Federal Emergency Management Agency (FEMA) National Flood Hazard

Layer Map shown in Appendix F of this drainage study. The site is not within a 100-year flood plain and does not impede, increase, or redirect flood flows.

SECTION III

Methodology

Rational Method

The Rational Method ($Q = C \times i \times A$) outlined in the San Diego County Hydrology Manual (2003) was applied in the calculation of the 10-, 25-, and 100-year storm event peak rate of runoff for each Drainage Management Area (DMA) using the Advanced Engineering Software (AES).

The 6-hour and 24-hour duration precipitation for the 10-, 25-, and 100-year storm frequencies were gathered from the rainfall isopluvial maps in the County Hydrology Manual. Per Figure 3-1, Intensity-Duration Design Chart, shown in Appendix A of this Drainage Study, the 6-hour precipitation for each frequency was adjusted so that the value is within the 45% to 65% range of the 24-hour precipitation value.

For each DMA in the existing and proposed conditions, the Runoff Coefficient, C and slope (%) were determined. Runoff Coefficient C was determined based on the land use for each DMA. Using the calculated Runoff Coefficient C with Table 3-1, the corresponding Soil Group and County Element Land Use was noted for inputting into AES software. Slope (%) for each DMA was determined by dividing the upstream and downstream elevations by the flow path length.

For each DMA in both the existing and proposed conditions for the 10-, 25-, and 100-year storm events, AES software applies the methods described in the County Hydrology Manual to calculate the time of concentration, T_c . AES software determines the initial time of concentration and maximum overland flow length based on Table 3-2 by inputting the subarea slope (%) and the County Element Land Use into the software. AES software calculates the total overland time of flow, or time of concentration for each subarea based on Figure 3-3 on flowpath length, slope (%), and runoff coefficient.

AES software is able to calculate rainfall intensity, i (inches/hour) based on Figure 3-1 of the County Hydrology Manual. From there, AES software calculates the peak flow rate, Q_{10} , Q_{25} , and Q_{100} with the calculated Runoff Coefficient, C ; rainfall intensity, i ; and the inputted acreage, A for each DMA.

Table 3-1
RUNOFF COEFFICIENTS FOR URBAN AREAS

Land Use		Runoff Coefficient "C"				
NRCS Elements	County Elements	% IMPER.	Soil Type			
			A	B	C	D
Undisturbed Natural Terrain (Natural)	Permanent Open Space	0*	0.20	0.25	0.30	0.35
Low Density Residential (LDR)	Residential, 1.0 DU/A or less	10	0.27	0.32	0.36	0.41
Low Density Residential (LDR)	Residential, 2.0 DU/A or less	20	0.34	0.38	0.42	0.46
Low Density Residential (LDR)	Residential, 2.9 DU/A or less	25	0.38	0.41	0.45	0.49
Medium Density Residential (MDR)	Residential, 4.3 DU/A or less	30	0.41	0.45	0.48	0.52
Medium Density Residential (MDR)	Residential, 7.3 DU/A or less	40	0.48	0.51	0.54	0.57
Medium Density Residential (MDR)	Residential, 10.9 DU/A or less	45	0.52	0.54	0.57	0.60
Medium Density Residential (MDR)	Residential, 14.5 DU/A or less	50	0.55	0.58	0.60	0.63
High Density Residential (HDR)	Residential, 24.0 DU/A or less	65	0.66	0.67	0.69	0.71
High Density Residential (HDR)	Residential, 43.0 DU/A or less	80	0.76	0.77	0.78	0.79
Commercial/Industrial (N. Com)	Neighborhood Commercial	80	0.76	0.77	0.78	0.79
Commercial/Industrial (G. Com)	General Commercial	85	0.80	0.80	0.81	0.82
Commercial/Industrial (O.P. Com)	Office Professional/Commercial	90	0.83	0.84	0.84	0.85
Commercial/Industrial (Limited I.)	Limited Industrial	90	0.83	0.84	0.84	0.85
Commercial/Industrial (General I.)	General Industrial	95	0.87	0.87	0.87	0.87

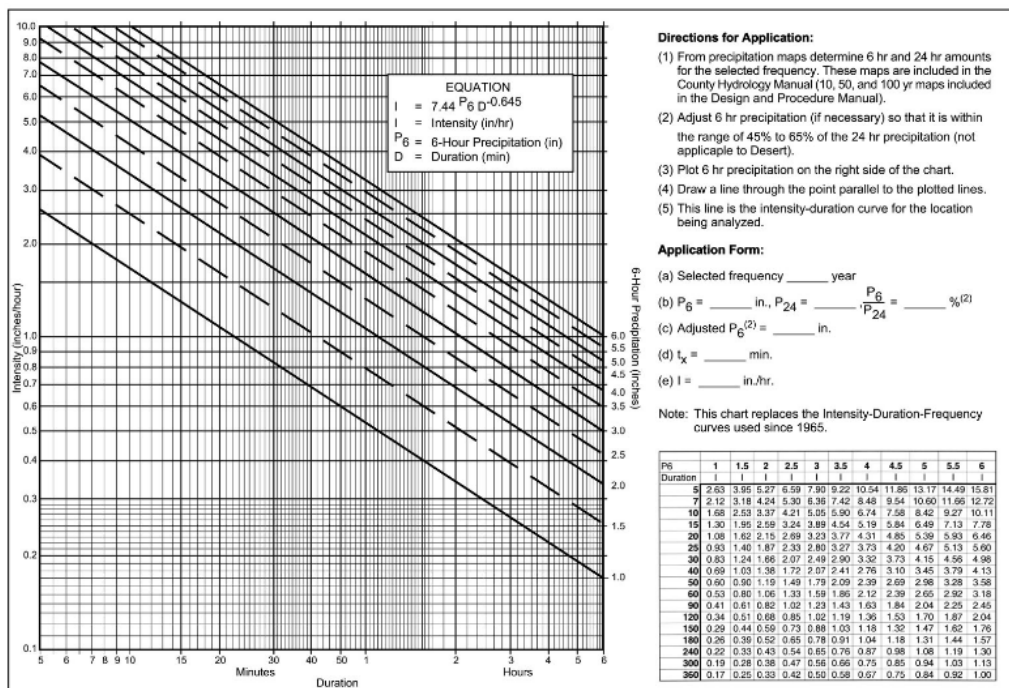
*The values associated with 0% impervious may be used for direct calculation of the runoff coefficient as described in Section 3.1.2 (representing the pervious runoff coefficient, C_p , for the soil type), or for areas that will remain undisturbed in perpetuity. Justification must be given that the area will remain natural forever (e.g., the area is located in Cleveland National Forest).

DU/A = dwelling units per acre

NRCS = National Resources Conservation Service

Table 3-2
MAXIMUM OVERLAND FLOW LENGTH (L_M)
& INITIAL TIME OF CONCENTRATION (T_i)

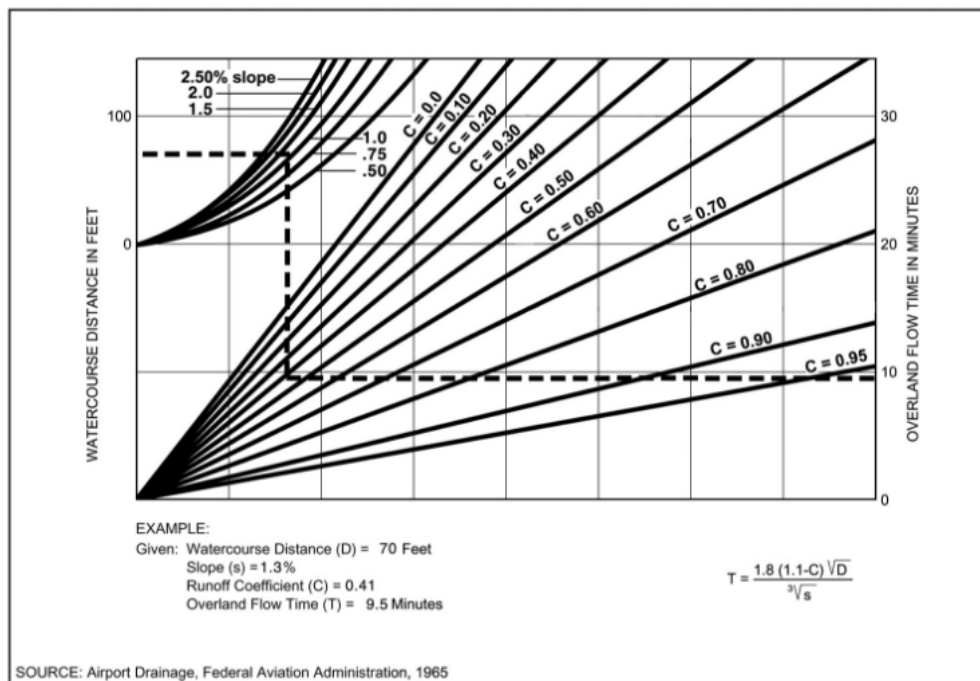
Element*	DU/ Acre	.5%		1%		2%		3%		5%		10%	
		L_M	T_i	L_M	T_i	L_M	T_i	L_M	T_i	L_M	T_i	L_M	T_i
Natural		50	13.2	70	12.5	85	10.9	100	10.3	100	8.7	100	6.9
LDR	1	50	12.2	70	11.5	85	10.0	100	9.5	100	8.0	100	6.4
LDR	2	50	11.3	70	10.5	85	9.2	100	8.8	100	7.4	100	5.8
LDR	2.9	50	10.7	70	10.0	85	8.8	95	8.1	100	7.0	100	5.6
MDR	4.3	50	10.2	70	9.6	80	8.1	95	7.8	100	6.7	100	5.3
MDR	7.3	50	9.2	65	8.4	80	7.4	95	7.0	100	6.0	100	4.8
MDR	10.9	50	8.7	65	7.9	80	6.9	90	6.4	100	5.7	100	4.5
MDR	14.5	50	8.2	65	7.4	80	6.5	90	6.0	100	5.4	100	4.3
HDR	24	50	6.7	65	6.1	75	5.1	90	4.9	95	4.3	100	3.5
HDR	43	50	5.3	65	4.7	75	4.0	85	3.8	95	3.4	100	2.7
N. Com		50	5.3	60	4.5	75	4.0	85	3.8	95	3.4	100	2.7
G. Com		50	4.7	60	4.1	75	3.6	85	3.4	90	2.9	100	2.4
O.P./Com		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
Limited I.		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
General I.		50	3.7	60	3.2	70	2.7	80	2.6	90	2.3	100	1.9



Intensity-Duration Design Chart - Template

FIGURE

3-1



Rational Formula - Overland Time of Flow Nomograph

FIGURE

3-3

Hydromodification

The stormwater runoff for the 100-year storm event generated from DMA-A and DMA-C for the existing and proposed conditions were calculated by AES software. The drainage subareas tributary to the two proposed underground detention systems and the runoff generated from those areas were determined since only a portion of the DMA drains into the underground detention system in the proposed condition. Hydrographs were generated with Rick Engineering Company's Rational Method Hydrograph software (RickRatHydro) by inputting the time of concentration values calculated with AES software, the adjusted 6-hour precipitation values calculated per Figure 3-1, the tributary DMA area in acres, the Rational Method Runoff Coefficient as previously determined, and 100-year peak discharge (cfs) calculated with AES software determined earlier.

The "pre-development (existing)" and "mitigated" scenarios for DMA-A and DMA-C for the 100-year storm event were analyzed by using hydrograph results generated from RickRatHydro in conjunction with Civil3D Hydraflow Hydrograph software.

The 100-year storm event hydrograph results for the existing and proposed conditions that were generated from RickRatHydro were inputted into Civil3D Hydraflow Hydrograph software. From there, a "pond" was created in Hydraflow software to simulate the proposed detention system with a weir and an orifice that control the outflow. Next, a "reservoir" was created to represent the "mitigated" scenario for both DMA-A and DMA-C. The results from RickRatHydro were used as the "inflow hydrograph". The size of the weir and orifice in the "pond" were adjusted until the discharge from the detention system is less than the allowable runoff in the "pre-development (existing)" condition.

The allowable runoff for the detention system (System #1) in DMA-A is calculated with steps below:

- 1) Determine total runoff not tributary to System #1 from result of AES software (stormwater runoff generated from drainage subareas that bypass System #1: 1A.1 & 1A.2, 8A, 9A.1 & 9A.2, and 10A.1 & 10A.2)
- 2) Determine total runoff generated from DMA-A in the pre-development (existing) condition from result of AES software
- 3) Allowable runoff = Runoff determined in Step 2 – Runoff calculated in Step 1

The allowable runoff for the detention system (System #2) in DMA-C is calculated with steps below:

- 1) Determine total runoff not tributary to System #2 from result of AES software (stormwater runoff generated from drainage subareas that bypass System #2: 9C.1 & 9C.2)
- 2) Determine total runoff generated from DMA-C in the pre-development (existing) condition from result of AES software
- 3) Allowable runoff = Runoff determined in Step 2 – Runoff calculated in Step 1

Point of Discharge (POC) Erosion Analysis

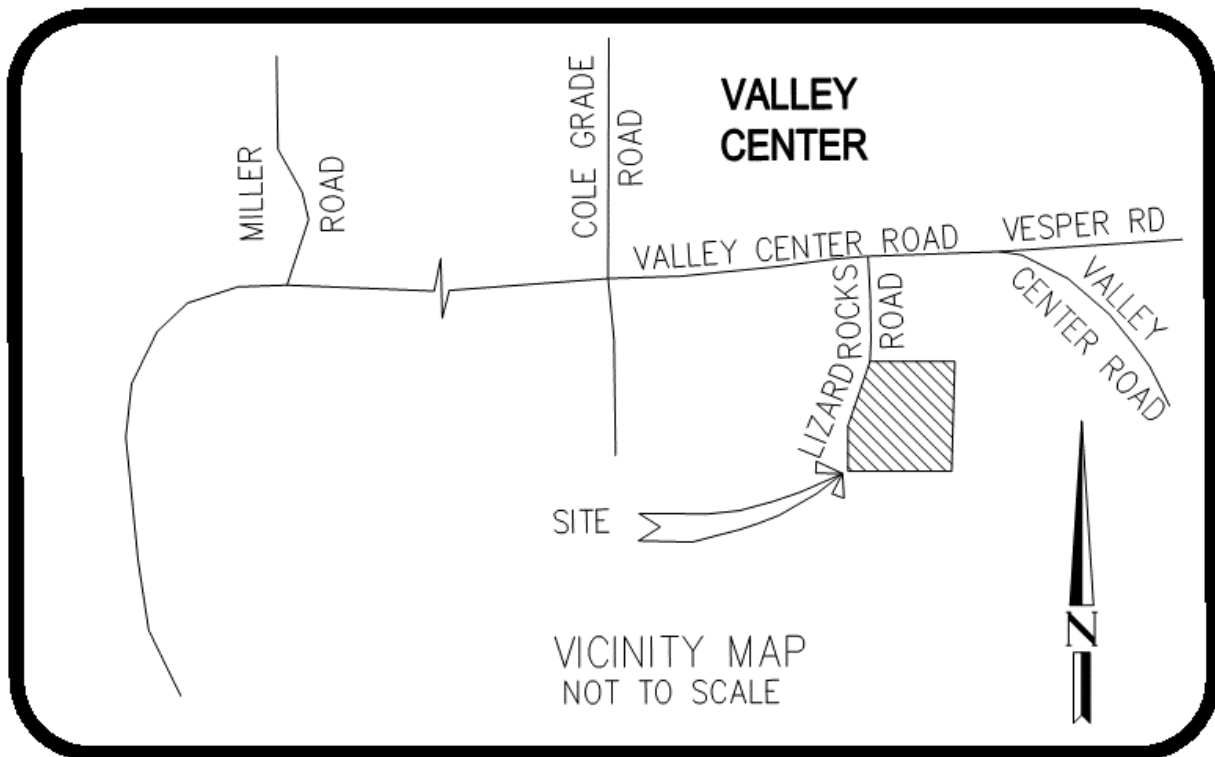
The drainage patterns for each DMA have been analyzed with Bentley FlowMaster V8i software to determine that each POC would not result in substantial erosion or siltation. The results are described in the Summary of this Drainage Study. Calculations are included as Appendix D in this drainage study.

Pipe Size Analysis

The AES software computes the size of the proposed storm drain system. Calculations are included as Appendix D in this drainage study.

SECTION IV

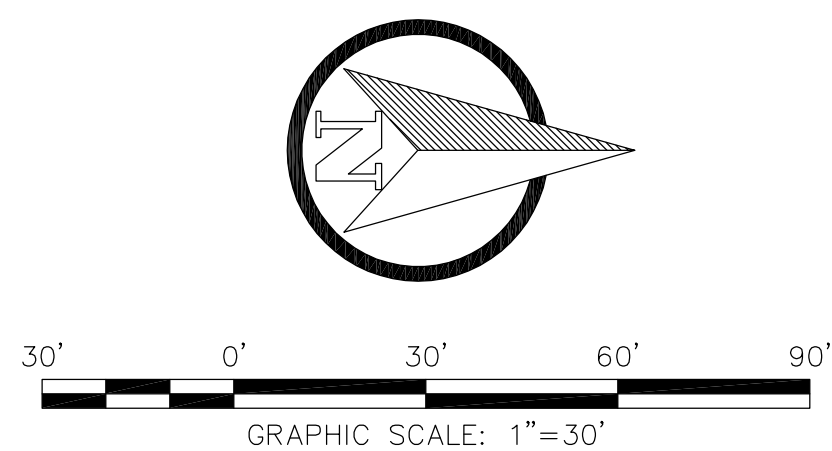
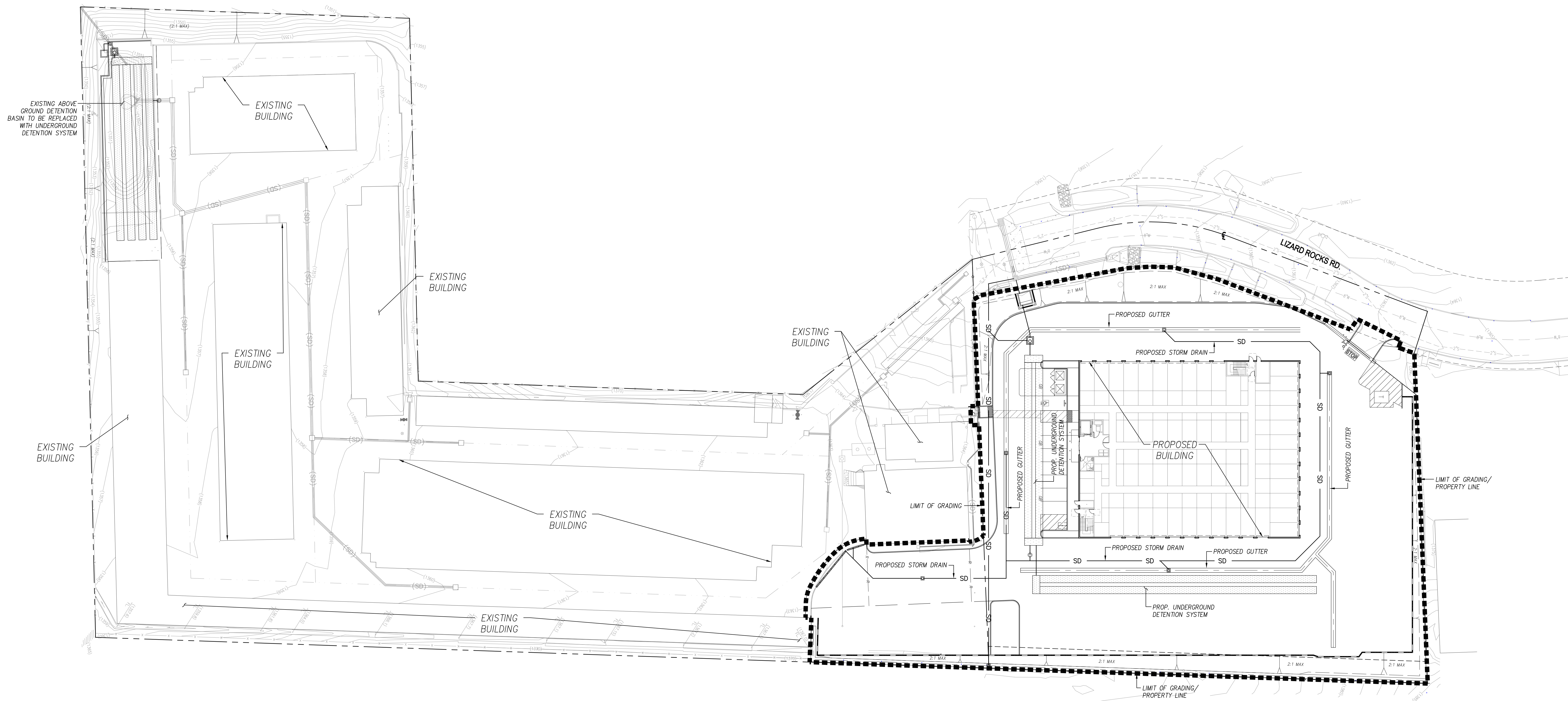
Vicinity Map



SECTION V

Site Map

A site map showing features of the project site is included on the next page.



GS VALLEY EXPANSION
28435 LIZARD ROCKS ROAD
VALLEY CENTER, CALIFORNIA
SITE MAP

PROJECT:	HYDROLOGY
DATE:	5/27/2021
CHECKED: JH	DRAWN: JH
DRAWING FILE:	19132SP
PROJECT NO.:	19-132
SHEET NUMBER:	1
OF	1 SHEETS
SCALE:	AS SHOWN

NO.:	REVISION:	DATE:

ORC Engineering, Inc.
Civil Engineering/Land Surveying/Land Planning

180 S. Old Springs Road
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Anheim Hills, CA 92808
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SECTION VI

Summary

Rational Method

From Figure 3-1 of the County Hydrology Manual, the following adjusted 6-hour precipitation was determined for each storm event:

- 10-year storm event = 2.7 in.
- 25-year storm event = 3.0 in.
- 100-year storm event = 4.5 in.

For each drainage management area (DMA), the runoff coefficients C, and slope (%) were determined. Although the site is classified as "limited industrial", the Runoff Coefficient, C was determined based on the percent impervious area calculated for each DMA. Based on the soil group of each DMA, the corresponding Land Use from Table 3-1 was noted for inputting into AES software.

Runoff Coefficients: Existing Condition

Subarea	Land Use Input for AES	C	Soil Group
1A.1	Natural Desert Landscaping	0.30	C
1A.2	Residential (2 DU/AC) or less	0.42	C
2A.1	Natural Desert Landscaping	0.30	C
2A.2	Natural Desert Landscaping	0.30	C
2A.3	Residential (1 DU/AC) or less	0.36	C
3A.1	Natural Desert Landscaping	0.30	C
3A.2	Residential (2 DU/AC) or less	0.42	C
1B	Residential (2.9 DU/AC) or less	0.45	C
2B	Limited Industrial	0.84	C
3B	Limited Industrial	0.84	C
4B	Limited Industrial	0.84	C
5B	Limited Industrial	0.84	C
6B	Limited Industrial	0.84	C
7B.1	Limited Industrial	0.84	C
7B.2	Limited Industrial	0.84	C
1C.1	Natural Desert Landscaping	0.30	C
1C.2	Residential (2 DU/AC) or less	0.42	C
1C.3	Limited Industrial	0.83	A
2C	Limited Industrial	0.84	C
3C.1	Artificial Desert Landscaping	0.87	C
3C.2	Limited Industrial	0.84	C
4C.1	Artificial Desert Landscaping	0.87	C
4C.2	Artificial Desert Landscaping	0.87	C
5C	Limited Industrial	0.84	C
6C.1	Limited Industrial	0.84	C
6C.2	Limited Industrial	0.84	C
7C	Limited Industrial	0.84	C
8C	Limited Industrial	0.83	A

Subarea	Land Use Input for AES	C	Soil Group
9C	Residential (2 DU/AC) or less	0.34	A
1D.1	Natural Desert Landscaping	0.30	C
1D.2	Residential (1 DU/AC) or less	0.27	A
1D.3	Residential (4.3 DU/AC) or less	0.48	C
2D	Natural Desert Landscaping	0.30	C
1E	Natural Desert Landscaping	0.20	A

Runoff Coefficients: Proposed Condition

Subarea	Land Use Input for AES	C	Soil Group
1A.1	Paved road with ditch	0.69	C
1A.2	Paved road with ditch	0.69	C
2A.1	Limited Industrial	0.84	C
2A.2	Limited Industrial	0.84	C
3A.1	Limited Industrial	0.84	C
3A.2	Limited Industrial	0.84	C
4A.1	Limited Industrial	0.84	C
4A.2	Limited Industrial	0.84	C
5A	Limited Industrial	0.84	C
6A	Limited Industrial	0.84	C
7A.1	Limited Industrial	0.84	C
7A.2	Limited Industrial	0.84	C
8A	Urban Newly Graded Areas	0.69	C
9A.1	Natural Desert Landscaping	0.30	C
9A.2	Natural Desert Landscaping	0.30	C
10A.1	Paved road with ditch	0.69	C
10A.2	Paved road with ditch	0.69	C
1B	Limited Industrial	0.84	C
2B	Limited Industrial	0.84	C
3B	Limited Industrial	0.84	C
4B	Limited Industrial	0.84	C
5B	Limited Industrial	0.84	C
6B	Limited Industrial	0.84	C
7B.1	Limited Industrial	0.84	C
7B.2	Limited Industrial	0.84	C
1C.1	Natural Desert Landscaping	0.30	C
1C.2	Residential (2DU/AC) or less	0.42	C
1C.3	Limited Industrial	0.83	A
2C	Limited Industrial	0.84	C
3C.1	Artificial Desert Landscaping	0.87	C
3C.2	Limited Industrial	0.84	C
4C.1	Artificial Desert Landscaping	0.87	C
4C.2	Artificial Desert Landscaping	0.87	C
5C	Residential (1 DU/AC) or less	0.84	C
6C.1	Limited Industrial	0.84	C
6C.2	Limited Industrial	0.84	C
7C	Residential (1 DU/AC) or less	0.84	C

Subarea	Land Use Input for AES	C	Soil Group
8C	Residential (1 DU/AC) or less	0.83	A
9C.1	Limited Industrial	0.83	A
9C.2	Limited Industrial	0.83	A
1D.1	Residential (4.3 DU/AC) or less	0.30	C
1D.2	Residential (1 DU/AC) or less	0.27	A
1D.3	Residential (4.3 DU/AC) or less	0.48	C
2D	Natural Desert Landscaping	0.30	C
1E	Natural Desert Landscaping	0.20	A

The Rational Method ($Q = C \times i \times A$) was applied with AES software to calculate the peak flow rates, Q_{10} , Q_{25} , and Q_{100} . For each DMA, the designated land use inputs listed in the tables on the previous page were inputted in order to model the correct Runoff Coefficient, C. For each DMA, the Soil Classification, total Area, A, flowpath lengths, upstream elevations, and downstream elevations were input.

AES calculates the rainfall intensity and time of concentration based on the methods described in the County Hydrology Manual.

Summary of the hydrologic analyses is tabulated on the following pages.

Existing Condition													
DMA	Tributary Area (acres)	Runoff Coefficient, C	Flowpath, D (ft)	Slope, s (%)	10-Year Storm			25-Year Storm			100-Year Storm		
					Q ₁₀	Tc ₁₀	i ₁₀	Q ₂₅	Tc ₂₅	i ₂₅	Q ₁₀₀	Tc ₁₀₀	i ₁₀₀
					cfs	min.	in/hr	cfs	min.	in/hr	cfs	min.	in/hr
1A.1	0.13	0.30	100	13.2	0.23	6.7	5.9	0.26	6.7	6.6	0.38	6.7	9.8
1A.2	0.73	0.42	305	5.1	1.18	13.0	3.8	1.36	12.2	4.4	2.14	11.4	7.0
2A.1	0.40	0.30	100	5.0	0.61	8.4	5.1	0.68	8.4	5.6	1.02	8.4	8.5
2A.2	3.55	0.30	465	5.9	3.81	14.5	3.6	4.30	14.2	4.0	6.63	13.6	6.2
2A.3	0.41	0.36	278	5.2	0.49	16.5	3.3	0.55	16.0	3.7	0.86	15.2	5.8
3A.1	0.17	0.30	100	11.2	0.30	6.7	5.9	0.33	6.7	6.6	0.50	6.7	9.8
3A.2	0.65	0.42	294	4.4	1.15	11.3	4.2	1.24	11.8	4.5	1.98	10.7	7.2
"A" Subtotal	6.03	0.33	--	--	6.60	16.6	--	7.48	16.1	--	11.63	15.3	--
1B	0.15	0.45	95	5.7	0.41	6.4	6.1	0.46	6.4	6.7	0.68	6.4	10.1
2B	0.05	0.84	--	--	0.24	5.0	5.8	0.27	5.0	6.4	0.41	5.0	9.7
3B	0.04	0.84	89	6.9	0.24	5.0	7.1	0.27	5.0	7.9	0.40	5.0	11.9
4B	0.14	0.84	60	10.3	0.84	5.0	7.1	0.93	5.0	7.9	1.39	5.0	11.9
5B	0.06	0.84	54	4.3	0.36	5.0	7.1	0.40	5.0	7.9	0.60	5.0	11.9
6B	0.00	0.84	--	--	0.03	5.0	7.1	0.03	5.0	7.9	0.05	5.0	11.9
7B.1	0.08	0.84	80	3.1	0.48	5.0	7.1	0.53	5.0	7.9	0.80	5.0	11.9
7B.2	0.02	0.84	36	3.6	0.12	5.0	7.1	0.13	5.0	7.9	0.20	5.0	11.9
"B" Subtotal	0.53	0.73	--	--	2.64	5.0	--	2.93	5.0	--	4.41	5.0	--
1C.1	0.31	0.30	100	4.0	0.45	9.1	4.8	0.50	9.1	5.4	0.75	9.1	8.1
1C.2	1.80	0.42	380	7.1	2.70	14.5	3.6	3.03	14.3	4.0	4.75	13.4	6.3
1C.3	0.44	0.83	371	2.7	1.21	16.3	3.3	1.36	16.0	3.7	7.09	15.0	5.8
2C	0.59	0.84	--	--	1.62	5.0	3.3	1.82	5.0	3.7	2.85	5.0	5.7
3C.1	0.04	0.87	90	5.1	0.25	5.0	7.1	0.28	5.0	7.9	0.41	5.0	11.9
3C.2	0.32	0.84	297	2.0	1.91	5.0	7.1	2.12	5.0	7.9	3.19	5.0	11.9
4C.1	0.03	0.87	60	8.5	0.19	5.0	7.1	0.21	5.0	7.9	0.31	5.0	11.9
4C.2	0.01	0.87	70	9.0	0.06	5.0	7.1	0.07	5.0	7.9	0.10	5.0	11.9
5C	0.44	0.84	--	--	1.19	5.0	3.2	1.34	5.0	3.6	2.09	5.0	5.7
6C.1	0.11	0.84	53	1.1	0.66	5.0	7.1	0.73	5.0	7.9	1.10	5.0	11.9
6C.2	0.35	0.84	188	1.1	2.09	5.0	7.1	2.32	5.0	7.9	3.49	5.0	11.9
7C	0.25	0.84	--	--	1.44	5.3	6.8	1.63	5.2	7.7	2.49	5.0	11.9
8C	0.33	0.83	--	--	0.87	5.0	3.2	0.98	5.0	3.6	1.53	5.0	5.6
9C	0.13	0.34	--	--	0.14	5.0	3.2	0.16	5.0	3.6	0.25	5.0	5.6
"C" Subtotal	5.16	0.65	--	--	10.56	17.5	--	11.85	17.2	--	18.58	16.1	--
1D.1	0.57	0.30	100	23.0	1.01	6.7	5.9	1.12	6.7	6.6	1.68	6.7	9.8
1D.2	5.18	0.27	651	9.4	4.91	15.0	3.5	5.70	14.0	4.1	9.07	12.8	6.5
1D.3	0.10	0.48	181	5.6	0.17	15.2	3.5	0.19	14.2	4.0	0.31	13.0	6.4
2D	0.12	0.30	22	4.5	0.26	5.0	7.1	0.28	5.0	7.9	0.43	5.0	11.9
"D" Subtotal	5.97	0.28	--	--	5.74	15.4	--	6.66	14.4	--	10.58	13.1	--
1E	0.12	0.20	86	8.0	0.13	7.5	5.5	0.15	7.5	6.1	0.22	7.5	9.1
"E" Subtotal	0.12	0.20	--	--	0.13	7.5	--	0.15	7.5	--	0.22	7.5	---
Study Total	17.81	0.42	--	--	25.67	--	--	29.07	--	--	45.42	--	--

Proposed Condition													
DMA	Tributary Area (acres)	Runoff Coefficient, C	Flowpath, D (ft)	Slope, s (%)	10-Year Storm			25-Year Storm			100-Year Storm		
					Q ₁₀	Tc ₁₀	i ₁₀	Q ₂₅	Tc ₂₅	i ₂₅	Q ₁₀₀	Tc ₁₀₀	i ₁₀₀
					cfs	min.	in/hr	cfs	min.	in/hr	cfs	min.	in/hr
1A.1	0.03	0.69	69.00	2.8	0.15	5.0	7.1	0.16	5.0	7.9	0.25	5.0	11.9
1A.2	0.12	0.69	162.00	3.9	0.41	8.8	5.0	0.47	8.2	5.7	0.71	8.3	8.5
2A.1	0.03	0.84	49.00	1.0	0.18	5.0	7.1	0.20	5.0	7.9	0.30	5.0	11.9
2A.2	0.08	0.84	33.00	0.6	0.48	5.0	7.1	0.53	5.0	7.9	0.80	5.0	11.9
3A.1	0.08	0.84	60.00	2.0	0.48	5.0	7.1	0.53	5.0	7.9	0.80	5.0	11.9
3A.2	0.44	0.84	90.00	0.6	2.63	5.0	7.1	2.92	5.0	7.9	4.38	5.0	11.9
4A.1	0.02	0.84	60.00	1.5	0.12	5.0	7.1	0.13	5.0	7.9	0.20	5.0	11.9
4A.2	0.47	0.84	168.00	0.5	2.79	5.0	7.1	3.12	5.0	7.9	4.68	5.0	11.9
5A	0.03	0.84	47.00	2.8	0.18	5.0	7.1	0.20	5.0	7.9	0.30	5.0	11.9
6A	0.12	0.84	64.00	1.6	0.72	5.0	7.1	0.80	5.0	7.9	1.20	5.0	11.9
7A.1	0.04	0.84	60.00	2.0	0.24	5.0	7.1	0.27	5.0	7.9	0.40	5.0	11.9
7A.2	0.48	0.84	83.00	1.2	3.11	5.0	7.1	3.19	5.0	7.9	4.78	5.0	11.9
8A	0.08	0.69	81.00	6.9	0.39	5.0	7.1	0.44	5.0	7.9	0.65	5.0	11.9
9A.1	0.40	0.30	100.00	5.0	0.61	8.4	5.1	0.68	8.4	5.6	1.02	8.4	8.5
9A.2	3.57	0.30	550.00	5.5	3.62	15.9	3.4	4.10	15.4	3.8	6.34	14.7	5.9
10A.1	0.03	0.69	64.00	2.2	0.15	5.0	7.1	0.16	5.0	7.9	0.25	5.0	11.9
10A.2	0.09	0.69	71.00	2.7	0.40	5.8	6.5	0.45	5.7	7.3	0.68	5.6	11.0
"A" Subtotal	6.10	0.48	--	--	15.35	5.0	--	17.18	5.0	--	26.27	5.0	--
1B	0.05	0.84	36.00	1.1	0.30	5.0	7.1	0.33	5.0	7.9	0.50	5.0	11.9
2B	0.05	0.84	--	--	0.30	5.0	7.1	0.33	5.0	7.9	0.50	5.0	11.9
3B	0.04	0.84	89.00	6.9	0.24	5.0	7.1	0.27	5.0	7.9	0.40	5.0	11.9
4B	0.11	0.84	49.00	1.6	0.66	5.0	7.1	0.73	5.0	7.9	1.10	5.0	11.9
5B	0.06	0.84	54.00	4.3	0.36	5.0	7.1	0.40	5.0	7.9	0.60	5.0	11.9
6B	0.00	0.84	--	--	0.03	5.0	7.1	0.03	5.0	7.9	0.05	5.0	11.9
7B.1	0.08	0.84	80.00	3.1	0.48	5.0	7.1	0.53	5.0	7.9	0.80	5.0	11.9
7B.2	0.02	0.84	36.00	3.6	0.12	5.0	7.1	0.13	5.0	7.9	0.20	5.0	11.9
"B" Subtotal	0.40	0.84	--	--	2.49	5.0	--	2.75	5.0	--	4.13	5.0	--
1C.1	0.31	0.30	100.00	4.0	0.45	9.1	4.8	0.50	9.1	5.4	0.75	9.1	8.1
1C.2	1.80	0.42	380.00	7.1	2.70	14.5	3.6	3.03	14.3	4.0	4.75	13.4	6.3
1C.3	0.50	0.83	385.00	2.6	1.37	16.4	3.3	1.54	16.1	3.7	2.42	15.1	5.8
2C	0.59	0.84	--	--	1.61	5.0	3.3	1.81	5.0	3.7	2.84	5.0	5.7
3C.1	0.04	0.87	90.00	5.1	0.25	5.0	7.1	0.28	5.0	7.9	0.41	5.0	11.9
3C.2	0.32	0.84	297.00	2.0	1.91	5.0	7.1	2.12	5.0	7.9	3.19	5.0	11.9
4C.1	0.03	0.87	60.00	8.5	0.19	5.0	7.1	0.21	5.0	7.9	0.31	5.0	11.9
4C.2	0.01	0.87	70.00	9.0	0.06	5.0	7.1	0.07	5.0	7.9	0.10	5.0	11.9
5C	0.44	0.84	--	--	1.19	5.0	3.2	1.33	5.0	3.6	2.09	5.0	5.6
6C.1	0.11	0.84	53.00	1.1	0.66	5.0	7.1	0.73	5.0	7.9	1.10	5.0	11.9
6C.2	0.35	0.84	188.00	1.1	2.09	5.0	7.1	2.32	5.0	7.9	3.49	5.0	11.9
7C	0.24	0.84	--	--	1.43	5.0	7.1	1.59	5.0	7.9	2.39	5.0	11.9
8C	0.33	0.83	--	--	0.87	5.0	3.2	0.97	5.0	3.6	1.53	5.0	5.6
9C.1	0.02	0.83	60.00	2.0	0.12	5.0	7.1	0.13	5.0	7.9	0.20	5.0	11.9
9C.2	0.11	0.83	129.00	0.8	0.65	5.0	7.1	0.72	5.0	7.9	1.08	5.0	11.9
"C" Subtotal	5.21	0.41	--	--	11.30	5.0	--	12.68	5.0	--	19.82	5.0	--



1D.1	0.57	0.30	100.00	23.0	1.01	6.7	5.9	1.12	6.7	6.6	1.68	6.7	9.8
1D.2	5.18	0.27	651.00	9.4	4.91	15.0	3.5	5.70	14.0	4.1	9.07	12.8	6.5
1D.3	0.10	0.48	181.00	5.6	0.17	15.2	3.5	0.19	14.2	4.0	0.31	13.0	6.4
2D	0.12	0.30	22.00	4.5	0.26	5.0	7.1	0.28	5.0	7.9	0.43	5.0	11.9
"D" Subtotal	5.97	0.28	--	--	5.74	15.4	--	6.66	14.4	--	10.58	13.1	--
1E	0.12	0.20	86.00	8.0	0.13	7.5	5.5	0.15	7.5	6.1	0.22	7.5	9.1
"E" Subtotal	0.12	0.20	--	--	0.13	7.5	--	0.15	7.5	--	0.22	7.5	--
Site Total	17.81	0.38	--	--	35.01	--	--	39.42	--	--	61.02	--	--

Comparing the runoff discharged from the overall Site Total in the existing and proposed conditions without flow control, the stormwater runoff will be increased by 9.3 cfs in the 10-year storm event, 10.4 cfs in the 25-year storm event and 15.6 cfs in the 100-year storm event. The table below summarizes the change in runoff without mitigation for the overall Site Total.

	Q ₁₀ (cfs)	Q ₂₅ (cfs)	Q ₁₀₀ (cfs)
Existing	25.67	29.07	45.42
Proposed	35.01	39.42	61.02
Difference	9.34	10.35	15.60
Change in %	36.4%	35.6%	34.3%

Since no changes are made to either DMA-D and DMA-E, comparisons in runoff are only made for DMA-A, DMA-B and DMA-C.

	DMA-A		DMA-B		DMA-C	
	Q ₁₀ (cfs)	Q ₂₅ (cfs)	Q ₁₀ (cfs)	Q ₂₅ (cfs)	Q ₁₀ (cfs)	Q ₂₅ (cfs)
Existing	6.60	7.48	2.64	2.93	10.56	11.85
Proposed	15.35	17.18	2.49	2.75	11.30	12.68
Difference	8.75	9.70	-0.15	-0.18	0.74	0.83

When comparing the existing versus proposed 10-year and 25-year runoff results, DMA-A has a significant increase in runoff whereas DMA-B has a slight decrease and DMA-C has a slight increase. Since discharge from DMA-A and DMA-C will be mitigated and all DMAs are tributary to the same downstream drainage system, there will be no increase in runoff from the overall site.

Underground detention systems in DMA-A and DMA-C with controlled outlets are proposed to temporarily detain stormwater runoff onsite and are designed to discharge runoff in the amount less or equal to the existing condition into the downstream drainage system.

Hydromodification/Mitigated Flow Analysis

For the 100-year storm event, Rick Engineering Company Rational Method Hydrograph software was used to generate the 100-year rational method hydrograph data for the existing and proposed Site conditions. The 100-year rational method hydrograph data and the proposed detention system data were

inputted into Civil 3D Hydraflow Hydrographs software to generate the pre-developed and mitigated flows from each underground detention system.

To mitigate the increase in runoff in DMA-A and DMA-C, a manhole with outflow control is proposed immediately downstream of both System #1 and System #2 to keep runoff discharged less than the existing condition. Both underground detention systems are sized to detain runoff generated from a 100-year storm event with discharged runoff less than the allowable Q.

Since the runoff generated from DMA-B does not increase in proposed condition, the existing underground detention system in DMA-B will remain the same.

Below are calculations to determine the allowable runoff for both underground detention systems.

- Allowable Q_{100} to discharge from proposed underground detention system (System #1) in DMA-A:

Areas tributary to System #1: subareas 2A thru 7A

Total runoff entering System #1 $Q_{100\text{-inflow}} = 16.78$ cfs

Other subareas bypass System #1 and discharges directly to POC #1.

Drainage areas bypassing System #1: 1A.1 & 1A.2, 8A, 9A.1 & 9A.2, and 10A.1 & 10A.2

Total runoff bypassing System #1 $Q_{100\text{-bypass}} = 0.88$ cfs + 0.65 cfs + 7.05 cfs + 0.91 cfs = 9.49 cfs

Existing Q_{100} discharged from DMA-A = 11.63 cfs

Allowable runoff to discharge from System #1 $Q_{100\text{-allowable}} = 11.63$ cfs – 9.49 cfs = 2.14 cfs

- Allowable Q_{100} to discharge from proposed underground detention system (System #2) in DMA-C:

Areas tributary to System #2: subareas 1C thru 8C

Total runoff entering System #2 $Q_{100\text{-inflow}} = 18.54$ cfs

Other subareas bypass System #2 and discharges directly to POC #2.

Drainage areas bypassing System #2: 9C.1 & 9C.2

Total runoff bypassing System #2 $Q_{100\text{-bypass}} = 1.28$ cfs

Existing Q_{100} Discharged from DMA-C = 18.58 cfs

Allowable runoff to discharge from System #2 $Q_{100\text{-allowable}} = 18.58$ cfs – 1.28 cfs = 17.30 cfs

The sizes of the outlet control for the two underground detention systems are summarized below:

	Size of Weir Structure	Size of Orifice Structure
System #1	0.67' wide x 0.80' high	3" diameter at bottom of System #1
System #2	1.00' wide x 2.75' high	6" diameter at bottom of System #2

With the outflow control proposed at each system, the results yielded from Hydraflow Hydrographs are shown below and shown in Appendix E.

	System #1 Outflow (cfs)	System #2 Outflow (cfs)
Mitigated	2.12	17.25

Total runoff discharged at each POC is tabulated below. Since the runoff discharged from each POC decreases, the proposed development will not have negative impact to downstream storm drain system or other flood control facilities.

	Pre-development Q ₁₀₀ (cfs)	Post-development Q ₁₀₀ (cfs)	Tributary DMA	Note for Post-development Q ₁₀₀
POC#1	16.04	15.74	A and B	7.05 cfs from 9A.1 & 9A.2 2.44 cfs from 1A.1 & 1A.2, 8A, and 10A.1 & 10A.2 2.12 cfs from System #1 4.13 cfs from DMA-B
POC#2	18.58	18.53	C and D	17.25 cfs from System #2 1.28 cfs from 9C.1 & 9C.2

Point of Discharge (POC) Erosion

Because the stormwater runoff discharged at each POC is reduced, erosion to the existing concrete ditch and dissipater is not expected.

SECTION VII***Declaration of Responsible Charge***

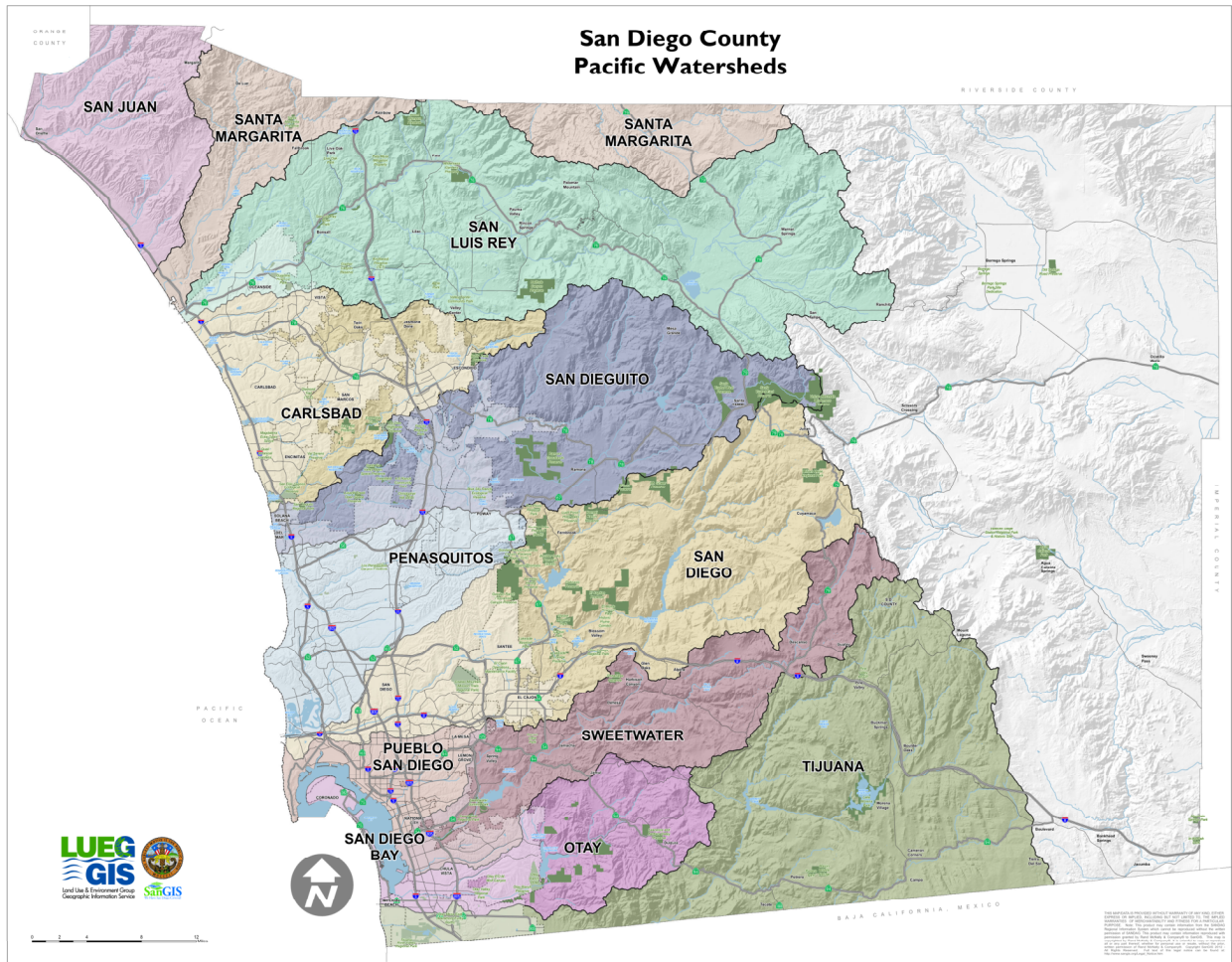
I hereby declare that I am the engineer of work for this project, that I have exercised responsible charge over the design of the project as defined in Section 6703 of the Business and Professions Code, and that the design is consistent with current standards.

I understand that the check of project drawings and specifications by the County of San Diego is confined to a review only and does not relieve me, as engineer of work, of my responsibilities for project design.

5/27/2021

Gregory R. Cooke, R.C.E. 39478

Date

SECTION VIII***Watershed Boundary/Topographic Map***

SECTION IX

Watershed Information

The project site is located within the lower San Luis Rey Hydrologic Area (HA) in the San Luis Rey Watershed.

The San Luis Rey Watershed Management Area (WMA) encompasses 560 square miles and lies in the northern portion of the San Diego County and neighbors Santa Margarita Watershed to the north and Carlsbad and San Dieguito Watersheds to the south. It can be further subdivided into three hydrological areas: Lower San Luis Rey, Monserate, and Warner Valley.

San Luis Rey Watershed supplies area residents with potable water source from Turner Reservoir and Lake Henshaw as well as a number of underground aquifers. The WMA also has six groundwater aquifers: Warner, Pauma, Pala, Bonsall, Moosa, and Mission Basins.

SECTION X

Watershed Geometric Information Map



SECTION XI

Watershed Point Rainfall Isohyetal Maps

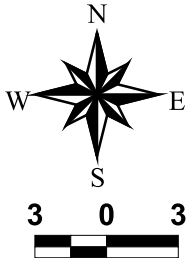
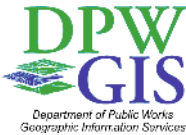
County of San Diego Hydrology Manual



Rainfall Isophluvials

2 Year Rainfall Event - 6 Hours

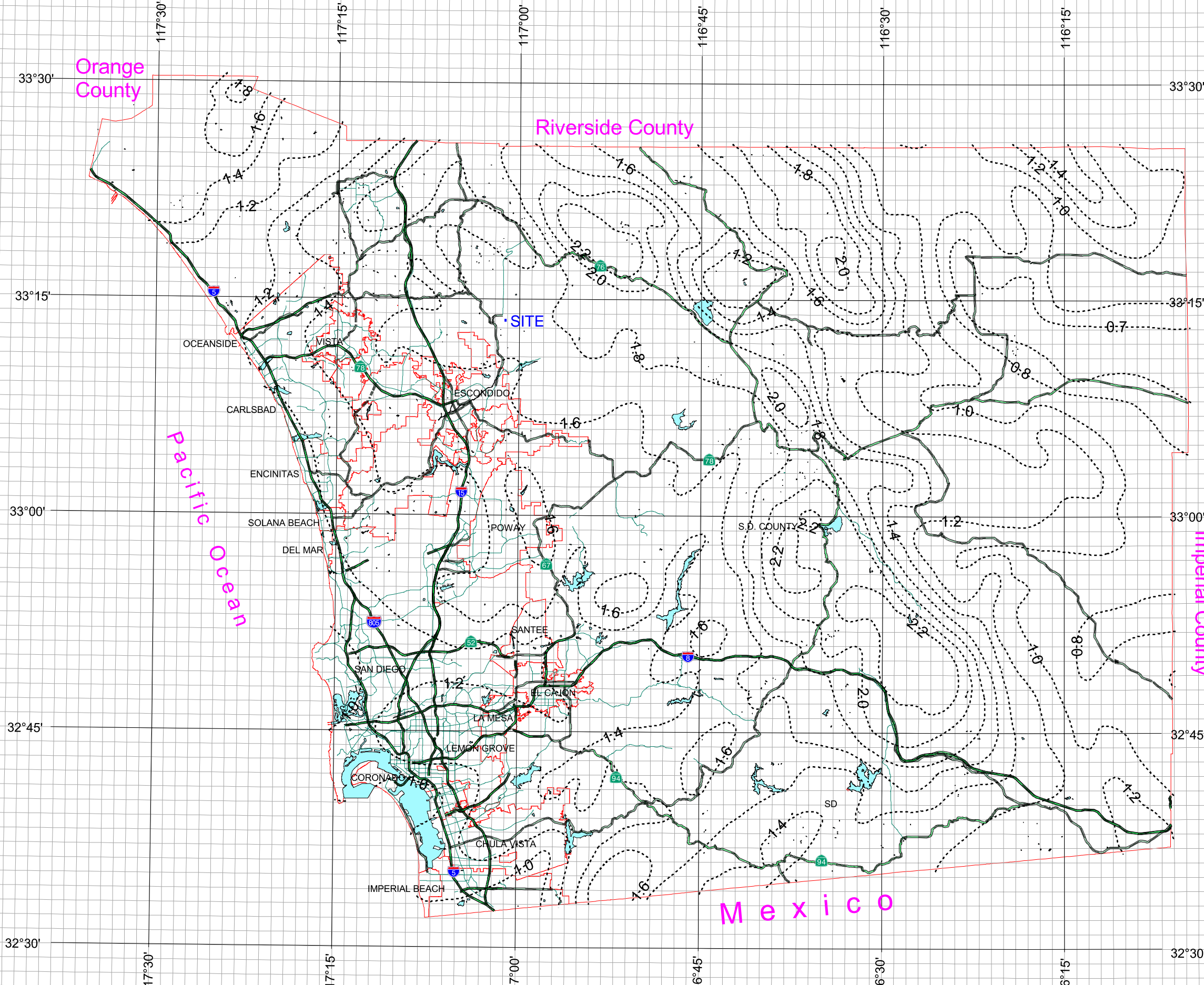
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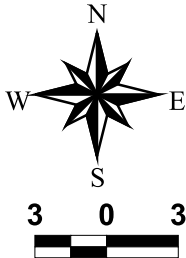
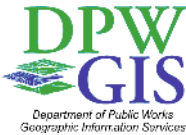
County of San Diego Hydrology Manual



Rainfall Isophuvials

2 Year Rainfall Event - 24 Hours

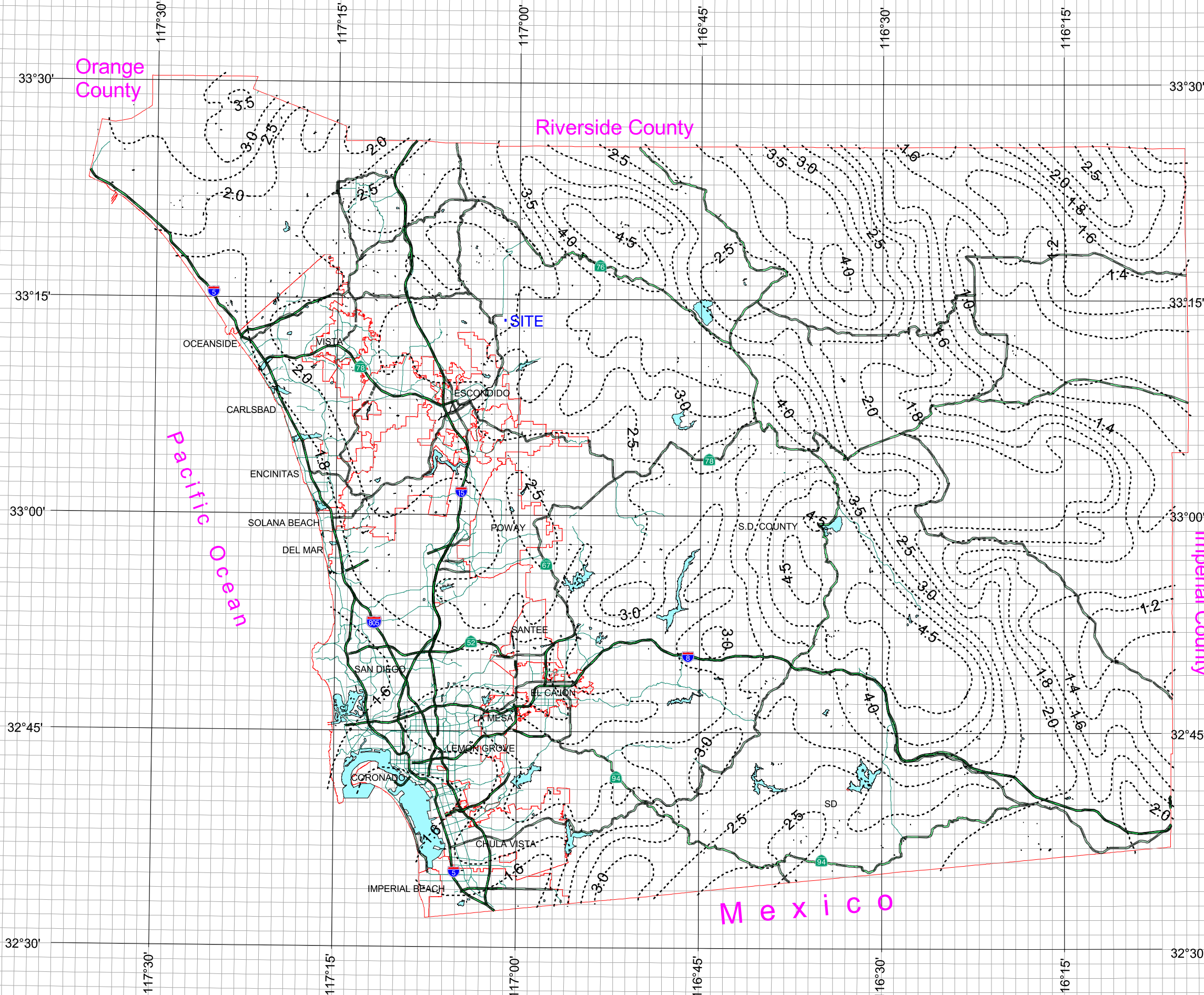
----- Isopluvial (inches)



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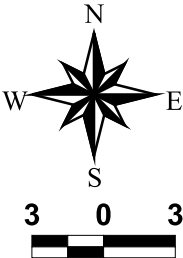
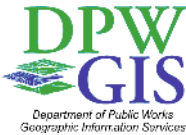
County of San Diego Hydrology Manual



Rainfall Isopluvials

10 Year Rainfall Event - 6 Hours

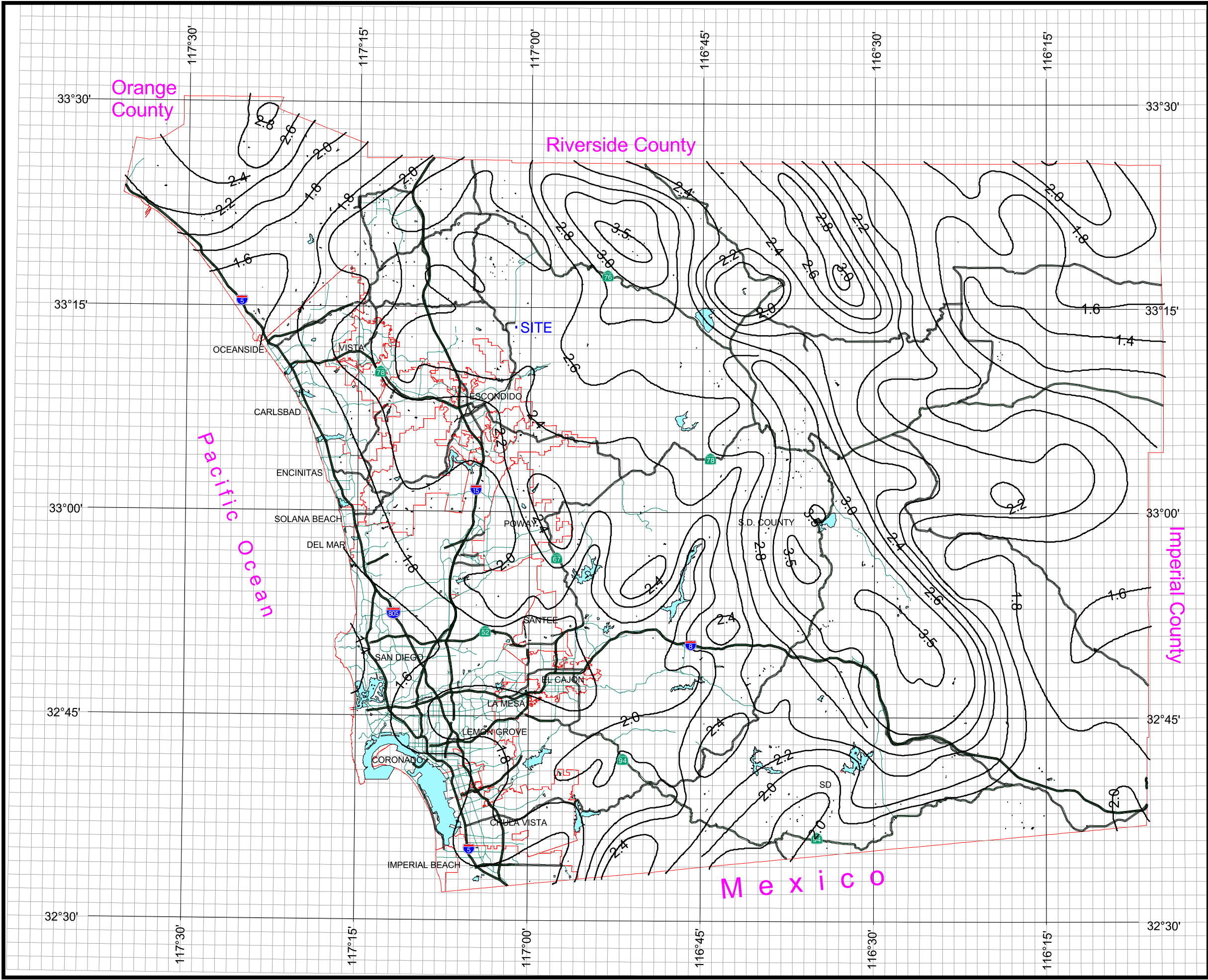
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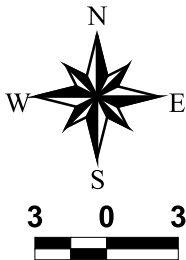
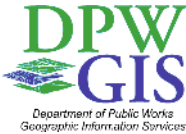
County of San Diego Hydrology Manual



Rainfall Isopluvials

10 Year Rainfall Event - 24 Hours

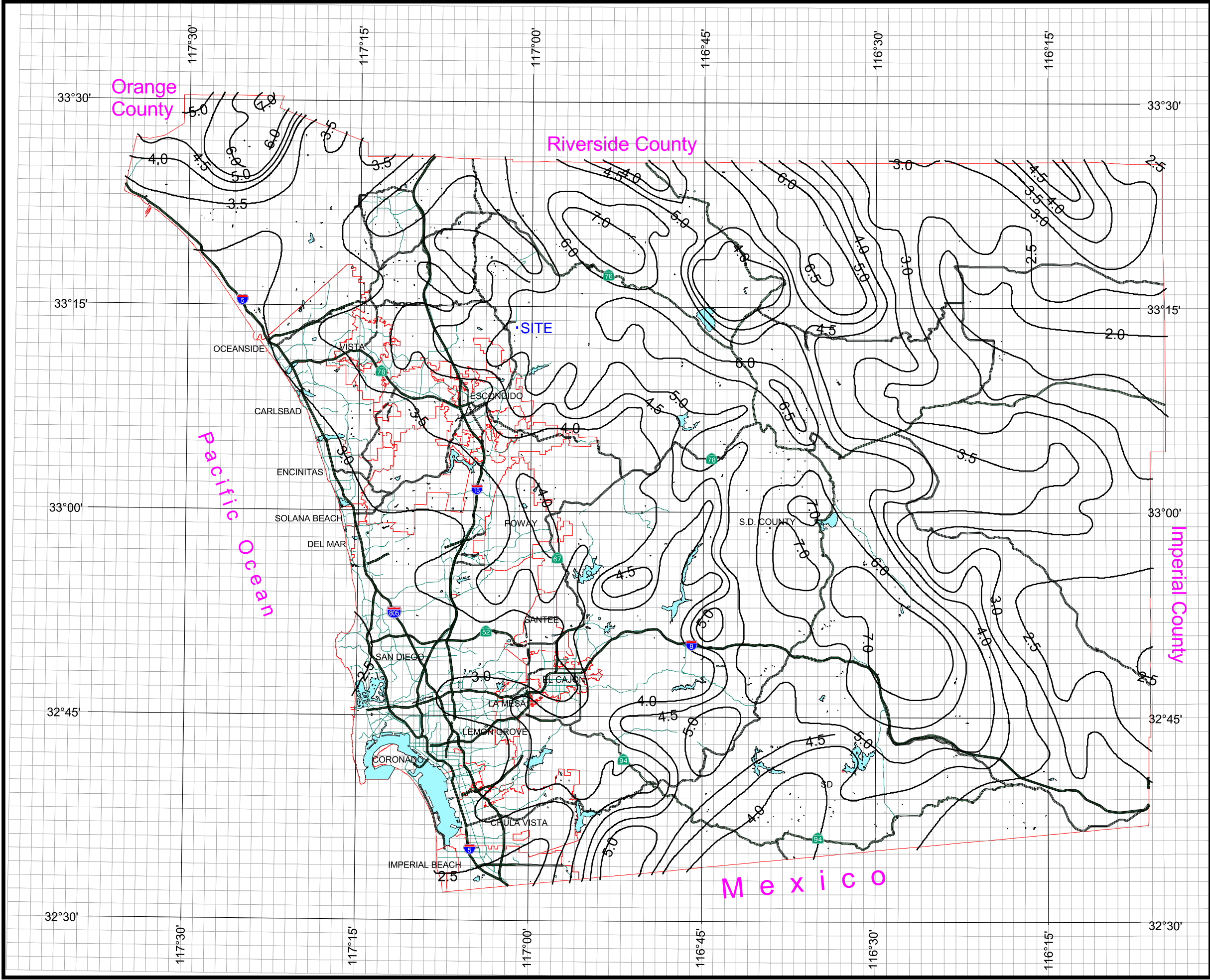
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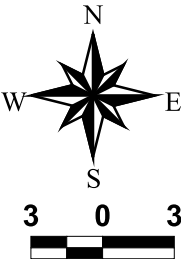
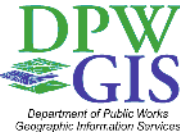
County of San Diego Hydrology Manual



Rainfall Isophuvials

25 Year Rainfall Event - 6 Hours

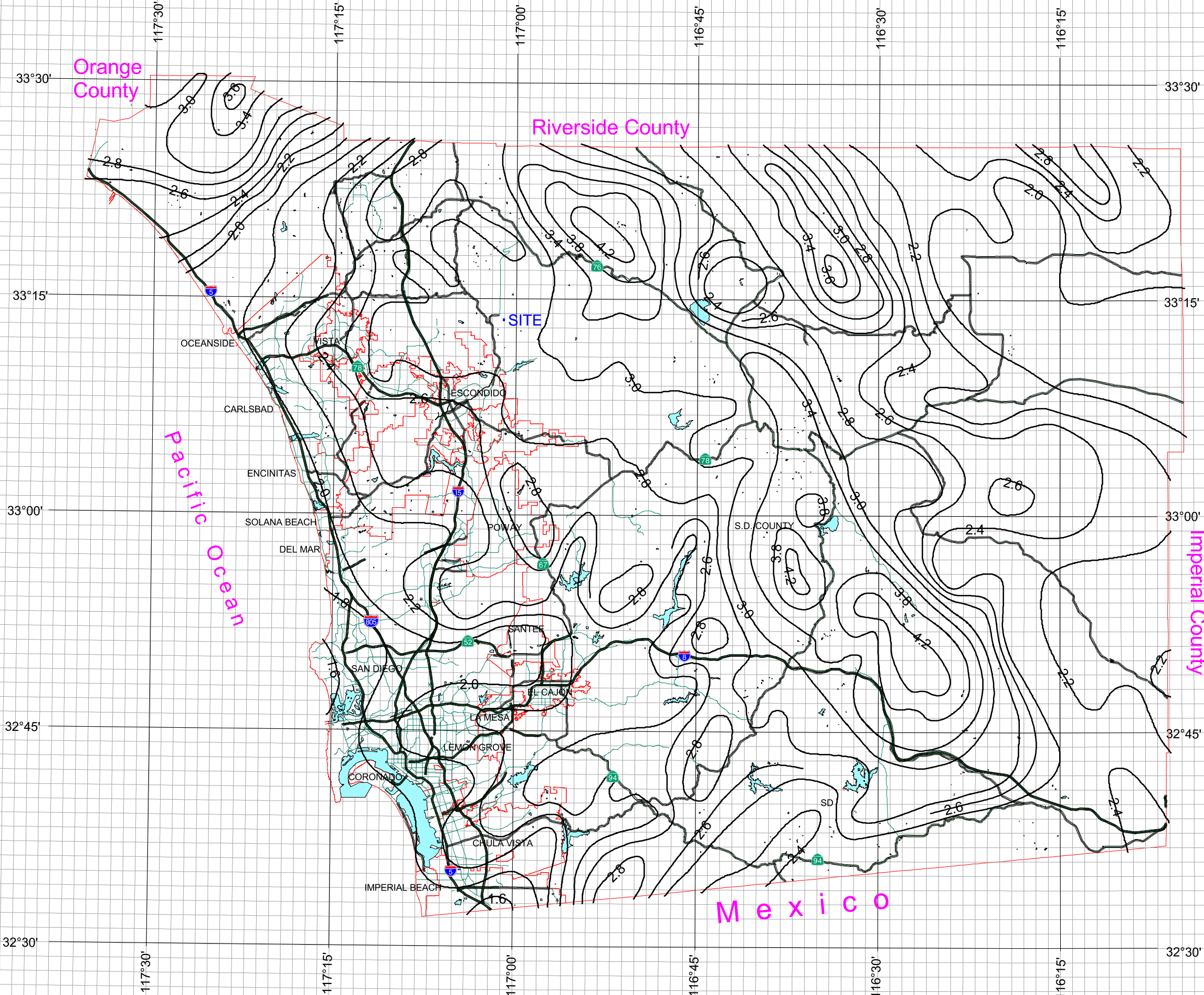
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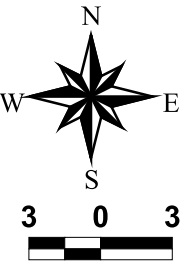
County of San Diego Hydrology Manual



Rainfall Isopluvials

25 Year Rainfall Event - 24 Hours

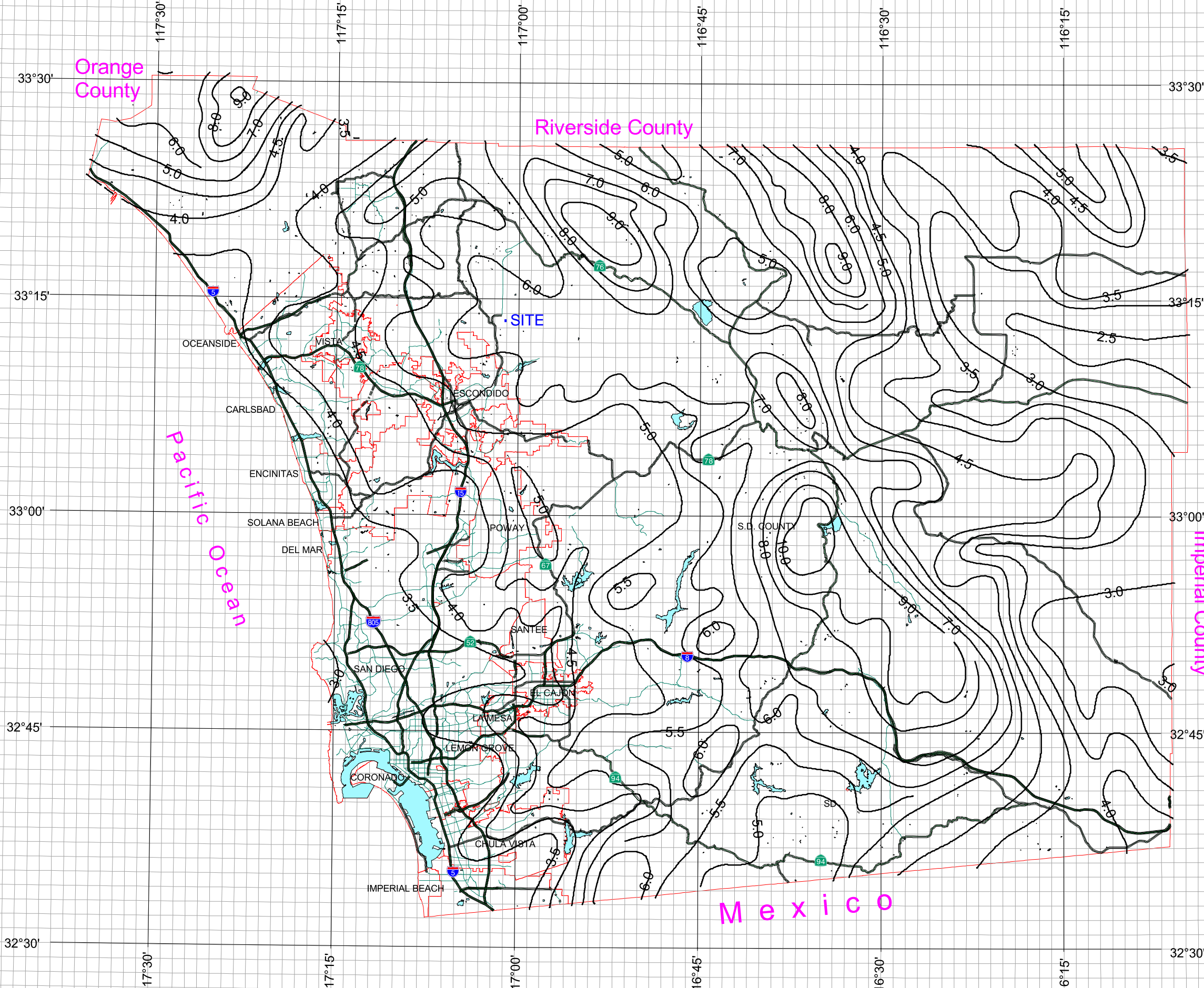
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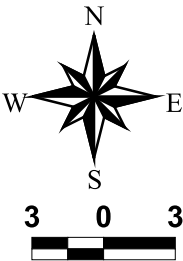
County of San Diego Hydrology Manual



Rainfall Isopluvials

100 Year Rainfall Event - 6 Hours

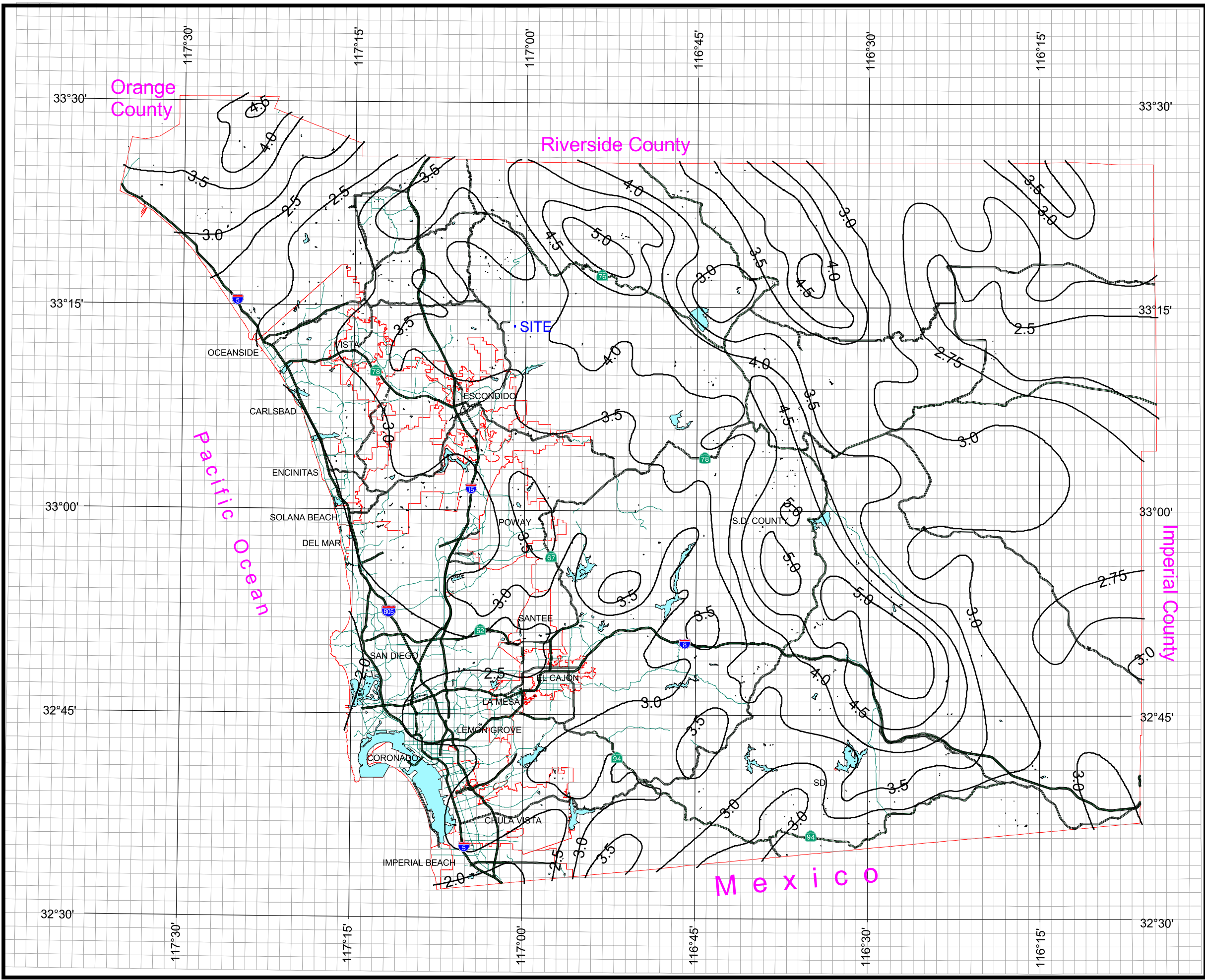
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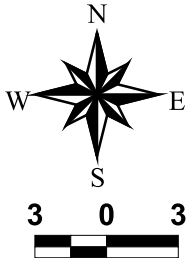
County of San Diego Hydrology Manual



Rainfall Isophuvials

100 Year Rainfall Event - 24 Hours

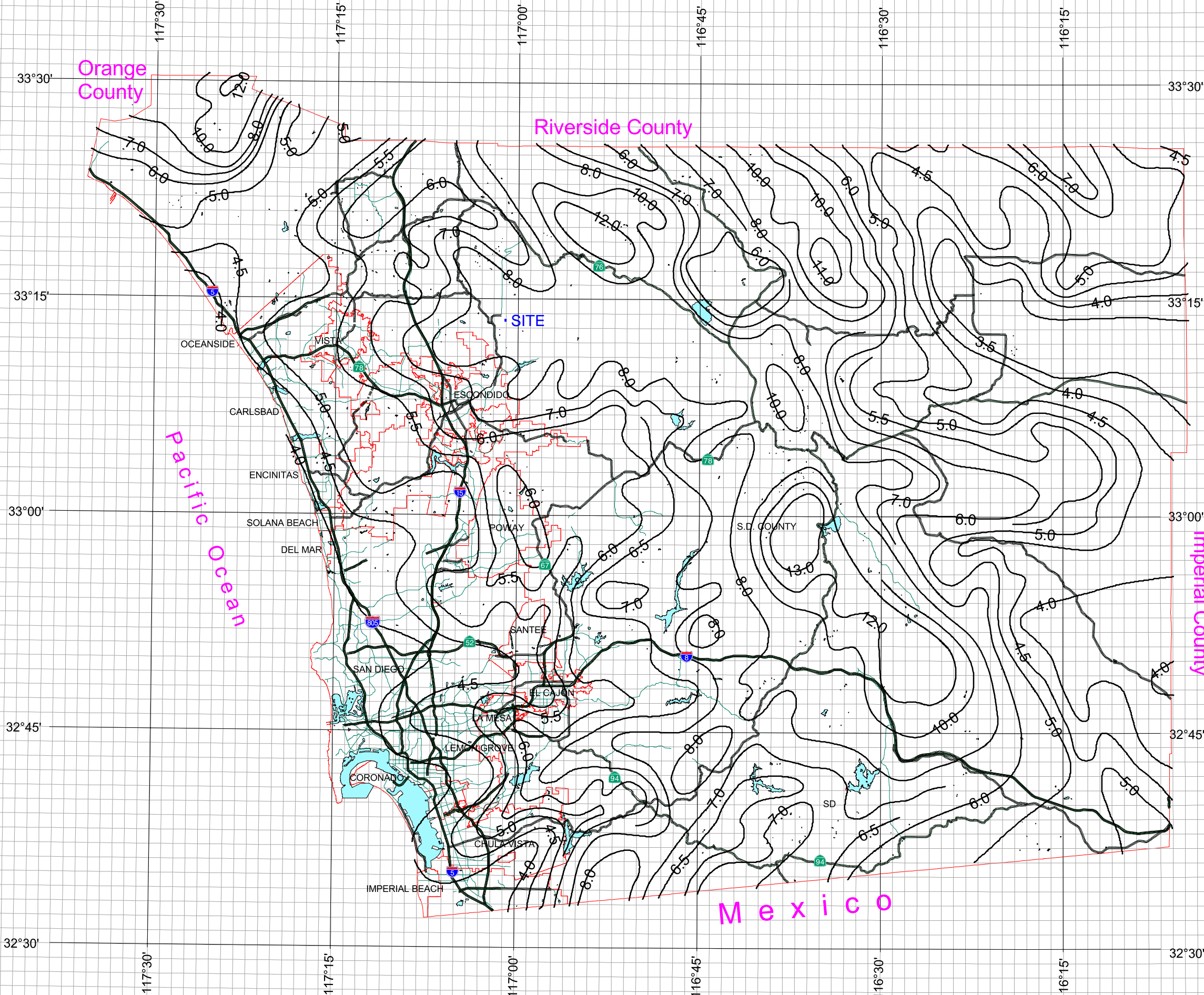
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APPENDIX A

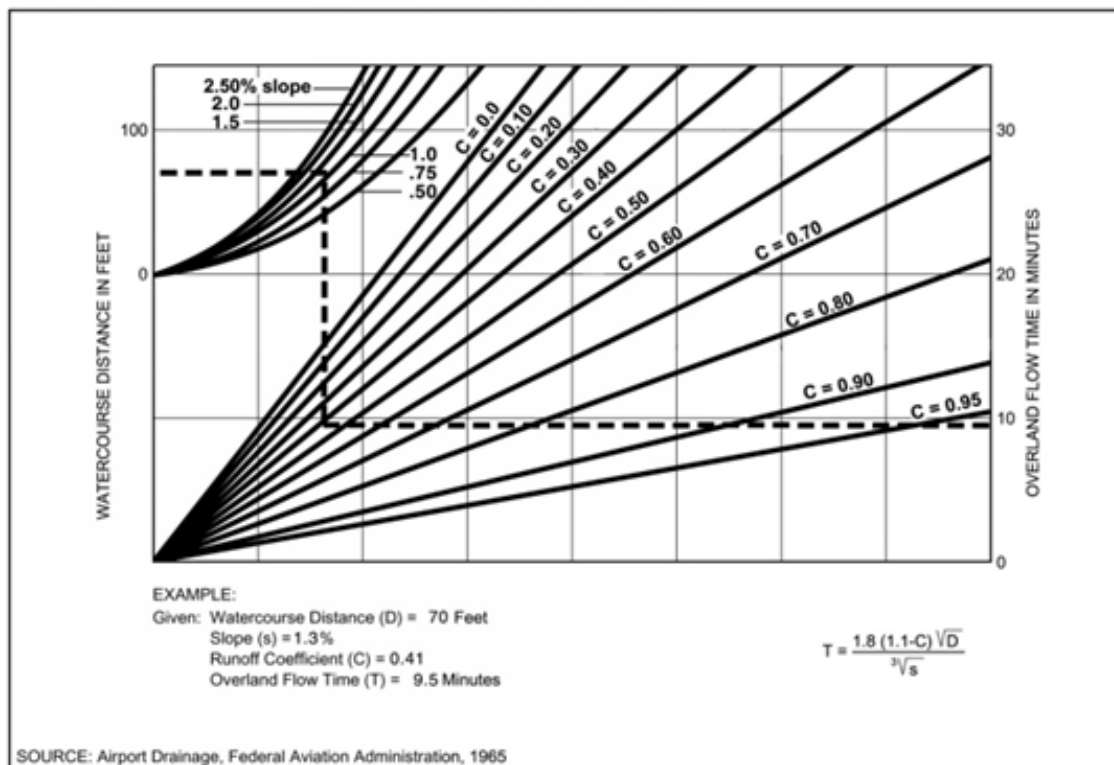
Intensity-Duration Design Charts

Table 3-2

**MAXIMUM OVERLAND FLOW LENGTH (L_M)
& INITIAL TIME OF CONCENTRATION (T_i)**

Element*	DU/ Acre	.5%		1%		2%		3%		5%		10%	
		L_M	T_i	L_M	T_i	L_M	T_i	L_M	T_i	L_M	T_i	L_M	T_i
Natural		50	13.2	70	12.5	85	10.9	100	10.3	100	8.7	100	6.9
LDR	1	50	12.2	70	11.5	85	10.0	100	9.5	100	8.0	100	6.4
LDR	2	50	11.3	70	10.5	85	9.2	100	8.8	100	7.4	100	5.8
LDR	2.9	50	10.7	70	10.0	85	8.8	95	8.1	100	7.0	100	5.6
MDR	4.3	50	10.2	70	9.6	80	8.1	95	7.8	100	6.7	100	5.3
MDR	7.3	50	9.2	65	8.4	80	7.4	95	7.0	100	6.0	100	4.8
MDR	10.9	50	8.7	65	7.9	80	6.9	90	6.4	100	5.7	100	4.5
MDR	14.5	50	8.2	65	7.4	80	6.5	90	6.0	100	5.4	100	4.3
HDR	24	50	6.7	65	6.1	75	5.1	90	4.9	95	4.3	100	3.5
HDR	43	50	5.3	65	4.7	75	4.0	85	3.8	95	3.4	100	2.7
N. Com		50	5.3	60	4.5	75	4.0	85	3.8	95	3.4	100	2.7
G. Com		50	4.7	60	4.1	75	3.6	85	3.4	90	2.9	100	2.4
O.P./Com		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
Limited I.		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
General I.		50	3.7	60	3.2	70	2.7	80	2.6	90	2.3	100	1.9

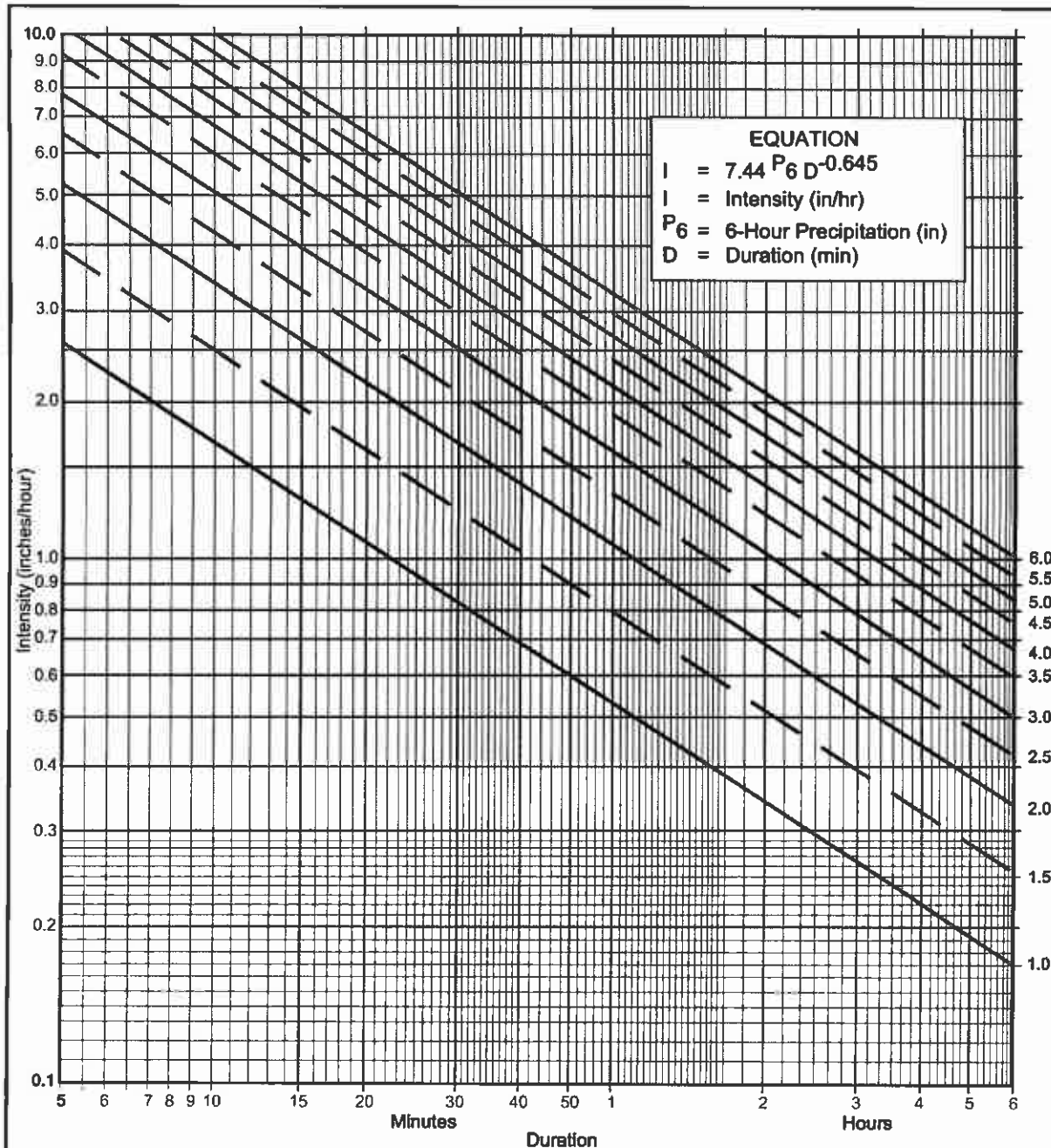
*See Table 3-1 for more detailed description



FIGURE

Rational Formula - Overland Time of Flow Nomograph

3-3



Directions for Application:

- (1) From precipitation maps determine 6 hr and 24 hr amounts for the selected frequency. These maps are included in the County Hydrology Manual (10, 50, and 100 yr maps included in the Design and Procedure Manual).
- (2) Adjust 6 hr precipitation (if necessary) so that it is within the range of 45% to 65% of the 24 hr precipitation (not applicable to Desert).
- (3) Plot 6 hr precipitation on the right side of the chart.
- (4) Draw a line through the point parallel to the plotted lines.
- (5) This line is the intensity-duration curve for the location being analyzed.

Application Form:

- (a) Selected frequency 2 year
- (b) $P_6 = \underline{1.8}$ in., $P_{24} = \underline{3.0}$ in., $\frac{P_6}{P_{24}} = \underline{60} \%$ ⁽²⁾
- (c) Adjusted P_6 ⁽²⁾ = 1.8 in.
- (d) $t_x = \underline{*}$ min. *Calculated with AES software, as described by the methods in Section III of this Drainage Study. Results are tabulated in Section VI.
- (e) $I = \underline{*}$ in./hr.

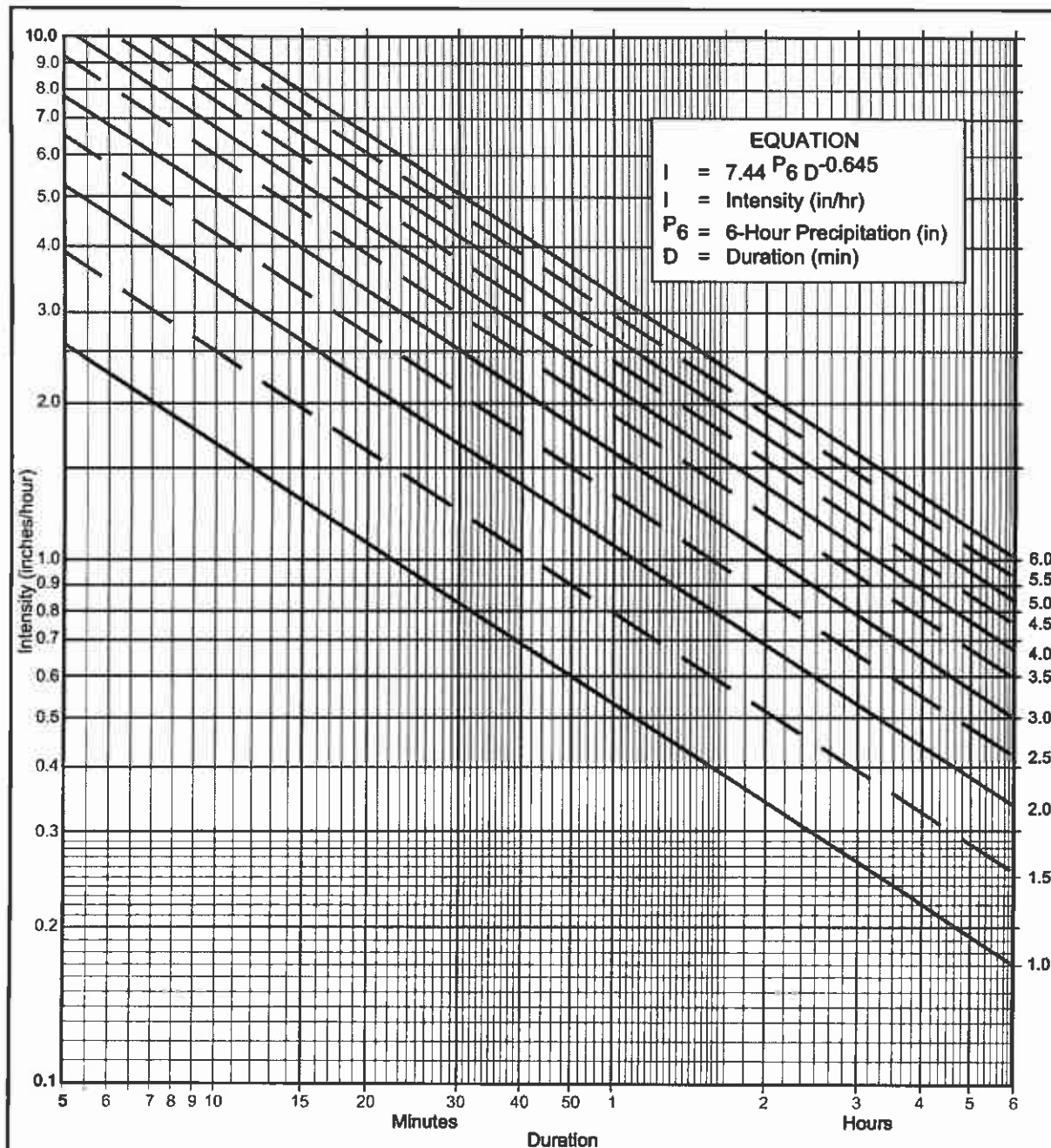
Note: This chart replaces the Intensity-Duration-Frequency curves used since 1965.

P6	1	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6
Duration	I	I	I	I	I	I	I	I	I	I	I
5	2.63	3.95	5.27	6.59	7.90	9.22	10.54	11.86	13.17	14.49	15.81
7	2.12	3.18	4.24	5.30	6.36	7.42	8.48	9.54	10.60	11.66	12.72
10	1.68	2.53	3.37	4.21	5.05	5.90	6.74	7.58	8.42	9.27	10.11
15	1.30	1.95	2.59	3.24	3.89	4.54	5.19	5.84	6.49	7.13	7.78
20	1.08	1.62	2.15	2.69	3.23	3.77	4.31	4.85	5.39	5.93	6.46
25	0.93	1.40	1.87	2.33	2.80	3.27	3.73	4.20	4.67	5.13	5.60
30	0.83	1.24	1.66	2.07	2.49	2.90	3.32	3.73	4.15	4.56	4.98
40	0.69	1.03	1.38	1.72	2.07	2.41	2.76	3.10	3.45	3.79	4.13
50	0.60	0.90	1.19	1.49	1.79	2.09	2.39	2.69	2.98	3.28	3.58
60	0.53	0.80	1.06	1.33	1.59	1.88	2.12	2.39	2.65	2.92	3.18
80	0.41	0.61	0.82	1.02	1.23	1.43	1.63	1.84	2.04	2.25	2.45
120	0.34	0.51	0.68	0.85	1.02	1.19	1.38	1.53	1.70	1.87	2.04
150	0.29	0.44	0.59	0.73	0.88	1.03	1.18	1.32	1.47	1.62	1.76
180	0.26	0.39	0.52	0.65	0.78	0.91	1.04	1.18	1.31	1.44	1.57
240	0.22	0.33	0.43	0.54	0.65	0.76	0.87	0.98	1.08	1.19	1.30
300	0.19	0.28	0.38	0.47	0.56	0.66	0.75	0.85	0.94	1.03	1.13
360	0.17	0.25	0.33	0.42	0.50	0.58	0.67	0.75	0.84	0.92	1.00

Intensity-Duration Design Chart - Template

FIGURE

3-1



Directions for Application:

- (1) From precipitation maps determine 6 hr and 24 hr amounts for the selected frequency. These maps are included in the County Hydrology Manual (10, 50, and 100 yr maps included in the Design and Procedure Manual).
- (2) Adjust 6 hr precipitation (if necessary) so that it is within the range of 45% to 65% of the 24 hr precipitation (not applicable to Desert).
- (3) Plot 6 hr precipitation on the right side of the chart.
- (4) Draw a line through the point parallel to the plotted lines.
- (5) This line is the intensity-duration curve for the location being analyzed.

Application Form:

- (a) Selected frequency 10 year
- (b) $P_6 = \underline{2.6}$ in., $P_{24} = \underline{6.0}$, $\frac{P_6}{P_{24}} = \underline{43} \%$ ⁽²⁾
- (c) Adjusted P_6 ⁽²⁾ = 2.7 in.
- (d) $t_x = \underline{*}$ min. *Calculated with AES software, as described by the methods in Section III of this Drainage Study. Results are tabulated in Section VI.
- (e) $I = \underline{*}$ in./hr.

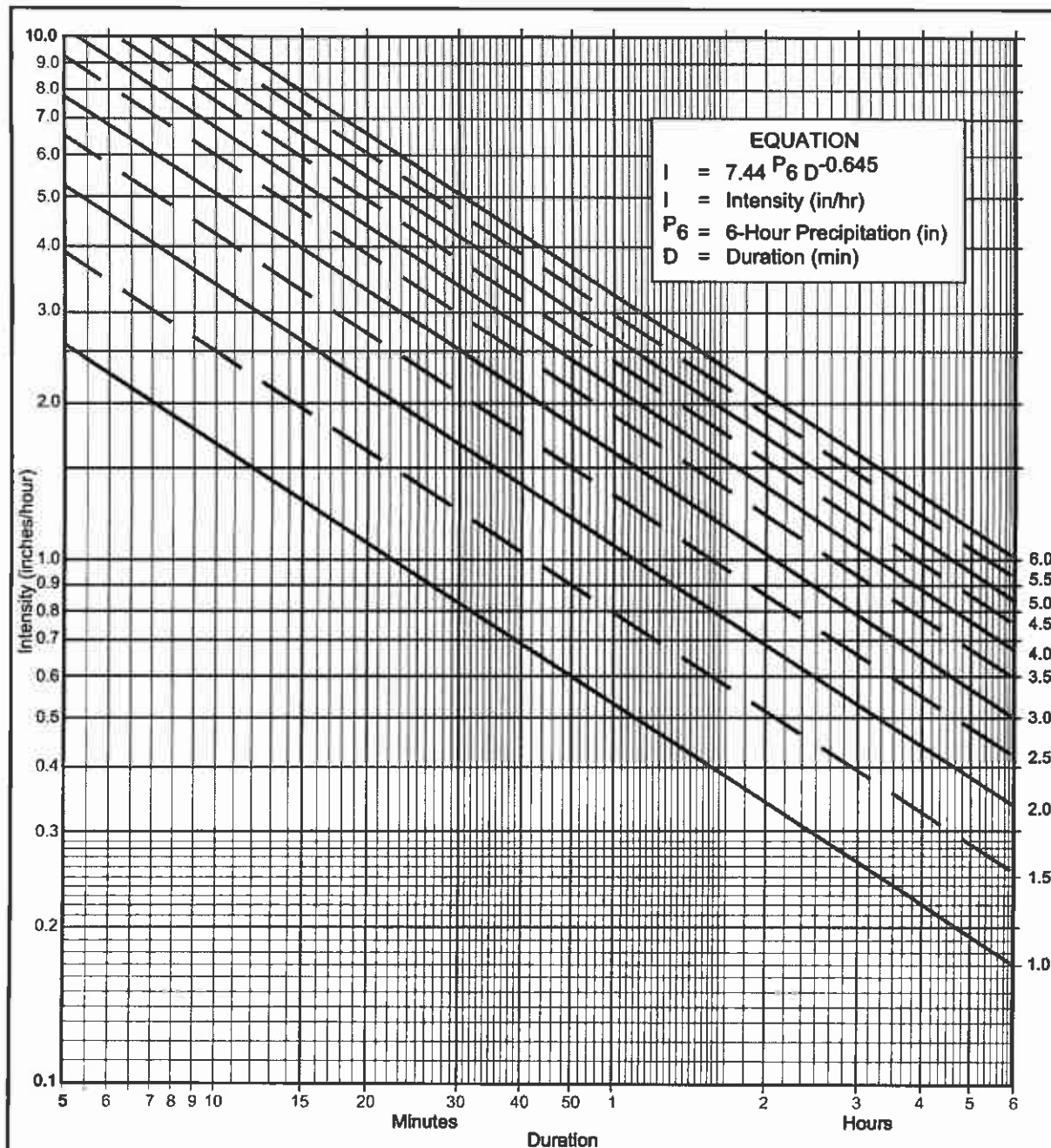
Note: This chart replaces the Intensity-Duration-Frequency curves used since 1965.

P6	1	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6
Duration	I	I	I	I	I	I	I	I	I	I	I
5	2.63	3.95	5.27	6.59	7.90	9.22	10.54	11.86	13.17	14.49	15.81
7	2.12	3.18	4.24	5.30	6.36	7.42	8.48	9.54	10.60	11.66	12.72
10	1.68	2.53	3.37	4.21	5.05	5.90	6.74	7.58	8.42	9.27	10.11
15	1.30	1.95	2.59	3.24	3.89	4.54	5.19	5.84	6.49	7.13	7.78
20	1.08	1.62	2.15	2.69	3.23	3.77	4.31	4.85	5.39	5.93	6.46
25	0.93	1.40	1.87	2.33	2.80	3.27	3.73	4.20	4.67	5.13	5.60
30	0.83	1.24	1.66	2.07	2.49	2.90	3.32	3.73	4.15	4.56	4.98
40	0.69	1.03	1.38	1.72	2.07	2.41	2.76	3.10	3.45	3.79	4.13
50	0.60	0.90	1.19	1.49	1.79	2.09	2.39	2.69	2.98	3.28	3.58
60	0.53	0.80	1.06	1.33	1.59	1.88	2.12	2.39	2.65	2.92	3.18
80	0.41	0.61	0.82	1.02	1.23	1.43	1.63	1.84	2.04	2.25	2.45
120	0.34	0.51	0.68	0.85	1.02	1.19	1.38	1.53	1.70	1.87	2.04
150	0.29	0.44	0.59	0.73	0.88	1.03	1.18	1.32	1.47	1.62	1.76
180	0.26	0.39	0.52	0.65	0.78	0.91	1.04	1.18	1.31	1.44	1.57
240	0.22	0.33	0.43	0.54	0.65	0.76	0.87	0.98	1.08	1.19	1.30
300	0.19	0.28	0.38	0.47	0.56	0.66	0.75	0.85	0.94	1.03	1.13
360	0.17	0.25	0.33	0.42	0.50	0.58	0.67	0.75	0.84	0.92	1.00

Intensity-Duration Design Chart - Template

FIGURE

3-1



Directions for Application:

- (1) From precipitation maps determine 6 hr and 24 hr amounts for the selected frequency. These maps are included in the County Hydrology Manual (10, 50, and 100 yr maps included in the Design and Procedure Manual).
- (2) Adjust 6 hr precipitation (if necessary) so that it is within the range of 45% to 65% of the 24 hr precipitation (not applicable to Desert).
- (3) Plot 6 hr precipitation on the right side of the chart.
- (4) Draw a line through the point parallel to the plotted lines.
- (5) This line is the intensity-duration curve for the location being analyzed.

Application Form:

- (a) Selected frequency 25 year
- (b) $P_6 = \underline{3.0}$ in., $P_{24} = \underline{6.5}$ in., $\frac{P_6}{P_{24}} = \underline{46} \%$ ⁽²⁾
- (c) Adjusted P_6 ⁽²⁾ = 3.0 in.
- (d) $t_x = \underline{*}$ min. *Calculated with AES software, as described by the methods in Section III of this Drainage Study. Results are tabulated in Section VI.
- (e) $I = \underline{*}$ in./hr.

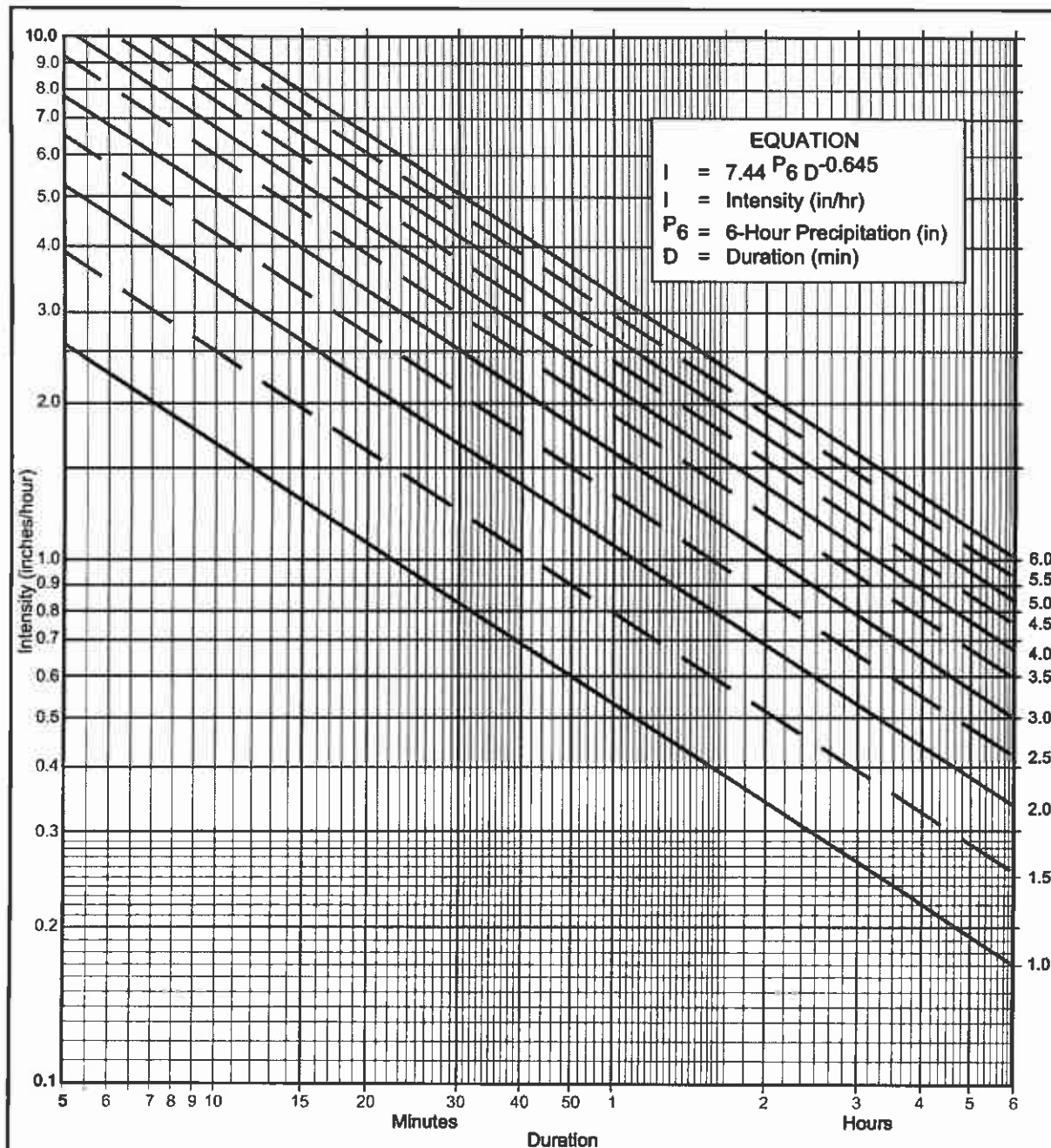
Note: This chart replaces the Intensity-Duration-Frequency curves used since 1965.

P6	1	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6
Duration	I	I	I	I	I	I	I	I	I	I	I
5	2.63	3.95	5.27	6.59	7.90	9.22	10.54	11.86	13.17	14.49	15.81
7	2.12	3.18	4.24	5.30	6.36	7.42	8.48	9.54	10.60	11.66	12.72
10	1.68	2.53	3.37	4.21	5.05	5.90	6.74	7.58	8.42	9.27	10.11
15	1.30	1.95	2.59	3.24	3.89	4.54	5.19	5.84	6.49	7.13	7.78
20	1.08	1.62	2.15	2.69	3.23	3.77	4.31	4.85	5.39	5.93	6.46
25	0.93	1.40	1.87	2.33	2.80	3.27	3.73	4.20	4.67	5.13	5.60
30	0.83	1.24	1.66	2.07	2.49	2.90	3.32	3.73	4.15	4.56	4.98
40	0.69	1.03	1.38	1.72	2.07	2.41	2.76	3.10	3.45	3.79	4.13
50	0.60	0.90	1.19	1.49	1.79	2.09	2.39	2.69	2.98	3.28	3.58
60	0.53	0.80	1.06	1.33	1.59	1.88	2.12	2.39	2.65	2.92	3.18
80	0.41	0.61	0.82	1.02	1.23	1.43	1.63	1.84	2.04	2.25	2.45
120	0.34	0.51	0.68	0.85	1.02	1.19	1.38	1.53	1.70	1.87	2.04
150	0.29	0.44	0.59	0.73	0.88	1.03	1.18	1.32	1.47	1.62	1.76
180	0.26	0.39	0.52	0.65	0.78	0.91	1.04	1.18	1.31	1.44	1.57
240	0.22	0.33	0.43	0.54	0.65	0.76	0.87	0.98	1.08	1.19	1.30
300	0.19	0.28	0.38	0.47	0.56	0.66	0.75	0.85	0.94	1.03	1.13
360	0.17	0.25	0.33	0.42	0.50	0.58	0.67	0.75	0.84	0.92	1.00

Intensity-Duration Design Chart - Template

FIGURE

3-1



Directions for Application:

- (1) From precipitation maps determine 6 hr and 24 hr amounts for the selected frequency. These maps are included in the County Hydrology Manual (10, 50, and 100 yr maps included in the Design and Procedure Manual).
- (2) Adjust 6 hr precipitation (if necessary) so that it is within the range of 45% to 65% of the 24 hr precipitation (not applicable to Desert).
- (3) Plot 6 hr precipitation on the right side of the chart.
- (4) Draw a line through the point parallel to the plotted lines.
- (5) This line is the intensity-duration curve for the location being analyzed.

Application Form:

- (a) Selected frequency 100 year
- (b) $P_6 = \underline{4.0}$ in., $P_{24} = \underline{10.0}$, $\frac{P_6}{P_{24}} = \underline{40} \%$ ⁽²⁾
- (c) Adjusted P_6 ⁽²⁾ = 4.5 in.
- (d) $t_x = \underline{*}$ min. *Calculated with AES software, as described by the methods in Section III of this Drainage Study. Results are tabulated in Section VI.
- (e) $I = \underline{*}$ in./hr.

Note: This chart replaces the Intensity-Duration-Frequency curves used since 1965.

P6	1	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6
Duration	I	I	I	I	I	I	I	I	I	I	I
5	2.63	3.95	5.27	6.59	7.90	9.22	10.54	11.86	13.17	14.49	15.81
7	2.12	3.18	4.24	5.30	6.36	7.42	8.48	9.54	10.60	11.66	12.72
10	1.68	2.53	3.37	4.21	5.05	5.90	6.74	7.58	8.42	9.27	10.11
15	1.30	1.95	2.59	3.24	3.89	4.54	5.19	5.84	6.49	7.13	7.78
20	1.08	1.62	2.15	2.69	3.23	3.77	4.31	4.85	5.39	5.93	6.46
25	0.93	1.40	1.87	2.33	2.80	3.27	3.73	4.20	4.67	5.13	5.60
30	0.83	1.24	1.66	2.07	2.49	2.90	3.32	3.73	4.15	4.56	4.98
40	0.69	1.03	1.38	1.72	2.07	2.41	2.76	3.10	3.45	3.79	4.13
50	0.60	0.90	1.19	1.49	1.79	2.09	2.39	2.69	2.98	3.28	3.58
60	0.53	0.80	1.06	1.33	1.59	1.88	2.12	2.39	2.65	2.92	3.18
80	0.41	0.61	0.82	1.02	1.23	1.43	1.63	1.84	2.04	2.25	2.45
120	0.34	0.51	0.68	0.85	1.02	1.19	1.38	1.53	1.70	1.87	2.04
150	0.29	0.44	0.59	0.73	0.88	1.03	1.18	1.32	1.47	1.62	1.76
180	0.26	0.39	0.52	0.65	0.78	0.91	1.04	1.18	1.31	1.44	1.57
240	0.22	0.33	0.43	0.54	0.65	0.76	0.87	0.98	1.08	1.19	1.30
300	0.19	0.28	0.38	0.47	0.56	0.66	0.75	0.85	0.94	1.03	1.13
360	0.17	0.25	0.33	0.42	0.50	0.58	0.67	0.75	0.84	0.92	1.00

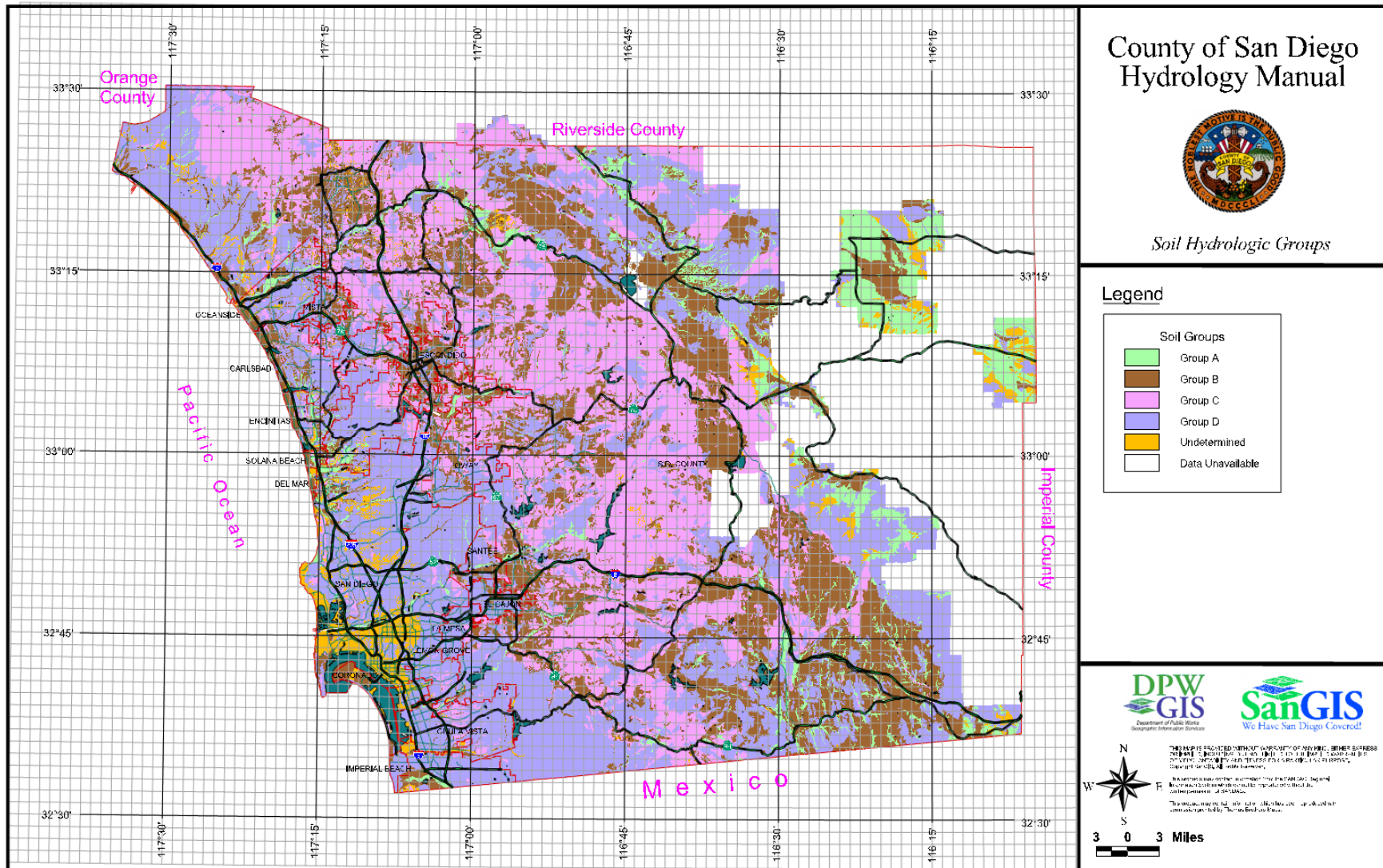
Intensity-Duration Design Chart - Template

FIGURE

3-1

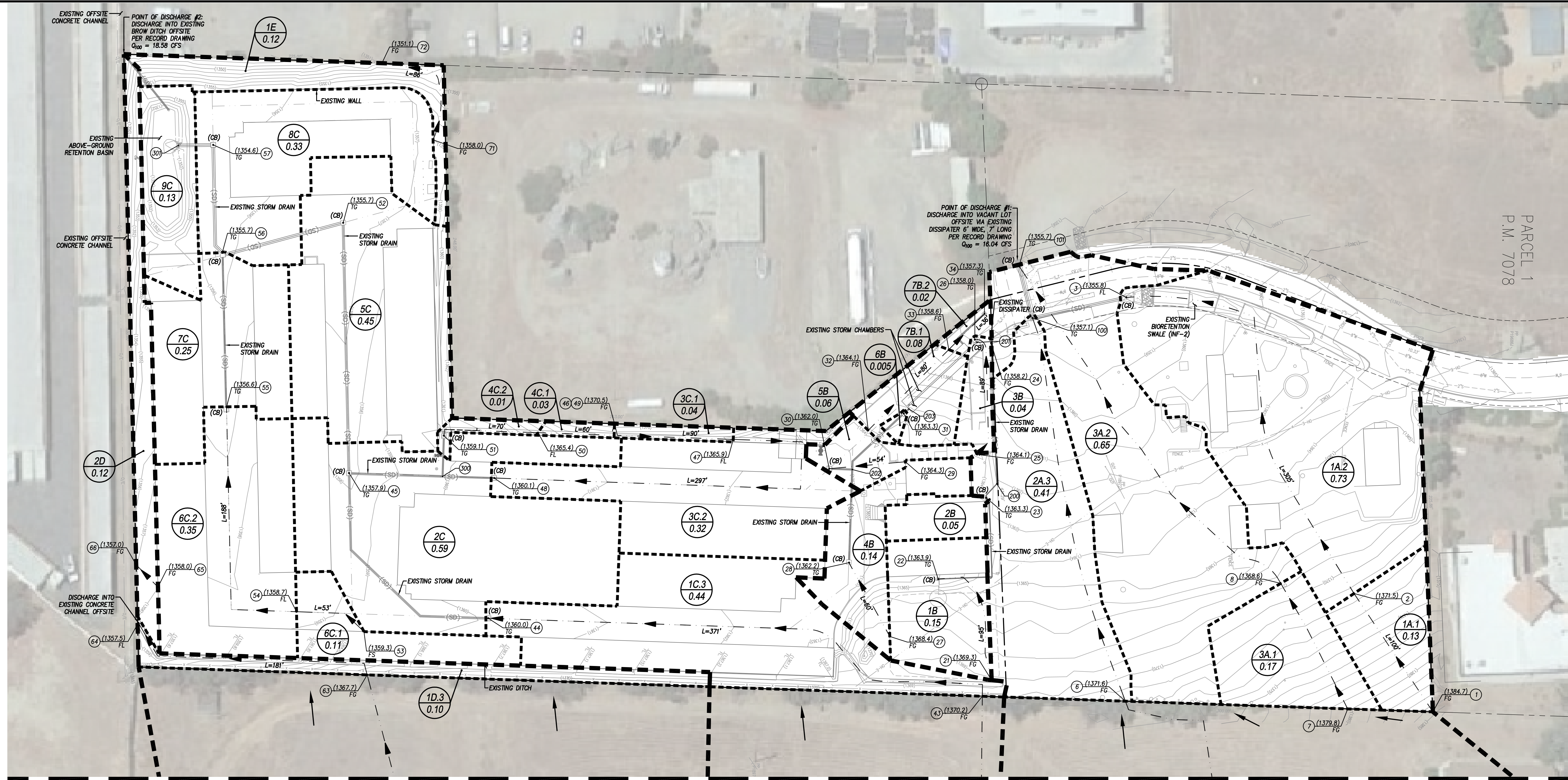
APPENDIX B

Hydrologic Soil Group Map



APPENDIX C

Hydrology Maps



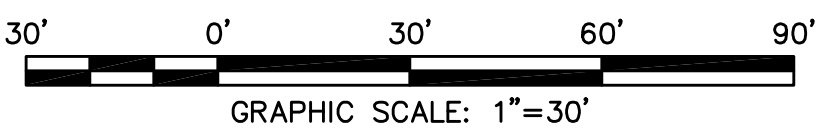
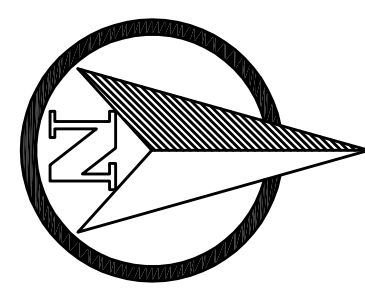
MATCH LINE - SEE SHEET 2

Drainage Area Summary

DMA	Tributary Area (acres)	Runoff Coefficient, C	Flowpath, D (ft)	Slope, s (%)	Existing Condition				25-Year Storm				100-Year Storm			
					Runoff Q ₁₀ cfs	T ₁₀ min.	Runoff Q ₂₅ cfs	T ₂₅ min.	Runoff Q ₁₀ cfs	T ₁₀ min.	Runoff Q ₂₅ cfs	T ₂₅ min.	Runoff Q ₁₀₀ cfs	T ₁₀₀ min.	Runoff Q ₂₅ cfs	T ₂₅ min.
1A.1	0.13	0.30	100	13.2	0.23	6.7	0.26	6.7	0.38	6.7	0.38	6.7	0.38	6.7	0.38	6.7
1A.2	0.73	0.42	305	5.1	1.18	13.0	1.36	12.2	2.14	11.4	2.14	11.4	2.14	11.4	2.14	11.4
2A.1	0.40	0.30	100	5.0	0.61	8.4	0.68	8.4	1.02	8.4	1.02	8.4	1.02	8.4	1.02	8.4
2A.2	3.55	0.30	465	5.9	3.81	14.5	4.30	14.2	6.63	13.6	6.63	13.6	6.63	13.6	6.63	13.6
2A.3	0.41	0.36	278	5.2	0.49	16.5	0.55	16.0	0.86	15.2	0.86	15.2	0.86	15.2	0.86	15.2
3A.1	0.17	0.30	100	11.2	0.30	6.7	0.33	6.7	0.50	6.7	0.50	6.7	0.50	6.7	0.50	6.7
3A.2	0.65	0.42	294	4.4	1.15	11.3	1.24	11.8	1.98	10.7	1.98	10.7	1.98	10.7	1.98	10.7
A Subtotal	6.60	0.33	—	—	6.60	16.6	7.48	16.1	11.63	15.3	11.63	15.3	11.63	15.3	11.63	15.3
1B	0.15	0.45	95	5.7	0.41	6.4	0.46	6.4	0.68	6.4	0.68	6.4	0.68	6.4	0.68	6.4
2B	0.05	0.84	—	—	0.24	5.0	0.27	5.0	0.41	5.0	0.41	5.0	0.41	5.0	0.41	5.0
3B	0.04	0.84	89	6.9	0.24	5.0	0.27	5.0	0.40	5.0	0.40	5.0	0.40	5.0	0.40	5.0
4B	0.14	0.84	60	10.3	0.84	5.0	0.93	5.0	1.39	5.0	1.39	5.0	1.39	5.0	1.39	5.0
5B	0.06	0.84	54	4.3	0.36	5.0	0.40	5.0	0.60	5.0	0.60	5.0	0.60	5.0	0.60	5.0
6B	0.005	0.84	—	—	0.03	5.0	0.03	5.0	0.05	5.0	0.05	5.0	0.05	5.0	0.05	5.0
7B.1	0.08	0.84	80	3.1	0.48	5.0	0.53	5.0	0.80	5.0	0.80	5.0	0.80	5.0	0.80	5.0
7B.2	0.02	0.84	36	3.6	0.12	5.0	0.13	5.0	0.20	5.0	0.20	5.0	0.20	5.0	0.20	5.0
B Subtotal	0.53	0.73	—	—	2.64	5.0	2.93	5.0	4.41	5.0	4.41	5.0	4.41	5.0	4.41	5.0
1C.1	0.31	0.30	100	4.0	0.45	9.1	0.50	9.1	0.75	9.1	0.75	9.1	0.75	9.1	0.75	9.1
1C.2	1.80	0.42	380	7.1	2.70	14.5	3.03	14.3	4.75	13.4	4.75	13.4	4.75	13.4	4.75	13.4
1C.3	0.40	0.83	371	2.7	1.21	16.3	1.36	16.0	7.05	15.0	7.05	15.0	7.05	15.0	7.05	15.0
2C	0.59	0.84	—	—	1.62	5.0	1.82	5.0	2.85	5.0	2.85	5.0	2.85	5.0	2.85	5.0
3C.1	0.04	0.87	90	5.1	0.25	5.0	0.28	5.0	0.41	5.0	0.41	5.0	0.41	5.0	0.41	5.0
3C.2	0.32	0.84	297	2.0	1.51	5.0	2.12	5.0	3.19	5.0	3.19	5.0	3.19	5.0	3.19	5.0
4C.1	0.03	0.87	60	8.5	0.19	5.0	0.21	5.0	0.31	5.0	0.31	5.0	0.31	5.0	0.31	5.0
4C.2	0.01	0.87	70	9.0	0.06	5.0	0.07	5.0	0.10	5.0	0.10	5.0	0.10	5.0	0.10	5.0
5C	0.44	0.84	—	—	1.19	5.0	1.34	5.0	2.09	5.0	2.09	5.0	2.09	5.0	2.09	5.0
6C.1	0.11	0.84	53	1.1	0.66	5.0	0.73	5.0	1.10	5.0	1.10	5.0	1.10	5.0	1.10	5.0
6C.2	0.35	0.84	188	1.1	2.09	5.0	2.32	5.0	3.49	5.0	3.49	5.0	3.49	5.0	3.49	5.0
7C	0.25	0.84	—	—	1.44	5.3	1.63	5.2	2.49	5.0	2.49	5.0	2.49	5.0	2.49	5.0
8C	0.33	0.83	—	—	0.87	5.0	0.98	5.0	1.53	5.0	1.53	5.0	1.53	5.0	1.53	5.0
9C	0.13	0.34	—	—	0.14	5.0	0.16	5.0	0.25	5.0	0.25	5.0	0.25	5.0	0.25	5.0
C Subtotal	5.16	0.65	—	—	10.56	17.5	11.85	17.2	18.58	16.1	18.58	16.1	18.58	16.1	18.58	16.1
1D.1	0.57	0.30	100	23.0	1.01	6.7	1.12	6.7	1.68	6.7	1.68	6.7	1.68	6.7	1.68	6.7
1D.2	5.18	0.27	651	9.4	4.93	15.0	5.70	14.0	9.07	12.8	9.07	12.8	9.07	12.8	9.07	12.8
1D.3	0.10	0.48	181	5.6	0.17	15.2	0.19	14.2	0.31	13.0	0.31	13.0	0.31	13.0	0.31	13.0
2D	0.12	0.30	22	4.4	0.26	5.0	0.28	5.0	0.43	5.0	0.43	5.0	0.43	5.0	0.43	5.0
D Subtotal	5.97	0.28	—	—	5.74	15.4	6.66	14.4	10.58	13.1	10.58	13.1	10.58	13.1	10.58	13.1
1E	0.12	0.20	—	—	0.13	7.5	0.15	7.5	0.22	7.5	0.22	7.5	0.22	7.5	0.22	7.5
E Subtotal	0.12	0.20	—	—	0.13	7.5	0.15	7.5	0.22	7.5	0.22	7.5	0.22	7.5	0.22	7.5
Study Total	17.81	0.42	—	—	25.67	—	29.07	—	45.42	—	45.42	—	45.42	—	45.42	—

Legend

- DRAINAGE STUDY AREA BOUNDARY
- DRAINAGE SUBAREA BOUNDARY
- LONGEST FLOW PATH
- ▲ DIRECTION OF FLOW
- 1A.1 SUBAREA DESIGNATION AREA (ACRES)
- (CB) EXISTING CATCH BASIN



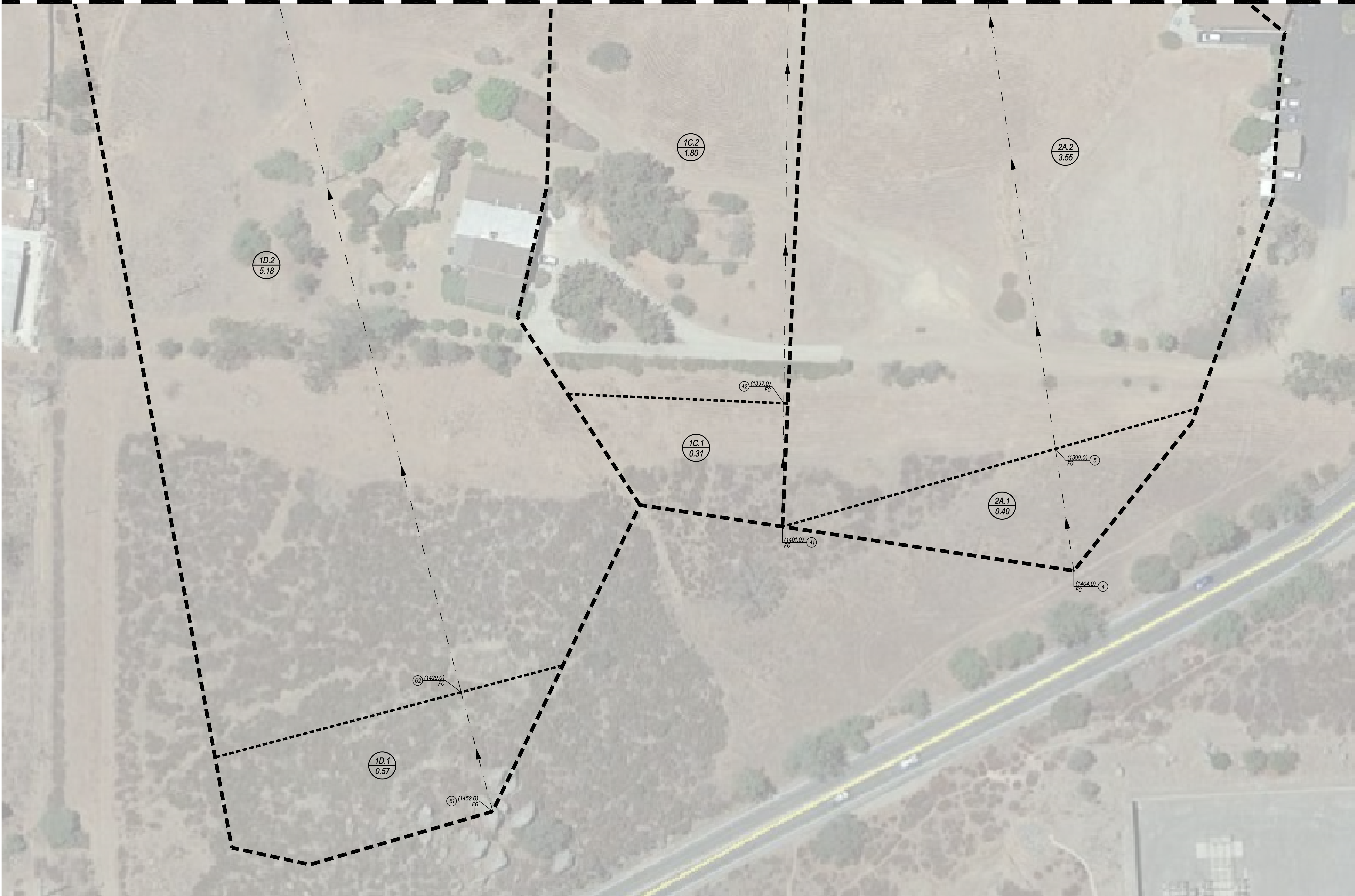
PROJECT: GS VALLEY EXPANSION
28435 LIZARD ROCKS ROAD
VALLEY CENTER, CALIFORNIA
DRAWING NAME: EXISTING HYDROLOGY MAP

ISSUE: HYDROLOGY
DATE: 5/27/2021
CHECKED: JH DRAWN: JH
DRAWING FILE: 19132EXHM
PROJECT NO.: 19-182
SHEET NUMBER: 1
OF 2 SHEETS
SCALE: AS SHOWN

180 S. Old Springs Road
Suite 210
Anheim Hills, CA 92808
714-665-6660
Engineering, Inc.
Civil Engineering/Land Surveying/Land Planning

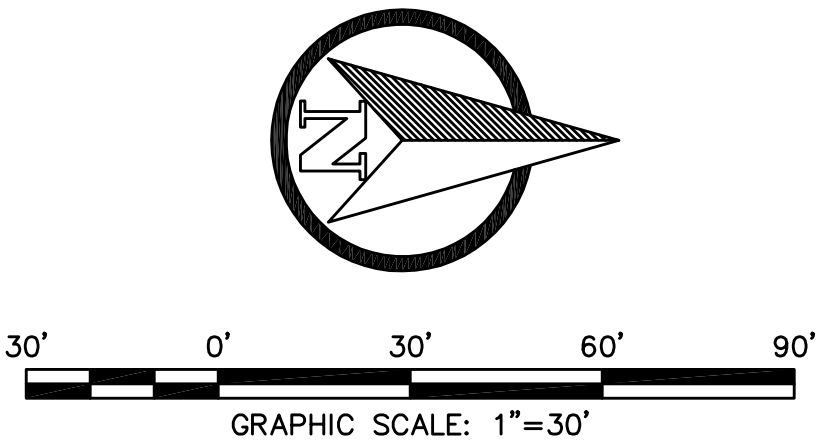
NOT FOR CONSTRUCTION

MATCH LINE - SEE SHEET 1



LEGEND:

- DRAINAGE STUDY AREA BOUNDARY
- DRAINAGE SUBAREA BOUNDARY
- - - LONGEST FLOW PATH
- ▲ DIRECTION OF FLOW
- 1A
0.11
(CB) SUBAREA DESIGNATION AREA (ACRES) EXISTING CATCH BASIN



160 S. Old Springs Road
Suite 210
Anheim Hills, CA 92808
714-665-6660

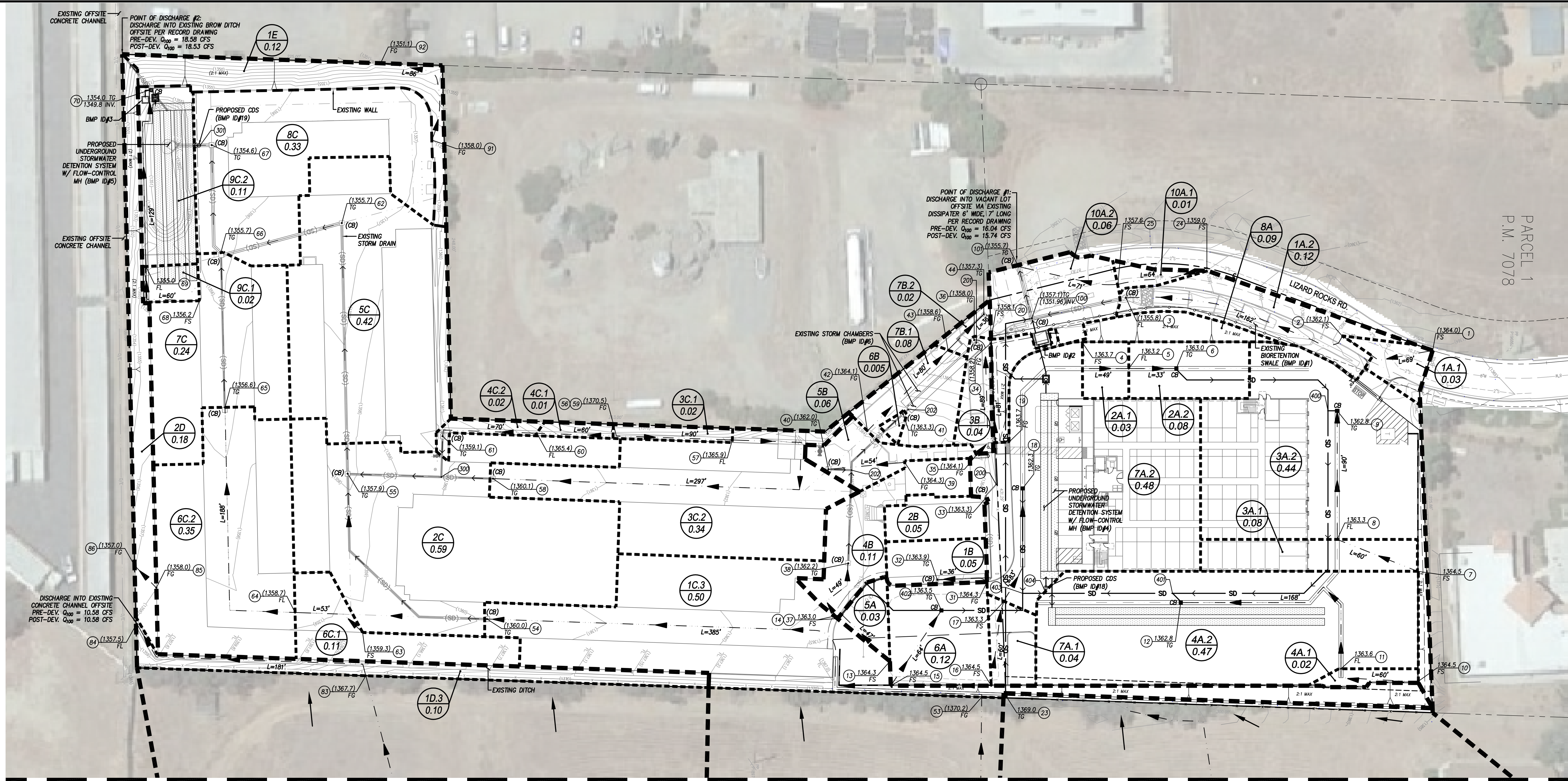
ORC
Engineering, Inc.
Civil Engineering/Land Surveying/Land Planning

NO.	REVISION:	DATE:

PROJECT: **GS VALLEY EXPANSION**
28435 LIZARD ROCKS ROAD
VALLEY CENTER, CALIFORNIA

DRAWING NAME: **EXISTING HYDROLOGY MAP**

ISSUE:	HYDROLOGY
DATE:	5/27/2021
CHECKED: JH	DRAWN: JH
DRAWING FILE:	19132EXHM
PROJECT NO.:	19-132
SHEET NUMBER:	2
OF	2 SHEETS
SCALE:	AS SHOWN



MATCH LINE - SEE SHEET 2

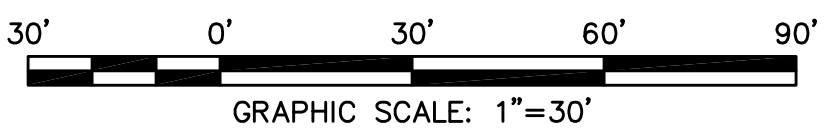
STRUCTURAL BMP IMPLEMENTED	
BMP ID#	BMP TYPE
1	INF-2: BIORETENTION (EXISTING)
2	BF-3: PROPRIETARY BIOFILTRATION (MODULAR WETLAND SYSTEM MWS-L-8-B-V)
3	BF-3: PROPRIETARY BIOFILTRATION (MODULAR WETLAND SYSTEM MWS-L-4-A-C)
4	HYDROMODIFICATION MANAGEMENT (DETENTION VAULT)
5	HYDROMODIFICATION MANAGEMENT (DETENTION VAULT)
6	HYDROMODIFICATION MANAGEMENT (DETENTION VAULT, EXISTING)

Drainage Area Summary

DMA	Tributary Area (acres)	Runoff Coefficient	Flowpath, D (ft)	Slope, s (ft)	Proposed Condition				100-Year Storm			
					15-Year Storm	25-Year Storm	100-Year Storm	Runoff Q ₁₀₀	15-Year Storm	25-Year Storm	100-Year Storm	Runoff Q ₁₀₀
1A.1	0.03	0.69	69	2.8	0.15	5.0	0.16	5.0	0.25	5.0	0.25	5.0
1A.2	0.12	0.69	162	3.9	0.41	8.8	0.47	8.2	0.71	8.3	0.71	8.3
2A.1	0.03	0.84	49	1.0	0.18	5.0	0.20	5.0	0.30	5.0	0.30	5.0
2A.2	0.08	0.84	33	0.6	0.48	5.0	0.53	5.0	0.80	5.0	0.80	5.0
3A.1	0.08	0.84	60	2.0	0.48	5.0	0.53	5.0	0.80	5.0	0.80	5.0
3A.2	0.44	0.84	90	0.6	2.63	5.0	2.92	5.0	4.38	5.0	4.38	5.0
4A.1	0.02	0.84	60	1.5	0.12	5.0	0.13	5.0	0.20	5.0	0.20	5.0
4A.2	0.47	0.84	168	0.5	2.79	5.0	3.12	5.0	4.68	5.0	4.68	5.0
5A	0.03	0.84	47	2.8	0.18	5.0	0.20	5.0	0.30	5.0	0.30	5.0
6A	0.12	0.84	64	1.6	0.72	5.0	0.80	5.0	1.20	5.0	1.20	5.0
7A.1	0.04	0.84	60	2.0	0.24	5.0	0.27	5.0	0.40	5.0	0.40	5.0
7A.2	0.48	0.84	83	1.2	3.11	5.0	3.19	5.0	4.78	5.0	4.78	5.0
8A	0.08	0.69	81	6.9	0.39	5.0	0.44	5.0	0.65	5.0	0.65	5.0
9A.1	0.40	0.30	100	5.0	0.61	8.4	0.68	8.4	1.02	8.4	1.02	8.4
9A.2	3.57	0.30	550	5.5	3.62	15.9	4.10	15.4	6.34	14.7	6.34	14.7
10A.1	0.03	0.69	64	2.2	0.15	5.0	0.16	5.0	0.25	5.0	0.25	5.0
10A.2	0.09	0.69	71	2.7	0.40	5.8	0.45	5.7	0.68	5.6	0.68	5.6
A Subtotal	6.40	0.48	--	--	15.35	5.0	17.18	5.0	26.27	5.0	26.27	5.0
1B	0.05	0.84	36	1.1	0.30	5.0	0.33	5.0	0.50	5.0	0.50	5.0
2B	0.05	0.84	--	--	0.30	5.0	0.33	5.0	0.50	5.0	0.50	5.0
3B	0.04	0.84	89	6.9	0.24	5.0	0.27	5.0	0.40	5.0	0.40	5.0
4B	0.11	0.84	49	1.6	0.66	5.0	0.73	5.0	1.10	5.0	1.10	5.0
5B	0.06	0.84	54	4.3	0.36	5.0	0.40	5.0	0.60	5.0	0.60	5.0
6B	0.005	0.84	--	--	0.03	5.0	0.03	5.0	0.05	5.0	0.05	5.0
7B.1	0.08	0.84	80	3.1	0.48	5.0	0.53	5.0	0.80	5.0	0.80	5.0
7B.2	0.02	0.84	36	3.6	0.12	5.0	0.13	5.0	0.20	5.0	0.20	5.0
B Subtotal	0.40	0.84	--	--	2.49	5.0	2.75	5.0	4.13	5.0	4.13	5.0
1C.1	0.31	0.30	100	4.0	0.45	9.1	0.50	9.1	0.75	9.1	0.75	9.1
1C.2	1.80	0.42	380	7.1	2.70	14.5	3.03	14.3	4.75	13.4	4.75	13.4
1C.3	0.50	0.83	385	2.6	1.37	16.4	1.54	16.1	2.42	15.1	2.42	15.1
2C	0.59	0.84	--	--	1.61	5.0	1.81	5.0	2.84	5.0	2.84	5.0
3C.1	0.04	0.87	90	5.1	0.25	5.0	0.28	5.0	0.41	5.0	0.41	5.0
3C.2	0.32	0.84	297	2.0	1.91	5.0	2.12	5.0	3.19	5.0	3.19	5.0
4C.1	0.03	0.87	60	8.5	0.19	5.0	0.21	5.0	0.31	5.0	0.31	5.0
4C.2	0.01	0.87	70	9.0	0.06	5.0	0.07	5.0	0.10	5.0	0.10	5.0
5C	0.44	0.84	--	--	1.19	5.0	1.33	5.0	2.09	5.0	2.09	5.0
6C.1	0.11	0.84	53	1.1	0.66	5.0	0.73	5.0	1.10	5.0	1.10	5.0
6C.2	0.35	0.84	188	1.1	2.09	5.0	2.32	5.0	3.49	5.0	3.49	5.0
7C	0.24	0.84	--	--	0.43	5.0	1.59	5.0	2.39	5.0	2.39	5.0
8C	0.33	0.83	--	--	0.87	5.0	0.97	5.0	1.53	5.0	1.53	5.0
9C.1	0.02	0.83	60	2.0	0.12	5.0	0.13	5.0	0.20	5.0	0.20	5.0
9C.2	0.11	0.83	129	0.8	0.65	5.0	0.72	5.0	1.08	5.0	1.08	5.0
C Subtotal	2.47	0.41	--	--	11.39	5.0	12.68	5.0	19.82	5.0	19.82	5.0
1D.1	0.57	0.30	100	23.0	1.01	6.7	1.12	6.7	1.68	6.7	1.68	6.7
1D.2	5.18	0.27	651	9.4	4.91	15.0	5.70	14.0	9.07	12.8	9.07	12.8
1D.3	0.10	0.48	181	5.6	0.17	15.2	0.19	14.2	0.31	13.0	0.31	13.0
2D	0.12	0.10	--	--	0.12	5.0	0.28	5.0	0.43	5.0	0.43	5.0
D Subtotal	5.97	0.28	--	--	5.74	15.4	6.66	14.4	10.58	13.1	10.58	13.1
1E	0.12	0.20	86	8.0	0.13	7.5	0.15	7.5	0.22	7.5	0.22	7.5
E Subtotal	0.12	0.20	--	--	0.13	7.5	0.15	7.5	0.22	7.5	0.22	7.5
Study Total	17.81	0.38	--	--	35.01	--	39.42	--	61.02	--	61.02	--

Legend

- DRAINAGE STUDY AREA BOUNDARY
- DRAINAGE SUBAREA BOUNDARY
- LONGEST FLOW PATH
- DIRECTION OF FLOW
- 1A 0.11 SUBAREA DESIGNATION AREA (ACRES)
- (CB) EXISTING CATCH BASIN
- CB PROPOSED CATCH BASIN
- SD DIRECTION OF PIPE FLOW



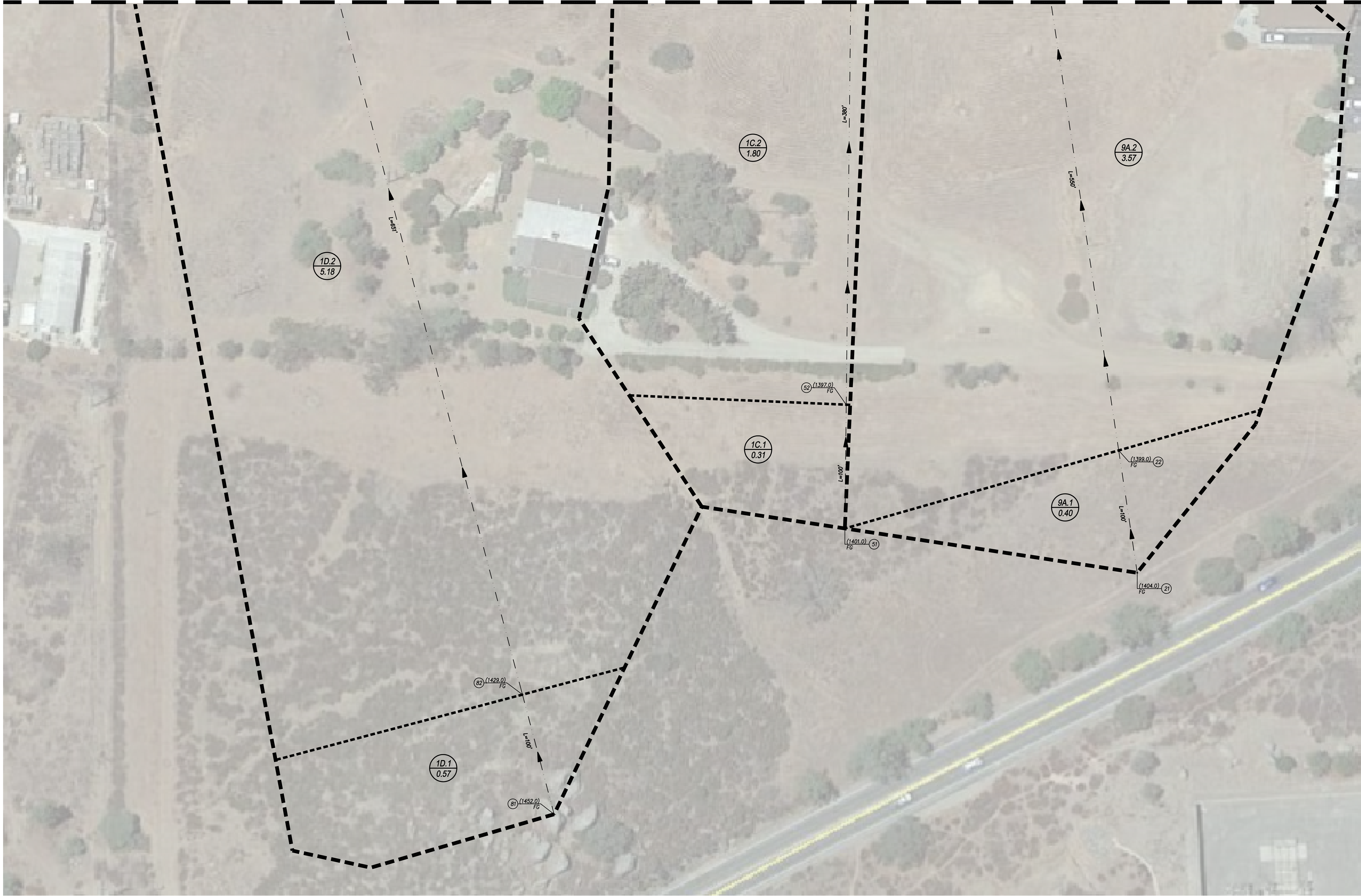
PROJECT: GS VALLEY EXPANSION
28435 LIZARD ROCKS ROAD
VALLEY CENTER, CALIFORNIA
DRAWING NAME: PROPOSED HYDROLOGY MAP

ISSUE: HYDROLOGY
DATE: 5/4/2021
CHECKED: JH DRAWN: JH
DRAWING FILE: 19132PRHM
PROJECT NO.: 19-02
SHEET NUMBER: 1
SCALE: AS SHOWN

160 S. Old Springs Road
Suite 210
Anheim Hills, CA 92808
714-665-6660
Engineering, Inc.
Civil Engineering/Land Surveying/Land Planning

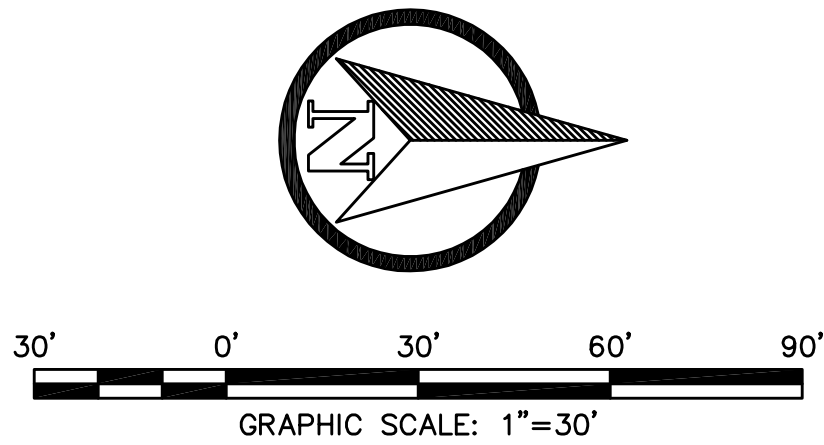
NOT FOR CONSTRUCTION

MATCH LINE - SEE SHEET 1



LEGEND:

- DRAINAGE STUDY AREA BOUNDARY
- DRAINAGE SUBAREA BOUNDARY
- LONGEST FLOW PATH
- ▲ DIRECTION OF FLOW
- 1A
0.11
SUBAREA DESIGNATION
AREA (ACRES)
- (CB) EXISTING CATCH BASIN
- CB PROPOSED CATCH BASIN
- SD DIRECTION OF PIPE FLOW



GS VALLEY EXPANSION
28435 LIZARD ROCKS ROAD
VALLEY CENTER, CALIFORNIA

PROPOSED HYDROLOGY MAP

PROJECT: 19132PRHM
DRAWING NAME: 19132PRHM

ISSUE: HYDROLOGY
DATE: 5/4/2021
CHECKED: JH DRAWN: JH
DRAWING FILE: 19132PRHM
PROJECT NO.: 19132
SHEET NUMBER:
2
OF 2 SHEETS
SCALE: AS SHOWN

ORC
Engineering, Inc.
Civil Engineering/Land Surveying/Land Planning

180 S. Old Springs Road
Suite 210
Anheim Hills, CA 92808
714-665-6660

NOT FOR CONSTRUCTION

APPENDIX D

Hydrology Calculations

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

DRC Engineering, Inc.
160 South Old Springs Road, Suite 210
Anaheim Hills, CA 92808
714-685-6860

***** DESCRIPTION OF STUDY *****
* 19-132 GS VALLEY EXPANSION (28435 LIZARD ROCKS ROAD) *
* EXISTING CONDITION *
* 10-YEAR STORM EVENT *

FILE NAME: 19132E10.DAT
TIME/DATE OF STUDY: 11:08 05/19/2021

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 10.00
6-HOUR DURATION PRECIPITATION (INCHES) = 2.700
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
===
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150
=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| DRAINAGE SUBAREA 1A.1 |
+-----+

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1384.70
DOWNSTREAM ELEVATION(FEET) = 1371.50
ELEVATION DIFFERENCE(FEET) = 13.20
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.899
SUBAREA RUNOFF(CFS) = 0.23

TOTAL AREA (ACRES) = 0.13 TOTAL RUNOFF (CFS) = 0.23

DRAINAGE SUBAREA 1A.2

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 305.00
REPRESENTATIVE CHANNEL SLOPE = 0.0510
CHANNEL BASE (FEET) = 40.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.841
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.83
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 0.80
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 6.32
Tc (MIN.) = 13.00
SUBAREA AREA (ACRES) = 0.73 SUBAREA RUNOFF (CFS) = 1.18
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 1.33

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.03 FLOW VELOCITY (FEET/SEC.) = 1.20
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 405.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 100.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 62.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.04
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.33
PIPE TRAVEL TIME (MIN.) = 0.26 Tc (MIN.) = 13.26
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 467.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 13.26
RAINFALL INTENSITY (INCH/HR) = 3.79
TOTAL STREAM AREA (ACRES) = 0.86
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.33

DRAINAGE SUBAREA 2A.1

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<


```
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1404.00
DOWNSTREAM ELEVATION(FEET) = 1399.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.422
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.082
SUBAREA RUNOFF(CFS) = 0.61
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 0.61
=====
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+-----+
| DRAINAGE SUBAREA 2A.2 |
+-----+
```

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*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 51
=====
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```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 465.00
REPRESENTATIVE CHANNEL SLOPE = 0.0590
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.576
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.54
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.27
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 6.10
Tc(MIN.) = 14.52
SUBAREA AREA(ACRES) = 3.55 SUBAREA RUNOFF(CFS) = 3.81
AREA-AVERAGE RUNOFF COEFFICIENT = 0.300
TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 4.24

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 1.45
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 565.00 FEET.
=====
```

```
+-----+
| DRAINAGE SUBAREA 2A.3 |
+-----+
```

```
*****
FLOW PROCESS FROM NODE 6.00 TO NODE 100.00 IS CODE = 51
=====
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 278.00
REPRESENTATIVE CHANNEL SLOPE = 0.0520
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.292
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3600
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 76
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.48
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.33
AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 1.99
Tc(MIN.) = 16.51
SUBAREA AREA(ACRES) = 0.41 SUBAREA RUNOFF(CFS) = 0.49
AREA-AVERAGE RUNOFF COEFFICIENT = 0.306
=====
```


TOTAL AREA (ACRES) = 4.4 PEAK FLOW RATE (CFS) = 4.39

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.09 FLOW VELOCITY (FEET/SEC.) = 2.32

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 100.00 = 843.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 16.51
RAINFALL INTENSITY (INCH/HR) = 3.29
TOTAL STREAM AREA (ACRES) = 4.36
PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.39

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.33	13.26	3.793	0.86
2	4.39	16.51	3.292	4.36

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	4.85	13.26	3.793
2	5.54	16.51	3.292

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 5.54 Tc (MIN.) = 16.51

TOTAL AREA (ACRES) = 5.2

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 100.00 = 843.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 35.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.73
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 5.54
PIPE TRAVEL TIME (MIN.) = 0.10 Tc (MIN.) = 16.62
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 101.00 = 878.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 16.62
RAINFALL INTENSITY (INCH/HR) = 3.28
TOTAL STREAM AREA (ACRES) = 5.22
PEAK FLOW RATE (CFS) AT CONFLUENCE = 5.54

+-----+
DRAINAGE SUBAREA 3A.1
+-----+

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1379.80
DOWNSTREAM ELEVATION (FEET) = 1368.60
ELEVATION DIFFERENCE (FEET) = 11.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
10 YEAR RAINFALL INTENSITY (INCH/ HOUR) = 5.899
SUBAREA RUNOFF (CFS) = 0.30
TOTAL AREA (ACRES) = 0.17 TOTAL RUNOFF (CFS) = 0.30

+-----+
| DRAINAGE SUBAREA 3A.2 |
+-----+

FLOW PROCESS FROM NODE 8.00 TO NODE 101.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 294.00
REPRESENTATIVE CHANNEL SLOPE = 0.0440
CHANNEL BASE (FEET) = 30.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/ HOUR) = 4.196
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.87
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.05
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 4.65
Tc (MIN.) = 11.34
SUBAREA AREA (ACRES) = 0.65 SUBAREA RUNOFF (CFS) = 1.15
AREA-AVERAGE RUNOFF COEFFICIENT = 0.395
TOTAL AREA (ACRES) = 0.8 PEAK FLOW RATE (CFS) = 1.36

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.10
LONGEST FLOWPATH FROM NODE 7.00 TO NODE 101.00 = 394.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 11.34
RAINFALL INTENSITY (INCH/HR) = 4.20
TOTAL STREAM AREA (ACRES) = 0.82
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.36

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/ HOUR)	AREA (ACRE)
1	5.54	16.62	3.279	5.22
2	1.36	11.34	4.196	0.82

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	5.69	11.34	4.196
2	6.60	16.62	3.279

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.60 Tc (MIN.) = 16.62
TOTAL AREA (ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 101.00 = 878.00 FEET.

DRAINAGE SUBAREA 1B

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4500
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 80
INITIAL SUBAREA FLOW-LENGTH (FEET) = 95.00
UPSTREAM ELEVATION (FEET) = 1369.30
DOWNSTREAM ELEVATION (FEET) = 1363.90
ELEVATION DIFFERENCE (FEET) = 5.40
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.390
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.073
SUBAREA RUNOFF (CFS) = 0.41
TOTAL AREA (ACRES) = 0.15 TOTAL RUNOFF (CFS) = 0.41

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 94.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.18
GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.41
PIPE TRAVEL TIME (MIN.) = 0.49 Tc (MIN.) = 6.88
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 189.00 FEET.

DRAINAGE SUBAREA 2B

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 5.789
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5475
SUBAREA AREA (ACRES) = 0.05 SUBAREA RUNOFF (CFS) = 0.24
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 0.63
TC (MIN.) = 6.88


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*****
FLOW PROCESS FROM NODE      23.00 TO NODE      200.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0640
FLOW LENGTH(FEET) =   13.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS   2.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   7.15
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       0.63
PIPE TRAVEL TIME(MIN.) =   0.03  Tc(MIN.) =   6.91
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      200.00 =      202.00 FEET.

*****
FLOW PROCESS FROM NODE      200.00 TO NODE      24.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0180
FLOW LENGTH(FEET) =   99.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS   4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   4.38
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       0.63
PIPE TRAVEL TIME(MIN.) =   0.38  Tc(MIN.) =   7.29
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      24.00 =      301.00 FEET.

*****
FLOW PROCESS FROM NODE      24.00 TO NODE      26.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =   12.00
REPRESENTATIVE CHANNEL SLOPE =  0.0170
CHANNEL BASE(FEET) =   6.00  "Z" FACTOR =  0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  0.50
CHANNEL FLOW THRU SUBAREA(CFS) =   0.63
FLOW VELOCITY(FEET/SEC.) =   1.88  FLOW DEPTH(FEET) =   0.06
TRAVEL TIME(MIN.) =   0.11  Tc(MIN.) =   7.40
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      26.00 =      313.00 FEET.

*****
FLOW PROCESS FROM NODE      26.00 TO NODE      26.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =   7.40
RAINFALL INTENSITY(INCH/HR) =   5.53
TOTAL STREAM AREA(ACRES) =   0.20
PEAK FLOW RATE(CFS) AT CONFLUENCE =   0.63

+-----+
| DRAINAGE SUBAREA 3B |
+-----+

*****
FLOW PROCESS FROM NODE      25.00 TO NODE      26.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96

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INITIAL SUBAREA FLOW-LENGTH(FEET) = 89.00
 UPSTREAM ELEVATION(FEET) = 1364.10
 DOWNSTREAM ELEVATION(FEET) = 1358.00
 ELEVATION DIFFERENCE(FEET) = 6.10
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.324
 10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.24
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.24

 FLOW PROCESS FROM NODE 26.00 TO NODE 26.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 2.32
 RAINFALL INTENSITY(INCH/HR) = 7.11
 TOTAL STREAM AREA(ACRES) = 0.04
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.24

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.63	7.40	5.526	0.20
2	0.24	2.32	7.114	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	0.44	2.32	7.114
2	0.82	7.40	5.526

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 0.82 Tc(MIN.) = 7.40
 TOTAL AREA(ACRES) = 0.2
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 26.00 = 313.00 FEET.

 FLOW PROCESS FROM NODE 26.00 TO NODE 201.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====
 REPRESENTATIVE SLOPE = 0.0200
 FLOW LENGTH(FEET) = 1.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.79
 GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.82
 PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 7.40
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 201.00 = 314.00 FEET.

+-----+
 | DRAINAGE SUBAREA 4B |
 +-----+

 FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"


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S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1368.40
DOWNSTREAM ELEVATION(FEET) = 1362.20
ELEVATION DIFFERENCE(FEET) = 6.20
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.683
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.84
TOTAL AREA(ACRES) = 0.14 TOTAL RUNOFF(CFS) = 0.84

*****
FLOW PROCESS FROM NODE 28.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.79
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.84
PIPE TRAVEL TIME(MIN.) = 0.28 Tc(MIN.) = 1.96
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 202.00 = 124.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 1.96
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.14
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.84

+-----+
| DRAINAGE SUBAREA 5B |
+-----+

*****
FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 54.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1362.00
ELEVATION DIFFERENCE(FEET) = 2.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.122
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.36
TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.36

*****
FLOW PROCESS FROM NODE 30.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0380
FLOW LENGTH(FEET) = 15.00 MANNING'S N = 0.012

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DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.70
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.36
PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 2.17
LONGEST FLOWPATH FROM NODE 29.00 TO NODE 202.00 = 69.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 2.17
RAINFALL INTENSITY (INCH/HR) = 7.11
TOTAL STREAM AREA (ACRES) = 0.06
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.36

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 0.84 1.96 7.114 0.14
2 0.36 2.17 7.114 0.06

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 1.16 1.96 7.114
2 1.20 2.17 7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE (CFS) = 1.20 Tc (MIN.) = 2.17
TOTAL AREA (ACRES) = 0.2
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 202.00 = 124.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.1100
FLOW LENGTH (FEET) = 54.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 9.82
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.20
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 2.27
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 203.00 = 178.00 FEET.

+-----+
| DRAINAGE SUBAREA 6B |
+-----+

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
10 YEAR RAINFALL INTENSITY (INCH/HR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96

```


AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA (ACRES) = 0.00 SUBAREA RUNOFF (CFS) = 0.03
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 1.22
TC (MIN.) = 2.27

DRAINAGE SUBAREA 7B.1

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 80.00
UPSTREAM ELEVATION (FEET) = 1361.10
DOWNSTREAM ELEVATION (FEET) = 1358.60
ELEVATION DIFFERENCE (FEET) = 2.50
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.863
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.48
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.48

DRAINAGE SUBAREA 7B.2

FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 36.00
REPRESENTATIVE CHANNEL SLOPE = 0.0360
CHANNEL BASE (FEET) = 8.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 0.50
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.54
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.01
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 0.30
Tc (MIN.) = 3.16
SUBAREA AREA (ACRES) = 0.02 SUBAREA RUNOFF (CFS) = 0.12
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 0.60

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 2.12
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 116.00 FEET.

DRAINAGE SUBAREA 1C.1

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1401.00
DOWNSTREAM ELEVATION (FEET) = 1397.00
ELEVATION DIFFERENCE (FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 9.072
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 4.844
SUBAREA RUNOFF (CFS) = 0.45
TOTAL AREA (ACRES) = 0.31 TOTAL RUNOFF (CFS) = 0.45

DRAINAGE SUBAREA 1C.2

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

CHANNEL LENGTH THRU SUBAREA (FEET) = 380.00
REPRESENTATIVE CHANNEL SLOPE = 0.0710
CHANNEL BASE (FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.576
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.80
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.16
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 5.45
Tc (MIN.) = 14.52
SUBAREA AREA (ACRES) = 1.80 SUBAREA RUNOFF (CFS) = 2.70
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA (ACRES) = 2.1 PEAK FLOW RATE (CFS) = 3.04

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.43
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 43.00 = 480.00 FEET.

DRAINAGE SUBAREA 1C.3

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

REPRESENTATIVE SLOPE = 0.0270
CHANNEL LENGTH THRU SUBAREA (FEET) = 371.00
"V" GUTTER WIDTH (FEET) = 5.00 GUTTER HIKE (FEET) = 0.090
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.325
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 3.64
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 3.57
AVERAGE FLOW DEPTH (FEET) = 0.18 FLOOD WIDTH (FEET) = 13.20
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.73 Tc (MIN.) = 16.26

SUBAREA AREA (ACRES) = 0.44 SUBAREA RUNOFF (CFS) = 1.21
AREA-AVERAGE RUNOFF COEFFICIENT = 0.476
TOTAL AREA (ACRES) = 2.5 PEAK FLOW RATE (CFS) = 4.04

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH (FEET) = 0.19 FLOOD WIDTH (FEET) = 13.98
FLOW VELOCITY (FEET/SEC.) = 3.58 DEPTH*VELOCITY (FT*FT/SEC) = 0.68
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 44.00 = 851.00 FEET.

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

REPRESENTATIVE SLOPE = 0.0130
FLOW LENGTH (FEET) = 166.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 7.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.27
GIVEN PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.04
PIPE TRAVEL TIME (MIN.) = 0.44 Tc (MIN.) = 16.70
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 1017.00 FEET.

+-----+
| DRAINAGE SUBAREA 2C |
+-----+

FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.268
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5445
SUBAREA AREA (ACRES) = 0.59 SUBAREA RUNOFF (CFS) = 1.62
TOTAL AREA (ACRES) = 3.1 TOTAL RUNOFF (CFS) = 5.59
TC (MIN.) = 16.70

FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

+-----+
| DRAINAGE SUBAREA 3C.1 |
+-----+

FLOW PROCESS FROM NODE 46.00 TO NODE 47.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 90.00
UPSTREAM ELEVATION (FEET) = 1370.50
DOWNSTREAM ELEVATION (FEET) = 1365.90
ELEVATION DIFFERENCE (FEET) = 4.60
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.280
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

SUBAREA RUNOFF(CFS) = 0.25
TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.25

DRAINAGE SUBAREA 3C.2

FLOW PROCESS FROM NODE 47.00 TO NODE 48.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
CHANNEL LENGTH THRU SUBAREA(FEET) = 297.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.20
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.54
AVERAGE FLOW DEPTH(FEET) = 0.13 FLOOD WIDTH(FEET) = 8.05
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.95 Tc(MIN.) = 4.23
SUBAREA AREA(ACRES) = 0.32 SUBAREA RUNOFF(CFS) = 1.91
AREA-AVERAGE RUNOFF COEFFICIENT = 0.843
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 2.16

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.16 FLOOD WIDTH(FEET) = 11.02
FLOW VELOCITY(FEET/SEC.) = 2.85 DEPTH*VELOCITY(FT*FT/SEC) = 0.46
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 48.00 = 387.00 FEET.

FLOW PROCESS FROM NODE 48.00 TO NODE 300.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
FLOW LENGTH(FEET) = 34.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.04
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.16
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 4.34
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 4.34
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.16

DRAINAGE SUBAREA 4C.1


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*****
FLOW PROCESS FROM NODE      49.00 TO NODE      50.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1370.50
DOWNSTREAM ELEVATION(FEET) = 1365.40
ELEVATION DIFFERENCE(FEET) = 5.10
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.571
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.19
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.19

+-----+
| DRAINAGE SUBAREA 4C.2 |
+-----+

*****
FLOW PROCESS FROM NODE      50.00 TO NODE      51.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 70.00
REPRESENTATIVE CHANNEL SLOPE = 0.0900
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.22
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FT/SEC.) = 2.60
AVERAGE FLOW DEPTH(FT) = 0.03 TRAVEL TIME(MIN.) = 0.45
Tc(MIN.) = 2.02
SUBAREA AREA(ACRES) = 0.01 SUBAREA RUNOFF(CFS) = 0.06
AREA-AVERAGE RUNOFF COEFFICIENT = 0.870
TOTAL AREA(ACRES) = 0.0 PEAK FLOW RATE(CFS) = 0.25

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FT) = 0.03 FLOW VELOCITY(FT/SEC.) = 2.97
LONGEST FLOWPATH FROM NODE 49.00 TO NODE 51.00 = 130.00 FEET.

*****
FLOW PROCESS FROM NODE      51.00 TO NODE      300.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FT) = 27.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.1 INCHES
PIPE-FLOW VELOCITY(FT/SEC.) = 2.65
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.25
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 2.19
LONGEST FLOWPATH FROM NODE 49.00 TO NODE 300.00 = 157.00 FEET.

*****
FLOW PROCESS FROM NODE      300.00 TO NODE      300.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

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=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.19
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.25

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HR)      (ACRE)
1            2.16      4.34      7.114          0.36
2            0.25      2.19      7.114          0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HR)
1            1.34      2.19      7.114
2            2.41      4.34      7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 2.41 Tc(MIN.) = 4.34
TOTAL AREA(ACRES) = 0.4
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 300.00 = 421.00 FEET.

*****
FLOW PROCESS FROM NODE 300.00 TO NODE 45.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 5.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.63
GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.41
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 4.50
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 45.00 = 485.00 FEET.

*****
FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HR)      (ACRE)
1            2.41      4.50      7.114          0.40
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 45.00 = 485.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HR)      (ACRE)
1            5.59      16.70      3.268          3.14
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 1017.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HR)
1            3.91      4.50      7.114
2            6.69      16.70      3.268

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 6.69 Tc(MIN.) = 16.70
TOTAL AREA(ACRES) = 3.5

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*****
FLOW PROCESS FROM NODE      45.00 TO NODE      45.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      45.00 TO NODE      52.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 173.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.50
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.69
PIPE TRAVEL TIME(MIN.) = 0.38 Tc(MIN.) = 17.08
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 52.00 = 1190.00 FEET.

+-----+
| DRAINAGE SUBAREA 5C |
+-----+

*****
FLOW PROCESS FROM NODE      52.00 TO NODE      52.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.221
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6075
SUBAREA AREA(ACRES) = 0.44 SUBAREA RUNOFF(CFS) = 1.19
TOTAL AREA(ACRES) = 4.0 TOTAL RUNOFF(CFS) = 7.79
TC(MIN.) = 17.08

*****
FLOW PROCESS FROM NODE      52.00 TO NODE      56.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 84.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 9.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.67
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.79
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 17.29
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 56.00 = 1274.00 FEET.

*****
FLOW PROCESS FROM NODE      56.00 TO NODE      56.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.29
RAINFALL INTENSITY(INCH/HR) = 3.20
TOTAL STREAM AREA(ACRES) = 3.98
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.79

+-----+
| DRAINAGE SUBAREA 6C.1 |
+-----+

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+-----+
*****
FLOW PROCESS FROM NODE      53.00 TO NODE      54.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
INITIAL SUBAREA FLOW-LENGTH(FEET) =      53.00
UPSTREAM ELEVATION(FEET) =    1359.30
DOWNSTREAM ELEVATION(FEET) =    1358.70
ELEVATION DIFFERENCE(FEET) =        0.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      3.269
  10 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =        0.66
TOTAL AREA(ACRES) =        0.11  TOTAL RUNOFF(CFS) =        0.66

+-----+
| DRAINAGE SUBAREA 6C.2 |
+-----+
*****
FLOW PROCESS FROM NODE      54.00 TO NODE      55.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =    188.00
REPRESENTATIVE CHANNEL SLOPE =    0.0110
CHANNEL BASE(FEET) =    12.00  "Z" FACTOR =    0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =    0.50
  10 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.097
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =        1.70
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    1.79
AVERAGE FLOW DEPTH(FEET) =    0.08  TRAVEL TIME(MIN.) =    1.75
Tc(MIN.) =    5.02
SUBAREA AREA(ACRES) =        0.35  SUBAREA RUNOFF(CFS) =    2.09
AREA-AVERAGE RUNOFF COEFFICIENT =    0.840
TOTAL AREA(ACRES) =        0.5  PEAK FLOW RATE(CFS) =        2.74

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =    0.10  FLOW VELOCITY(FEET/SEC.) =    2.24
LONGEST FLOWPATH FROM NODE      53.00 TO NODE      55.00 =    241.00 FEET.

*****
FLOW PROCESS FROM NODE      55.00 TO NODE      56.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =    0.0150
FLOW LENGTH(FEET) =    105.00  MANNING'S N =    0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS    6.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    6.00
GIVEN PIPE DIAMETER(INCH) =    12.00  NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =        2.74
PIPE TRAVEL TIME(MIN.) =    0.29  Tc(MIN.) =    5.31
LONGEST FLOWPATH FROM NODE      53.00 TO NODE      56.00 =    346.00 FEET.

+-----+
| DRAINAGE SUBAREA 7C |
+-----+

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+-----+
*****
FLOW PROCESS FROM NODE      56.00 TO NODE      56.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
    10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.843
    LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
    SOIL CLASSIFICATION IS "C"
    S.C.S. CURVE NUMBER (AMC II) = 96
    AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
    SUBAREA AREA(ACRES) = 0.25 SUBAREA RUNOFF(CFS) = 1.44
    TOTAL AREA(ACRES) = 0.7 TOTAL RUNOFF(CFS) = 4.08
    TC(MIN.) = 5.31
*****
FLOW PROCESS FROM NODE      56.00 TO NODE      56.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.31
RAINFALL INTENSITY(INCH/HR) = 6.84
TOTAL STREAM AREA(ACRES) = 0.71
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.08

** CONFLUENCE DATA **
STREAM    RUNOFF      Tc      INTENSITY      AREA
NUMBER    (CFS)      (MIN.)  (INCH/HOUR)    (ACRE)
    1      7.79     17.29      3.196         3.98
    2      4.08      5.31      6.843         0.71

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF      Tc      INTENSITY
NUMBER    (CFS)      (MIN.)  (INCH/HOUR)
    1      6.47      5.31      6.843
    2      9.69     17.29      3.196

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 9.69 Tc(MIN.) = 17.29
TOTAL AREA(ACRES) = 4.7
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 56.00 = 1274.00 FEET.
*****
FLOW PROCESS FROM NODE      56.00 TO NODE      57.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.06
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.69
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 17.47
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 57.00 = 1350.00 FEET.
+-----+
| DRAINAGE SUBAREA 8C |
+-----+

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*****
FLOW PROCESS FROM NODE      57.00 TO NODE      57.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.174
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6550
SUBAREA AREA(ACRES) = 0.33 SUBAREA RUNOFF(CFS) = 0.87
TOTAL AREA(ACRES) = 5.0 TOTAL RUNOFF(CFS) = 10.44
TC(MIN.) = 17.47

*****
FLOW PROCESS FROM NODE      57.00 TO NODE      301.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0130
FLOW LENGTH(FEET) = 23.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.94
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.44
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 17.52
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 301.00 = 1373.00 FEET.

+-----+
| DRAINAGE SUBAREA 9C |
+-----+

*****
FLOW PROCESS FROM NODE      301.00 TO NODE      301.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.169
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .3400
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 51
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6470
SUBAREA AREA(ACRES) = 0.13 SUBAREA RUNOFF(CFS) = 0.14
TOTAL AREA(ACRES) = 5.2 TOTAL RUNOFF(CFS) = 10.56
TC(MIN.) = 17.52

+-----+
| DRAINAGE SUBAREA 1D.1 |
+-----+

*****
FLOW PROCESS FROM NODE      61.00 TO NODE      62.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1452.00
DOWNSTREAM ELEVATION(FEET) = 1429.00
ELEVATION DIFFERENCE(FEET) = 23.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.899
SUBAREA RUNOFF(CFS) = 1.01

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TOTAL AREA (ACRES) = 0.57 TOTAL RUNOFF (CFS) = 1.01

DRAINAGE SUBAREA 1D.2

FLOW PROCESS FROM NODE 62.00 TO NODE 63.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 651.00
REPRESENTATIVE CHANNEL SLOPE = 0.0940
CHANNEL BASE (FEET) = 100.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.508
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .2700
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 45
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 3.63
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.31
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 8.27
Tc (MIN.) = 14.96
SUBAREA AREA (ACRES) = 5.18 SUBAREA RUNOFF (CFS) = 4.91
AREA-AVERAGE RUNOFF COEFFICIENT = 0.273
TOTAL AREA (ACRES) = 5.8 PEAK FLOW RATE (CFS) = 5.51

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.56
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 63.00 = 751.00 FEET.

DRAINAGE SUBAREA 1D.3

FLOW PROCESS FROM NODE 63.00 TO NODE 64.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0560
CHANNEL LENGTH THRU SUBAREA (FEET) = 181.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.800
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.470
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4800
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 81
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 5.59
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 11.71
AVERAGE FLOW DEPTH (FEET) = 0.80 FLOOD WIDTH (FEET) = 3.00
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.26 Tc (MIN.) = 15.22
SUBAREA AREA (ACRES) = 0.10 SUBAREA RUNOFF (CFS) = 0.17
AREA-AVERAGE RUNOFF COEFFICIENT = 0.277
TOTAL AREA (ACRES) = 5.8 PEAK FLOW RATE (CFS) = 5.61

NOTE: TRAVEL TIME ESTIMATES BASED ON NORMAL DEPTH
IN A FLOWING-FULL GUTTER (NORMAL DEPTH = GUTTER HIKE)

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.80 FLOOD WIDTH (FEET) = 3.00
FLOW VELOCITY (FEET/SEC.) = 11.71 DEPTH*VELOCITY (FT*FT/SEC) = 9.37
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 64.00 = 932.00 FEET.

FLOW PROCESS FROM NODE 64.00 TO NODE 66.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 41.00
REPRESENTATIVE CHANNEL SLOPE = 0.0120
CHANNEL BASE (FEET) = 4.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 1.00
CHANNEL FLOW THRU SUBAREA (CFS) = 5.61
FLOW VELOCITY (FEET/SEC.) = 4.53 FLOW DEPTH (FEET) = 0.30
TRAVEL TIME (MIN.) = 0.15 Tc (MIN.) = 15.37
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 66.00 = 973.00 FEET.

FLOW PROCESS FROM NODE 66.00 TO NODE 66.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 15.37
RAINFALL INTENSITY (INCH/HR) = 3.45
TOTAL STREAM AREA (ACRES) = 5.85
PEAK FLOW RATE (CFS) AT CONFLUENCE = 5.61

+-----+
| DRAINAGE SUBAREA 2D |
+-----+

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 22.00
UPSTREAM ELEVATION (FEET) = 1358.00
DOWNSTREAM ELEVATION (FEET) = 1357.00
ELEVATION DIFFERENCE (FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 4.078
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.26
TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 0.26

FLOW PROCESS FROM NODE 66.00 TO NODE 66.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 4.08
RAINFALL INTENSITY (INCH/HR) = 7.11
TOTAL STREAM AREA (ACRES) = 0.12
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.26

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	5.61	15.37	3.448	5.85
2	0.26	4.08	7.114	0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.75	4.08	7.114
2	5.74	15.37	3.448

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 5.74 Tc(MIN.) = 15.37
TOTAL AREA(ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 66.00 = 973.00 FEET.

DRAINAGE SUBAREA 1E

FLOW PROCESS FROM NODE 71.00 TO NODE 72.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .2000
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(FEET) = 86.00
UPSTREAM ELEVATION(FEET) = 1358.00
DOWNSTREAM ELEVATION(FEET) = 1351.10
ELEVATION DIFFERENCE(FEET) = 6.90
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.505
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.474
SUBAREA RUNOFF(CFS) = 0.13
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.13

=====

END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 0.1 TC(MIN.) = 7.50
PEAK FLOW RATE(CFS) = 0.13

=====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
* 19-132 GS VALLEY EXPANSION (28435 LIZARD ROCKS ROAD) *
* EXISTING CONDITION *
* 25-YEAR STORM EVENT *

FILE NAME: 19132E25.DAT
TIME/DATE OF STUDY: 11:09 05/19/2021

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 25.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
===
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150
=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| DRAINAGE SUBAREA 1A.1 |
+-----+

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(Feet) = 100.00
UPSTREAM ELEVATION(Feet) = 1384.70
DOWNSTREAM ELEVATION(Feet) = 1371.50
ELEVATION DIFFERENCE(Feet) = 13.20
SUBAREA OVERLAND TIME OF FLOW(Min.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.554
SUBAREA RUNOFF(CFS) = 0.26

TOTAL AREA (ACRES) = 0.13 TOTAL RUNOFF (CFS) = 0.26

DRAINAGE SUBAREA 1A.2

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 305.00
REPRESENTATIVE CHANNEL SLOPE = 0.0510
CHANNEL BASE (FEET) = 40.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 4.437
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.94
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 0.91
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 5.56
Tc (MIN.) = 12.24
SUBAREA AREA (ACRES) = 0.73 SUBAREA RUNOFF (CFS) = 1.36
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 1.53

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.03 FLOW VELOCITY (FEET/SEC.) = 1.15
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 405.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 100.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 62.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 4.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.22
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.53
PIPE TRAVEL TIME (MIN.) = 0.24 Tc (MIN.) = 12.49
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 467.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 12.49
RAINFALL INTENSITY (INCH/HR) = 4.38
TOTAL STREAM AREA (ACRES) = 0.86
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.53

DRAINAGE SUBAREA 2A.1

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<


```
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1404.00
DOWNSTREAM ELEVATION(FEET) = 1399.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.422
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.647
SUBAREA RUNOFF(CFS) = 0.68
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 0.68
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+-----+
| DRAINAGE SUBAREA 2A.2 |
+-----+
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*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 51
=====
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 465.00
REPRESENTATIVE CHANNEL SLOPE = 0.0590
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.039
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.86
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.35
AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 5.74
Tc(MIN.) = 14.16
SUBAREA AREA(ACRES) = 3.55 SUBAREA RUNOFF(CFS) = 4.30
AREA-AVERAGE RUNOFF COEFFICIENT = 0.300
TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 4.79
=====
```

```
END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 1.64
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 565.00 FEET.
```

```
+-----+
| DRAINAGE SUBAREA 2A.3 |
+-----+
```

```
*****
FLOW PROCESS FROM NODE 6.00 TO NODE 100.00 IS CODE = 51
=====
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 278.00
REPRESENTATIVE CHANNEL SLOPE = 0.0520
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.729
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3600
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 76
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.06
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.48
AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 1.87
Tc(MIN.) = 16.03
SUBAREA AREA(ACRES) = 0.41 SUBAREA RUNOFF(CFS) = 0.55
AREA-AVERAGE RUNOFF COEFFICIENT = 0.306
=====
```


TOTAL AREA (ACRES) = 4.4 PEAK FLOW RATE (CFS) = 4.97

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.10 FLOW VELOCITY (FEET/SEC.) = 2.43

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 100.00 = 843.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 16.03
RAINFALL INTENSITY (INCH/HR) = 3.73
TOTAL STREAM AREA (ACRES) = 4.36
PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.97

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.53	12.49	4.380	0.86
2	4.97	16.03	3.729	4.36

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	5.40	12.49	4.380
2	6.27	16.03	3.729

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.27 Tc (MIN.) = 16.03

TOTAL AREA (ACRES) = 5.2

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 100.00 = 843.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 35.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.93
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 6.27
PIPE TRAVEL TIME (MIN.) = 0.10 Tc (MIN.) = 16.12
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 101.00 = 878.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 16.12
RAINFALL INTENSITY (INCH/HR) = 3.71
TOTAL STREAM AREA (ACRES) = 5.22
PEAK FLOW RATE (CFS) AT CONFLUENCE = 6.27

+-----+
DRAINAGE SUBAREA 3A.1
+-----+

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1379.80
DOWNSTREAM ELEVATION (FEET) = 1368.60
ELEVATION DIFFERENCE (FEET) = 11.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.554
SUBAREA RUNOFF (CFS) = 0.33
TOTAL AREA (ACRES) = 0.17 TOTAL RUNOFF (CFS) = 0.33

+-----+
| DRAINAGE SUBAREA 3A.2 |
+-----+

FLOW PROCESS FROM NODE 8.00 TO NODE 101.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 294.00
REPRESENTATIVE CHANNEL SLOPE = 0.0440
CHANNEL BASE (FEET) = 30.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 4.540
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.96
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 0.96
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 5.13
Tc (MIN.) = 11.81
SUBAREA AREA (ACRES) = 0.65 SUBAREA RUNOFF (CFS) = 1.24
AREA-AVERAGE RUNOFF COEFFICIENT = 0.395
TOTAL AREA (ACRES) = 0.8 PEAK FLOW RATE (CFS) = 1.47

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.20
LONGEST FLOWPATH FROM NODE 7.00 TO NODE 101.00 = 394.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 11.81
RAINFALL INTENSITY (INCH/HR) = 4.54
TOTAL STREAM AREA (ACRES) = 0.82
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.47

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.27	16.12	3.714	5.22
2	1.47	11.81	4.540	0.82

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	T _c (MIN.)	INTENSITY (INCH/HOUR)
1	6.60	11.81	4.540
2	7.48	16.12	3.714

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.48 T_c (MIN.) = 16.12
TOTAL AREA (ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 101.00 = 878.00 FEET.

DRAINAGE SUBAREA 1B

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4500
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 80
INITIAL SUBAREA FLOW-LENGTH (FEET) = 95.00
UPSTREAM ELEVATION (FEET) = 1369.30
DOWNSTREAM ELEVATION (FEET) = 1363.90
ELEVATION DIFFERENCE (FEET) = 5.40
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.390
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.747
SUBAREA RUNOFF (CFS) = 0.46
TOTAL AREA (ACRES) = 0.15 TOTAL RUNOFF (CFS) = 0.46

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 94.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 6.0 INCH PIPE IS 4.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.28
GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.46
PIPE TRAVEL TIME (MIN.) = 0.48 T_c (MIN.) = 6.87
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 189.00 FEET.

DRAINAGE SUBAREA 2B

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.440
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5475
SUBAREA AREA (ACRES) = 0.05 SUBAREA RUNOFF (CFS) = 0.27
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 0.71
TC (MIN.) = 6.87


```

*****
FLOW PROCESS FROM NODE      23.00 TO NODE      200.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0640
FLOW LENGTH(FEET) =   13.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS   2.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   7.40
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       0.71
PIPE TRAVEL TIME(MIN.) =   0.03  Tc(MIN.) =   6.90
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      200.00 =      202.00 FEET.

*****
FLOW PROCESS FROM NODE      200.00 TO NODE      24.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0180
FLOW LENGTH(FEET) =   99.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS   4.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   4.48
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       0.71
PIPE TRAVEL TIME(MIN.) =   0.37  Tc(MIN.) =   7.27
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      24.00 =      301.00 FEET.

*****
FLOW PROCESS FROM NODE      24.00 TO NODE      26.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =   12.00
REPRESENTATIVE CHANNEL SLOPE =  0.0170
CHANNEL BASE(FEET) =   6.00  "Z" FACTOR =  0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  0.50
CHANNEL FLOW THRU SUBAREA(CFS) =   0.71
FLOW VELOCITY(FEET/SEC.) =   2.02  FLOW DEPTH(FEET) =   0.06
TRAVEL TIME(MIN.) =   0.10  Tc(MIN.) =   7.36
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      26.00 =      313.00 FEET.

*****
FLOW PROCESS FROM NODE      26.00 TO NODE      26.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =   7.36
RAINFALL INTENSITY(INCH/HR) =   6.16
TOTAL STREAM AREA(ACRES) =   0.20
PEAK FLOW RATE(CFS) AT CONFLUENCE =   0.71

+-----+
| DRAINAGE SUBAREA 3B |
+-----+

*****
FLOW PROCESS FROM NODE      25.00 TO NODE      26.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96

```


INITIAL SUBAREA FLOW-LENGTH(FEET) = 89.00
 UPSTREAM ELEVATION(FEET) = 1364.10
 DOWNSTREAM ELEVATION(FEET) = 1358.00
 ELEVATION DIFFERENCE(FEET) = 6.10
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.324
 25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.27
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.27

 FLOW PROCESS FROM NODE 26.00 TO NODE 26.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 2.32
 RAINFALL INTENSITY(INCH/HR) = 7.90
 TOTAL STREAM AREA(ACRES) = 0.04
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.27

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.71	7.36	6.157	0.20
2	0.27	2.32	7.904	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	0.49	2.32	7.904
2	0.91	7.36	6.157

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 0.91 Tc(MIN.) = 7.36
 TOTAL AREA(ACRES) = 0.2
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 26.00 = 313.00 FEET.

 FLOW PROCESS FROM NODE 26.00 TO NODE 201.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====
 REPRESENTATIVE SLOPE = 0.0200
 FLOW LENGTH(FEET) = 1.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.98
 GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.91
 PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 7.37
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 201.00 = 314.00 FEET.

+-----+
 | DRAINAGE SUBAREA 4B |
 +-----+

 FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"


```

S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1368.40
DOWNSTREAM ELEVATION(FEET) = 1362.20
ELEVATION DIFFERENCE(FEET) = 6.20
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.683
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.93
TOTAL AREA(ACRES) = 0.14 TOTAL RUNOFF(CFS) = 0.93

*****
FLOW PROCESS FROM NODE 28.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.90
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.93
PIPE TRAVEL TIME(MIN.) = 0.27 Tc(MIN.) = 1.96
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 202.00 = 124.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 1.96
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.14
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.93

+-----+
| DRAINAGE SUBAREA 5B |
+-----+

*****
FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 54.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1362.00
ELEVATION DIFFERENCE(FEET) = 2.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.122
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.40
TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.40

*****
FLOW PROCESS FROM NODE 30.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0380
FLOW LENGTH(FEET) = 15.00 MANNING'S N = 0.012

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DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.92
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.40
PIPE TRAVEL TIME (MIN.) = 0.05 Tc (MIN.) = 2.17
LONGEST FLOWPATH FROM NODE 29.00 TO NODE 202.00 = 69.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 2.17
RAINFALL INTENSITY (INCH/HR) = 7.90
TOTAL STREAM AREA (ACRES) = 0.06
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.40

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HR) (ACRE)
1 0.93 1.96 7.904 0.14
2 0.40 2.17 7.904 0.06

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HR)
1 1.29 1.96 7.904
2 1.33 2.17 7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE (CFS) = 1.33 Tc (MIN.) = 2.17
TOTAL AREA (ACRES) = 0.2
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 202.00 = 124.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.1100
FLOW LENGTH (FEET) = 54.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 10.16
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.33
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 2.26
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 203.00 = 178.00 FEET.

+-----+
| DRAINAGE SUBAREA 6B |
+-----+

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
25 YEAR RAINFALL INTENSITY (INCH/HR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96

```


AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA (ACRES) = 0.00 SUBAREA RUNOFF (CFS) = 0.03
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 1.36
TC (MIN.) = 2.26

DRAINAGE SUBAREA 7B.1

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 80.00
UPSTREAM ELEVATION (FEET) = 1361.10
DOWNSTREAM ELEVATION (FEET) = 1358.60
ELEVATION DIFFERENCE (FEET) = 2.50
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.863
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.53
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.53

DRAINAGE SUBAREA 7B.2

FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 36.00
REPRESENTATIVE CHANNEL SLOPE = 0.0360
CHANNEL BASE (FEET) = 8.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.60
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.12
AVERAGE FLOW DEPTH (FEET) = 0.04 TRAVEL TIME (MIN.) = 0.28
Tc (MIN.) = 3.15
SUBAREA AREA (ACRES) = 0.02 SUBAREA RUNOFF (CFS) = 0.13
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 0.66

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 2.02
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 116.00 FEET.

DRAINAGE SUBAREA 1C.1

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1401.00
DOWNSTREAM ELEVATION (FEET) = 1397.00
ELEVATION DIFFERENCE (FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 9.072
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 5.382
SUBAREA RUNOFF (CFS) = 0.50
TOTAL AREA (ACRES) = 0.31 TOTAL RUNOFF (CFS) = 0.50

DRAINAGE SUBAREA 1C.2

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

CHANNEL LENGTH THRU SUBAREA (FEET) = 380.00
REPRESENTATIVE CHANNEL SLOPE = 0.0710
CHANNEL BASE (FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 4.012
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 2.01
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.21
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 5.24
Tc (MIN.) = 14.31
SUBAREA AREA (ACRES) = 1.80 SUBAREA RUNOFF (CFS) = 3.03
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA (ACRES) = 2.1 PEAK FLOW RATE (CFS) = 3.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.61
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 43.00 = 480.00 FEET.

DRAINAGE SUBAREA 1C.3

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

REPRESENTATIVE SLOPE = 0.0270
CHANNEL LENGTH THRU SUBAREA (FEET) = 371.00
"V" GUTTER WIDTH (FEET) = 5.00 GUTTER HIKE (FEET) = 0.090
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.731
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 4.09
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 3.62
AVERAGE FLOW DEPTH (FEET) = 0.19 FLOOD WIDTH (FEET) = 13.98
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.71 Tc (MIN.) = 16.01

SUBAREA AREA (ACRES) = 0.44 SUBAREA RUNOFF (CFS) = 1.36
AREA-AVERAGE RUNOFF COEFFICIENT = 0.476
TOTAL AREA (ACRES) = 2.5 PEAK FLOW RATE (CFS) = 4.53

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH (FEET) = 0.20 FLOOD WIDTH (FEET) = 14.77
FLOW VELOCITY (FEET/SEC.) = 3.65 DEPTH*VELOCITY (FT*FT/SEC) = 0.72
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 44.00 = 851.00 FEET.

FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0130
FLOW LENGTH (FEET) = 166.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.44
GIVEN PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.53
PIPE TRAVEL TIME (MIN.) = 0.43 Tc (MIN.) = 16.44
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 1017.00 FEET.

+-----+
| DRAINAGE SUBAREA 2C |
+-----+

FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.668
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5445
SUBAREA AREA (ACRES) = 0.59 SUBAREA RUNOFF (CFS) = 1.82
TOTAL AREA (ACRES) = 3.1 TOTAL RUNOFF (CFS) = 6.27
Tc (MIN.) = 16.44

FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

+-----+
| DRAINAGE SUBAREA 3C.1 |
+-----+

FLOW PROCESS FROM NODE 46.00 TO NODE 47.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 90.00
UPSTREAM ELEVATION (FEET) = 1370.50
DOWNSTREAM ELEVATION (FEET) = 1365.90
ELEVATION DIFFERENCE (FEET) = 4.60
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.280
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

SUBAREA RUNOFF(CFS) = 0.28
TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.28

DRAINAGE SUBAREA 3C.2

FLOW PROCESS FROM NODE 47.00 TO NODE 48.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
CHANNEL LENGTH THRU SUBAREA(FEET) = 297.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.34
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.61
AVERAGE FLOW DEPTH(FEET) = 0.14 FLOOD WIDTH(FEET) = 8.52
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.90 Tc(MIN.) = 4.18
SUBAREA AREA(ACRES) = 0.32 SUBAREA RUNOFF(CFS) = 2.12
AREA-AVERAGE RUNOFF COEFFICIENT = 0.843
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 2.40

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.17 FLOOD WIDTH(FEET) = 11.64
FLOW VELOCITY(FEET/SEC.) = 2.90 DEPTH*VELOCITY(FT*FT/SEC) = 0.48
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 48.00 = 387.00 FEET.

FLOW PROCESS FROM NODE 48.00 TO NODE 300.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
FLOW LENGTH(FEET) = 34.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.16
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.40
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 4.29
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 4.29
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.40

DRAINAGE SUBAREA 4C.1


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*****
FLOW PROCESS FROM NODE      49.00 TO NODE      50.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1370.50
DOWNSTREAM ELEVATION(FEET) = 1365.40
ELEVATION DIFFERENCE(FEET) = 5.10
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.571
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.21
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.21

+-----+
| DRAINAGE SUBAREA 4C.2 |
+-----+

*****
FLOW PROCESS FROM NODE      50.00 TO NODE      51.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 70.00
REPRESENTATIVE CHANNEL SLOPE = 0.0900
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.24
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.89
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 0.40
Tc(MIN.) = 1.97
SUBAREA AREA(ACRES) = 0.01 SUBAREA RUNOFF(CFS) = 0.07
AREA-AVERAGE RUNOFF COEFFICIENT = 0.870
TOTAL AREA(ACRES) = 0.0 PEAK FLOW RATE(CFS) = 0.28

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.03 FLOW VELOCITY(FEET/SEC.) = 2.74
LONGEST FLOWPATH FROM NODE 49.00 TO NODE 51.00 = 130.00 FEET.

*****
FLOW PROCESS FROM NODE      51.00 TO NODE      300.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 27.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 2.72
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.28
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 2.14
LONGEST FLOWPATH FROM NODE 49.00 TO NODE 300.00 = 157.00 FEET.

*****
FLOW PROCESS FROM NODE      300.00 TO NODE      300.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

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=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.14
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.28

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1            2.40      4.29      7.904      0.36
2            0.28      2.14      7.904      0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
1            1.47      2.14      7.904
2            2.67      4.29      7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 2.67 Tc(MIN.) = 4.29
TOTAL AREA(ACRES) = 0.4
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 300.00 = 421.00 FEET.

*****
FLOW PROCESS FROM NODE 300.00 TO NODE 45.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 5.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.83
GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.67
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 4.44
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 45.00 = 485.00 FEET.

*****
FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1            2.67      4.44      7.904      0.40
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 45.00 = 485.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1            6.27      16.44      3.668      3.14
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 1017.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
1            4.37      4.44      7.904
2            7.51      16.44      3.668

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 7.51 Tc(MIN.) = 16.44
TOTAL AREA(ACRES) = 3.5

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*****
FLOW PROCESS FROM NODE      45.00 TO NODE      45.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      45.00 TO NODE      52.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 173.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.72
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.51
PIPE TRAVEL TIME(MIN.) = 0.37 Tc(MIN.) = 16.82
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 52.00 = 1190.00 FEET.

+-----+
| DRAINAGE SUBAREA 5C |
+-----+

*****
FLOW PROCESS FROM NODE      52.00 TO NODE      52.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.615
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6075
SUBAREA AREA(ACRES) = 0.44 SUBAREA RUNOFF(CFS) = 1.34
TOTAL AREA(ACRES) = 4.0 TOTAL RUNOFF(CFS) = 8.74
TC(MIN.) = 16.82

*****
FLOW PROCESS FROM NODE      52.00 TO NODE      56.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 84.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.88
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 8.74
PIPE TRAVEL TIME(MIN.) = 0.20 Tc(MIN.) = 17.02
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 56.00 = 1274.00 FEET.

*****
FLOW PROCESS FROM NODE      56.00 TO NODE      56.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.02
RAINFALL INTENSITY(INCH/HR) = 3.59
TOTAL STREAM AREA(ACRES) = 3.98
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.74

+-----+
| DRAINAGE SUBAREA 6C.1 |
+-----+

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+-----+
*****
FLOW PROCESS FROM NODE      53.00 TO NODE      54.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
INITIAL SUBAREA FLOW-LENGTH(FEET) =      53.00
UPSTREAM ELEVATION(FEET) =    1359.30
DOWNSTREAM ELEVATION(FEET) =    1358.70
ELEVATION DIFFERENCE(FEET) =        0.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      3.269
  25 YEAR RAINFALL INTENSITY(INCH/HOUR) =  7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =        0.73
TOTAL AREA (ACRES) =        0.11  TOTAL RUNOFF (CFS) =        0.73

+-----+
| DRAINAGE SUBAREA 6C.2 |
+-----+
*****
FLOW PROCESS FROM NODE      54.00 TO NODE      55.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =    188.00
REPRESENTATIVE CHANNEL SLOPE =  0.0110
CHANNEL BASE(FEET) =    12.00  "Z" FACTOR =    0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =    0.50
  25 YEAR RAINFALL INTENSITY(INCH/HOUR) =  7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =        1.89
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    1.95
AVERAGE FLOW DEPTH(FEET) =    0.08  TRAVEL TIME(MIN.) =    1.61
Tc(MIN.) =    4.88
SUBAREA AREA(ACRES) =        0.35  SUBAREA RUNOFF(CFS) =    2.32
AREA-AVERAGE RUNOFF COEFFICIENT =  0.840
TOTAL AREA(ACRES) =        0.5  PEAK FLOW RATE(CFS) =        3.05

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.11  FLOW VELOCITY(FEET/SEC.) =    2.32
LONGEST FLOWPATH FROM NODE      53.00 TO NODE      55.00 =    241.00 FEET.

*****
FLOW PROCESS FROM NODE      55.00 TO NODE      56.00 IS CODE =  41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE =  0.0150
FLOW LENGTH(FEET) =    105.00  MANNING'S N =  0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS  7.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    6.15
GIVEN PIPE DIAMETER(INCH) =    12.00  NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =        3.05
PIPE TRAVEL TIME(MIN.) =    0.28  Tc(MIN.) =    5.16
LONGEST FLOWPATH FROM NODE      53.00 TO NODE      56.00 =    346.00 FEET.

+-----+
| DRAINAGE SUBAREA 7C |
+-----+

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FLOW PROCESS FROM NODE 56.00 TO NODE 56.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.741
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) = 0.25 SUBAREA RUNOFF(CFS) = 1.63
TOTAL AREA(ACRES) = 0.7 TOTAL RUNOFF(CFS) = 4.62
TC(MIN.) = 5.16

FLOW PROCESS FROM NODE 56.00 TO NODE 56.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.16
RAINFALL INTENSITY(INCH/HR) = 7.74
TOTAL STREAM AREA(ACRES) = 0.71
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.62

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	8.74	17.02	3.587	3.98
2	4.62	5.16	7.741	0.71

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.27	5.16	7.741
2	10.88	17.02	3.587

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 10.88 Tc(MIN.) = 17.02
TOTAL AREA(ACRES) = 4.7
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 56.00 = 1274.00 FEET.

FLOW PROCESS FROM NODE 56.00 TO NODE 57.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.28
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.88
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 17.19
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 57.00 = 1350.00 FEET.

DRAINAGE SUBAREA 8C

```

*****
FLOW PROCESS FROM NODE      57.00 TO NODE      57.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
    25 YEAR RAINFALL INTENSITY(INCH/HOUR) =  3.563
    LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
    SOIL CLASSIFICATION IS "A"
    S.C.S. CURVE NUMBER (AMC II) =  92
    AREA-AVERAGE RUNOFF COEFFICIENT = 0.6550
    SUBAREA AREA(ACRES) =      0.33  SUBAREA RUNOFF(CFS) =      0.98
    TOTAL AREA(ACRES) =      5.0    TOTAL RUNOFF(CFS) =      11.72
    TC(MIN.) =      17.19

*****
FLOW PROCESS FROM NODE      57.00 TO NODE      301.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
    REPRESENTATIVE SLOPE =  0.0130
    FLOW LENGTH(FEET) =      23.00  MANNING'S N =  0.012
    DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.2 INCHES
    PIPE-FLOW VELOCITY(FEET/SEC.) =  8.18
    GIVEN PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES =  1
    PIPE-FLOW(CFS) =      11.72
    PIPE TRAVEL TIME(MIN.) =  0.05    Tc(MIN.) =  17.24
    LONGEST FLOWPATH FROM NODE      41.00 TO NODE      301.00 =      1373.00 FEET.

+-----+
| DRAINAGE SUBAREA 9C |
+-----+

*****
FLOW PROCESS FROM NODE      301.00 TO NODE      301.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
    25 YEAR RAINFALL INTENSITY(INCH/HOUR) =  3.557
    RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .3400
    SOIL CLASSIFICATION IS "A"
    S.C.S. CURVE NUMBER (AMC II) =  51
    AREA-AVERAGE RUNOFF COEFFICIENT = 0.6470
    SUBAREA AREA(ACRES) =      0.13  SUBAREA RUNOFF(CFS) =      0.16
    TOTAL AREA(ACRES) =      5.2    TOTAL RUNOFF(CFS) =      11.85
    TC(MIN.) =      17.24

+-----+
| DRAINAGE SUBAREA 1D.1 |
+-----+

*****
FLOW PROCESS FROM NODE      61.00 TO NODE      62.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
    NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
    SOIL CLASSIFICATION IS "C"
    S.C.S. CURVE NUMBER (AMC II) =  85
    INITIAL SUBAREA FLOW-LENGTH(FEET) =  100.00
    UPSTREAM ELEVATION(FEET) =  1452.00
    DOWNSTREAM ELEVATION(FEET) =  1429.00
    ELEVATION DIFFERENCE(FEET) =  23.00
    SUBAREA OVERLAND TIME OF FLOW(MIN.) =  6.684
    WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
    25 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.554

```


SUBAREA RUNOFF(CFS) = 1.12
TOTAL AREA(ACRES) = 0.57 TOTAL RUNOFF(CFS) = 1.12

DRAINAGE SUBAREA 1D.2

FLOW PROCESS FROM NODE 62.00 TO NODE 63.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 651.00
REPRESENTATIVE CHANNEL SLOPE = 0.0940
CHANNEL BASE(FEET) = 100.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.077
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .2700
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 45
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.13
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.49
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 7.27
Tc(MIN.) = 13.95
SUBAREA AREA(ACRES) = 5.18 SUBAREA RUNOFF(CFS) = 5.70
AREA-AVERAGE RUNOFF COEFFICIENT = 0.273
TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 6.40

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.81
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 63.00 = 751.00 FEET.

DRAINAGE SUBAREA 1D.3

FLOW PROCESS FROM NODE 63.00 TO NODE 64.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0560
CHANNEL LENGTH THRU SUBAREA(FEET) = 181.00
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.800
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.030
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4800
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 81
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.50
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 11.71
AVERAGE FLOW DEPTH(FEET) = 0.80 FLOOD WIDTH(FEET) = 3.00
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 0.26 Tc(MIN.) = 14.21
SUBAREA AREA(ACRES) = 0.10 SUBAREA RUNOFF(CFS) = 0.19
AREA-AVERAGE RUNOFF COEFFICIENT = 0.277
TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 6.52

NOTE:TRAVEL TIME ESTIMATES BASED ON NORMAL DEPTH
IN A FLOWING-FULL GUTTER(NORMAL DEPTH = GUTTER HIKE)

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.80 FLOOD WIDTH(FEET) = 3.00
FLOW VELOCITY(FEET/SEC.) = 11.71 DEPTH*VELOCITY(FT*FT/SEC) = 9.37


```

LONGEST FLOWPATH FROM NODE      61.00 TO NODE      64.00 =      932.00 FEET.

*****
FLOW PROCESS FROM NODE      64.00 TO NODE      66.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =    41.00
REPRESENTATIVE CHANNEL SLOPE =    0.0120
CHANNEL BASE(FEET) =    4.00  "Z" FACTOR =    0.330
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =    1.00
CHANNEL FLOW THRU SUBAREA(CFS) =    6.52
FLOW VELOCITY(FEET/SEC.) =    4.77  FLOW DEPTH(FEET) =    0.33
TRAVEL TIME(MIN.) =    0.14  Tc(MIN.) =    14.35
LONGEST FLOWPATH FROM NODE      61.00 TO NODE      66.00 =    973.00 FEET.

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =    14.35
RAINFALL INTENSITY(INCH/HR) =    4.00
TOTAL STREAM AREA(ACRES) =    5.85
PEAK FLOW RATE(CFS) AT CONFLUENCE =    6.52

+-----+
| DRAINAGE SUBAREA 2D |
+-----+

*****
FLOW PROCESS FROM NODE      65.00 TO NODE      66.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  85
INITIAL SUBAREA FLOW-LENGTH(FEET) =    22.00
UPSTREAM ELEVATION(FEET) =    1358.00
DOWNSTREAM ELEVATION(FEET) =    1357.00
ELEVATION DIFFERENCE(FEET) =    1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =    4.078
25 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =    0.28
TOTAL AREA(ACRES) =    0.12  TOTAL RUNOFF(CFS) =    0.28

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
TIME OF CONCENTRATION(MIN.) =    4.08
RAINFALL INTENSITY(INCH/HR) =    7.90
TOTAL STREAM AREA(ACRES) =    0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE =    0.28

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
  1          6.52      14.35      4.004        5.85
  2          0.28       4.08      7.904        0.12

```


RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	2.14	4.08	7.904
2	6.66	14.35	4.004

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 6.66 Tc (MIN.) = 14.35
TOTAL AREA (ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 66.00 = 973.00 FEET.

DRAINAGE SUBAREA 1E

FLOW PROCESS FROM NODE 71.00 TO NODE 72.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .2000
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH (FEET) = 86.00
UPSTREAM ELEVATION (FEET) = 1358.00
DOWNSTREAM ELEVATION (FEET) = 1351.10
ELEVATION DIFFERENCE (FEET) = 6.90
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 7.505
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.083
SUBAREA RUNOFF (CFS) = 0.15
TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 0.15

END OF STUDY SUMMARY:
TOTAL AREA (ACRES) = 0.1 TC (MIN.) = 7.50
PEAK FLOW RATE (CFS) = 0.15

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

DRC Engineering, Inc.
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Anaheim Hills, CA 92808
714-685-6860

***** DESCRIPTION OF STUDY *****
* 19-132 GS VALLEY EXPANSION (28435 LIZARD ROCKS ROAD) *
* EXISTING CONDITION *
* 100-YEAR STORM EVENT *

FILE NAME: 19132E00.DAT
TIME/DATE OF STUDY: 11:10 05/19/2021

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 4.500
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| DRAINAGE SUBAREA 1A.1 |
+-----+

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(Feet) = 100.00
UPSTREAM ELEVATION(Feet) = 1384.70
DOWNSTREAM ELEVATION(Feet) = 1371.50
ELEVATION DIFFERENCE(Feet) = 13.20
SUBAREA OVERLAND TIME OF FLOW(Min.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 9.832
SUBAREA RUNOFF(CFS) = 0.38

TOTAL AREA (ACRES) = 0.13 TOTAL RUNOFF (CFS) = 0.38

DRAINAGE SUBAREA 1A.2

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 305.00
REPRESENTATIVE CHANNEL SLOPE = 0.0510
CHANNEL BASE (FEET) = 40.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.989
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.46
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.09
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 4.66
Tc (MIN.) = 11.35
SUBAREA AREA (ACRES) = 0.73 SUBAREA RUNOFF (CFS) = 2.14
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 2.42

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.41
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 405.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 100.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 62.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 5.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.83
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.42
PIPE TRAVEL TIME (MIN.) = 0.21 Tc (MIN.) = 11.56
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 467.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 11.56
RAINFALL INTENSITY (INCH/HR) = 6.90
TOTAL STREAM AREA (ACRES) = 0.86
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.42

DRAINAGE SUBAREA 2A.1

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<


```
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1404.00
DOWNSTREAM ELEVATION(FEET) = 1399.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.422
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.470
SUBAREA RUNOFF(CFS) = 1.02
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 1.02
=====
```

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+-----+
| DRAINAGE SUBAREA 2A.2 |
+-----+
```

```
*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 51
=====
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```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 465.00
REPRESENTATIVE CHANNEL SLOPE = 0.0590
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.229
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.40
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.51
AVERAGE FLOW DEPTH(FEET) = 0.05 TRAVEL TIME(MIN.) = 5.14
Tc(MIN.) = 13.56
SUBAREA AREA(ACRES) = 3.55 SUBAREA RUNOFF(CFS) = 6.63
AREA-AVERAGE RUNOFF COEFFICIENT = 0.300
TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 7.38
=====
```

```
END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 1.93
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 565.00 FEET.
```

```
+-----+
| DRAINAGE SUBAREA 2A.3 |
+-----+
```

```
*****
FLOW PROCESS FROM NODE 6.00 TO NODE 100.00 IS CODE = 51
=====
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 278.00
REPRESENTATIVE CHANNEL SLOPE = 0.0520
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.798
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3600
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 76
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.81
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.90
AVERAGE FLOW DEPTH(FEET) = 0.13 TRAVEL TIME(MIN.) = 1.60
Tc(MIN.) = 15.16
SUBAREA AREA(ACRES) = 0.41 SUBAREA RUNOFF(CFS) = 0.86
AREA-AVERAGE RUNOFF COEFFICIENT = 0.306
=====
```


TOTAL AREA (ACRES) = 4.4 PEAK FLOW RATE (CFS) = 7.73

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH (FEET) = 0.13 FLOW VELOCITY (FEET/SEC.) = 2.91

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 100.00 = 843.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 15.16
RAINFALL INTENSITY (INCH/HR) = 5.80
TOTAL STREAM AREA (ACRES) = 4.36
PEAK FLOW RATE (CFS) AT CONFLUENCE = 7.73

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.42	11.56	6.905	0.86
2	7.73	15.16	5.798	4.36

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	8.31	11.56	6.905
2	9.75	15.16	5.798

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.75 Tc (MIN.) = 15.16

TOTAL AREA (ACRES) = 5.2

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 100.00 = 843.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 35.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.67
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 9.75
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 15.25
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 101.00 = 878.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 15.25
RAINFALL INTENSITY (INCH/HR) = 5.78
TOTAL STREAM AREA (ACRES) = 5.22
PEAK FLOW RATE (CFS) AT CONFLUENCE = 9.75

+-----+
DRAINAGE SUBAREA 3A.1
+-----+

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1379.80
DOWNSTREAM ELEVATION (FEET) = 1368.60
ELEVATION DIFFERENCE (FEET) = 11.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY (INCH/ HOUR) = 9.832
SUBAREA RUNOFF (CFS) = 0.50
TOTAL AREA (ACRES) = 0.17 TOTAL RUNOFF (CFS) = 0.50

+-----+
| DRAINAGE SUBAREA 3A.2 |
+-----+

FLOW PROCESS FROM NODE 8.00 TO NODE 101.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 294.00
REPRESENTATIVE CHANNEL SLOPE = 0.0440
CHANNEL BASE (FEET) = 30.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
100 YEAR RAINFALL INTENSITY (INCH/ HOUR) = 7.240
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.49
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.21
AVERAGE FLOW DEPTH (FEET) = 0.04 TRAVEL TIME (MIN.) = 4.06
Tc (MIN.) = 10.74
SUBAREA AREA (ACRES) = 0.65 SUBAREA RUNOFF (CFS) = 1.98
AREA-AVERAGE RUNOFF COEFFICIENT = 0.395
TOTAL AREA (ACRES) = 0.8 PEAK FLOW RATE (CFS) = 2.35

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.05 FLOW VELOCITY (FEET/SEC.) = 1.55
LONGEST FLOWPATH FROM NODE 7.00 TO NODE 101.00 = 394.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 10.74
RAINFALL INTENSITY (INCH/HR) = 7.24
TOTAL STREAM AREA (ACRES) = 0.82
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.35

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	9.75	15.25	5.776	5.22
2	2.35	10.74	7.240	0.82

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	T _c (MIN.)	INTENSITY (INCH/HOUR)
1	10.13	10.74	7.240
2	11.63	15.25	5.776

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 11.63 T_c (MIN.) = 15.25
TOTAL AREA (ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 101.00 = 878.00 FEET.

DRAINAGE SUBAREA 1B

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

RESIDENTIAL (2.9 DU/AC OR LESS) RUNOFF COEFFICIENT = .4500
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 80
INITIAL SUBAREA FLOW-LENGTH (FEET) = 95.00
UPSTREAM ELEVATION (FEET) = 1369.30
DOWNSTREAM ELEVATION (FEET) = 1363.90
ELEVATION DIFFERENCE (FEET) = 5.40
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.390
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 10.121
SUBAREA RUNOFF (CFS) = 0.68
TOTAL AREA (ACRES) = 0.15 TOTAL RUNOFF (CFS) = 0.68

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 94.00 MANNING'S N = 0.012
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.48
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.68
PIPE TRAVEL TIME (MIN.) = 0.45 T_c (MIN.) = 6.84
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 189.00 FEET.

DRAINAGE SUBAREA 2B

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 9.686
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5475
SUBAREA AREA (ACRES) = 0.05 SUBAREA RUNOFF (CFS) = 0.41
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 1.06
T_c (MIN.) = 6.84


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*****
FLOW PROCESS FROM NODE      23.00 TO NODE      200.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0640
FLOW LENGTH(FEET) =  13.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS  3.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  8.10
GIVEN PIPE DIAMETER(INCH) =  6.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  1.06
PIPE TRAVEL TIME(MIN.) =  0.03  Tc(MIN.) =  6.87
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      200.00 =  202.00 FEET.

*****
FLOW PROCESS FROM NODE      200.00 TO NODE      24.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0180
FLOW LENGTH(FEET) =  99.00  MANNING'S N =  0.012
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) =  5.40
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) =  6.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  1.06
PIPE TRAVEL TIME(MIN.) =  0.31  Tc(MIN.) =  7.17
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      24.00 =  301.00 FEET.

*****
FLOW PROCESS FROM NODE      24.00 TO NODE      26.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =  12.00
REPRESENTATIVE CHANNEL SLOPE =  0.0170
CHANNEL BASE(FEET) =  6.00  "Z" FACTOR =  0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  0.50
CHANNEL FLOW THRU SUBAREA(CFS) =  1.06
FLOW VELOCITY(FEET/SEC.) =  2.23  FLOW DEPTH(FEET) =  0.08
TRAVEL TIME(MIN.) =  0.09  Tc(MIN.) =  7.26
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      26.00 =  313.00 FEET.

*****
FLOW PROCESS FROM NODE      26.00 TO NODE      26.00 IS CODE =  1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =  7.26
RAINFALL INTENSITY(INCH/HR) =  9.32
TOTAL STREAM AREA(ACRES) =  0.20
PEAK FLOW RATE(CFS) AT CONFLUENCE =  1.06

+-----+
| DRAINAGE SUBAREA 3B |
+-----+

*****
FLOW PROCESS FROM NODE      25.00 TO NODE      26.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400

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SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 89.00
UPSTREAM ELEVATION(FEET) = 1364.10
DOWNSTREAM ELEVATION(FEET) = 1358.00
ELEVATION DIFFERENCE(FEET) = 6.10
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.324
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.40
TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.40

*****
FLOW PROCESS FROM NODE 26.00 TO NODE 26.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.32
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.40

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 1.06 7.26 9.320 0.20
2 0.40 2.32 11.856 0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 0.74 2.32 11.856
2 1.37 7.26 9.320

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1.37 Tc(MIN.) = 7.26
TOTAL AREA(ACRES) = 0.2
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 26.00 = 313.00 FEET.

*****
FLOW PROCESS FROM NODE 26.00 TO NODE 201.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0200
FLOW LENGTH(FEET) = 1.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.57
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.37
PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 7.27
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 201.00 = 314.00 FEET.

+-----+
| DRAINAGE SUBAREA 4B |
+-----+

*****
FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
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LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1368.40
DOWNSTREAM ELEVATION(FEET) = 1362.20
ELEVATION DIFFERENCE(FEET) = 6.20
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.683
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.39
TOTAL AREA(ACRES) = 0.14 TOTAL RUNOFF(CFS) = 1.39

*****
FLOW PROCESS FROM NODE 28.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 5.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.34
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.39
PIPE TRAVEL TIME(MIN.) = 0.25 Tc(MIN.) = 1.93
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 202.00 = 124.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 1.93
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.14
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.39

+-----+
| DRAINAGE SUBAREA 5B |
+-----+

*****
FLOW PROCESS FROM NODE 29.00 TO NODE 30.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 54.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1362.00
ELEVATION DIFFERENCE(FEET) = 2.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.122
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.60
TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.60

*****
FLOW PROCESS FROM NODE 30.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
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REPRESENTATIVE SLOPE = 0.0380
FLOW LENGTH(FEET) = 15.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.53
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.60
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 2.17
LONGEST FLOWPATH FROM NODE 29.00 TO NODE 202.00 = 69.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.17
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.06
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.60

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 1.39 1.93 11.856 0.14
2 0.60 2.17 11.856 0.06

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 1.93 1.93 11.856
2 1.99 2.17 11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1.99 Tc(MIN.) = 2.17
TOTAL AREA(ACRES) = 0.2
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 202.00 = 124.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.1100
FLOW LENGTH(FEET) = 54.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.42
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.99
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 2.25
LONGEST FLOWPATH FROM NODE 27.00 TO NODE 203.00 = 178.00 FEET.

+-----+
| DRAINAGE SUBAREA 6B |
+-----+

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400

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SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA (ACRES) = 0.00 SUBAREA RUNOFF (CFS) = 0.05
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 2.04
TC (MIN.) = 2.25

DRAINAGE SUBAREA 7B.1

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 80.00
UPSTREAM ELEVATION (FEET) = 1361.10
DOWNSTREAM ELEVATION (FEET) = 1358.60
ELEVATION DIFFERENCE (FEET) = 2.50
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.863
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.80
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.80

DRAINAGE SUBAREA 7B.2

FLOW PROCESS FROM NODE 33.00 TO NODE 34.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 36.00
REPRESENTATIVE CHANNEL SLOPE = 0.0360
CHANNEL BASE (FEET) = 8.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 0.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.90
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.30
AVERAGE FLOW DEPTH (FEET) = 0.05 TRAVEL TIME (MIN.) = 0.26
Tc (MIN.) = 3.12
SUBAREA AREA (ACRES) = 0.02 SUBAREA RUNOFF (CFS) = 0.20
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 1.00

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.05 FLOW VELOCITY (FEET/SEC.) = 2.56
LONGEST FLOWPATH FROM NODE 32.00 TO NODE 34.00 = 116.00 FEET.

DRAINAGE SUBAREA 1C.1

FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1401.00
DOWNSTREAM ELEVATION (FEET) = 1397.00
ELEVATION DIFFERENCE (FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 9.072
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 8.074
SUBAREA RUNOFF (CFS) = 0.75
TOTAL AREA (ACRES) = 0.31 TOTAL RUNOFF (CFS) = 0.75

DRAINAGE SUBAREA 1C.2

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

CHANNEL LENGTH THRU SUBAREA (FEET) = 380.00
REPRESENTATIVE CHANNEL SLOPE = 0.0710
CHANNEL BASE (FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.277
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 3.10
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.46
AVERAGE FLOW DEPTH (FEET) = 0.04 TRAVEL TIME (MIN.) = 4.33
Tc (MIN.) = 13.40
SUBAREA AREA (ACRES) = 1.80 SUBAREA RUNOFF (CFS) = 4.75
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA (ACRES) = 2.1 PEAK FLOW RATE (CFS) = 5.33

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.05 FLOW VELOCITY (FEET/SEC.) = 1.76
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 43.00 = 480.00 FEET.

DRAINAGE SUBAREA 1C.3

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

REPRESENTATIVE SLOPE = 0.0270
CHANNEL LENGTH THRU SUBAREA (FEET) = 371.00
"V" GUTTER WIDTH (FEET) = 5.00 GUTTER HIKE (FEET) = 0.090
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 5.840
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 6.40
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 3.90


```

AVERAGE FLOW DEPTH(FEET) = 0.22 FLOOD WIDTH(FEET) = 17.27
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.59 Tc(MIN.) = 14.99
SUBAREA AREA(ACRES) = 0.44 SUBAREA RUNOFF(CFS) = 2.13
AREA-AVERAGE RUNOFF COEFFICIENT = 0.476
TOTAL AREA(ACRES) = 2.5 PEAK FLOW RATE(CFS) = 7.09

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.23 FLOOD WIDTH(FEET) = 18.05
FLOW VELOCITY(FEET/SEC.) = 3.99 DEPTH*VELOCITY(FT*FT/SEC) = 0.92
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 44.00 = 851.00 FEET.

*****
FLOW PROCESS FROM NODE 44.00 TO NODE 45.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0130
FLOW LENGTH(FEET) = 166.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 11.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.01
GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.09
PIPE TRAVEL TIME(MIN.) = 0.39 Tc(MIN.) = 15.38
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 1017.00 FEET.

+-----+
| DRAINAGE SUBAREA 2C |
+-----+

*****
FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.743
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5445
SUBAREA AREA(ACRES) = 0.59 SUBAREA RUNOFF(CFS) = 2.85
TOTAL AREA(ACRES) = 3.1 TOTAL RUNOFF(CFS) = 9.82
TC(MIN.) = 15.38

*****
FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

+-----+
| DRAINAGE SUBAREA 3C.1 |
+-----+

*****
FLOW PROCESS FROM NODE 46.00 TO NODE 47.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ARTIICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 90.00
UPSTREAM ELEVATION(FEET) = 1370.50
DOWNSTREAM ELEVATION(FEET) = 1365.90
ELEVATION DIFFERENCE(FEET) = 4.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.280

```


100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.41
TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.41

DRAINAGE SUBAREA 3C.2

FLOW PROCESS FROM NODE 47.00 TO NODE 48.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
CHANNEL LENGTH THRU SUBAREA(FEET) = 297.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.01
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.78
AVERAGE FLOW DEPTH(FEET) = 0.16 FLOOD WIDTH(FEET) = 10.70
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.78 Tc(MIN.) = 4.06
SUBAREA AREA(ACRES) = 0.32 SUBAREA RUNOFF(CFS) = 3.19
AREA-AVERAGE RUNOFF COEFFICIENT = 0.843
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 3.60

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.19 FLOOD WIDTH(FEET) = 14.14
FLOW VELOCITY(FEET/SEC.) = 3.13 DEPTH*VELOCITY(FT*FT/SEC) = 0.60
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 48.00 = 387.00 FEET.

FLOW PROCESS FROM NODE 48.00 TO NODE 300.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
FLOW LENGTH(FEET) = 34.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 9.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.56
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.60
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 4.17
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 4.17
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.60

DRAINAGE SUBAREA 4C.1


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+-----+
*****
FLOW PROCESS FROM NODE      49.00 TO NODE      50.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) =      60.00
UPSTREAM ELEVATION(FEET) =      1370.50
DOWNSTREAM ELEVATION(FEET) =      1365.40
ELEVATION DIFFERENCE(FEET) =       5.10
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      1.571
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =       0.31
TOTAL AREA(ACRES) =       0.03   TOTAL RUNOFF(CFS) =       0.31

+-----+
| DRAINAGE SUBAREA 4C.2 |
+-----+
*****
FLOW PROCESS FROM NODE      50.00 TO NODE      51.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =      70.00
REPRESENTATIVE CHANNEL SLOPE =      0.0900
CHANNEL BASE(FEET) =      3.00   "Z" FACTOR =      0.330
MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =      1.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =       0.36
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =      3.40
AVERAGE FLOW DEPTH(FEET) =      0.04   TRAVEL TIME(MIN.) =      0.34
Tc(MIN.) =      1.91
SUBAREA AREA(ACRES) =      0.01   SUBAREA RUNOFF(CFS) =      0.10
AREA-AVERAGE RUNOFF COEFFICIENT =      0.870
TOTAL AREA(ACRES) =      0.0   PEAK FLOW RATE(CFS) =      0.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.04   FLOW VELOCITY(FEET/SEC.) =      3.34
LONGEST FLOWPATH FROM NODE      49.00 TO NODE      51.00 =      130.00 FEET.

*****
FLOW PROCESS FROM NODE      51.00 TO NODE      300.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =      0.0100
FLOW LENGTH(FEET) =      27.00   MANNING'S N =      0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS      2.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      3.10
GIVEN PIPE DIAMETER(INCH) =      12.00   NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      0.41
PIPE TRAVEL TIME(MIN.) =      0.15   Tc(MIN.) =      2.06
LONGEST FLOWPATH FROM NODE      49.00 TO NODE      300.00 =      157.00 FEET.

*****
FLOW PROCESS FROM NODE      300.00 TO NODE      300.00 IS CODE = 1
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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.06
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.41

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HR)      (ACRE)
1           3.60      4.17      11.856        0.36
2           0.41      2.06      11.856        0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HR)
1           2.19      2.06      11.856
2           4.01      4.17      11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 4.01 Tc(MIN.) = 4.17
TOTAL AREA(ACRES) = 0.4
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 300.00 = 421.00 FEET.

*****
FLOW PROCESS FROM NODE 300.00 TO NODE 45.00 IS CODE = 41
=====
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 6.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.61
GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.01
PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 4.31
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 45.00 = 485.00 FEET.

*****
FLOW PROCESS FROM NODE 45.00 TO NODE 45.00 IS CODE = 11
=====
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HR)      (ACRE)
1           4.01      4.31      11.856        0.40
LONGEST FLOWPATH FROM NODE 46.00 TO NODE 45.00 = 485.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HR)      (ACRE)
1           9.82      15.38      5.743        3.14
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 45.00 = 1017.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HR)
1           6.76      4.31      11.856
2          11.76      15.38      5.743

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 11.76 Tc(MIN.) = 15.38

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TOTAL AREA (ACRES) =          3.5

*****
FLOW PROCESS FROM NODE      45.00 TO NODE      45.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      45.00 TO NODE      52.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH (FEET) = 173.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.46
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 11.76
PIPE TRAVEL TIME (MIN.) = 0.34 Tc (MIN.) = 15.72
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 52.00 = 1190.00 FEET.

+-----+
| DRAINAGE SUBAREA 5C |
+-----+

*****
FLOW PROCESS FROM NODE      52.00 TO NODE      52.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 5.662
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6075
SUBAREA AREA (ACRES) = 0.44 SUBAREA RUNOFF (CFS) = 2.09
TOTAL AREA (ACRES) = 4.0 TOTAL RUNOFF (CFS) = 13.69
TC (MIN.) = 15.72

*****
FLOW PROCESS FROM NODE      52.00 TO NODE      56.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 84.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.70
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 13.69
PIPE TRAVEL TIME (MIN.) = 0.18 Tc (MIN.) = 15.91
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 56.00 = 1274.00 FEET.

*****
FLOW PROCESS FROM NODE      56.00 TO NODE      56.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 15.91
RAINFALL INTENSITY (INCH/HR) = 5.62
TOTAL STREAM AREA (ACRES) = 3.98
PEAK FLOW RATE (CFS) AT CONFLUENCE = 13.69
+-----+

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DRAINAGE SUBAREA 6C.1

FLOW PROCESS FROM NODE 53.00 TO NODE 54.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 53.00
UPSTREAM ELEVATION (FEET) = 1359.30
DOWNSTREAM ELEVATION (FEET) = 1358.70
ELEVATION DIFFERENCE (FEET) = 0.60
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.269
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 1.10
TOTAL AREA (ACRES) = 0.11 TOTAL RUNOFF (CFS) = 1.10

DRAINAGE SUBAREA 6C.2

FLOW PROCESS FROM NODE 54.00 TO NODE 55.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 188.00
REPRESENTATIVE CHANNEL SLOPE = 0.0110
CHANNEL BASE (FEET) = 12.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 0.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 2.84
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.28
AVERAGE FLOW DEPTH (FEET) = 0.10 TRAVEL TIME (MIN.) = 1.38
Tc (MIN.) = 4.65
SUBAREA AREA (ACRES) = 0.35 SUBAREA RUNOFF (CFS) = 3.49
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 4.58

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.14 FLOW VELOCITY (FEET/SEC.) = 2.72
LONGEST FLOWPATH FROM NODE 53.00 TO NODE 55.00 = 241.00 FEET.

FLOW PROCESS FROM NODE 55.00 TO NODE 56.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH (FEET) = 105.00 MANNING'S N = 0.012
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.83
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.58
PIPE TRAVEL TIME (MIN.) = 0.30 Tc (MIN.) = 4.95
LONGEST FLOWPATH FROM NODE 53.00 TO NODE 56.00 = 346.00 FEET.

DRAINAGE SUBAREA 7C

FLOW PROCESS FROM NODE 56.00 TO NODE 56.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) = 0.25 SUBAREA RUNOFF(CFS) = 2.49
TOTAL AREA(ACRES) = 0.7 TOTAL RUNOFF(CFS) = 7.07
TC(MIN.) = 4.95

FLOW PROCESS FROM NODE 56.00 TO NODE 56.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.95
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.71
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.07

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	13.69	15.91	5.621	3.98
2	7.07	4.95	11.856	0.71

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	11.33	4.95	11.856
2	17.04	15.91	5.621

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 17.04 Tc(MIN.) = 15.91
TOTAL AREA(ACRES) = 4.7
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 56.00 = 1274.00 FEET.

FLOW PROCESS FROM NODE 56.00 TO NODE 57.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.08
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.04
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 16.06
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 57.00 = 1350.00 FEET.

DRAINAGE SUBAREA 8C

FLOW PROCESS FROM NODE 57.00 TO NODE 57.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR)	=	5.585
LIMITED INDUSTRIAL RUNOFF COEFFICIENT	=	.8300
SOIL CLASSIFICATION IS "A"		
S.C.S. CURVE NUMBER (AMC II)	=	92
AREA-AVERAGE RUNOFF COEFFICIENT	=	0.6550
SUBAREA AREA(ACRES)	=	0.33
SUBAREA RUNOFF(CFS)	=	1.53
TOTAL AREA(ACRES)	=	5.0
TOTAL RUNOFF(CFS)	=	18.36
TC(MIN.)	=	16.06

FLOW PROCESS FROM NODE 57.00 TO NODE 301.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE	=	0.0130
FLOW LENGTH(Feet)	=	23.00
MANNING'S N	=	0.012
DEPTH OF FLOW IN 24.0 INCH PIPE	IS	14.7 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.)	=	9.11
GIVEN PIPE DIAMETER(INCH)	=	24.00
NUMBER OF PIPES	=	1
PIPE-FLOW(CFS)	=	18.36
PIPE TRAVEL TIME(MIN.)	=	0.04
Tc(MIN.)	=	16.10
LONGEST FLOWPATH FROM NODE 41.00 TO NODE 301.00	=	1373.00 FEET.

DRAINAGE SUBAREA 9C

FLOW PROCESS FROM NODE 301.00 TO NODE 301.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR)	=	5.576
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT	=	.3400
SOIL CLASSIFICATION IS "A"		
S.C.S. CURVE NUMBER (AMC II)	=	51
AREA-AVERAGE RUNOFF COEFFICIENT	=	0.6470
SUBAREA AREA(ACRES)	=	0.13
SUBAREA RUNOFF(CFS)	=	0.25
TOTAL AREA(ACRES)	=	5.2
TOTAL RUNOFF(CFS)	=	18.58
TC(MIN.)	=	16.10

DRAINAGE SUBAREA 1D.1

FLOW PROCESS FROM NODE 61.00 TO NODE 62.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT	=	.3000
SOIL CLASSIFICATION IS "C"		
S.C.S. CURVE NUMBER (AMC II)	=	85
INITIAL SUBAREA FLOW-LENGTH(Feet)	=	100.00
UPSTREAM ELEVATION(Feet)	=	1452.00
DOWNSTREAM ELEVATION(Feet)	=	1429.00

ELEVATION DIFFERENCE (FEET) = 23.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 9.832
SUBAREA RUNOFF (CFS) = 1.68
TOTAL AREA (ACRES) = 0.57 TOTAL RUNOFF (CFS) = 1.68

DRAINAGE SUBAREA 1D.2

FLOW PROCESS FROM NODE 62.00 TO NODE 63.00 IS CODE = 51

>>>> COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>> TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 651.00
REPRESENTATIVE CHANNEL SLOPE = 0.0940
CHANNEL BASE (FEET) = 100.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.482
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .2700
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 45
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 6.31
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.79
AVERAGE FLOW DEPTH (FEET) = 0.04 TRAVEL TIME (MIN.) = 6.07
Tc (MIN.) = 12.75
SUBAREA AREA (ACRES) = 5.18 SUBAREA RUNOFF (CFS) = 9.07
AREA-AVERAGE RUNOFF COEFFICIENT = 0.273
TOTAL AREA (ACRES) = 5.8 PEAK FLOW RATE (CFS) = 10.17

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.05 FLOW VELOCITY (FEET/SEC.) = 2.01
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 63.00 = 751.00 FEET.

DRAINAGE SUBAREA 1D.3

FLOW PROCESS FROM NODE 63.00 TO NODE 64.00 IS CODE = 91

>>>> COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0560
CHANNEL LENGTH THRU SUBAREA (FEET) = 181.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.800
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 1.00
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.398
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4800
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 81
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 10.33
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 11.71
AVERAGE FLOW DEPTH (FEET) = 0.80 FLOOD WIDTH (FEET) = 3.00
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.26 Tc (MIN.) = 13.01
SUBAREA AREA (ACRES) = 0.10 SUBAREA RUNOFF (CFS) = 0.31
AREA-AVERAGE RUNOFF COEFFICIENT = 0.277
TOTAL AREA (ACRES) = 5.8 PEAK FLOW RATE (CFS) = 10.35

NOTE: TRAVEL TIME ESTIMATES BASED ON NORMAL DEPTH
IN A FLOWING-FULL GUTTER (NORMAL DEPTH = GUTTER HIKE)


```

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH( FEET ) = 0.80  FLOOD WIDTH( FEET ) = 3.00
FLOW VELOCITY( FEET/SEC. ) = 11.71  DEPTH*VELOCITY( FT*FT/SEC ) = 9.37
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 64.00 = 932.00 FEET.

*****
FLOW PROCESS FROM NODE 64.00 TO NODE 66.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA( FEET ) = 41.00
REPRESENTATIVE CHANNEL SLOPE = 0.0120
CHANNEL BASE( FEET ) = 4.00  "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH( FEET ) = 1.00
CHANNEL FLOW THRU SUBAREA( CFS ) = 10.35
FLOW VELOCITY( FEET/SEC. ) = 5.60  FLOW DEPTH( FEET ) = 0.45
TRAVEL TIME( MIN. ) = 0.12  Tc( MIN. ) = 13.13
LONGEST FLOWPATH FROM NODE 61.00 TO NODE 66.00 = 973.00 FEET.

*****
FLOW PROCESS FROM NODE 66.00 TO NODE 66.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION( MIN. ) = 13.13
RAINFALL INTENSITY( INCH/HR ) = 6.36
TOTAL STREAM AREA( ACRES ) = 5.85
PEAK FLOW RATE( CFS ) AT CONFLUENCE = 10.35

+-----+
| DRAINAGE SUBAREA 2D |
+-----+

*****
FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH( FEET ) = 22.00
UPSTREAM ELEVATION( FEET ) = 1358.00
DOWNSTREAM ELEVATION( FEET ) = 1357.00
ELEVATION DIFFERENCE( FEET ) = 1.00
SUBAREA OVERLAND TIME OF FLOW( MIN. ) = 4.078
100 YEAR RAINFALL INTENSITY( INCH/HOUR ) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF( CFS ) = 0.43
TOTAL AREA( ACRES ) = 0.12  TOTAL RUNOFF( CFS ) = 0.43

*****
FLOW PROCESS FROM NODE 66.00 TO NODE 66.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION( MIN. ) = 4.08
RAINFALL INTENSITY( INCH/HR ) = 11.86
TOTAL STREAM AREA( ACRES ) = 0.12
PEAK FLOW RATE( CFS ) AT CONFLUENCE = 0.43

** CONFLUENCE DATA **

```


STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	10.35	13.13	6.360	5.85
2	0.43	4.08	11.856	0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	3.64	4.08	11.856
2	10.58	13.13	6.360

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE (CFS) = 10.58 Tc (MIN.) = 13.13
 TOTAL AREA (ACRES) = 6.0
 LONGEST FLOWPATH FROM NODE 61.00 TO NODE 66.00 = 973.00 FEET.

-----+-----
 | DRAINAGE SUBAREA 1E |
 |-----+-----|
 -----+-----

 FLOW PROCESS FROM NODE 71.00 TO NODE 72.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 =====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .2000
 SOIL CLASSIFICATION IS "A"
 S.C.S. CURVE NUMBER (AMC II) = 63
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 86.00
 UPSTREAM ELEVATION (FEET) = 1358.00
 DOWNSTREAM ELEVATION (FEET) = 1351.10
 ELEVATION DIFFERENCE (FEET) = 6.90
 SUBAREA OVERLAND TIME OF FLOW (MIN.) = 7.505
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 9.124
 SUBAREA RUNOFF (CFS) = 0.22
 TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 0.22
 =====

END OF STUDY SUMMARY:
 TOTAL AREA (ACRES) = 0.1 TC (MIN.) = 7.50
 PEAK FLOW RATE (CFS) = 0.22
 =====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

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Anaheim Hills, CA 92808
714-685-6860

***** DESCRIPTION OF STUDY *****
* 19-132 GS VALLEY EXPANSION (28435 LIZARD ROCKS ROAD) *
* PROPOSED CONDITION *
* 10-YEAR STORM EVENT *

FILE NAME: 19132P10.DAT
TIME/DATE OF STUDY: 11:12 05/19/2021

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 10.00
6-HOUR DURATION PRECIPITATION (INCHES) = 2.700
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
===
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150
=====

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| DRAINAGE SUBAREA 1A.1 |
+-----+

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(FEET) = 69.00
UPSTREAM ELEVATION(FEET) = 1364.00
DOWNSTREAM ELEVATION(FEET) = 1362.10
ELEVATION DIFFERENCE(FEET) = 1.90
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.374
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.15

TOTAL AREA (ACRES) = 0.03 TOTAL RUNOFF (CFS) = 0.15

DRAINAGE SUBAREA 1A.2

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 162.00
REPRESENTATIVE CHANNEL SLOPE = 0.0390
CHANNEL BASE (FEET) = 45.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.50
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 4.956
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.34
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 0.62
AVERAGE FLOW DEPTH (FEET) = 0.01 TRAVEL TIME (MIN.) = 4.38
Tc (MIN.) = 8.76
SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.41
AREA-AVERAGE RUNOFF COEFFICIENT = 0.690
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 0.51

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.02 FLOW VELOCITY (FEET/SEC.) = 0.63
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 231.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 100.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 62.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 2.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.05
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.51
PIPE TRAVEL TIME (MIN.) = 0.34 Tc (MIN.) = 9.10
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 293.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

DRAINAGE SUBAREA 2A.1

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 49.00
UPSTREAM ELEVATION (FEET) = 1363.70

DOWNSTREAM ELEVATION(FEET) = 1363.20
ELEVATION DIFFERENCE(FEET) = 0.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.254
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.18
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.18

DRAINAGE SUBAREA 2A.2

FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0060
CHANNEL LENGTH THRU SUBAREA(FEET) = 33.00
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.100
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.42
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.37
AVERAGE FLOW DEPTH(FEET) = 0.14 FLOOD WIDTH(FEET) = 5.82
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 0.40 Tc(MIN.) = 3.65
SUBAREA AREA(ACRES) = 0.08 SUBAREA RUNOFF(CFS) = 0.48
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.66

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.16 FLOOD WIDTH(FEET) = 7.95
FLOW VELOCITY(FEET/SEC.) = 1.46 DEPTH*VELOCITY(FT*FT/SEC) = 0.23
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 82.00 FEET.

FLOW PROCESS FROM NODE 6.00 TO NODE 400.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH(FEET) = 127.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 9.0 INCH PIPE IS 4.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 2.78
ESTIMATED PIPE DIAMETER(INCH) = 9.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.66
PIPE TRAVEL TIME(MIN.) = 0.76 Tc(MIN.) = 4.42
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 400.00 = 209.00 FEET.

FLOW PROCESS FROM NODE 400.00 TO NODE 400.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 4.42
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.11
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.66

+-----+

DRAINAGE SUBAREA 3A.1

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.30
ELEVATION DIFFERENCE (FEET) = 1.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.877
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.48
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.48

DRAINAGE SUBAREA 3A.2

FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0060
CHANNEL LENGTH THRU SUBAREA (FEET) = 90.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.79
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.66
AVERAGE FLOW DEPTH (FEET) = 0.22 FLOOD WIDTH (FEET) = 13.74
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.90 Tc (MIN.) = 3.78
SUBAREA AREA (ACRES) = 0.44 SUBAREA RUNOFF (CFS) = 2.63
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 3.11

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.26 FLOOD WIDTH (FEET) = 17.70
FLOW VELOCITY (FEET/SEC.) = 1.83 DEPTH*VELOCITY (FT*FT/SEC) = 0.47
LONGEST FLOWPATH FROM NODE 7.00 TO NODE 9.00 = 150.00 FEET.

FLOW PROCESS FROM NODE 9.00 TO NODE 400.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0170
FLOW LENGTH (FEET) = 6.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.1 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.48
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.11
PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 3.80


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LONGEST FLOWPATH FROM NODE      7.00 TO NODE      400.00 =      156.00 FEET.
*****
FLOW PROCESS FROM NODE      400.00 TO NODE      400.00 IS CODE =      1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      2 ARE:
TIME OF CONCENTRATION(MIN.) =      3.80
RAINFALL INTENSITY(INCH/HR) =      7.11
TOTAL STREAM AREA(ACRES) =      0.52
PEAK FLOW RATE(CFS) AT CONFLUENCE =      3.11

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            0.66      4.42      7.114      0.11
2            3.11      3.80      7.114      0.52

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR      2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            3.67      3.80      7.114
2            3.76      4.42      7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      3.76      Tc(MIN.) =      4.42
TOTAL AREA(ACRES) =      0.6
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      400.00 =      209.00 FEET.
*****
FLOW PROCESS FROM NODE      400.00 TO NODE      401.00 IS CODE =      31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE =      0.0050
FLOW LENGTH(FEET) =      227.00      MANNING'S N =      0.012
DEPTH OF FLOW IN      15.0 INCH PIPE IS      10.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      4.25
ESTIMATED PIPE DIAMETER(INCH) =      15.00      NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      3.76
PIPE TRAVEL TIME(MIN.) =      0.89      Tc(MIN.) =      5.31
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      401.00 =      436.00 FEET.
*****
FLOW PROCESS FROM NODE      401.00 TO NODE      401.00 IS CODE =      1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      1 ARE:
TIME OF CONCENTRATION(MIN.) =      5.31
RAINFALL INTENSITY(INCH/HR) =      6.85
TOTAL STREAM AREA(ACRES) =      0.63
PEAK FLOW RATE(CFS) AT CONFLUENCE =      3.76

+-----+
| DRAINAGE SUBAREA 4A.1 |
+-----+
*****
FLOW PROCESS FROM NODE      10.00 TO NODE      11.00 IS CODE =      21
-----

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>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.60
ELEVATION DIFFERENCE (FEET) = 0.90
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.167
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.12
TOTAL AREA (ACRES) = 0.02 TOTAL RUNOFF (CFS) = 0.12

+-----+
| DRAINAGE SUBAREA 4A.2 |
+-----+

*****
FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 91
-----
>>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
CHANNEL LENGTH THRU SUBAREA (FEET) = 168.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.074
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.52
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.49
AVERAGE FLOW DEPTH (FEET) = 0.21 FLOOD WIDTH (FEET) = 13.28
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.88 Tc (MIN.) = 5.04
SUBAREA AREA (ACRES) = 0.47 SUBAREA RUNOFF (CFS) = 2.79
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 2.91

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.26 FLOOD WIDTH (FEET) = 17.85
FLOW VELOCITY (FEET/SEC.) = 1.68 DEPTH*VELOCITY (FT*FT/SEC) = 0.44
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 12.00 = 228.00 FEET.

*****
FLOW PROCESS FROM NODE 12.00 TO NODE 401.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 8.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.57
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.91
PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 5.07
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 401.00 = 236.00 FEET.

*****
FLOW PROCESS FROM NODE 401.00 TO NODE 401.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2

```


CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.07
RAINFALL INTENSITY(INCH/HR) = 7.05
TOTAL STREAM AREA(ACRES) = 0.49
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.91

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	3.76	5.31	6.846	0.63
2	2.91	5.07	7.052	0.49

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	6.57	5.07	7.052
2	6.59	5.31	6.846

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 6.59 Tc(MIN.) = 5.31
TOTAL AREA(ACRES) = 1.1
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 401.00 = 436.00 FEET.

FLOW PROCESS FROM NODE 401.00 TO NODE 404.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH(FEET) = 89.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.86
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.59
PIPE TRAVEL TIME(MIN.) = 0.31 Tc(MIN.) = 5.61
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 404.00 = 525.00 FEET.

FLOW PROCESS FROM NODE 404.00 TO NODE 404.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<

-----+-----+
| DRAINAGE SUBAREA 5A |
+-----+-----+

FLOW PROCESS FROM NODE 13.00 TO NODE 14.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 47.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1363.00
ELEVATION DIFFERENCE(FEET) = 1.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.286
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.18
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.18


```

*****
FLOW PROCESS FROM NODE      14.00 TO NODE      402.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE =  0.0050
FLOW LENGTH(FEET) =   60.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS   2.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   2.05
ESTIMATED PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       0.18
PIPE TRAVEL TIME(MIN.) =   0.49  Tc(MIN.) =   2.77
LONGEST FLOWPATH FROM NODE      13.00 TO NODE      402.00 =      107.00 FEET.

*****
FLOW PROCESS FROM NODE      402.00 TO NODE      402.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =   2.77
RAINFALL INTENSITY(INCH/HR) =   7.11
TOTAL STREAM AREA(ACRES) =    0.03
PEAK FLOW RATE(CFS) AT CONFLUENCE =      0.18

+-----+
| DRAINAGE SUBAREA 6A |
+-----+

*****
FLOW PROCESS FROM NODE      15.00 TO NODE      402.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
INITIAL SUBAREA FLOW-LENGTH(FEET) =   64.00
UPSTREAM ELEVATION(FEET) =   1364.50
DOWNSTREAM ELEVATION(FEET) =   1363.50
ELEVATION DIFFERENCE(FEET) =    1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =   3.227
  10 YEAR RAINFALL INTENSITY(INCH/HOUR) =   7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =       0.72
TOTAL AREA(ACRES) =    0.12  TOTAL RUNOFF(CFS) =      0.72

*****
FLOW PROCESS FROM NODE      402.00 TO NODE      402.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  2 ARE:
TIME OF CONCENTRATION(MIN.) =   3.23
RAINFALL INTENSITY(INCH/HR) =   7.11
TOTAL STREAM AREA(ACRES) =    0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE =      0.72

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
  1          0.18      2.77      7.114      0.03
  2          0.72      3.23      7.114      0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

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CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	T _c (MIN.)	INTENSITY (INCH/HOUR)
1	0.80	2.77	7.114
2	0.90	3.23	7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 0.90 T_c (MIN.) = 3.23
TOTAL AREA (ACRES) = 0.1
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 402.00 = 107.00 FEET.

FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH (FEET) = 71.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 9.0 INCH PIPE IS 3.7 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.23
ESTIMATED PIPE DIAMETER (INCH) = 9.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.90
PIPE TRAVEL TIME (MIN.) = 0.23 T_c (MIN.) = 3.45
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 403.00 = 178.00 FEET.

FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<
=====

+-----+
| DRAINAGE SUBAREA 7A.1 |
+-----+

FLOW PROCESS FROM NODE 16.00 TO NODE 17.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.30
ELEVATION DIFFERENCE (FEET) = 1.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.877
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON T_c = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.24
TOTAL AREA (ACRES) = 0.04 TOTAL RUNOFF (CFS) = 0.24

+-----+
| DRAINAGE SUBAREA 7A.2 |
+-----+

FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<
=====

REPRESENTATIVE SLOPE = 0.0120
CHANNEL LENGTH THRU SUBAREA (FEET) = 83.00


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"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.100
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.67
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.21
AVERAGE FLOW DEPTH(FEET) = 0.19 FLOOD WIDTH(FEET) = 11.15
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 0.63 Tc(MIN.) = 3.50
SUBAREA AREA(ACRES) = 0.48 SUBAREA RUNOFF(CFS) = 2.87
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.5 PEAK FLOW RATE(CFS) = 3.11

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.23 FLOOD WIDTH(FEET) = 15.11
FLOW VELOCITY(FEET/SEC.) = 2.43 DEPTH*VELOCITY(FT*FT/SEC) = 0.56
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 18.00 = 143.00 FEET.

*****
FLOW PROCESS FROM NODE 18.00 TO NODE 403.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.09
ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.11
PIPE TRAVEL TIME(MIN.) = 0.31 Tc(MIN.) = 3.81
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 218.00 FEET.

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 3.11 3.81 7.114 0.52
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 218.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 0.90 3.45 7.114 0.15
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 403.00 = 178.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 3.71 3.45 7.114
2 4.00 3.81 7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 4.00 Tc(MIN.) = 3.81
TOTAL AREA(ACRES) = 0.7

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 3 <<<<
=====

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*****
FLOW PROCESS FROM NODE      403.00 TO NODE      403.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE =  0.0600
FLOW LENGTH(FEET) =   13.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   9.0 INCH PIPE IS   7.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  10.74
ESTIMATED PIPE DIAMETER(INCH) =   9.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =       4.00
PIPE TRAVEL TIME(MIN.) =   0.02  Tc(MIN.) =   3.83
LONGEST FLOWPATH FROM NODE      16.00 TO NODE      403.00 =      231.00 FEET.

*****
FLOW PROCESS FROM NODE      404.00 TO NODE      404.00 IS CODE =  11
-----
>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
   1         4.00       3.83       7.114       0.67
LONGEST FLOWPATH FROM NODE      16.00 TO NODE      404.00 =      231.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
   1         6.59       5.61       6.603       1.12
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      404.00 =      525.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
   1         8.50       3.83       7.114
   2        10.31       5.61       6.603

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      10.31  Tc(MIN.) =   5.61
TOTAL AREA(ACRES) =       1.8

*****
FLOW PROCESS FROM NODE      404.00 TO NODE      404.00 IS CODE =  12
-----
>>>>CLEAR MEMORY BANK # 2 <<<<
=====

+-----+
| DRAINAGE SUBAREA 8A |
+-----+

*****
FLOW PROCESS FROM NODE      19.00 TO NODE      20.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  91
INITIAL SUBAREA FLOW-LENGTH(FEET) =   81.00
UPSTREAM ELEVATION(FEET) =  1363.70
DOWNSTREAM ELEVATION(FEET) =  1358.10
ELEVATION DIFFERENCE(FEET) =   5.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) =   3.487
10 YEAR RAINFALL INTENSITY(INCH/HOUR) =   7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =       0.39

```


TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.39

DRAINAGE SUBAREA 9A.1

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH (FEET) = 100.00
UPSTREAM ELEVATION (FEET) = 1404.00
DOWNSTREAM ELEVATION (FEET) = 1399.00
ELEVATION DIFFERENCE (FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 8.422
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 5.082
SUBAREA RUNOFF (CFS) = 0.61
TOTAL AREA (ACRES) = 0.40 TOTAL RUNOFF (CFS) = 0.61

DRAINAGE SUBAREA 9A.2

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 550.00
REPRESENTATIVE CHANNEL SLOPE = 0.0550
CHANNEL BASE (FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.379
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 2.47
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.23
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 7.43
Tc (MIN.) = 15.85
SUBAREA AREA (ACRES) = 3.57 SUBAREA RUNOFF (CFS) = 3.62
AREA-AVERAGE RUNOFF COEFFICIENT = 0.300
TOTAL AREA (ACRES) = 4.0 PEAK FLOW RATE (CFS) = 4.02

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 1.56
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 100.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0300
FLOW LENGTH (FEET) = 278.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.55
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 4.02
PIPE TRAVEL TIME (MIN.) = 0.54 Tc (MIN.) = 16.40
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 100.00 = 928.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.02	16.40	3.307	3.97

LONGEST FLOWPATH FROM NODE 21.00 TO NODE 100.00 = 928.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.51	9.10	4.836	0.15

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 293.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	2.75	9.10	4.836
2	4.38	16.40	3.307

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 4.38 Tc(MIN.) = 16.40
TOTAL AREA(ACRES) = 4.1

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<
=====

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(Feet) = 35.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 7.4 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 5.36
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.38
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 16.50
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 101.00 = 963.00 FEET.

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 16.50
RAINFALL INTENSITY(INCH/HR) = 3.29
TOTAL STREAM AREA(ACRES) = 4.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.38

+-----+
| DRAINAGE SUBAREA 10A.1 |
+-----+

FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 21

```

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(FEET) = 64.00
UPSTREAM ELEVATION(FEET) = 1359.00
DOWNSTREAM ELEVATION(FEET) = 1357.60
ELEVATION DIFFERENCE(FEET) = 1.40
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.548
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.15
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.15

+-----+
| DRAINAGE SUBAREA 10A.2 |
+-----+

*****
FLOW PROCESS FROM NODE 25.00 TO NODE 101.00 IS CODE = 51
-----

>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 71.00
REPRESENTATIVE CHANNEL SLOPE = 0.0270
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.458
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.35
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.94
AVERAGE FLOW DEPTH(FEET) = 0.01 TRAVEL TIME(MIN.) = 1.26
Tc(MIN.) = 5.81
SUBAREA AREA(ACRES) = 0.09 SUBAREA RUNOFF(CFS) = 0.40
AREA-AVERAGE RUNOFF COEFFICIENT = 0.690
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.53

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.02 FLOW VELOCITY(FEET/SEC.) = 0.98
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 101.00 = 135.00 FEET.

*****
FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1
-----

>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.81
RAINFALL INTENSITY(INCH/HR) = 6.46
TOTAL STREAM AREA(ACRES) = 0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.53

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 4.38 16.50 3.293 4.12
2 0.53 5.81 6.458 0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY

```


NUMBER	(CFS)	(MIN.)	(INCH/HOUR)
1	2.07	5.81	6.458
2	4.65	16.50	3.293

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE (CFS) = 4.65 Tc (MIN.) = 16.50
 TOTAL AREA (ACRES) = 4.2
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 101.00 = 963.00 FEET.

DRAINAGE SUBAREA 1B

 FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 36.00
 UPSTREAM ELEVATION (FEET) = 1364.30
 DOWNSTREAM ELEVATION (FEET) = 1363.90
 ELEVATION DIFFERENCE (FEET) = 0.40
 SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.711
 10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF (CFS) = 0.30
 TOTAL AREA (ACRES) = 0.05 TOTAL RUNOFF (CFS) = 0.30

 FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
 FLOW LENGTH (FEET) = 94.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.1 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.96
 GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.30
 PIPE TRAVEL TIME (MIN.) = 0.53 Tc (MIN.) = 3.24
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 33.00 = 130.00 FEET.

DRAINAGE SUBAREA 2B

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
 SUBAREA AREA (ACRES) = 0.05 SUBAREA RUNOFF (CFS) = 0.30
 TOTAL AREA (ACRES) = 0.1 TOTAL RUNOFF (CFS) = 0.60
 TC (MIN.) = 3.24

 FLOW PROCESS FROM NODE 33.00 TO NODE 200.00 IS CODE = 41

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-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0640
FLOW LENGTH(FEET) = 13.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 6.0 INCH PIPE IS 2.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.09
GIVEN PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.60
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 3.27
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 200.00 = 143.00 FEET.

*****
FLOW PROCESS FROM NODE 200.00 TO NODE 34.00 IS CODE = 41
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0180
FLOW LENGTH(FEET) = 99.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.36
GIVEN PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.60
PIPE TRAVEL TIME(MIN.) = 0.38 Tc(MIN.) = 3.65
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 34.00 = 242.00 FEET.

*****
FLOW PROCESS FROM NODE 34.00 TO NODE 36.00 IS CODE = 51
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 12.00
REPRESENTATIVE CHANNEL SLOPE = 0.0170
CHANNEL BASE(FEET) = 6.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50
CHANNEL FLOW THRU SUBAREA(CFS) = 0.60
FLOW VELOCITY(FEET/SEC.) = 1.77 FLOW DEPTH(FEET) = 0.06
TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 3.76
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 36.00 = 254.00 FEET.

*****
FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 3.76
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.10
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.60

+-----+
| DRAINAGE SUBAREA 3B |
+-----+

*****
FLOW PROCESS FROM NODE 35.00 TO NODE 36.00 IS CODE = 21
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 89.00
UPSTREAM ELEVATION(FEET) = 1364.10

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DOWNSTREAM ELEVATION(FEET) = 1358.00
ELEVATION DIFFERENCE(FEET) = 6.10
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.324
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.24
TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.24

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.32
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.24

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.60	3.76	7.114	0.10
2	0.24	2.32	7.114	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	0.61	2.32	7.114
2	0.84	3.76	7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 0.84 Tc(MIN.) = 3.76
TOTAL AREA(ACRES) = 0.1
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 36.00 = 254.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 201.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
FLOW LENGTH(FEET) = 1.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.84
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.84
PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 3.77
LONGEST FLOWPATH FROM NODE 31.00 TO NODE 201.00 = 255.00 FEET.

+-----+
| DRAINAGE SUBAREA 4B |
+-----+

FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 49.00


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UPSTREAM ELEVATION(FEET) = 1363.00
DOWNSTREAM ELEVATION(FEET) = 1362.20
ELEVATION DIFFERENCE(FEET) = 0.80
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.782
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.66
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.66

*****
FLOW PROCESS FROM NODE 38.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.52
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.66
PIPE TRAVEL TIME(MIN.) = 0.30 Tc(MIN.) = 3.09
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 202.00 = 113.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 3.09
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.11
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.66

+-----+
| DRAINAGE SUBAREA 5B |
+-----+

*****
FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 54.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1362.00
ELEVATION DIFFERENCE(FEET) = 2.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.122
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.36
TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.36

*****
FLOW PROCESS FROM NODE 40.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0380
FLOW LENGTH(FEET) = 15.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.70
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1

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PIPE-FLOW(CFS) =          0.36
PIPE TRAVEL TIME(MIN.) =    0.05    Tc(MIN.) =    2.17
LONGEST FLOWPATH FROM NODE    39.00 TO NODE    202.00 =    69.00 FEET.

*****
FLOW PROCESS FROM NODE    202.00 TO NODE    202.00 IS CODE =    1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS =    2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM    2 ARE:
TIME OF CONCENTRATION(MIN.) =    2.17
RAINFALL INTENSITY(INCH/HR) =    7.11
TOTAL STREAM AREA(ACRES) =    0.06
PEAK FLOW RATE(CFS) AT CONFLUENCE =          0.36

** CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/ HOUR)    (ACRE)
    1        0.66    3.09        7.114        0.11
    2        0.36    2.17        7.114        0.06

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR    2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/ HOUR)
    1        0.82    2.17        7.114
    2        1.02    3.09        7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =          1.02    Tc(MIN.) =    3.09
TOTAL AREA(ACRES) =          0.2
LONGEST FLOWPATH FROM NODE    37.00 TO NODE    202.00 =    113.00 FEET.

*****
FLOW PROCESS FROM NODE    202.00 TO NODE    203.00 IS CODE =    41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =    0.1100
FLOW LENGTH(FEET) =    54.00    MANNING'S N =    0.012
DEPTH OF FLOW IN    12.0 INCH PIPE IS    2.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    9.40
GIVEN PIPE DIAMETER(INCH) =    12.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =          1.02
PIPE TRAVEL TIME(MIN.) =    0.10    Tc(MIN.) =    3.18
LONGEST FLOWPATH FROM NODE    37.00 TO NODE    203.00 =    167.00 FEET.

+-----+
| DRAINAGE SUBAREA 6B |
+-----+

*****
FLOW PROCESS FROM NODE    203.00 TO NODE    203.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
    10 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =    96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) =          0.00    SUBAREA RUNOFF(CFS) =          0.03
TOTAL AREA(ACRES) =          0.2    TOTAL RUNOFF(CFS) =          1.05

```


TC(MIN.) = 3.18

DRAINAGE SUBAREA 7B.1

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(Feet) = 80.00
UPSTREAM ELEVATION(Feet) = 1361.10
DOWNSTREAM ELEVATION(Feet) = 1358.60
ELEVATION DIFFERENCE(Feet) = 2.50
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.863
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.48
TOTAL AREA(ACRES) = 0.08 TOTAL RUNOFF(CFS) = 0.48

DRAINAGE SUBAREA 7B.2

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(Feet) = 36.00
REPRESENTATIVE CHANNEL SLOPE = 0.0360
CHANNEL BASE(Feet) = 8.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(Feet) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.54
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 2.01
AVERAGE FLOW DEPTH(Feet) = 0.03 TRAVEL TIME(MIN.) = 0.30
Tc(MIN.) = 3.16
SUBAREA AREA(ACRES) = 0.02 SUBAREA RUNOFF(CFS) = 0.12
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.60

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(Feet) = 0.04 FLOW VELOCITY(Feet/Sec.) = 2.12
LONGEST FLOWPATH FROM NODE 42.00 TO NODE 44.00 = 116.00 FEET.

DRAINAGE SUBAREA 1C.1

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000

SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 85
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 1401.00
 DOWNSTREAM ELEVATION(FEET) = 1397.00
 ELEVATION DIFFERENCE(FEET) = 4.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.072
 10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.844
 SUBAREA RUNOFF(CFS) = 0.45
 TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.45

DRAINAGE SUBAREA 1C.2

FLOW PROCESS FROM NODE 52.00 TO NODE 53.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

CHANNEL LENGTH THRU SUBAREA(FEET) = 380.00
 REPRESENTATIVE CHANNEL SLOPE = 0.0710
 CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
 10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.576
 RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 79
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.80
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FT/SEC.) = 1.16
 AVERAGE FLOW DEPTH(FT) = 0.03 TRAVEL TIME(MIN.) = 5.45
 Tc(MIN.) = 14.52
 SUBAREA AREA(ACRES) = 1.80 SUBAREA RUNOFF(CFS) = 2.70
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
 TOTAL AREA(ACRES) = 2.1 PEAK FLOW RATE(CFS) = 3.04

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FT) = 0.04 FLOW VELOCITY(FT/SEC.) = 1.43
 LONGEST FLOWPATH FROM NODE 51.00 TO NODE 53.00 = 480.00 FEET.

DRAINAGE SUBAREA 1C.3

FLOW PROCESS FROM NODE 53.00 TO NODE 54.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

REPRESENTATIVE SLOPE = 0.0260
 CHANNEL LENGTH THRU SUBAREA(FT) = 385.00
 "V" GUTTER WIDTH(FT) = 5.00 GUTTER HIKE(FT) = 0.090
 PAVEMENT LIP(FT) = 0.010 MANNING'S N = .0150
 PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
 MAXIMUM DEPTH(FT) = 0.50
 10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.312
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
 SOIL CLASSIFICATION IS "A"
 S.C.S. CURVE NUMBER (AMC II) = 92
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.72
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FT/SEC.) = 3.50
 AVERAGE FLOW DEPTH(FT) = 0.19 FLOOD WIDTH(FT) = 13.52
 "V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.83 Tc(MIN.) = 16.35
 SUBAREA AREA(ACRES) = 0.50 SUBAREA RUNOFF(CFS) = 1.37
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.484
 TOTAL AREA(ACRES) = 2.6 PEAK FLOW RATE(CFS) = 4.19

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH(FEET) = 0.19 FLOOD WIDTH(FEET) = 14.30

FLOW VELOCITY(FEET/SEC.) = 3.57 DEPTH*VELOCITY(FT*FT/SEC) = 0.69

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 54.00 = 865.00 FEET.

FLOW PROCESS FROM NODE 54.00 TO NODE 55.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0130

FLOW LENGTH(FEET) = 166.00 MANNING'S N = 0.012

DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.0 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 6.32

GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 4.19

PIPE TRAVEL TIME(MIN.) = 0.44 Tc(MIN.) = 16.79

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 55.00 = 1031.00 FEET.

+-----+
| DRAINAGE SUBAREA 2C |
+-----+

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.257

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400

SOIL CLASSIFICATION IS "C"

S.C.S. CURVE NUMBER (AMC II) = 96

AREA-AVERAGE RUNOFF COEFFICIENT = 0.5499

SUBAREA AREA(ACRES) = 0.59 SUBAREA RUNOFF(CFS) = 1.61

TOTAL AREA(ACRES) = 3.2 TOTAL RUNOFF(CFS) = 5.73

Tc(MIN.) = 16.79

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<

=====

+-----+
| DRAINAGE SUBAREA 3C.1 |
+-----+

FLOW PROCESS FROM NODE 56.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700

SOIL CLASSIFICATION IS "C"

S.C.S. CURVE NUMBER (AMC II) = 96

INITIAL SUBAREA FLOW-LENGTH(FEET) = 90.00

UPSTREAM ELEVATION(FEET) = 1370.50

DOWNSTREAM ELEVATION(FEET) = 1365.90

ELEVATION DIFFERENCE(FEET) = 4.60

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.280

10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114

NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

SUBAREA RUNOFF(CFS) = 0.25

TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.25

DRAINAGE SUBAREA 3C.2

FLOW PROCESS FROM NODE 57.00 TO NODE 58.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
CHANNEL LENGTH THRU SUBAREA(Feet) = 297.00
"V" GUTTER WIDTH(Feet) = 5.00 GUTTER HIKE(Feet) = 0.090
PAVEMENT LIP(Feet) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.20000
MAXIMUM DEPTH(Feet) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.20
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 2.73
AVERAGE FLOW DEPTH(Feet) = 0.13 FLOOD WIDTH(Feet) = 5.32
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.81 Tc(MIN.) = 4.09
SUBAREA AREA(ACRES) = 0.32 SUBAREA RUNOFF(CFS) = 1.91
AREA-AVERAGE RUNOFF COEFFICIENT = 0.843
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 2.16

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(Feet) = 0.17 FLOOD WIDTH(Feet) = 5.68
FLOW VELOCITY(Feet/Sec.) = 3.39 DEPTH*VELOCITY(Ft*Ft/Sec) = 0.57
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 58.00 = 387.00 FEET.

FLOW PROCESS FROM NODE 58.00 TO NODE 300.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
FLOW LENGTH(Feet) = 34.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.4 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 5.02
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.16
PIPE TRAVEL TIME(MIN.) = 0.11 Tc(MIN.) = 4.20
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 4.20
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.16

DRAINAGE SUBAREA 4C.1

FLOW PROCESS FROM NODE 59.00 TO NODE 60.00 IS CODE = 21

```

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1370.50
DOWNSTREAM ELEVATION (FEET) = 1365.40
ELEVATION DIFFERENCE (FEET) = 5.10
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 1.571
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.19
TOTAL AREA (ACRES) = 0.03 TOTAL RUNOFF (CFS) = 0.19

+-----+
| DRAINAGE SUBAREA 4C.2 |
+-----+

*****
FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
CHANNEL LENGTH THRU SUBAREA (FEET) = 70.00
REPRESENTATIVE CHANNEL SLOPE = 0.0900
CHANNEL BASE (FEET) = 3.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 1.00
10 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.22
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.60
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 0.45
Tc (MIN.) = 2.02
SUBAREA AREA (ACRES) = 0.01 SUBAREA RUNOFF (CFS) = 0.06
AREA-AVERAGE RUNOFF COEFFICIENT = 0.870
TOTAL AREA (ACRES) = 0.0 PEAK FLOW RATE (CFS) = 0.25

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.03 FLOW VELOCITY (FEET/SEC.) = 2.97
LONGEST FLOWPATH FROM NODE 59.00 TO NODE 61.00 = 130.00 FEET.

*****
FLOW PROCESS FROM NODE 61.00 TO NODE 300.00 IS CODE = 41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 27.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.1 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.65
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.25
PIPE TRAVEL TIME (MIN.) = 0.17 Tc (MIN.) = 2.19
LONGEST FLOWPATH FROM NODE 59.00 TO NODE 300.00 = 157.00 FEET.

*****
FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

```


TIME OF CONCENTRATION(MIN.) = 2.19
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.25

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.16	4.20	7.114	0.36
2	0.25	2.19	7.114	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.37	2.19	7.114
2	2.41	4.20	7.114

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2.41 Tc(MIN.) = 4.20
TOTAL AREA(ACRES) = 0.4
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 55.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 5.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.63
GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.41
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 4.36
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 55.00 = 485.00 FEET.

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.41	4.36	7.114	0.40

LONGEST FLOWPATH FROM NODE 56.00 TO NODE 55.00 = 485.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	5.73	16.79	3.257	3.20

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 55.00 = 1031.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	3.90	4.36	7.114
2	6.83	16.79	3.257

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 6.83 Tc(MIN.) = 16.79
TOTAL AREA(ACRES) = 3.6

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 12

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>>>>CLEAR MEMORY BANK # 2 <<<<
=====
*****
FLOW PROCESS FROM NODE      55.00 TO NODE      62.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 173.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.54
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.83
PIPE TRAVEL TIME(MIN.) = 0.38 Tc(MIN.) = 17.17
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 62.00 = 1204.00 FEET.

+-----+
| DRAINAGE SUBAREA 5C |
+-----+

*****
FLOW PROCESS FROM NODE      62.00 TO NODE      62.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.210
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6108
SUBAREA AREA(ACRES) = 0.44 SUBAREA RUNOFF(CFS) = 1.19
TOTAL AREA(ACRES) = 4.0 TOTAL RUNOFF(CFS) = 7.92
TC(MIN.) = 17.17

*****
FLOW PROCESS FROM NODE      62.00 TO NODE      66.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 84.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 9.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.70
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.92
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 17.38
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 66.00 = 1288.00 FEET.

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.38
RAINFALL INTENSITY(INCH/HR) = 3.18
TOTAL STREAM AREA(ACRES) = 4.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.92

+-----+
| DRAINAGE SUBAREA 6C.1 |
+-----+

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FLOW PROCESS FROM NODE 63.00 TO NODE 64.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 53.00
UPSTREAM ELEVATION(FEET) = 1359.30
DOWNSTREAM ELEVATION(FEET) = 1358.70
ELEVATION DIFFERENCE(FEET) = 0.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.269
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.66
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.66

+-----+
| DRAINAGE SUBAREA 6C.2 |
+-----+

FLOW PROCESS FROM NODE 64.00 TO NODE 65.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<
=====

REPRESENTATIVE SLOPE = 0.0110
CHANNEL LENGTH THRU SUBAREA(FEET) = 188.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.70
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.15
AVERAGE FLOW DEPTH(FEET) = 0.16 FLOOD WIDTH(FEET) = 11.33
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.46 Tc(MIN.) = 4.73
SUBAREA AREA(ACRES) = 0.35 SUBAREA RUNOFF(CFS) = 2.09
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.5 PEAK FLOW RATE(CFS) = 2.75

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.19 FLOOD WIDTH(FEET) = 14.30
FLOW VELOCITY(FEET/SEC.) = 2.35 DEPTH*VELOCITY(FT*FT/SEC) = 0.45
LONGEST FLOWPATH FROM NODE 63.00 TO NODE 65.00 = 241.00 FEET.

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 105.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.00
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.75
PIPE TRAVEL TIME(MIN.) = 0.29 Tc(MIN.) = 5.02
LONGEST FLOWPATH FROM NODE 63.00 TO NODE 66.00 = 346.00 FEET.

+-----+
| DRAINAGE SUBAREA 7C |
+-----+


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+-----+
*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.098
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) = 0.24 SUBAREA RUNOFF(CFS) = 1.43
TOTAL AREA(ACRES) = 0.7 TOTAL RUNOFF(CFS) = 4.17
TC(MIN.) = 5.02

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.02
RAINFALL INTENSITY(INCH/HR) = 7.10
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.17

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 7.92 17.38 3.185 4.04
2 4.17 5.02 7.098 0.70

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 6.46 5.02 7.098
2 9.79 17.38 3.185

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 9.79 Tc(MIN.) = 17.38
TOTAL AREA(ACRES) = 4.7
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 66.00 = 1288.00 FEET.

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      67.00 IS CODE = 41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.08
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.79
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 17.56
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 67.00 = 1364.00 FEET.

+-----+
| DRAINAGE SUBAREA 8C |
+-----+

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*****
FLOW PROCESS FROM NODE      67.00 TO NODE      67.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.164
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6567
SUBAREA AREA(ACRES) = 0.33 SUBAREA RUNOFF(CFS) = 0.87
TOTAL AREA(ACRES) = 5.1 TOTAL RUNOFF(CFS) = 10.53
TC(MIN.) = 17.56

*****
FLOW PROCESS FROM NODE      67.00 TO NODE      301.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0130
FLOW LENGTH(FEET) = 6.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 10.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.96
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.53
PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 17.57
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 301.00 = 1370.00 FEET.

+-----+
| DRAINAGE SUBAREA 9C.1 |
+-----+

*****
FLOW PROCESS FROM NODE      68.00 TO NODE      69.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1356.20
DOWNSTREAM ELEVATION(FEET) = 1355.00
ELEVATION DIFFERENCE(FEET) = 1.20
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.988
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.12
TOTAL AREA(ACRES) = 0.02 TOTAL RUNOFF(CFS) = 0.12

+-----+
| DRAINAGE SUBAREA 9C.2 |
+-----+

*****
FLOW PROCESS FROM NODE      69.00 TO NODE      70.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 129.00
REPRESENTATIVE CHANNEL SLOPE = 0.0080
CHANNEL BASE(FEET) = 1.50 "Z" FACTOR = 13.090
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

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LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.44
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.49
AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 1.44
Tc(MIN.) = 4.43
SUBAREA AREA(ACRES) = 0.11 SUBAREA RUNOFF(CFS) = 0.65
AREA-AVERAGE RUNOFF COEFFICIENT = 0.830
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.77

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.13 FLOW VELOCITY(FEET/SEC.) = 1.75
LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 189.00 FEET.

-----+-----
DRAINAGE SUBAREA 1D.1
-----+-----

FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1452.00
DOWNSTREAM ELEVATION(FEET) = 1429.00
ELEVATION DIFFERENCE(FEET) = 23.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.684
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.899
SUBAREA RUNOFF(CFS) = 1.01
TOTAL AREA(ACRES) = 0.57 TOTAL RUNOFF(CFS) = 1.01

-----+-----
DRAINAGE SUBAREA 1D.2
-----+-----

FLOW PROCESS FROM NODE 82.00 TO NODE 83.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 651.00
REPRESENTATIVE CHANNEL SLOPE = 0.0940
CHANNEL BASE(FEET) = 100.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.508
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .2700
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 45
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.63
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.31
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 8.27
Tc(MIN.) = 14.96
SUBAREA AREA(ACRES) = 5.18 SUBAREA RUNOFF(CFS) = 4.91
AREA-AVERAGE RUNOFF COEFFICIENT = 0.273
TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 5.51

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.56
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 83.00 = 751.00 FEET.

DRAINAGE SUBAREA 1D.3

FLOW PROCESS FROM NODE 83.00 TO NODE 84.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0560
CHANNEL LENGTH THRU SUBAREA(Feet) = 181.00
"V" GUTTER WIDTH(Feet) = 3.00 GUTTER HIKE(Feet) = 0.800
PAVEMENT LIP(Feet) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.20000
MAXIMUM DEPTH(Feet) = 1.00
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.470
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4800
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 81
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.59
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 11.71
AVERAGE FLOW DEPTH(Feet) = 0.80 FLOOD WIDTH(Feet) = 3.00
"V" GUTTER FLOW TRAVEL TIME(Min.) = 0.26 Tc(Min.) = 15.22
SUBAREA AREA(ACRES) = 0.10 SUBAREA RUNOFF(CFS) = 0.17
AREA-AVERAGE RUNOFF COEFFICIENT = 0.277
TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 5.61

NOTE:TRAVEL TIME ESTIMATES BASED ON NORMAL DEPTH
IN A FLOWING-FULL GUTTER(NORMAL DEPTH = GUTTER HIKE)

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(Feet) = 0.80 FLOOD WIDTH(Feet) = 3.00
FLOW VELOCITY(Feet/Sec.) = 11.71 DEPTH*VELOCITY(Ft*Ft/Sec) = 9.37
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 84.00 = 932.00 FEET.

FLOW PROCESS FROM NODE 84.00 TO NODE 86.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(Feet) = 41.00
REPRESENTATIVE CHANNEL SLOPE = 0.0120
CHANNEL BASE(Feet) = 4.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(Feet) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 5.61
FLOW VELOCITY(Feet/Sec.) = 4.53 FLOW DEPTH(Feet) = 0.30
TRAVEL TIME(Min.) = 0.15 Tc(Min.) = 15.37
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 86.00 = 973.00 FEET.

FLOW PROCESS FROM NODE 86.00 TO NODE 86.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(Min.) = 15.37
RAINFALL INTENSITY(INCH/HR) = 3.45
TOTAL STREAM AREA(ACRES) = 5.85
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.61

DRAINAGE SUBAREA 2D

FLOW PROCESS FROM NODE 85.00 TO NODE 86.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 22.00
UPSTREAM ELEVATION(FEET) = 1358.00
DOWNSTREAM ELEVATION(FEET) = 1357.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.078
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.114
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.26
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.26

FLOW PROCESS FROM NODE 86.00 TO NODE 86.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.08
RAINFALL INTENSITY(INCH/HR) = 7.11
TOTAL STREAM AREA(ACRES) = 0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.26

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	5.61	15.37	3.448	5.85
2	0.26	4.08	7.114	0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.75	4.08	7.114
2	5.74	15.37	3.448

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 5.74 Tc(MIN.) = 15.37
TOTAL AREA(ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 86.00 = 973.00 FEET.

+-----+
| DRAINAGE SUBAREA 1E |
+-----+

FLOW PROCESS FROM NODE 91.00 TO NODE 92.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .2000
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(FEET) = 86.00
UPSTREAM ELEVATION(FEET) = 1358.00
DOWNSTREAM ELEVATION(FEET) = 1351.10
ELEVATION DIFFERENCE(FEET) = 6.90
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.505
10 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.474

SUBAREA RUNOFF (CFS) = 0.13
TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 0.13

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 0.1 TC (MIN.) = 7.50
PEAK FLOW RATE (CFS) = 0.13

=====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

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Anaheim Hills, CA 92808
714-685-6860

***** DESCRIPTION OF STUDY *****
* 19-132 GS VALLEY EXPANSION (28435 LIZARD ROCKS ROAD) *
* PROPOSED CONDITION *
* 25-YEAR STORM EVENT *

FILE NAME: 19132P25.DAT
TIME/DATE OF STUDY: 11:12 05/19/2021

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 25.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| DRAINAGE SUBAREA 1A.1 |
+-----+

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(Feet) = 69.00
UPSTREAM ELEVATION(Feet) = 1364.00
DOWNSTREAM ELEVATION(Feet) = 1362.10
ELEVATION DIFFERENCE(Feet) = 1.90
SUBAREA OVERLAND TIME OF FLOW(Min.) = 4.374
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.16

TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.16

DRAINAGE SUBAREA 1A.2

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 162.00
REPRESENTATIVE CHANNEL SLOPE = 0.0390
CHANNEL BASE(FEET) = 45.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.50
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.727
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.39
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.70
AVERAGE FLOW DEPTH(FEET) = 0.01 TRAVEL TIME(MIN.) = 3.87
Tc(MIN.) = 8.24
SUBAREA AREA(ACRES) = 0.12 SUBAREA RUNOFF(CFS) = 0.47
AREA-AVERAGE RUNOFF COEFFICIENT = 0.690
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.59

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.02 FLOW VELOCITY(FEET/SEC.) = 0.73
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 231.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 100.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH(FEET) = 62.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 2.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.17
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.59
PIPE TRAVEL TIME(MIN.) = 0.33 Tc(MIN.) = 8.57
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 293.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

DRAINAGE SUBAREA 2A.1

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 49.00
UPSTREAM ELEVATION(FEET) = 1363.70

DOWNSTREAM ELEVATION (FEET) = 1363.20
 ELEVATION DIFFERENCE (FEET) = 0.50
 SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.254
 25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF (CFS) = 0.20
 TOTAL AREA (ACRES) = 0.03 TOTAL RUNOFF (CFS) = 0.20

DRAINAGE SUBAREA 2A.2

 FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

REPRESENTATIVE SLOPE = 0.0060
 CHANNEL LENGTH THRU SUBAREA (FEET) = 33.00
 "V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
 PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
 PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
 MAXIMUM DEPTH (FEET) = 0.50
 25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.46
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.40
 AVERAGE FLOW DEPTH (FEET) = 0.14 FLOOD WIDTH (FEET) = 6.28
 "V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.39 Tc (MIN.) = 3.65
 SUBAREA AREA (ACRES) = 0.08 SUBAREA RUNOFF (CFS) = 0.53
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
 TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 0.73

END OF SUBAREA "V" GUTTER HYDRAULICS:
 DEPTH (FEET) = 0.17 FLOOD WIDTH (FEET) = 8.56
 FLOW VELOCITY (FEET/SEC.) = 1.46 DEPTH*VELOCITY (FT*FT/SEC) = 0.24
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 82.00 FEET.

 FLOW PROCESS FROM NODE 6.00 TO NODE 400.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

REPRESENTATIVE SLOPE = 0.0050
 FLOW LENGTH (FEET) = 127.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 9.0 INCH PIPE IS 5.1 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 2.85
 ESTIMATED PIPE DIAMETER (INCH) = 9.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.73
 PIPE TRAVEL TIME (MIN.) = 0.74 Tc (MIN.) = 4.39
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 400.00 = 209.00 FEET.

 FLOW PROCESS FROM NODE 400.00 TO NODE 400.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 4.39
 RAINFALL INTENSITY (INCH/HR) = 7.90
 TOTAL STREAM AREA (ACRES) = 0.11
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.73

DRAINAGE SUBAREA 3A.1

 FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400	
SOIL CLASSIFICATION IS "C"	
S.C.S. CURVE NUMBER (AMC II) = 96	
INITIAL SUBAREA FLOW-LENGTH (FEET) =	60.00
UPSTREAM ELEVATION (FEET) =	1364.50
DOWNSTREAM ELEVATION (FEET) =	1363.30
ELEVATION DIFFERENCE (FEET) =	1.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) =	2.877
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904	
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.	
SUBAREA RUNOFF (CFS) =	0.53
TOTAL AREA (ACRES) =	0.08
TOTAL RUNOFF (CFS) =	0.53

DRAINAGE SUBAREA 3A.2

 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0060	
CHANNEL LENGTH THRU SUBAREA (FEET) =	90.00
"V" GUTTER WIDTH (FEET) =	3.00
GUTTER HIKE (FEET) =	0.100
PAVEMENT LIP (FEET) =	0.010
MANNING'S N =	.0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000	
MAXIMUM DEPTH (FEET) =	0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904	
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.	
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400	
SOIL CLASSIFICATION IS "C"	
S.C.S. CURVE NUMBER (AMC II) = 96	
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) =	1.99
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) =	1.68
AVERAGE FLOW DEPTH (FEET) =	0.23
FLOOD WIDTH (FEET) =	14.50
"V" GUTTER FLOW TRAVEL TIME (MIN.) =	0.89
Tc (MIN.) =	3.77
SUBAREA AREA (ACRES) =	0.44
SUBAREA RUNOFF (CFS) =	2.92
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840	
TOTAL AREA (ACRES) =	0.5
PEAK FLOW RATE (CFS) =	3.45

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH (FEET) =	0.26	FLOOD WIDTH (FEET) =	18.46
FLOW VELOCITY (FEET/SEC.) =	1.88	DEPTH*VELOCITY (FT*FT/SEC) =	0.50
LONGEST FLOWPATH FROM NODE	7.00 TO NODE	9.00 =	150.00 FEET.

 FLOW PROCESS FROM NODE 9.00 TO NODE 400.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0170	
FLOW LENGTH (FEET) =	6.00
MANNING'S N =	0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.6 INCHES	
PIPE-FLOW VELOCITY (FEET/SEC.) =	6.61
ESTIMATED PIPE DIAMETER (INCH) =	12.00
NUMBER OF PIPES =	1
PIPE-FLOW (CFS) =	3.45
PIPE TRAVEL TIME (MIN.) =	0.02
Tc (MIN.) =	3.79


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LONGEST FLOWPATH FROM NODE      7.00 TO NODE      400.00 =      156.00 FEET.

*****
FLOW PROCESS FROM NODE      400.00 TO NODE      400.00 IS CODE =      1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      2 ARE:
TIME OF CONCENTRATION(MIN.) =      3.79
RAINFALL INTENSITY(INCH/HR) =      7.90
TOTAL STREAM AREA(ACRES) =      0.52
PEAK FLOW RATE(CFS) AT CONFLUENCE =      3.45

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
1            0.73      4.39      7.904      0.11
2            3.45      3.79      7.904      0.52

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR      2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
1            4.08      3.79      7.904
2            4.18      4.39      7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      4.18      Tc(MIN.) =      4.39
TOTAL AREA(ACRES) =      0.6
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      400.00 =      209.00 FEET.

*****
FLOW PROCESS FROM NODE      400.00 TO NODE      401.00 IS CODE =      31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE =      0.0050
FLOW LENGTH(FEET) =      227.00      MANNING'S N =      0.012
DEPTH OF FLOW IN      15.0 INCH PIPE IS      11.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      4.32
ESTIMATED PIPE DIAMETER(INCH) =      15.00      NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      4.18
PIPE TRAVEL TIME(MIN.) =      0.87      Tc(MIN.) =      5.27
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      401.00 =      436.00 FEET.

*****
FLOW PROCESS FROM NODE      401.00 TO NODE      401.00 IS CODE =      1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      1 ARE:
TIME OF CONCENTRATION(MIN.) =      5.27
RAINFALL INTENSITY(INCH/HR) =      7.65
TOTAL STREAM AREA(ACRES) =      0.63
PEAK FLOW RATE(CFS) AT CONFLUENCE =      4.18

+-----+
| DRAINAGE SUBAREA 4A.1 |
+-----+

*****
FLOW PROCESS FROM NODE      10.00 TO NODE      11.00 IS CODE =      21
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>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.60
ELEVATION DIFFERENCE (FEET) = 0.90
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.167
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.13
TOTAL AREA (ACRES) = 0.02 TOTAL RUNOFF (CFS) = 0.13

+-----+
| DRAINAGE SUBAREA 4A.2 |
+-----+

*****
FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 91
-----
>>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
CHANNEL LENGTH THRU SUBAREA (FEET) = 168.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.69
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.54
AVERAGE FLOW DEPTH (FEET) = 0.22 FLOOD WIDTH (FEET) = 13.89
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.82 Tc (MIN.) = 4.99
SUBAREA AREA (ACRES) = 0.47 SUBAREA RUNOFF (CFS) = 3.12
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 3.25

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.27 FLOOD WIDTH (FEET) = 18.77
FLOW VELOCITY (FEET/SEC.) = 1.72 DEPTH*VELOCITY (FT*FT/SEC) = 0.46
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 12.00 = 228.00 FEET.

*****
FLOW PROCESS FROM NODE 12.00 TO NODE 401.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 8.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 8.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.69
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.25
PIPE TRAVEL TIME (MIN.) = 0.02 Tc (MIN.) = 5.01
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 401.00 = 236.00 FEET.

*****
FLOW PROCESS FROM NODE 401.00 TO NODE 401.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

```


TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 5.01
 RAINFALL INTENSITY(INCH/HR) = 7.89
 TOTAL STREAM AREA(ACRES) = 0.49
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.25

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.18	5.27	7.645	0.63
2	3.25	5.01	7.895	0.49

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.30	5.01	7.895
2	7.33	5.27	7.645

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 7.33 Tc(MIN.) = 5.27
 TOTAL AREA(ACRES) = 1.1
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 401.00 = 436.00 FEET.

 FLOW PROCESS FROM NODE 401.00 TO NODE 404.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0050
 FLOW LENGTH(FEET) = 89.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 14.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.92
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 7.33
 PIPE TRAVEL TIME(MIN.) = 0.30 Tc(MIN.) = 5.57
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 404.00 = 525.00 FEET.

 FLOW PROCESS FROM NODE 404.00 TO NODE 404.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<

-----+-----
 | DRAINAGE SUBAREA 5A |
 -----+-----

 FLOW PROCESS FROM NODE 13.00 TO NODE 14.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 47.00
 UPSTREAM ELEVATION(FEET) = 1364.30
 DOWNSTREAM ELEVATION(FEET) = 1363.00
 ELEVATION DIFFERENCE(FEET) = 1.30
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.286
 25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.20
 TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.20

FLOW PROCESS FROM NODE 14.00 TO NODE 402.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH (FEET) = 60.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.07
ESTIMATED PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.20
PIPE TRAVEL TIME (MIN.) = 0.48 Tc (MIN.) = 2.77
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 402.00 = 107.00 FEET.

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 2.77
RAINFALL INTENSITY (INCH/HR) = 7.90
TOTAL STREAM AREA (ACRES) = 0.03
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.20

+-----+
| DRAINAGE SUBAREA 6A |
+-----+

FLOW PROCESS FROM NODE 15.00 TO NODE 402.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 64.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.50
ELEVATION DIFFERENCE (FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.227
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.80
TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 0.80

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 3.23
RAINFALL INTENSITY (INCH/HR) = 7.90
TOTAL STREAM AREA (ACRES) = 0.12
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.80

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.20	2.77	7.904	0.03
2	0.80	3.23	7.904	0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	0.88	2.77	7.904
2	1.00	3.23	7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.00 Tc (MIN.) = 3.23
TOTAL AREA (ACRES) = 0.1
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 402.00 = 107.00 FEET.

FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH (FEET) = 71.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 9.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.39
ESTIMATED PIPE DIAMETER (INCH) = 9.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.00
PIPE TRAVEL TIME (MIN.) = 0.22 Tc (MIN.) = 3.45
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 403.00 = 178.00 FEET.

FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<

+-----+
DRAINAGE SUBAREA 7A.1
+-----+

FLOW PROCESS FROM NODE 16.00 TO NODE 17.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.30
ELEVATION DIFFERENCE (FEET) = 1.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.877
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.27
TOTAL AREA (ACRES) = 0.04 TOTAL RUNOFF (CFS) = 0.27

+-----+
DRAINAGE SUBAREA 7A.2
+-----+

FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0120


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CHANNEL LENGTH THRU SUBAREA (FEET) = 83.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.86
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.25
AVERAGE FLOW DEPTH (FEET) = 0.20 FLOOD WIDTH (FEET) = 11.76
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.61 Tc (MIN.) = 3.49
SUBAREA AREA (ACRES) = 0.48 SUBAREA RUNOFF (CFS) = 3.19
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 3.45

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.24 FLOOD WIDTH (FEET) = 15.87
FLOW VELOCITY (FEET/SEC.) = 2.48 DEPTH*VELOCITY (FT*FT/SEC) = 0.59
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 18.00 = 143.00 FEET.

*****
FLOW PROCESS FROM NODE 18.00 TO NODE 403.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH (FEET) = 75.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 9.6 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.18
ESTIMATED PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.45
PIPE TRAVEL TIME (MIN.) = 0.30 Tc (MIN.) = 3.79
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 218.00 FEET.

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 3.45 3.79 7.904 0.52
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 218.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 1.00 3.45 7.904 0.15
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 403.00 = 178.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 4.13 3.45 7.904
2 4.45 3.79 7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE (CFS) = 4.45 Tc (MIN.) = 3.79
TOTAL AREA (ACRES) = 0.7

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 3 <<<<
=====

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FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0600
FLOW LENGTH(FEET) = 13.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.40
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.45
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 3.81
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 231.00 FEET.

FLOW PROCESS FROM NODE 404.00 TO NODE 404.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.45	3.81	7.904	0.67

LONGEST FLOWPATH FROM NODE 16.00 TO NODE 404.00 = 231.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	7.33	5.57	7.375	1.12

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 404.00 = 525.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.47	3.81	7.904
2	11.48	5.57	7.375

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 11.48 Tc(MIN.) = 5.57
TOTAL AREA(ACRES) = 1.8

FLOW PROCESS FROM NODE 404.00 TO NODE 404.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<

+-----+
| DRAINAGE SUBAREA 8A |
+-----+

FLOW PROCESS FROM NODE 19.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 91
INITIAL SUBAREA FLOW-LENGTH(FEET) = 81.00
UPSTREAM ELEVATION(FEET) = 1363.70
DOWNSTREAM ELEVATION(FEET) = 1358.10
ELEVATION DIFFERENCE(FEET) = 5.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.487
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

SUBAREA RUNOFF(CFS) = 0.44
TOTAL AREA(ACRES) = 0.08 TOTAL RUNOFF(CFS) = 0.44

DRAINAGE SUBAREA 9A.1

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1404.00
DOWNSTREAM ELEVATION(FEET) = 1399.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.422
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.647
SUBAREA RUNOFF(CFS) = 0.68
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 0.68

DRAINAGE SUBAREA 9A.2

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 550.00
REPRESENTATIVE CHANNEL SLOPE = 0.0550
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.825
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.78
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.31
AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 6.98
Tc(MIN.) = 15.40
SUBAREA AREA(ACRES) = 3.57 SUBAREA RUNOFF(CFS) = 4.10
AREA-AVERAGE RUNOFF COEFFICIENT = 0.300
TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 4.56

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 1.56
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 100.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0300
FLOW LENGTH(FEET) = 278.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.78
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.56
PIPE TRAVEL TIME(MIN.) = 0.53 Tc(MIN.) = 15.93


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LONGEST FLOWPATH FROM NODE      21.00 TO NODE      100.00 =      928.00 FEET.

*****
FLOW PROCESS FROM NODE      100.00 TO NODE      100.00 IS CODE =  11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
  1          4.56      15.93      3.743      3.97
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      100.00 =      928.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
  1          0.59      8.57      5.586      0.15
LONGEST FLOWPATH FROM NODE      1.00 TO NODE      100.00 =      293.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
  1          3.04      8.57      5.586
  2          4.95      15.93      3.743

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      4.95  Tc(MIN.) =  15.93
TOTAL AREA(ACRES) =      4.1

*****
FLOW PROCESS FROM NODE      100.00 TO NODE      100.00 IS CODE =  12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      100.00 TO NODE      101.00 IS CODE =  41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE =  0.0100
FLOW LENGTH(FEET) =  35.00  MANNING'S N =  0.013
DEPTH OF FLOW IN  24.0 INCH PIPE IS  7.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  5.55
GIVEN PIPE DIAMETER(INCH) =  24.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  4.95
PIPE TRAVEL TIME(MIN.) =  0.11  Tc(MIN.) =  16.04
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      101.00 =      963.00 FEET.

*****
FLOW PROCESS FROM NODE      101.00 TO NODE      101.00 IS CODE =  1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =  16.04
RAINFALL INTENSITY(INCH/HR) =  3.73
TOTAL STREAM AREA(ACRES) =  4.12
PEAK FLOW RATE(CFS) AT CONFLUENCE =      4.95

+-----+
| DRAINAGE SUBAREA 10A.1 |
+-----+

*****
FLOW PROCESS FROM NODE      24.00 TO NODE      25.00 IS CODE =  21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(FEET) = 64.00
UPSTREAM ELEVATION(FEET) = 1359.00
DOWNSTREAM ELEVATION(FEET) = 1357.60
ELEVATION DIFFERENCE(FEET) = 1.40
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.548
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.16
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.16

+-----+
| DRAINAGE SUBAREA 10A.2 |
+-----+

*****
FLOW PROCESS FROM NODE 25.00 TO NODE 101.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 71.00
REPRESENTATIVE CHANNEL SLOPE = 0.0270
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.286
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.39
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.05
AVERAGE FLOW DEPTH(FEET) = 0.01 TRAVEL TIME(MIN.) = 1.13
Tc(MIN.) = 5.67
SUBAREA AREA(ACRES) = 0.09 SUBAREA RUNOFF(CFS) = 0.45
AREA-AVERAGE RUNOFF COEFFICIENT = 0.690
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.60

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.02 FLOW VELOCITY(FEET/SEC.) = 1.11
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 101.00 = 135.00 FEET.

*****
FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.67
RAINFALL INTENSITY(INCH/HR) = 7.29
TOTAL STREAM AREA(ACRES) = 0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.60

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 4.95 16.04 3.727 4.12
2 0.60 5.67 7.286 0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

```


STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	2.36	5.67	7.286
2	5.26	16.04	3.727

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 5.26 Tc(MIN.) = 16.04
 TOTAL AREA(ACRES) = 4.2
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 101.00 = 963.00 FEET.

DRAINAGE SUBAREA 1B

 FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 36.00
 UPSTREAM ELEVATION(FEET) = 1364.30
 DOWNSTREAM ELEVATION(FEET) = 1363.90
 ELEVATION DIFFERENCE(FEET) = 0.40
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.711
 25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.33
 TOTAL AREA(ACRES) = 0.05 TOTAL RUNOFF(CFS) = 0.33

 FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
 FLOW LENGTH(FEET) = 94.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 3.03
 GIVEN PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.33
 PIPE TRAVEL TIME(MIN.) = 0.52 Tc(MIN.) = 3.23
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 33.00 = 130.00 FEET.

DRAINAGE SUBAREA 2B

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
 SUBAREA AREA(ACRES) = 0.05 SUBAREA RUNOFF(CFS) = 0.33
 TOTAL AREA(ACRES) = 0.1 TOTAL RUNOFF(CFS) = 0.66
 TC(MIN.) = 3.23

```

FLOW PROCESS FROM NODE      33.00 TO NODE      200.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0640
FLOW LENGTH(FEET) =   13.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS  2.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   7.22
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =           0.66
PIPE TRAVEL TIME(MIN.) =   0.03  Tc(MIN.) =   3.26
LONGEST FLOWPATH FROM NODE      31.00 TO NODE      200.00 =      143.00 FEET.

*****
FLOW PROCESS FROM NODE      200.00 TO NODE      34.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0180
FLOW LENGTH(FEET) =   99.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS  4.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   4.42
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =           0.66
PIPE TRAVEL TIME(MIN.) =   0.37  Tc(MIN.) =   3.63
LONGEST FLOWPATH FROM NODE      31.00 TO NODE      34.00 =      242.00 FEET.

*****
FLOW PROCESS FROM NODE      34.00 TO NODE      36.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =   12.00
REPRESENTATIVE CHANNEL SLOPE =  0.0170
CHANNEL BASE(FEET) =   6.00  "Z" FACTOR =  0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  0.50
CHANNEL FLOW THRU SUBAREA(CFS) =           0.66
FLOW VELOCITY(FEET/SEC.) =   1.90  FLOW DEPTH(FEET) =   0.06
TRAVEL TIME(MIN.) =   0.11  Tc(MIN.) =   3.74
LONGEST FLOWPATH FROM NODE      31.00 TO NODE      36.00 =      254.00 FEET.

*****
FLOW PROCESS FROM NODE      36.00 TO NODE      36.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =   3.74
RAINFALL INTENSITY(INCH/HR) =   7.90
TOTAL STREAM AREA(ACRES) =   0.10
PEAK FLOW RATE(CFS) AT CONFLUENCE =           0.66

+-----+
| DRAINAGE SUBAREA 3B |
+-----+

*****
FLOW PROCESS FROM NODE      35.00 TO NODE      36.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
INITIAL SUBAREA FLOW-LENGTH(FEET) =   89.00

```


UPSTREAM ELEVATION (FEET) = 1364.10
 DOWNSTREAM ELEVATION (FEET) = 1358.00
 ELEVATION DIFFERENCE (FEET) = 6.10
 SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.324
 25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF (CFS) = 0.27
 TOTAL AREA (ACRES) = 0.04 TOTAL RUNOFF (CFS) = 0.27

 FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION (MIN.) = 2.32
 RAINFALL INTENSITY (INCH/HR) = 7.90
 TOTAL STREAM AREA (ACRES) = 0.04
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.27

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.66	3.74	7.904	0.10
2	0.27	2.32	7.904	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	0.68	2.32	7.904
2	0.93	3.74	7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE (CFS) = 0.93 Tc (MIN.) = 3.74
 TOTAL AREA (ACRES) = 0.1
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 36.00 = 254.00 FEET.

 FLOW PROCESS FROM NODE 36.00 TO NODE 201.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====
 REPRESENTATIVE SLOPE = 0.0200
 FLOW LENGTH (FEET) = 1.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.4 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.00
 GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 0.93
 PIPE TRAVEL TIME (MIN.) = 0.00 Tc (MIN.) = 3.74
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 201.00 = 255.00 FEET.

+-----+
 | DRAINAGE SUBAREA 4B |
 +-----+

 FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96


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INITIAL SUBAREA FLOW-LENGTH(FEET) = 49.00
UPSTREAM ELEVATION(FEET) = 1363.00
DOWNSTREAM ELEVATION(FEET) = 1362.20
ELEVATION DIFFERENCE(FEET) = 0.80
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.782
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.73
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.73

*****
FLOW PROCESS FROM NODE 38.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.62
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.73
PIPE TRAVEL TIME(MIN.) = 0.29 Tc(MIN.) = 3.08
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 202.00 = 113.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 3.08
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.11
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.73

+-----+
| DRAINAGE SUBAREA 5B |
+-----+

*****
FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 54.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1362.00
ELEVATION DIFFERENCE(FEET) = 2.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.122
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.40
TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.40

*****
FLOW PROCESS FROM NODE 40.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0380
FLOW LENGTH(FEET) = 15.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 1.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.92

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GIVEN PIPE DIAMETER(INCH) = 12.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.40
PIPE TRAVEL TIME(MIN.) = 0.05    Tc(MIN.) = 2.17
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 202.00 = 69.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.17
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.06
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.40

** CONFLUENCE DATA **
STREAM    RUNOFF      Tc      INTENSITY      AREA
NUMBER    (CFS)      (MIN.)  (INCH/ HOUR)   (ACRE)
  1        0.73      3.08      7.904         0.11
  2        0.40      2.17      7.904         0.06

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF      Tc      INTENSITY
NUMBER    (CFS)      (MIN.)  (INCH/ HOUR)
  1        0.91      2.17      7.904
  2        1.13      3.08      7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1.13    Tc(MIN.) = 3.08
TOTAL AREA(ACRES) = 0.2
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 202.00 = 113.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.1100
FLOW LENGTH(FEET) = 54.00    MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.68
GIVEN PIPE DIAMETER(INCH) = 12.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.13
PIPE TRAVEL TIME(MIN.) = 0.09    Tc(MIN.) = 3.17
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 203.00 = 167.00 FEET.

+-----+
| DRAINAGE SUBAREA 6B |
+-----+

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) = 0.00    SUBAREA RUNOFF(CFS) = 0.03

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TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 1.16
TC (MIN.) = 3.17

DRAINAGE SUBAREA 7B.1

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 80.00
UPSTREAM ELEVATION (FEET) = 1361.10
DOWNSTREAM ELEVATION (FEET) = 1358.60
ELEVATION DIFFERENCE (FEET) = 2.50
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.863
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.53
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.53

DRAINAGE SUBAREA 7B.2

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 36.00
REPRESENTATIVE CHANNEL SLOPE = 0.0360
CHANNEL BASE (FEET) = 8.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.60
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.12
AVERAGE FLOW DEPTH (FEET) = 0.04 TRAVEL TIME (MIN.) = 0.28
Tc (MIN.) = 3.15
SUBAREA AREA (ACRES) = 0.02 SUBAREA RUNOFF (CFS) = 0.13
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 0.66

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.04 FLOW VELOCITY (FEET/SEC.) = 2.02
LONGEST FLOWPATH FROM NODE 42.00 TO NODE 44.00 = 116.00 FEET.

DRAINAGE SUBAREA 1C.1

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1401.00
DOWNSTREAM ELEVATION(FEET) = 1397.00
ELEVATION DIFFERENCE(FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.072
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.382
SUBAREA RUNOFF(CFS) = 0.50
TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.50

DRAINAGE SUBAREA 1C.2

FLOW PROCESS FROM NODE 52.00 TO NODE 53.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 380.00
REPRESENTATIVE CHANNEL SLOPE = 0.0710
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.012
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.01
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.21
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 5.24
Tc(MIN.) = 14.31
SUBAREA AREA(ACRES) = 1.80 SUBAREA RUNOFF(CFS) = 3.03
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA(ACRES) = 2.1 PEAK FLOW RATE(CFS) = 3.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.61
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 53.00 = 480.00 FEET.

DRAINAGE SUBAREA 1C.3

FLOW PROCESS FROM NODE 53.00 TO NODE 54.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0260
CHANNEL LENGTH THRU SUBAREA(FEET) = 385.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.717
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.18
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.56
AVERAGE FLOW DEPTH(FEET) = 0.19 FLOOD WIDTH(FEET) = 14.30
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.80 Tc(MIN.) = 16.11
SUBAREA AREA(ACRES) = 0.50 SUBAREA RUNOFF(CFS) = 1.54
AREA-AVERAGE RUNOFF COEFFICIENT = 0.484

TOTAL AREA (ACRES) = 2.6 PEAK FLOW RATE (CFS) = 4.70

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH (FEET) = 0.20 FLOOD WIDTH (FEET) = 15.08

FLOW VELOCITY (FEET/SEC.) = 3.65 DEPTH*VELOCITY (FT*FT/SEC) = 0.73

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 54.00 = 865.00 FEET.

FLOW PROCESS FROM NODE 54.00 TO NODE 55.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

REPRESENTATIVE SLOPE = 0.0130

FLOW LENGTH (FEET) = 166.00 MANNING'S N = 0.012

DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.6 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 6.50

GIVEN PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 4.70

PIPE TRAVEL TIME (MIN.) = 0.43 Tc (MIN.) = 16.53

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 55.00 = 1031.00 FEET.

DRAINAGE SUBAREA 2C

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.655

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400

SOIL CLASSIFICATION IS "C"

S.C.S. CURVE NUMBER (AMC II) = 96

AREA-AVERAGE RUNOFF COEFFICIENT = 0.5499

SUBAREA AREA (ACRES) = 0.59 SUBAREA RUNOFF (CFS) = 1.81

TOTAL AREA (ACRES) = 3.2 TOTAL RUNOFF (CFS) = 6.43

Tc (MIN.) = 16.53

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<

DRAINAGE SUBAREA 3C.1

FLOW PROCESS FROM NODE 56.00 TO NODE 57.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700

SOIL CLASSIFICATION IS "C"

S.C.S. CURVE NUMBER (AMC II) = 96

INITIAL SUBAREA FLOW-LENGTH (FEET) = 90.00

UPSTREAM ELEVATION (FEET) = 1370.50

DOWNSTREAM ELEVATION (FEET) = 1365.90

ELEVATION DIFFERENCE (FEET) = 4.60

SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.280

25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904

NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

SUBAREA RUNOFF (CFS) = 0.28

TOTAL AREA (ACRES) = 0.04 TOTAL RUNOFF (CFS) = 0.28

DRAINAGE SUBAREA 3C.2

FLOW PROCESS FROM NODE 57.00 TO NODE 58.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
CHANNEL LENGTH THRU SUBAREA (FEET) = 297.00
"V" GUTTER WIDTH (FEET) = 5.00 GUTTER HIKE (FEET) = 0.090
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.20000
MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.34
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.82
AVERAGE FLOW DEPTH (FEET) = 0.14 FLOOD WIDTH (FEET) = 5.38
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.75 Tc (MIN.) = 4.03
SUBAREA AREA (ACRES) = 0.32 SUBAREA RUNOFF (CFS) = 2.12
AREA-AVERAGE RUNOFF COEFFICIENT = 0.843
TOTAL AREA (ACRES) = 0.4 PEAK FLOW RATE (CFS) = 2.40

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.18 FLOOD WIDTH (FEET) = 5.76
FLOW VELOCITY (FEET/SEC.) = 3.52 DEPTH*VELOCITY (FT*FT/SEC) = 0.62
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 58.00 = 387.00 FEET.

FLOW PROCESS FROM NODE 58.00 TO NODE 300.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
FLOW LENGTH (FEET) = 34.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.16
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 2.40
PIPE TRAVEL TIME (MIN.) = 0.11 Tc (MIN.) = 4.14
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 4.14
RAINFALL INTENSITY (INCH/HR) = 7.90
TOTAL STREAM AREA (ACRES) = 0.36
PEAK FLOW RATE (CFS) AT CONFLUENCE = 2.40

DRAINAGE SUBAREA 4C.1

FLOW PROCESS FROM NODE 59.00 TO NODE 60.00 IS CODE = 21


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-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1370.50
DOWNSTREAM ELEVATION (FEET) = 1365.40
ELEVATION DIFFERENCE (FEET) = 5.10
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 1.571
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.21
TOTAL AREA (ACRES) = 0.03 TOTAL RUNOFF (CFS) = 0.21

+-----+
| DRAINAGE SUBAREA 4C.2 |
+-----+

*****
FLOW PROCESS FROM NODE 60.00 TO NODE 61.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA (FEET) = 70.00
REPRESENTATIVE CHANNEL SLOPE = 0.0900
CHANNEL BASE (FEET) = 3.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 1.00
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.24
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.89
AVERAGE FLOW DEPTH (FEET) = 0.03 TRAVEL TIME (MIN.) = 0.40
Tc (MIN.) = 1.97
SUBAREA AREA (ACRES) = 0.01 SUBAREA RUNOFF (CFS) = 0.07
AREA-AVERAGE RUNOFF COEFFICIENT = 0.870
TOTAL AREA (ACRES) = 0.0 PEAK FLOW RATE (CFS) = 0.28

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.03 FLOW VELOCITY (FEET/SEC.) = 2.74
LONGEST FLOWPATH FROM NODE 59.00 TO NODE 61.00 = 130.00 FEET.

*****
FLOW PROCESS FROM NODE 61.00 TO NODE 300.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 27.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.72
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.28
PIPE TRAVEL TIME (MIN.) = 0.17 Tc (MIN.) = 2.14
LONGEST FLOWPATH FROM NODE 59.00 TO NODE 300.00 = 157.00 FEET.

*****
FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2

```


CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.14
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.28

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.40	4.14	7.904	0.36
2	0.28	2.14	7.904	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.51	2.14	7.904
2	2.67	4.14	7.904

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 2.67 Tc(MIN.) = 4.14
TOTAL AREA(ACRES) = 0.4
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 55.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 5.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.83
GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.67
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 4.30
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 55.00 = 485.00 FEET.

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.67	4.30	7.904	0.40

LONGEST FLOWPATH FROM NODE 56.00 TO NODE 55.00 = 485.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.43	16.53	3.655	3.20

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 55.00 = 1031.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	4.35	4.30	7.904
2	7.67	16.53	3.655

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 7.67 Tc(MIN.) = 16.53
TOTAL AREA(ACRES) = 3.6

FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 12


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-----
>>>>CLEAR MEMORY BANK # 2 <<<<
=====

*****
FLOW PROCESS FROM NODE      55.00 TO NODE      62.00 IS CODE =  41
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

REPRESENTATIVE SLOPE =  0.0150
FLOW LENGTH(FEET) =  173.00  MANNING'S N =  0.012
DEPTH OF FLOW IN  18.0 INCH PIPE IS  9.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  7.76
GIVEN PIPE DIAMETER(INCH) =  18.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  7.67
PIPE TRAVEL TIME(MIN.) =  0.37  Tc(MIN.) =  16.91
LONGEST FLOWPATH FROM NODE      51.00 TO NODE      62.00 =  1204.00 FEET.

+-----+
| DRAINAGE SUBAREA 5C |
+-----+

*****
FLOW PROCESS FROM NODE      62.00 TO NODE      62.00 IS CODE =  81
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====

25 YEAR RAINFALL INTENSITY(INCH/HOUR) =  3.603
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6108
SUBAREA AREA(ACRES) =  0.44  SUBAREA RUNOFF(CFS) =  1.33
TOTAL AREA(ACRES) =  4.0  TOTAL RUNOFF(CFS) =  8.89
TC(MIN.) =  16.91

*****
FLOW PROCESS FROM NODE      62.00 TO NODE      66.00 IS CODE =  41
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

REPRESENTATIVE SLOPE =  0.0100
FLOW LENGTH(FEET) =  84.00  MANNING'S N =  0.012
DEPTH OF FLOW IN  24.0 INCH PIPE IS  10.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  6.91
GIVEN PIPE DIAMETER(INCH) =  24.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  8.89
PIPE TRAVEL TIME(MIN.) =  0.20  Tc(MIN.) =  17.11
LONGEST FLOWPATH FROM NODE      51.00 TO NODE      66.00 =  1288.00 FEET.

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE =  1
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====

TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =  17.11
RAINFALL INTENSITY(INCH/HR) =  3.58
TOTAL STREAM AREA(ACRES) =  4.04
PEAK FLOW RATE(CFS) AT CONFLUENCE =  8.89

+-----+
| DRAINAGE SUBAREA 6C.1 |
+-----+

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FLOW PROCESS FROM NODE 63.00 TO NODE 64.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 53.00
UPSTREAM ELEVATION (FEET) = 1359.30
DOWNSTREAM ELEVATION (FEET) = 1358.70
ELEVATION DIFFERENCE (FEET) = 0.60
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.269
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.73
TOTAL AREA (ACRES) = 0.11 TOTAL RUNOFF (CFS) = 0.73

+-----+
| DRAINAGE SUBAREA 6C.2 |
+-----+

FLOW PROCESS FROM NODE 64.00 TO NODE 65.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
CHANNEL LENGTH THRU SUBAREA (FEET) = 188.00
"V" GUTTER WIDTH (FEET) = 5.00 GUTTER HIKE (FEET) = 0.090
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
25 YEAR RAINFALL INTENSITY (INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 1.89
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.19
AVERAGE FLOW DEPTH (FEET) = 0.17 FLOOD WIDTH (FEET) = 11.95
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.43 Tc (MIN.) = 4.70
SUBAREA AREA (ACRES) = 0.35 SUBAREA RUNOFF (CFS) = 2.32
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 3.05

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.20 FLOOD WIDTH (FEET) = 15.08
FLOW VELOCITY (FEET/SEC.) = 2.37 DEPTH*VELOCITY (FT*FT/SEC) = 0.48
LONGEST FLOWPATH FROM NODE 63.00 TO NODE 65.00 = 241.00 FEET.

FLOW PROCESS FROM NODE 65.00 TO NODE 66.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH (FEET) = 105.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.15
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.05
PIPE TRAVEL TIME (MIN.) = 0.28 Tc (MIN.) = 4.98
LONGEST FLOWPATH FROM NODE 63.00 TO NODE 66.00 = 346.00 FEET.

+-----+
| DRAINAGE SUBAREA 7C |
+-----+


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*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE = 81
*****
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) = 0.24 SUBAREA RUNOFF(CFS) = 1.59
TOTAL AREA(ACRES) = 0.7 TOTAL RUNOFF(CFS) = 4.65
TC(MIN.) = 4.98

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE = 1
*****
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.98
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.65

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           8.89      17.11      3.575           4.04
2           4.65      4.98       7.904           0.70

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           7.24      4.98       7.904
2          10.99      17.11      3.575

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 10.99 Tc(MIN.) = 17.11
TOTAL AREA(ACRES) = 4.7
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 66.00 = 1288.00 FEET.

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      67.00 IS CODE = 41
*****
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 76.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.30
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.99
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 17.28
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 67.00 = 1364.00 FEET.

*****
DRAINAGE SUBAREA 8C
*****

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+-----+
*****
FLOW PROCESS FROM NODE      67.00 TO NODE      67.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
    25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.552
    LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
    SOIL CLASSIFICATION IS "A"
    S.C.S. CURVE NUMBER (AMC II) = 92
    AREA-AVERAGE RUNOFF COEFFICIENT = 0.6567
    SUBAREA AREA(ACRES) = 0.33 SUBAREA RUNOFF(CFS) = 0.97
    TOTAL AREA(ACRES) = 5.1 TOTAL RUNOFF(CFS) = 11.83
    TC(MIN.) = 17.28

*****
FLOW PROCESS FROM NODE      67.00 TO NODE      301.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
    REPRESENTATIVE SLOPE = 0.0130
    FLOW LENGTH(FEET) = 6.00 MANNING'S N = 0.012
    DEPTH OF FLOW IN 24.0 INCH PIPE IS 11.2 INCHES
    PIPE-FLOW VELOCITY(FEET/SEC.) = 8.20
    GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
    PIPE-FLOW(CFS) = 11.83
    PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 17.29
    LONGEST FLOWPATH FROM NODE 51.00 TO NODE 301.00 = 1370.00 FEET.

+-----+
| DRAINAGE SUBAREA 9C.1 |
+-----+

*****
FLOW PROCESS FROM NODE      68.00 TO NODE      69.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
    LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
    SOIL CLASSIFICATION IS "A"
    S.C.S. CURVE NUMBER (AMC II) = 92
    INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
    UPSTREAM ELEVATION(FEET) = 1356.20
    DOWNSTREAM ELEVATION(FEET) = 1355.00
    ELEVATION DIFFERENCE(FEET) = 1.20
    SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.988
    25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
    NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
    SUBAREA RUNOFF(CFS) = 0.13
    TOTAL AREA(ACRES) = 0.02 TOTAL RUNOFF(CFS) = 0.13

+-----+
| DRAINAGE SUBAREA 9C.2 |
+-----+

*****
FLOW PROCESS FROM NODE      69.00 TO NODE      70.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
    CHANNEL LENGTH THRU SUBAREA(FEET) = 129.00
    REPRESENTATIVE CHANNEL SLOPE = 0.0080
    CHANNEL BASE(FEET) = 1.50 "Z" FACTOR = 13.090
    MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50

```


25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
 SOIL CLASSIFICATION IS "A"
 S.C.S. CURVE NUMBER (AMC II) = 92
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.49
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.53
 AVERAGE FLOW DEPTH(FEET) = 0.11 TRAVEL TIME(MIN.) = 1.41
 Tc(MIN.) = 4.39
 SUBAREA AREA(ACRES) = 0.11 SUBAREA RUNOFF(CFS) = 0.72
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.830
 TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.85

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.14 FLOW VELOCITY(FEET/SEC.) = 1.79
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 189.00 FEET.

DRAINAGE SUBAREA 1D.1

 FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 85
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 1452.00
 DOWNSTREAM ELEVATION(FEET) = 1429.00
 ELEVATION DIFFERENCE(FEET) = 23.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.684
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.554
 SUBAREA RUNOFF(CFS) = 1.12
 TOTAL AREA(ACRES) = 0.57 TOTAL RUNOFF(CFS) = 1.12

DRAINAGE SUBAREA 1D.2

 FLOW PROCESS FROM NODE 82.00 TO NODE 83.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

CHANNEL LENGTH THRU SUBAREA(FEET) = 651.00
 REPRESENTATIVE CHANNEL SLOPE = 0.0940
 CHANNEL BASE(FEET) = 100.00 "Z" FACTOR = 0.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
 25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.077
 RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .2700
 SOIL CLASSIFICATION IS "A"
 S.C.S. CURVE NUMBER (AMC II) = 45
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.13
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.49
 AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 7.27
 Tc(MIN.) = 13.95
 SUBAREA AREA(ACRES) = 5.18 SUBAREA RUNOFF(CFS) = 5.70
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.273
 TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 6.40

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.81

LONGEST FLOWPATH FROM NODE 81.00 TO NODE 83.00 = 751.00 FEET.

DRAINAGE SUBAREA 1D.3

FLOW PROCESS FROM NODE 83.00 TO NODE 84.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0560
CHANNEL LENGTH THRU SUBAREA(FEET) = 181.00
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.800
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.20000
MAXIMUM DEPTH(FEET) = 1.00
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.030
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4800
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 81
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.50
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 11.71
AVERAGE FLOW DEPTH(FEET) = 0.80 FLOOD WIDTH(FEET) = 3.00
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 0.26 Tc(MIN.) = 14.21
SUBAREA AREA(ACRES) = 0.10 SUBAREA RUNOFF(CFS) = 0.19
AREA-AVERAGE RUNOFF COEFFICIENT = 0.277
TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 6.52

NOTE:TRAVEL TIME ESTIMATES BASED ON NORMAL DEPTH
IN A FLOWING-FULL GUTTER(NORMAL DEPTH = GUTTER HIKE)

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH(FEET) = 0.80 FLOOD WIDTH(FEET) = 3.00
FLOW VELOCITY(FEET/SEC.) = 11.71 DEPTH*VELOCITY(FT*FT/SEC) = 9.37
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 84.00 = 932.00 FEET.

FLOW PROCESS FROM NODE 84.00 TO NODE 86.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 41.00
REPRESENTATIVE CHANNEL SLOPE = 0.0120
CHANNEL BASE(FEET) = 4.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
CHANNEL FLOW THRU SUBAREA(CFS) = 6.52
FLOW VELOCITY(FEET/SEC.) = 4.77 FLOW DEPTH(FEET) = 0.33
TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 14.35
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 86.00 = 973.00 FEET.

FLOW PROCESS FROM NODE 86.00 TO NODE 86.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 14.35
RAINFALL INTENSITY(INCH/HR) = 4.00
TOTAL STREAM AREA(ACRES) = 5.85
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.52

DRAINAGE SUBAREA 2D


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+-----+
*****
FLOW PROCESS FROM NODE      85.00 TO NODE      86.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 22.00
UPSTREAM ELEVATION(FEET) = 1358.00
DOWNSTREAM ELEVATION(FEET) = 1357.00
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.078
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.904
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.28
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.28
*****
FLOW PROCESS FROM NODE      86.00 TO NODE      86.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.08
RAINFALL INTENSITY(INCH/HR) = 7.90
TOTAL STREAM AREA(ACRES) = 0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.28

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1           6.52      14.35      4.004           5.85
2           0.28      4.08      7.904           0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1           2.14      4.08      7.904
2           6.66      14.35      4.004

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 6.66 Tc(MIN.) = 14.35
TOTAL AREA(ACRES) = 6.0
LONGEST FLOWPATH FROM NODE      81.00 TO NODE      86.00 = 973.00 FEET.

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+-----+
| DRAINAGE SUBAREA 1E |
+-----+

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*****
FLOW PROCESS FROM NODE      91.00 TO NODE      92.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .2000
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(FEET) = 86.00
UPSTREAM ELEVATION(FEET) = 1358.00
DOWNSTREAM ELEVATION(FEET) = 1351.10
ELEVATION DIFFERENCE(FEET) = 6.90

```


SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.505
25 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.083
SUBAREA RUNOFF(CFS) = 0.15
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.15

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 0.1 TC(MIN.) = 7.50
PEAK FLOW RATE(CFS) = 0.15

=====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1510

Analysis prepared by:

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Anaheim Hills, CA 92808
714-685-6860

***** DESCRIPTION OF STUDY *****
* 19-132 GS VALLEY EXPANSION (28435 LIZARD ROCKS ROAD) *
* PROPOSED CONDITION *
* 100-YEAR STORM EVENT *

FILE NAME: 19132P00.DAT
TIME/DATE OF STUDY: 11:13 05/19/2021

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 4.500
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| DRAINAGE SUBAREA 1A.1 |
+-----+

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(FEET) = 69.00
UPSTREAM ELEVATION(FEET) = 1364.00
DOWNSTREAM ELEVATION(FEET) = 1362.10
ELEVATION DIFFERENCE(FEET) = 1.90
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.374
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.25

TOTAL AREA (ACRES) = 0.03 TOTAL RUNOFF (CFS) = 0.25

DRAINAGE SUBAREA 1A.2

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 162.00
REPRESENTATIVE CHANNEL SLOPE = 0.0390
CHANNEL BASE (FEET) = 45.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH (FEET) = 1.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 8.539
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.62
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 0.68
AVERAGE FLOW DEPTH (FEET) = 0.02 TRAVEL TIME (MIN.) = 3.94
Tc (MIN.) = 8.32
SUBAREA AREA (ACRES) = 0.12 SUBAREA RUNOFF (CFS) = 0.71
AREA-AVERAGE RUNOFF COEFFICIENT = 0.690
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 0.88

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.03 FLOW VELOCITY (FEET/SEC.) = 0.76
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 231.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 100.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH (FEET) = 62.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 3.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 3.58
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.88
PIPE TRAVEL TIME (MIN.) = 0.29 Tc (MIN.) = 8.61
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 100.00 = 293.00 FEET.

FLOW PROCESS FROM NODE 100.00 TO NODE 100.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

DRAINAGE SUBAREA 2A.1

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 49.00
UPSTREAM ELEVATION (FEET) = 1363.70

DOWNSTREAM ELEVATION (FEET) = 1363.20
 ELEVATION DIFFERENCE (FEET) = 0.50
 SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.254
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF (CFS) = 0.30
 TOTAL AREA (ACRES) = 0.03 TOTAL RUNOFF (CFS) = 0.30

DRAINAGE SUBAREA 2A.2

 FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 91

>>>> COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

REPRESENTATIVE SLOPE = 0.0060
 CHANNEL LENGTH THRU SUBAREA (FEET) = 33.00
 "V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
 PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
 PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
 MAXIMUM DEPTH (FEET) = 0.50
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.70
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.47
 AVERAGE FLOW DEPTH (FEET) = 0.16 FLOOD WIDTH (FEET) = 8.26
 "V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.38 Tc (MIN.) = 3.63
 SUBAREA AREA (ACRES) = 0.08 SUBAREA RUNOFF (CFS) = 0.80
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
 TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 1.10

END OF SUBAREA "V" GUTTER HYDRAULICS:
 DEPTH (FEET) = 0.19 FLOOD WIDTH (FEET) = 10.69
 FLOW VELOCITY (FEET/SEC.) = 1.55 DEPTH*VELOCITY (FT*FT/SEC) = 0.29
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 82.00 FEET.

 FLOW PROCESS FROM NODE 6.00 TO NODE 400.00 IS CODE = 31

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>> USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

REPRESENTATIVE SLOPE = 0.0050
 FLOW LENGTH (FEET) = 127.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 9.0 INCH PIPE IS 6.7 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.10
 ESTIMATED PIPE DIAMETER (INCH) = 9.00 NUMBER OF PIPES = 1
 PIPE-FLOW (CFS) = 1.10
 PIPE TRAVEL TIME (MIN.) = 0.68 Tc (MIN.) = 4.31
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 400.00 = 209.00 FEET.

 FLOW PROCESS FROM NODE 400.00 TO NODE 400.00 IS CODE = 1

>>>> DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION (MIN.) = 4.31
 RAINFALL INTENSITY (INCH/HR) = 11.86
 TOTAL STREAM AREA (ACRES) = 0.11
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.10

DRAINAGE SUBAREA 3A.1

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.30
ELEVATION DIFFERENCE (FEET) = 1.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.877
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.80
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.80

DRAINAGE SUBAREA 3A.2

FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0060
CHANNEL LENGTH THRU SUBAREA (FEET) = 90.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.100
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH (FEET) = 0.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 2.99
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 1.81
AVERAGE FLOW DEPTH (FEET) = 0.25 FLOOD WIDTH (FEET) = 17.40
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.83 Tc (MIN.) = 3.70
SUBAREA AREA (ACRES) = 0.44 SUBAREA RUNOFF (CFS) = 4.38
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.5 PEAK FLOW RATE (CFS) = 5.18

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.30 FLOOD WIDTH (FEET) = 22.12
FLOW VELOCITY (FEET/SEC.) = 2.01 DEPTH*VELOCITY (FT*FT/SEC) = 0.60
LONGEST FLOWPATH FROM NODE 7.00 TO NODE 9.00 = 150.00 FEET.

FLOW PROCESS FROM NODE 9.00 TO NODE 400.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0170
FLOW LENGTH (FEET) = 6.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.4 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.37
ESTIMATED PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 5.18
PIPE TRAVEL TIME (MIN.) = 0.01 Tc (MIN.) = 3.72


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LONGEST FLOWPATH FROM NODE      7.00 TO NODE      400.00 =      156.00 FEET.
*****
FLOW PROCESS FROM NODE      400.00 TO NODE      400.00 IS CODE =      1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      2 ARE:
TIME OF CONCENTRATION(MIN.) =      3.72
RAINFALL INTENSITY(INCH/HR) =     11.86
TOTAL STREAM AREA(ACRES) =      0.52
PEAK FLOW RATE(CFS) AT CONFLUENCE =      5.18

** CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HR)      (ACRE)
    1         1.10      4.31      11.856         0.11
    2         5.18      3.72      11.856         0.52

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR      2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HR)
    1         6.12      3.72      11.856
    2         6.27      4.31      11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      6.27      Tc(MIN.) =      4.31
TOTAL AREA(ACRES) =      0.6
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      400.00 =      209.00 FEET.
*****
FLOW PROCESS FROM NODE      400.00 TO NODE      401.00 IS CODE =     31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE =      0.0050
FLOW LENGTH(FEET) =     227.00      MANNING'S N =      0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =      4.82
ESTIMATED PIPE DIAMETER(INCH) =     18.00      NUMBER OF PIPES =      1
PIPE-FLOW(CFS) =      6.27
PIPE TRAVEL TIME(MIN.) =      0.78      Tc(MIN.) =      5.10
LONGEST FLOWPATH FROM NODE      4.00 TO NODE      401.00 =     436.00 FEET.
*****
FLOW PROCESS FROM NODE      401.00 TO NODE      401.00 IS CODE =      1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =      2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM      1 ARE:
TIME OF CONCENTRATION(MIN.) =      5.10
RAINFALL INTENSITY(INCH/HR) =     11.71
TOTAL STREAM AREA(ACRES) =      0.63
PEAK FLOW RATE(CFS) AT CONFLUENCE =      6.27

+-----+
| DRAINAGE SUBAREA 4A.1 |
+-----+
*****
FLOW PROCESS FROM NODE      10.00 TO NODE      11.00 IS CODE =     21
-----

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>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 60.00
UPSTREAM ELEVATION(FEET) = 1364.50
DOWNSTREAM ELEVATION(FEET) = 1363.60
ELEVATION DIFFERENCE(FEET) = 0.90
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.167
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.20
TOTAL AREA(ACRES) = 0.02 TOTAL RUNOFF(CFS) = 0.20

+-----+
| DRAINAGE SUBAREA 4A.2 |
+-----+

*****
FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 91
-----
>>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
CHANNEL LENGTH THRU SUBAREA(FEET) = 168.00
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.100
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.54
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.64
AVERAGE FLOW DEPTH(FEET) = 0.25 FLOOD WIDTH(FEET) = 16.79
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.70 Tc(MIN.) = 4.87
SUBAREA AREA(ACRES) = 0.47 SUBAREA RUNOFF(CFS) = 4.68
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.5 PEAK FLOW RATE(CFS) = 4.88

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.30 FLOOD WIDTH(FEET) = 22.27
FLOW VELOCITY(FEET/SEC.) = 1.87 DEPTH*VELOCITY(FT*FT/SEC) = 0.56
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 12.00 = 228.00 FEET.

*****
FLOW PROCESS FROM NODE 12.00 TO NODE 401.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0120
FLOW LENGTH(FEET) = 8.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 9.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.35
ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.88
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 4.89
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 401.00 = 236.00 FEET.

*****
FLOW PROCESS FROM NODE 401.00 TO NODE 401.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

```



```

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 47.00
UPSTREAM ELEVATION(FEET) = 1364.30
DOWNSTREAM ELEVATION(FEET) = 1363.00
ELEVATION DIFFERENCE(FEET) = 1.30
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.286
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.30
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.30

```

FLOW PROCESS FROM NODE 14.00 TO NODE 402.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH (FEET) = 60.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.8 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 2.28
ESTIMATED PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 0.30
PIPE TRAVEL TIME (MIN.) = 0.44 Tc (MIN.) = 2.72
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 402.00 = 107.00 FEET.

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 2.72
RAINFALL INTENSITY (INCH/HR) = 11.86
TOTAL STREAM AREA (ACRES) = 0.03
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.30

+-----+
| DRAINAGE SUBAREA 6A |
+-----+

FLOW PROCESS FROM NODE 15.00 TO NODE 402.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 64.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.50
ELEVATION DIFFERENCE (FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 3.227
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 1.20
TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 1.20

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION (MIN.) = 3.23
RAINFALL INTENSITY (INCH/HR) = 11.86
TOTAL STREAM AREA (ACRES) = 0.12
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.20

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	0.30	2.72	11.856	0.03
2	1.20	3.23	11.856	0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.31	2.72	11.856
2	1.49	3.23	11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 1.49 Tc (MIN.) = 3.23
TOTAL AREA (ACRES) = 0.1
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 402.00 = 107.00 FEET.

FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0220
FLOW LENGTH (FEET) = 71.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 9.0 INCH PIPE IS 5.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.94
ESTIMATED PIPE DIAMETER (INCH) = 9.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.49
PIPE TRAVEL TIME (MIN.) = 0.20 Tc (MIN.) = 3.43
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 403.00 = 178.00 FEET.

FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<

-----+-----+
DRAINAGE SUBAREA 7A.1
-----+-----+

FLOW PROCESS FROM NODE 16.00 TO NODE 17.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 60.00
UPSTREAM ELEVATION (FEET) = 1364.50
DOWNSTREAM ELEVATION (FEET) = 1363.30
ELEVATION DIFFERENCE (FEET) = 1.20
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.877
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.40
TOTAL AREA (ACRES) = 0.04 TOTAL RUNOFF (CFS) = 0.40

-----+-----+
DRAINAGE SUBAREA 7A.2
-----+-----+

FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0120


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CHANNEL LENGTH THRU SUBAREA(FEET) = 83.00
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.100
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.79
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.39
AVERAGE FLOW DEPTH(FEET) = 0.22 FLOOD WIDTH(FEET) = 14.35
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 0.58 Tc(MIN.) = 3.46
SUBAREA AREA(ACRES) = 0.48 SUBAREA RUNOFF(CFS) = 4.78
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.5 PEAK FLOW RATE(CFS) = 5.18

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.27 FLOOD WIDTH(FEET) = 19.07
FLOW VELOCITY(FEET/SEC.) = 2.65 DEPTH*VELOCITY(FT*FT/SEC) = 0.72
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 18.00 = 143.00 FEET.

*****
FLOW PROCESS FROM NODE 18.00 TO NODE 403.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
FLOW LENGTH(FEET) = 75.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.64
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.18
PIPE TRAVEL TIME(MIN.) = 0.27 Tc(MIN.) = 3.72
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 218.00 FEET.

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 5.18 3.72 11.856 0.52
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 218.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 1.49 3.43 11.856 0.15
LONGEST FLOWPATH FROM NODE 13.00 TO NODE 403.00 = 178.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (MIN.) (INCH/HOUR)
1 6.26 3.43 11.856
2 6.67 3.72 11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 6.67 Tc(MIN.) = 3.72
TOTAL AREA(ACRES) = 0.7

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 3 <<<<
=====

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FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

REPRESENTATIVE SLOPE = 0.0600
FLOW LENGTH(FEET) = 13.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.51
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.67
PIPE TRAVEL TIME(MIN.) = 0.02 Tc(MIN.) = 3.74
LONGEST FLOWPATH FROM NODE 16.00 TO NODE 403.00 = 231.00 FEET.

FLOW PROCESS FROM NODE 404.00 TO NODE 404.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.67	3.74	11.856	0.67

LONGEST FLOWPATH FROM NODE 16.00 TO NODE 404.00 = 231.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	11.09	5.37	11.323	1.12

LONGEST FLOWPATH FROM NODE 4.00 TO NODE 404.00 = 525.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	14.40	3.74	11.856
2	17.47	5.37	11.323

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 17.47 Tc(MIN.) = 5.37
TOTAL AREA(ACRES) = 1.8

FLOW PROCESS FROM NODE 404.00 TO NODE 404.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<
=====

+-----+
| DRAINAGE SUBAREA 8A |
+-----+

FLOW PROCESS FROM NODE 19.00 TO NODE 20.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====

URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 91
INITIAL SUBAREA FLOW-LENGTH(FEET) = 81.00
UPSTREAM ELEVATION(FEET) = 1363.70
DOWNSTREAM ELEVATION(FEET) = 1358.10
ELEVATION DIFFERENCE(FEET) = 5.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.487
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

SUBAREA RUNOFF(CFS) = 0.65
TOTAL AREA(ACRES) = 0.08 TOTAL RUNOFF(CFS) = 0.65

DRAINAGE SUBAREA 9A.1

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1404.00
DOWNSTREAM ELEVATION(FEET) = 1399.00
ELEVATION DIFFERENCE(FEET) = 5.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.422
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.470
SUBAREA RUNOFF(CFS) = 1.02
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 1.02

DRAINAGE SUBAREA 9A.2

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(FEET) = 550.00
REPRESENTATIVE CHANNEL SLOPE = 0.0550
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.923
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.29
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.47
AVERAGE FLOW DEPTH(FEET) = 0.05 TRAVEL TIME(MIN.) = 6.24
Tc(MIN.) = 14.66
SUBAREA AREA(ACRES) = 3.57 SUBAREA RUNOFF(CFS) = 6.34
AREA-AVERAGE RUNOFF COEFFICIENT = 0.300
TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 7.05

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 1.84
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 650.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 100.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

REPRESENTATIVE SLOPE = 0.0300
FLOW LENGTH(FEET) = 278.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.84
ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.05
PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 15.13


```

LONGEST FLOWPATH FROM NODE      21.00 TO NODE      100.00 =      928.00 FEET.

*****
FLOW PROCESS FROM NODE      100.00 TO NODE      100.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1            7.05      15.13      5.804      3.97
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      100.00 =      928.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)      (ACRE)
1            0.88      8.61      8.353      0.15
LONGEST FLOWPATH FROM NODE      1.00 TO NODE      100.00 =      293.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)      (INCH/HOUR)
1            4.90      8.61      8.353
2            7.67      15.13      5.804

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      7.67      Tc(MIN.) =      15.13
TOTAL AREA(ACRES) =      4.1

*****
FLOW PROCESS FROM NODE      100.00 TO NODE      100.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE      100.00 TO NODE      101.00 IS CODE = 41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 35.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 9.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.26
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.67
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 15.23
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      101.00 =      963.00 FEET.

*****
FLOW PROCESS FROM NODE      101.00 TO NODE      101.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.23
RAINFALL INTENSITY(INCH/HR) = 5.78
TOTAL STREAM AREA(ACRES) = 4.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.67

+-----+
| DRAINAGE SUBAREA 10A.1 |
+-----+

*****
FLOW PROCESS FROM NODE      24.00 TO NODE      25.00 IS CODE = 21

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-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
INITIAL SUBAREA FLOW-LENGTH(FEET) = 64.00
UPSTREAM ELEVATION(FEET) = 1359.00
DOWNSTREAM ELEVATION(FEET) = 1357.60
ELEVATION DIFFERENCE(FEET) = 1.40
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 4.548
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.25
TOTAL AREA(ACRES) = 0.03 TOTAL RUNOFF(CFS) = 0.25

+-----+
| DRAINAGE SUBAREA 10A.2 |
+-----+

*****
FLOW PROCESS FROM NODE 25.00 TO NODE 101.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) = 71.00
REPRESENTATIVE CHANNEL SLOPE = 0.0270
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 10.967
STREETS & ROADS (DITCHES) RUNOFF COEFFICIENT = .6900
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.59
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.08
AVERAGE FLOW DEPTH(FEET) = 0.02 TRAVEL TIME(MIN.) = 1.09
Tc(MIN.) = 5.64
SUBAREA AREA(ACRES) = 0.09 SUBAREA RUNOFF(CFS) = 0.68
AREA-AVERAGE RUNOFF COEFFICIENT = 0.690
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.91

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.03 FLOW VELOCITY(FEET/SEC.) = 1.18
LONGEST FLOWPATH FROM NODE 24.00 TO NODE 101.00 = 135.00 FEET.

*****
FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 5.64
RAINFALL INTENSITY(INCH/HR) = 10.97
TOTAL STREAM AREA(ACRES) = 0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.91

** CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 7.67 15.23 5.781 4.12
2 0.91 5.64 10.967 0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

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STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	3.75	5.64	10.967
2	8.15	15.23	5.781

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 8.15 Tc(MIN.) = 15.23
 TOTAL AREA(ACRES) = 4.2
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 101.00 = 963.00 FEET.

DRAINAGE SUBAREA 1B

 FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 36.00
 UPSTREAM ELEVATION(FEET) = 1364.30
 DOWNSTREAM ELEVATION(FEET) = 1363.90
 ELEVATION DIFFERENCE(FEET) = 0.40
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.711
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.50
 TOTAL AREA(ACRES) = 0.05 TOTAL RUNOFF(CFS) = 0.50

 FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
 FLOW LENGTH(FEET) = 94.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 6.0 INCH PIPE IS 4.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 3.31
 GIVEN PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 0.50
 PIPE TRAVEL TIME(MIN.) = 0.47 Tc(MIN.) = 3.18
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 33.00 = 130.00 FEET.

DRAINAGE SUBAREA 2B

 FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 96
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
 SUBAREA AREA(ACRES) = 0.05 SUBAREA RUNOFF(CFS) = 0.50
 TOTAL AREA(ACRES) = 0.1 TOTAL RUNOFF(CFS) = 1.00
 TC(MIN.) = 3.18

```

FLOW PROCESS FROM NODE      33.00 TO NODE      200.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0640
FLOW LENGTH(FEET) =   13.00  MANNING'S N =  0.012
DEPTH OF FLOW IN   6.0 INCH PIPE IS   3.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   7.98
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =           1.00
PIPE TRAVEL TIME(MIN.) =   0.03  Tc(MIN.) =   3.21
LONGEST FLOWPATH FROM NODE   31.00 TO NODE   200.00 =   143.00 FEET.

*****
FLOW PROCESS FROM NODE      200.00 TO NODE      34.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0180
FLOW LENGTH(FEET) =   99.00  MANNING'S N =  0.012
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) =   5.07
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) =   6.00  NUMBER OF PIPES =   1
PIPE-FLOW(CFS) =           1.00
PIPE TRAVEL TIME(MIN.) =   0.33  Tc(MIN.) =   3.54
LONGEST FLOWPATH FROM NODE   31.00 TO NODE   34.00 =   242.00 FEET.

*****
FLOW PROCESS FROM NODE      34.00 TO NODE      36.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =   12.00
REPRESENTATIVE CHANNEL SLOPE =  0.0170
CHANNEL BASE(FEET) =   6.00  "Z" FACTOR =  0.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  0.50
CHANNEL FLOW THRU SUBAREA(CFS) =   1.00
FLOW VELOCITY(FEET/SEC.) =   2.26  FLOW DEPTH(FEET) =   0.07
TRAVEL TIME(MIN.) =   0.09  Tc(MIN.) =   3.63
LONGEST FLOWPATH FROM NODE   31.00 TO NODE   36.00 =   254.00 FEET.

*****
FLOW PROCESS FROM NODE      36.00 TO NODE      36.00 IS CODE =   1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS =  2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM  1 ARE:
TIME OF CONCENTRATION(MIN.) =   3.63
RAINFALL INTENSITY(INCH/HR) =  11.86
TOTAL STREAM AREA(ACRES) =   0.10
PEAK FLOW RATE(CFS) AT CONFLUENCE =   1.00

+-----+
| DRAINAGE SUBAREA 3B |
+-----+

*****
FLOW PROCESS FROM NODE      35.00 TO NODE      36.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96

```


INITIAL SUBAREA FLOW-LENGTH(FEET) = 89.00
 UPSTREAM ELEVATION(FEET) = 1364.10
 DOWNSTREAM ELEVATION(FEET) = 1358.00
 ELEVATION DIFFERENCE(FEET) = 6.10
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 2.324
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.40
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.40

 FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 2.32
 RAINFALL INTENSITY(INCH/HR) = 11.86
 TOTAL STREAM AREA(ACRES) = 0.04
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.40

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.00	3.63	11.856	0.10
2	0.40	2.32	11.856	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.04	2.32	11.856
2	1.39	3.63	11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 1.39 Tc(MIN.) = 3.63
 TOTAL AREA(ACRES) = 0.1
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 36.00 = 254.00 FEET.

 FLOW PROCESS FROM NODE 36.00 TO NODE 201.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====
 REPRESENTATIVE SLOPE = 0.0200
 FLOW LENGTH(FEET) = 1.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.59
 GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1.39
 PIPE TRAVEL TIME(MIN.) = 0.00 Tc(MIN.) = 3.63
 LONGEST FLOWPATH FROM NODE 31.00 TO NODE 201.00 = 255.00 FEET.

+-----+
 | DRAINAGE SUBAREA 4B |
 +-----+

 FLOW PROCESS FROM NODE 37.00 TO NODE 38.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
 SOIL CLASSIFICATION IS "C"


```

S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 49.00
UPSTREAM ELEVATION (FEET) = 1363.00
DOWNSTREAM ELEVATION (FEET) = 1362.20
ELEVATION DIFFERENCE (FEET) = 0.80
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.782
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 1.10
TOTAL AREA (ACRES) = 0.11 TOTAL RUNOFF (CFS) = 1.10

*****
FLOW PROCESS FROM NODE 38.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 64.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.5 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.07
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 1.10
PIPE TRAVEL TIME (MIN.) = 0.26 Tc (MIN.) = 3.04
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 202.00 = 113.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 3.04
RAINFALL INTENSITY (INCH/HR) = 11.86
TOTAL STREAM AREA (ACRES) = 0.11
PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.10

+-----+
| DRAINAGE SUBAREA 5B |
+-----+

*****
FLOW PROCESS FROM NODE 39.00 TO NODE 40.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 54.00
UPSTREAM ELEVATION (FEET) = 1364.30
DOWNSTREAM ELEVATION (FEET) = 1362.00
ELEVATION DIFFERENCE (FEET) = 2.30
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.122
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.60
TOTAL AREA (ACRES) = 0.06 TOTAL RUNOFF (CFS) = 0.60

*****
FLOW PROCESS FROM NODE 40.00 TO NODE 202.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0380
FLOW LENGTH (FEET) = 15.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 2.3 INCHES

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PIPE-FLOW VELOCITY(FEET/SEC.) = 5.53
GIVEN PIPE DIAMETER(INCH) = 12.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 0.60
PIPE TRAVEL TIME(MIN.) = 0.05    Tc(MIN.) = 2.17
LONGEST FLOWPATH FROM NODE 39.00 TO NODE 202.00 = 69.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 202.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 2.17
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.06
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.60

** CONFLUENCE DATA **
STREAM    RUNOFF      Tc      INTENSITY      AREA
NUMBER    (CFS)      (MIN.)  (INCH/HR)    (ACRE)
1          1.10      3.04      11.856      0.11
2          0.60      2.17      11.856      0.06

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF      Tc      INTENSITY
NUMBER    (CFS)      (MIN.)  (INCH/HR)
1          1.38      2.17      11.856
2          1.69      3.04      11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1.69    Tc(MIN.) = 3.04
TOTAL AREA(ACRES) = 0.2
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 202.00 = 113.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.1100
FLOW LENGTH(FEET) = 54.00    MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 3.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.88
GIVEN PIPE DIAMETER(INCH) = 12.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.69
PIPE TRAVEL TIME(MIN.) = 0.08    Tc(MIN.) = 3.13
LONGEST FLOWPATH FROM NODE 37.00 TO NODE 203.00 = 167.00 FEET.

+-----+
| DRAINAGE SUBAREA 6B |
+-----+

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400

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SUBAREA AREA (ACRES) = 0.00 SUBAREA RUNOFF (CFS) = 0.05
TOTAL AREA (ACRES) = 0.2 TOTAL RUNOFF (CFS) = 1.74
TC (MIN.) = 3.13

DRAINAGE SUBAREA 7B.1

FLOW PROCESS FROM NODE 42.00 TO NODE 43.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 80.00
UPSTREAM ELEVATION (FEET) = 1361.10
DOWNSTREAM ELEVATION (FEET) = 1358.60
ELEVATION DIFFERENCE (FEET) = 2.50
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.863
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.80
TOTAL AREA (ACRES) = 0.08 TOTAL RUNOFF (CFS) = 0.80

DRAINAGE SUBAREA 7B.2

FLOW PROCESS FROM NODE 43.00 TO NODE 44.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 36.00
REPRESENTATIVE CHANNEL SLOPE = 0.0360
CHANNEL BASE (FEET) = 8.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 0.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.90
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.30
AVERAGE FLOW DEPTH (FEET) = 0.05 TRAVEL TIME (MIN.) = 0.26
Tc (MIN.) = 3.12
SUBAREA AREA (ACRES) = 0.02 SUBAREA RUNOFF (CFS) = 0.20
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA (ACRES) = 0.1 PEAK FLOW RATE (CFS) = 1.00

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.05 FLOW VELOCITY (FEET/SEC.) = 2.56
LONGEST FLOWPATH FROM NODE 42.00 TO NODE 44.00 = 116.00 FEET.

DRAINAGE SUBAREA 1C.1

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<


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=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 1401.00
DOWNSTREAM ELEVATION(FEET) = 1397.00
ELEVATION DIFFERENCE(FEET) = 4.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.072
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.074
SUBAREA RUNOFF(CFS) = 0.75
TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.75
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+-----+
| DRAINAGE SUBAREA 1C.2 |
+-----+
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*****
FLOW PROCESS FROM NODE 52.00 TO NODE 53.00 IS CODE = 51
=====
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```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
```

```
CHANNEL LENGTH THRU SUBAREA(FEET) = 380.00
REPRESENTATIVE CHANNEL SLOPE = 0.0710
CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.277
RESIDENTIAL (2. DU/AC OR LESS) RUNOFF COEFFICIENT = .4200
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 79
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.10
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.46
AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 4.33
Tc(MIN.) = 13.40
SUBAREA AREA(ACRES) = 1.80 SUBAREA RUNOFF(CFS) = 4.75
AREA-AVERAGE RUNOFF COEFFICIENT = 0.402
TOTAL AREA(ACRES) = 2.1 PEAK FLOW RATE(CFS) = 5.33
=====
```

```
END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 1.76
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 53.00 = 480.00 FEET.
```

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+-----+
| DRAINAGE SUBAREA 1C.3 |
+-----+
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*****
FLOW PROCESS FROM NODE 53.00 TO NODE 54.00 IS CODE = 91
=====
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```
>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<
=====
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```
REPRESENTATIVE SLOPE = 0.0260
CHANNEL LENGTH THRU SUBAREA(FEET) = 385.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.821
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 92
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.54
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.86
AVERAGE FLOW DEPTH(FEET) = 0.23 FLOOD WIDTH(FEET) = 17.58
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.66 Tc(MIN.) = 15.07
SUBAREA AREA(ACRES) = 0.50 SUBAREA RUNOFF(CFS) = 2.42
=====
```



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AREA-AVERAGE RUNOFF COEFFICIENT = 0.484
TOTAL AREA (ACRES) = 2.6 PEAK FLOW RATE (CFS) = 7.36

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.24 FLOOD WIDTH (FEET) = 18.52
FLOW VELOCITY (FEET/SEC.) = 3.95 DEPTH*VELOCITY (FT*FT/SEC) = 0.93
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 54.00 = 865.00 FEET.

*****
FLOW PROCESS FROM NODE 54.00 TO NODE 55.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0130
FLOW LENGTH (FEET) = 166.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 15.0 INCH PIPE IS 11.9 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.03
GIVEN PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 7.36
PIPE TRAVEL TIME (MIN.) = 0.39 Tc (MIN.) = 15.46
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 55.00 = 1031.00 FEET.

+-----+
| DRAINAGE SUBAREA 2C |
+-----+

*****
FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 5.725
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5499
SUBAREA AREA (ACRES) = 0.59 SUBAREA RUNOFF (CFS) = 2.84
TOTAL AREA (ACRES) = 3.2 TOTAL RUNOFF (CFS) = 10.07
Tc (MIN.) = 15.46

*****
FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<
=====

+-----+
| DRAINAGE SUBAREA 3C.1 |
+-----+

*****
FLOW PROCESS FROM NODE 56.00 TO NODE 57.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH (FEET) = 90.00
UPSTREAM ELEVATION (FEET) = 1370.50
DOWNSTREAM ELEVATION (FEET) = 1365.90
ELEVATION DIFFERENCE (FEET) = 4.60
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 2.280
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF (CFS) = 0.41

```


TOTAL AREA (ACRES) = 0.04 TOTAL RUNOFF (CFS) = 0.41

DRAINAGE SUBAREA 3C.2

FLOW PROCESS FROM NODE 57.00 TO NODE 58.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

REPRESENTATIVE SLOPE = 0.0200
CHANNEL LENGTH THRU SUBAREA (FEET) = 297.00
"V" GUTTER WIDTH (FEET) = 5.00 GUTTER HIKE (FEET) = 0.090
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.20000
MAXIMUM DEPTH (FEET) = 0.50
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 2.01
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 3.28
AVERAGE FLOW DEPTH (FEET) = 0.16 FLOOD WIDTH (FEET) = 5.63
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.51 Tc (MIN.) = 3.79
SUBAREA AREA (ACRES) = 0.32 SUBAREA RUNOFF (CFS) = 3.19
AREA-AVERAGE RUNOFF COEFFICIENT = 0.843
TOTAL AREA (ACRES) = 0.4 PEAK FLOW RATE (CFS) = 3.60

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH (FEET) = 0.21 FLOOD WIDTH (FEET) = 6.10
FLOW VELOCITY (FEET/SEC.) = 4.06 DEPTH*VELOCITY (FT*FT/SEC) = 0.85
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 58.00 = 387.00 FEET.

FLOW PROCESS FROM NODE 58.00 TO NODE 300.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0110
FLOW LENGTH (FEET) = 34.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS 9.2 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.56
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 3.60
PIPE TRAVEL TIME (MIN.) = 0.10 Tc (MIN.) = 3.89
LONGEST FLOWPATH FROM NODE 56.00 TO NODE 300.00 = 421.00 FEET.

FLOW PROCESS FROM NODE 300.00 TO NODE 300.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 3.89
RAINFALL INTENSITY (INCH/HR) = 11.86
TOTAL STREAM AREA (ACRES) = 0.36
PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.60

DRAINAGE SUBAREA 4C.1

```

FLOW PROCESS FROM NODE      59.00 TO NODE      60.00 IS CODE =  21
=====
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
INITIAL SUBAREA FLOW-LENGTH(FEET) =      60.00
UPSTREAM ELEVATION(FEET) =    1370.50
DOWNSTREAM ELEVATION(FEET) =    1365.40
ELEVATION DIFFERENCE(FEET) =        5.10
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      1.571
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =      0.31
TOTAL AREA(ACRES) =      0.03  TOTAL RUNOFF(CFS) =      0.31

+-----+
| DRAINAGE SUBAREA 4C.2 |
+-----+

*****
FLOW PROCESS FROM NODE      60.00 TO NODE      61.00 IS CODE =  51
=====
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
CHANNEL LENGTH THRU SUBAREA(FEET) =      70.00
REPRESENTATIVE CHANNEL SLOPE =  0.0900
CHANNEL BASE(FEET) =      3.00  "Z" FACTOR =  0.330
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
ARTIFICIAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .8700
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) =  96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      0.36
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  3.40
AVERAGE FLOW DEPTH(FEET) =  0.04  TRAVEL TIME(MIN.) =  0.34
Tc(MIN.) =  1.91
SUBAREA AREA(ACRES) =      0.01  SUBAREA RUNOFF(CFS) =      0.10
AREA-AVERAGE RUNOFF COEFFICIENT =  0.870
TOTAL AREA(ACRES) =      0.0  PEAK FLOW RATE(CFS) =      0.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.04  FLOW VELOCITY(FEET/SEC.) =  3.34
LONGEST FLOWPATH FROM NODE      59.00 TO NODE      61.00 =      130.00 FEET.

*****
FLOW PROCESS FROM NODE      61.00 TO NODE      300.00 IS CODE =  41
=====
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE =  0.0100
FLOW LENGTH(FEET) =      27.00  MANNING'S N =  0.012
DEPTH OF FLOW IN 12.0 INCH PIPE IS  2.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  3.10
GIVEN PIPE DIAMETER(INCH) = 12.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =      0.41
PIPE TRAVEL TIME(MIN.) =  0.15  Tc(MIN.) =  2.06
LONGEST FLOWPATH FROM NODE      59.00 TO NODE      300.00 =      157.00 FEET.

*****
FLOW PROCESS FROM NODE      300.00 TO NODE      300.00 IS CODE =  1
=====
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
=====

```


TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 2.06
 RAINFALL INTENSITY(INCH/HR) = 11.86
 TOTAL STREAM AREA(ACRES) = 0.04
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.41

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	3.60	3.89	11.856	0.36
2	0.41	2.06	11.856	0.04

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	2.32	2.06	11.856
2	4.01	3.89	11.856

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 4.01 Tc(MIN.) = 3.89
 TOTAL AREA(ACRES) = 0.4
 LONGEST FLOWPATH FROM NODE 56.00 TO NODE 300.00 = 421.00 FEET.

 FLOW PROCESS FROM NODE 300.00 TO NODE 55.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0220
 FLOW LENGTH(FEET) = 64.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 15.0 INCH PIPE IS 6.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.61
 GIVEN PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 4.01
 PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 4.03
 LONGEST FLOWPATH FROM NODE 56.00 TO NODE 55.00 = 485.00 FEET.

 FLOW PROCESS FROM NODE 55.00 TO NODE 55.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.01	4.03	11.856	0.40

LONGEST FLOWPATH FROM NODE 56.00 TO NODE 55.00 = 485.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	10.07	15.46	5.725	3.20

LONGEST FLOWPATH FROM NODE 51.00 TO NODE 55.00 = 1031.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	6.64	4.03	11.856
2	12.01	15.46	5.725

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 12.01 Tc(MIN.) = 15.46
 TOTAL AREA(ACRES) = 3.6

```

FLOW PROCESS FROM NODE      55.00 TO NODE      55.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 2 <<<<
=====
*****
FLOW PROCESS FROM NODE      55.00 TO NODE      62.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 173.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 13.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.48
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.01
PIPE TRAVEL TIME(MIN.) = 0.34 Tc(MIN.) = 15.80
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 62.00 = 1204.00 FEET.

+-----+
| DRAINAGE SUBAREA 5C |
+-----+

*****
FLOW PROCESS FROM NODE      62.00 TO NODE      62.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.645
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6108
SUBAREA AREA(ACRES) = 0.44 SUBAREA RUNOFF(CFS) = 2.09
TOTAL AREA(ACRES) = 4.0 TOTAL RUNOFF(CFS) = 13.93
TC(MIN.) = 15.80

*****
FLOW PROCESS FROM NODE      62.00 TO NODE      66.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(FEET) = 84.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 13.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.73
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 13.93
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 15.98
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 66.00 = 1288.00 FEET.

*****
FLOW PROCESS FROM NODE      66.00 TO NODE      66.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.98
RAINFALL INTENSITY(INCH/HR) = 5.60
TOTAL STREAM AREA(ACRES) = 4.04
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.93

+-----+
| DRAINAGE SUBAREA 6C.1 |
+-----+

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+-----+
*****
FLOW PROCESS FROM NODE      63.00 TO NODE      64.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
INITIAL SUBAREA FLOW-LENGTH(FEET) = 53.00
UPSTREAM ELEVATION(FEET) = 1359.30
DOWNSTREAM ELEVATION(FEET) = 1358.70
ELEVATION DIFFERENCE(FEET) = 0.60
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 3.269
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 1.10
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 1.10

+-----+
| DRAINAGE SUBAREA 6C.2 |
+-----+

*****
FLOW PROCESS FROM NODE      64.00 TO NODE      65.00 IS CODE = 91
-----
>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<
=====
REPRESENTATIVE SLOPE = 0.0110
CHANNEL LENGTH THRU SUBAREA(FEET) = 188.00
"V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.090
PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.84
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.33
AVERAGE FLOW DEPTH(FEET) = 0.20 FLOOD WIDTH(FEET) = 14.61
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 1.34 Tc(MIN.) = 4.61
SUBAREA AREA(ACRES) = 0.35 SUBAREA RUNOFF(CFS) = 3.49
AREA-AVERAGE RUNOFF COEFFICIENT = 0.840
TOTAL AREA(ACRES) = 0.5 PEAK FLOW RATE(CFS) = 4.58

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.23 FLOOD WIDTH(FEET) = 18.05
FLOW VELOCITY(FEET/SEC.) = 2.58 DEPTH*VELOCITY(FT*FT/SEC) = 0.59
LONGEST FLOWPATH FROM NODE 63.00 TO NODE 65.00 = 241.00 FEET.

*****
FLOW PROCESS FROM NODE      65.00 TO NODE      66.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 105.00 MANNING'S N = 0.012
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.83
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.58
PIPE TRAVEL TIME(MIN.) = 0.30 Tc(MIN.) = 4.91
LONGEST FLOWPATH FROM NODE 63.00 TO NODE 66.00 = 346.00 FEET.

```


DRAINAGE SUBAREA 7C

FLOW PROCESS FROM NODE 66.00 TO NODE 66.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8400
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.8400
SUBAREA AREA(ACRES) = 0.24 SUBAREA RUNOFF(CFS) = 2.39
TOTAL AREA(ACRES) = 0.7 TOTAL RUNOFF(CFS) = 6.97
TC(MIN.) = 4.91

FLOW PROCESS FROM NODE 66.00 TO NODE 66.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 4.91
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.97

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	13.93	15.98	5.603	4.04
2	6.97	4.91	11.856	0.70

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	11.25	4.91	11.856
2	17.22	15.98	5.603

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 17.22 Tc(MIN.) = 15.98
TOTAL AREA(ACRES) = 4.7
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 66.00 = 1288.00 FEET.

FLOW PROCESS FROM NODE 66.00 TO NODE 67.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH(Feet) = 76.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.4 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 8.10
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.22
PIPE TRAVEL TIME(MIN.) = 0.16 Tc(MIN.) = 16.14
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 67.00 = 1364.00 FEET.

DRAINAGE SUBAREA 8C

FLOW PROCESS FROM NODE 67.00 TO NODE 67.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR)	=	5.568
LIMITED INDUSTRIAL RUNOFF COEFFICIENT	=	.8300
SOIL CLASSIFICATION IS	"A"	
S.C.S. CURVE NUMBER (AMC II)	=	92
AREA-AVERAGE RUNOFF COEFFICIENT	=	0.6567
SUBAREA AREA(ACRES)	=	0.33
SUBAREA RUNOFF(CFS)	=	1.53
TOTAL AREA(ACRES)	=	5.1
TOTAL RUNOFF(CFS)	=	18.54
TC(MIN.)	=	16.14

FLOW PROCESS FROM NODE 67.00 TO NODE 301.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE	=	0.0130
FLOW LENGTH(Feet)	=	6.00
MANNING'S N	=	0.012
DEPTH OF FLOW IN 24.0 INCH PIPE	=	14.8 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.)	=	9.13
GIVEN PIPE DIAMETER(INCH)	=	24.00
NUMBER OF PIPES	=	1
PIPE-FLOW(CFS)	=	18.54
PIPE TRAVEL TIME(MIN.)	=	0.01
Tc(MIN.)	=	16.15
LONGEST FLOWPATH FROM NODE 51.00 TO NODE 301.00	=	1370.00 FEET.

-----+
DRAINAGE SUBAREA 9C.1
-----+

FLOW PROCESS FROM NODE 68.00 TO NODE 69.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

LIMITED INDUSTRIAL RUNOFF COEFFICIENT	=	.8300
SOIL CLASSIFICATION IS	"A"	
S.C.S. CURVE NUMBER (AMC II)	=	92
INITIAL SUBAREA FLOW-LENGTH(Feet)	=	60.00
UPSTREAM ELEVATION(Feet)	=	1356.20
DOWNSTREAM ELEVATION(Feet)	=	1355.00
ELEVATION DIFFERENCE(Feet)	=	1.20
SUBAREA OVERLAND TIME OF FLOW(MIN.)	=	2.988
100 YEAR RAINFALL INTENSITY(INCH/HOUR)	=	11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.		
SUBAREA RUNOFF(CFS)	=	0.20
TOTAL AREA(ACRES)	=	0.02
TOTAL RUNOFF(CFS)	=	0.20

-----+
DRAINAGE SUBAREA 9C.2
-----+

FLOW PROCESS FROM NODE 69.00 TO NODE 70.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

CHANNEL LENGTH THRU SUBAREA(Feet)	=	129.00
REPRESENTATIVE CHANNEL SLOPE	=	0.0080

CHANNEL BASE(FEET) = 1.50 "Z" FACTOR = 13.090
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 0.50
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 LIMITED INDUSTRIAL RUNOFF COEFFICIENT = .8300
 SOIL CLASSIFICATION IS "A"
 S.C.S. CURVE NUMBER (AMC II) = 92
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.74
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.72
 AVERAGE FLOW DEPTH(FEET) = 0.13 TRAVEL TIME(MIN.) = 1.25
 Tc(MIN.) = 4.24
 SUBAREA AREA(ACRES) = 0.11 SUBAREA RUNOFF(CFS) = 1.08
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.830
 TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 1.28

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.17 FLOW VELOCITY(FEET/SEC.) = 2.01
 LONGEST FLOWPATH FROM NODE 68.00 TO NODE 70.00 = 189.00 FEET.

-----+
 | DRAINAGE SUBAREA 1D.1 |
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 |-----+
 +-----+

 FLOW PROCESS FROM NODE 81.00 TO NODE 82.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 =====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
 SOIL CLASSIFICATION IS "C"
 S.C.S. CURVE NUMBER (AMC II) = 85
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 1452.00
 DOWNSTREAM ELEVATION(FEET) = 1429.00
 ELEVATION DIFFERENCE(FEET) = 23.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.684
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 9.832
 SUBAREA RUNOFF(CFS) = 1.68
 TOTAL AREA(ACRES) = 0.57 TOTAL RUNOFF(CFS) = 1.68

-----+
 | DRAINAGE SUBAREA 1D.2 |
 |-----+
 |-----+
 +-----+

 FLOW PROCESS FROM NODE 82.00 TO NODE 83.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
 =====

CHANNEL LENGTH THRU SUBAREA(FEET) = 651.00
 REPRESENTATIVE CHANNEL SLOPE = 0.0940
 CHANNEL BASE(FEET) = 100.00 "Z" FACTOR = 0.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.482
 RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .2700
 SOIL CLASSIFICATION IS "A"
 S.C.S. CURVE NUMBER (AMC II) = 45
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.31
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.79
 AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 6.07
 Tc(MIN.) = 12.75
 SUBAREA AREA(ACRES) = 5.18 SUBAREA RUNOFF(CFS) = 9.07
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.273
 TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 10.17

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH (FEET) = 0.05 FLOW VELOCITY (FEET/SEC.) = 2.01
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 83.00 = 751.00 FEET.

-----+-----+
| DRAINAGE SUBAREA 1D.3 |
-----+-----+
|

FLOW PROCESS FROM NODE 83.00 TO NODE 84.00 IS CODE = 91

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<
=====

REPRESENTATIVE SLOPE = 0.0560
CHANNEL LENGTH THRU SUBAREA (FEET) = 181.00
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.800
PAVEMENT LIP (FEET) = 0.010 MANNING'S N = .0150
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.20000
MAXIMUM DEPTH (FEET) = 1.00
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 6.398
RESIDENTIAL (4.3 DU/AC OR LESS) RUNOFF COEFFICIENT = .4800
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 81
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 10.33
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 11.71
AVERAGE FLOW DEPTH (FEET) = 0.80 FLOOD WIDTH (FEET) = 3.00
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.26 Tc (MIN.) = 13.01
SUBAREA AREA (ACRES) = 0.10 SUBAREA RUNOFF (CFS) = 0.31
AREA-AVERAGE RUNOFF COEFFICIENT = 0.277
TOTAL AREA (ACRES) = 5.8 PEAK FLOW RATE (CFS) = 10.35

NOTE: TRAVEL TIME ESTIMATES BASED ON NORMAL DEPTH
IN A FLOWING-FULL GUTTER (NORMAL DEPTH = GUTTER HIKE)

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH (FEET) = 0.80 FLOOD WIDTH (FEET) = 3.00
FLOW VELOCITY (FEET/SEC.) = 11.71 DEPTH*VELOCITY (FT*FT/SEC) = 9.37
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 84.00 = 932.00 FEET.

FLOW PROCESS FROM NODE 84.00 TO NODE 86.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====

CHANNEL LENGTH THRU SUBAREA (FEET) = 41.00
REPRESENTATIVE CHANNEL SLOPE = 0.0120
CHANNEL BASE (FEET) = 4.00 "Z" FACTOR = 0.330
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH (FEET) = 1.00
CHANNEL FLOW THRU SUBAREA (CFS) = 10.35
FLOW VELOCITY (FEET/SEC.) = 5.60 FLOW DEPTH (FEET) = 0.45
TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 13.13
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 86.00 = 973.00 FEET.

FLOW PROCESS FROM NODE 86.00 TO NODE 86.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION (MIN.) = 13.13
RAINFALL INTENSITY (INCH/HR) = 6.36
TOTAL STREAM AREA (ACRES) = 5.85
PEAK FLOW RATE (CFS) AT CONFLUENCE = 10.35

-----+-----+
| DRAINAGE SUBAREA 2D |
-----+-----+
|

FLOW PROCESS FROM NODE 85.00 TO NODE 86.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3000
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 85
INITIAL SUBAREA FLOW-LENGTH(Feet) = 22.00
UPSTREAM ELEVATION(Feet) = 1358.00
DOWNSTREAM ELEVATION(Feet) = 1357.00
ELEVATION DIFFERENCE(Feet) = 1.00
SUBAREA OVERLAND TIME OF FLOW(Min.) = 4.078
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 11.856
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) = 0.43
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.43

FLOW PROCESS FROM NODE 86.00 TO NODE 86.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(Min.) = 4.08
RAINFALL INTENSITY(INCH/HR) = 11.86
TOTAL STREAM AREA(ACRES) = 0.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.43

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (Min.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	10.35	13.13	6.360	5.85
2	0.43	4.08	11.856	0.12

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (Min.)	INTENSITY (INCH/HOUR)
1	3.64	4.08	11.856
2	10.58	13.13	6.360

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 10.58 Tc(Min.) = 13.13
TOTAL AREA(ACRES) = 6.0
LONGEST FLOWPATH FROM NODE 81.00 TO NODE 86.00 = 973.00 FEET.

-----+-----+
DRAINAGE SUBAREA 1E
-----+-----+

FLOW PROCESS FROM NODE 91.00 TO NODE 92.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .2000
SOIL CLASSIFICATION IS "A"
S.C.S. CURVE NUMBER (AMC II) = 63
INITIAL SUBAREA FLOW-LENGTH(Feet) = 86.00
UPSTREAM ELEVATION(Feet) = 1358.00

DOWNSTREAM ELEVATION (FEET) = 1351.10
ELEVATION DIFFERENCE (FEET) = 6.90
SUBAREA OVERLAND TIME OF FLOW (MIN.) = 7.505
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 9.124
SUBAREA RUNOFF (CFS) = 0.22
TOTAL AREA (ACRES) = 0.12 TOTAL RUNOFF (CFS) = 0.22

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 0.1 TC (MIN.) = 7.50
PEAK FLOW RATE (CFS) = 0.22

=====

END OF RATIONAL METHOD ANALYSIS

APPENDIX E

Hydromodification Analysis

RUN DATE 5/19/2021
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 15 MIN.
6 HOUR RAINFALL 4.5 INCHES
BASIN AREA 6.03 ACRES
RUNOFF COEFFICIENT 0.33
PEAK DISCHARGE 11.63 CFS

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 15	DISCHARGE (CFS) = 0.5
TIME (MIN) = 30	DISCHARGE (CFS) = 0.6
TIME (MIN) = 45	DISCHARGE (CFS) = 0.6
TIME (MIN) = 60	DISCHARGE (CFS) = 0.6
TIME (MIN) = 75	DISCHARGE (CFS) = 0.7
TIME (MIN) = 90	DISCHARGE (CFS) = 0.7
TIME (MIN) = 105	DISCHARGE (CFS) = 0.7
TIME (MIN) = 120	DISCHARGE (CFS) = 0.8
TIME (MIN) = 135	DISCHARGE (CFS) = 0.9
TIME (MIN) = 150	DISCHARGE (CFS) = 0.9
TIME (MIN) = 165	DISCHARGE (CFS) = 1
TIME (MIN) = 180	DISCHARGE (CFS) = 1.1
TIME (MIN) = 195	DISCHARGE (CFS) = 1.4
TIME (MIN) = 210	DISCHARGE (CFS) = 1.6
TIME (MIN) = 225	DISCHARGE (CFS) = 2.3
TIME (MIN) = 240	DISCHARGE (CFS) = 3.2
TIME (MIN) = 255	DISCHARGE (CFS) = 11.63
TIME (MIN) = 270	DISCHARGE (CFS) = 1.8
TIME (MIN) = 285	DISCHARGE (CFS) = 1.2
TIME (MIN) = 300	DISCHARGE (CFS) = 1
TIME (MIN) = 315	DISCHARGE (CFS) = 0.8
TIME (MIN) = 330	DISCHARGE (CFS) = 0.7
TIME (MIN) = 345	DISCHARGE (CFS) = 0.6
TIME (MIN) = 360	DISCHARGE (CFS) = 0.6
TIME (MIN) = 375	DISCHARGE (CFS) = 0

RUN DATE 5/19/2021
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 5 MIN.
6 HOUR RAINFALL 4.5 INCHES
BASIN AREA 1.78 ACRES
RUNOFF COEFFICIENT 0.33
PEAK DISCHARGE 16.78 CFS

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 5	DISCHARGE (CFS) = 0.2
TIME (MIN) = 10	DISCHARGE (CFS) = 0.2
TIME (MIN) = 15	DISCHARGE (CFS) = 0.2
TIME (MIN) = 20	DISCHARGE (CFS) = 0.2
TIME (MIN) = 25	DISCHARGE (CFS) = 0.2
TIME (MIN) = 30	DISCHARGE (CFS) = 0.2
TIME (MIN) = 35	DISCHARGE (CFS) = 0.2
TIME (MIN) = 40	DISCHARGE (CFS) = 0.2
TIME (MIN) = 45	DISCHARGE (CFS) = 0.2
TIME (MIN) = 50	DISCHARGE (CFS) = 0.2
TIME (MIN) = 55	DISCHARGE (CFS) = 0.2
TIME (MIN) = 60	DISCHARGE (CFS) = 0.2
TIME (MIN) = 65	DISCHARGE (CFS) = 0.2
TIME (MIN) = 70	DISCHARGE (CFS) = 0.2
TIME (MIN) = 75	DISCHARGE (CFS) = 0.2
TIME (MIN) = 80	DISCHARGE (CFS) = 0.2
TIME (MIN) = 85	DISCHARGE (CFS) = 0.2
TIME (MIN) = 90	DISCHARGE (CFS) = 0.2
TIME (MIN) = 95	DISCHARGE (CFS) = 0.2
TIME (MIN) = 100	DISCHARGE (CFS) = 0.2
TIME (MIN) = 105	DISCHARGE (CFS) = 0.2
TIME (MIN) = 110	DISCHARGE (CFS) = 0.2
TIME (MIN) = 115	DISCHARGE (CFS) = 0.2
TIME (MIN) = 120	DISCHARGE (CFS) = 0.2
TIME (MIN) = 125	DISCHARGE (CFS) = 0.2
TIME (MIN) = 130	DISCHARGE (CFS) = 0.3
TIME (MIN) = 135	DISCHARGE (CFS) = 0.3
TIME (MIN) = 140	DISCHARGE (CFS) = 0.3
TIME (MIN) = 145	DISCHARGE (CFS) = 0.3
TIME (MIN) = 150	DISCHARGE (CFS) = 0.3
TIME (MIN) = 155	DISCHARGE (CFS) = 0.3
TIME (MIN) = 160	DISCHARGE (CFS) = 0.3
TIME (MIN) = 165	DISCHARGE (CFS) = 0.3
TIME (MIN) = 170	DISCHARGE (CFS) = 0.3
TIME (MIN) = 175	DISCHARGE (CFS) = 0.4
TIME (MIN) = 180	DISCHARGE (CFS) = 0.4
TIME (MIN) = 185	DISCHARGE (CFS) = 0.4
TIME (MIN) = 190	DISCHARGE (CFS) = 0.4
TIME (MIN) = 195	DISCHARGE (CFS) = 0.4
TIME (MIN) = 200	DISCHARGE (CFS) = 0.5
TIME (MIN) = 205	DISCHARGE (CFS) = 0.5
TIME (MIN) = 210	DISCHARGE (CFS) = 0.5
TIME (MIN) = 215	DISCHARGE (CFS) = 0.6
TIME (MIN) = 220	DISCHARGE (CFS) = 0.7
TIME (MIN) = 225	DISCHARGE (CFS) = 0.8
TIME (MIN) = 230	DISCHARGE (CFS) = 0.9
TIME (MIN) = 235	DISCHARGE (CFS) = 1.4
TIME (MIN) = 240	DISCHARGE (CFS) = -7.9
TIME (MIN) = 245	DISCHARGE (CFS) = 16.78
TIME (MIN) = 250	DISCHARGE (CFS) = 1.1
TIME (MIN) = 255	DISCHARGE (CFS) = 0.7
TIME (MIN) = 260	DISCHARGE (CFS) = 0.6
TIME (MIN) = 265	DISCHARGE (CFS) = 0.5
TIME (MIN) = 270	DISCHARGE (CFS) = 0.4
TIME (MIN) = 275	DISCHARGE (CFS) = 0.4
TIME (MIN) = 280	DISCHARGE (CFS) = 0.3
TIME (MIN) = 285	DISCHARGE (CFS) = 0.3
TIME (MIN) = 290	DISCHARGE (CFS) = 0.3
TIME (MIN) = 295	DISCHARGE (CFS) = 0.3
TIME (MIN) = 300	DISCHARGE (CFS) = 0.3
TIME (MIN) = 305	DISCHARGE (CFS) = 0.2
TIME (MIN) = 310	DISCHARGE (CFS) = 0.2
TIME (MIN) = 315	DISCHARGE (CFS) = 0.2
TIME (MIN) = 320	DISCHARGE (CFS) = 0.2
TIME (MIN) = 325	DISCHARGE (CFS) = 0.2
TIME (MIN) = 330	DISCHARGE (CFS) = 0.2
TIME (MIN) = 335	DISCHARGE (CFS) = 0.2
TIME (MIN) = 340	DISCHARGE (CFS) = 0.2
TIME (MIN) = 345	DISCHARGE (CFS) = 0.2
TIME (MIN) = 350	DISCHARGE (CFS) = 0.2
TIME (MIN) = 355	DISCHARGE (CFS) = 0.2
TIME (MIN) = 360	DISCHARGE (CFS) = 0.2
TIME (MIN) = 365	DISCHARGE (CFS) = 0

RUN DATE 5/19/2021
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 16 MIN.
6 HOUR RAINFALL 4.5 INCHES
BASIN AREA 5.16 ACRES
RUNOFF COEFFICIENT 0.65
PEAK DISCHARGE 18.58 CFS

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 16	DISCHARGE (CFS) = 0.9
TIME (MIN) = 32	DISCHARGE (CFS) = 1
TIME (MIN) = 48	DISCHARGE (CFS) = 1
TIME (MIN) = 64	DISCHARGE (CFS) = 1.1
TIME (MIN) = 80	DISCHARGE (CFS) = 1.1
TIME (MIN) = 96	DISCHARGE (CFS) = 1.2
TIME (MIN) = 112	DISCHARGE (CFS) = 1.2
TIME (MIN) = 128	DISCHARGE (CFS) = 1.4
TIME (MIN) = 144	DISCHARGE (CFS) = 1.5
TIME (MIN) = 160	DISCHARGE (CFS) = 1.7
TIME (MIN) = 176	DISCHARGE (CFS) = 1.8
TIME (MIN) = 192	DISCHARGE (CFS) = 2.2
TIME (MIN) = 208	DISCHARGE (CFS) = 2.5
TIME (MIN) = 224	DISCHARGE (CFS) = 3.7
TIME (MIN) = 240	DISCHARGE (CFS) = 5.4
TIME (MIN) = 256	DISCHARGE (CFS) = 18.58
TIME (MIN) = 272	DISCHARGE (CFS) = 3
TIME (MIN) = 288	DISCHARGE (CFS) = 2
TIME (MIN) = 304	DISCHARGE (CFS) = 1.6
TIME (MIN) = 320	DISCHARGE (CFS) = 1.3
TIME (MIN) = 336	DISCHARGE (CFS) = 1.1
TIME (MIN) = 352	DISCHARGE (CFS) = 1
TIME (MIN) = 368	DISCHARGE (CFS) = 0.9
TIME (MIN) = 384	DISCHARGE (CFS) = 0

RUN DATE 5/19/2021
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 16 MIN.
6 HOUR RAINFALL 4.5 INCHES
BASIN AREA 5.08 ACRES
RUNOFF COEFFICIENT 0.66
PEAK DISCHARGE 18.54 CFS

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 16	DISCHARGE (CFS) = 0.9
TIME (MIN) = 32	DISCHARGE (CFS) = 1
TIME (MIN) = 48	DISCHARGE (CFS) = 1
TIME (MIN) = 64	DISCHARGE (CFS) = 1.1
TIME (MIN) = 80	DISCHARGE (CFS) = 1.1
TIME (MIN) = 96	DISCHARGE (CFS) = 1.2
TIME (MIN) = 112	DISCHARGE (CFS) = 1.2
TIME (MIN) = 128	DISCHARGE (CFS) = 1.4
TIME (MIN) = 144	DISCHARGE (CFS) = 1.5
TIME (MIN) = 160	DISCHARGE (CFS) = 1.7
TIME (MIN) = 176	DISCHARGE (CFS) = 1.8
TIME (MIN) = 192	DISCHARGE (CFS) = 2.2
TIME (MIN) = 208	DISCHARGE (CFS) = 2.5
TIME (MIN) = 224	DISCHARGE (CFS) = 3.7
TIME (MIN) = 240	DISCHARGE (CFS) = 5.5
TIME (MIN) = 256	DISCHARGE (CFS) = 18.54
TIME (MIN) = 272	DISCHARGE (CFS) = 3
TIME (MIN) = 288	DISCHARGE (CFS) = 2
TIME (MIN) = 304	DISCHARGE (CFS) = 1.6
TIME (MIN) = 320	DISCHARGE (CFS) = 1.3
TIME (MIN) = 336	DISCHARGE (CFS) = 1.1
TIME (MIN) = 352	DISCHARGE (CFS) = 1
TIME (MIN) = 368	DISCHARGE (CFS) = 0.9
TIME (MIN) = 384	DISCHARGE (CFS) = 0

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	11.63	DMA-A Exist 100-Yr
2	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	18.58	DMA-C Exist 100-Yr
4	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	16.78	DMA-A Prop 100-Yr to System #1
5	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	18.54	DMA-C Prop 100-Yr to System #2
7	Reservoir	4	-----	0.000	-----	-----	0.000	0.000	-----	0.442	System #1 (DMA-A)
8	Reservoir	5	-----	0.000	-----	-----	0.000	0.000	-----	13.68	System #2 (DMA-C)
Proj. file: 19132 Detention.gpw										Tuesday, 05 / 25 / 2021	

Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	11.63	15	255	32,337	-----	-----	-----	DMA-A Exist 100-Yr
2	Manual	18.58	16	256	54,893	-----	-----	-----	DMA-C Exist 100-Yr
4	Manual	16.78	5	245	14,364	-----	-----	-----	DMA-A Prop 100-Yr to System #1
5	Manual	18.54	16	256	54,950	-----	-----	-----	DMA-C Prop 100-Yr to System #2
7	Reservoir	0.442	5	270	14,348	4	103.63	10,263	System #1 (DMA-A)
8	Reservoir	13.68	16	256	54,942	5	105.00	9,280	System #2 (DMA-C)
19132 Detention.gpw					Return Period: 100 Year			Tuesday, 05 / 25 / 2021	

Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

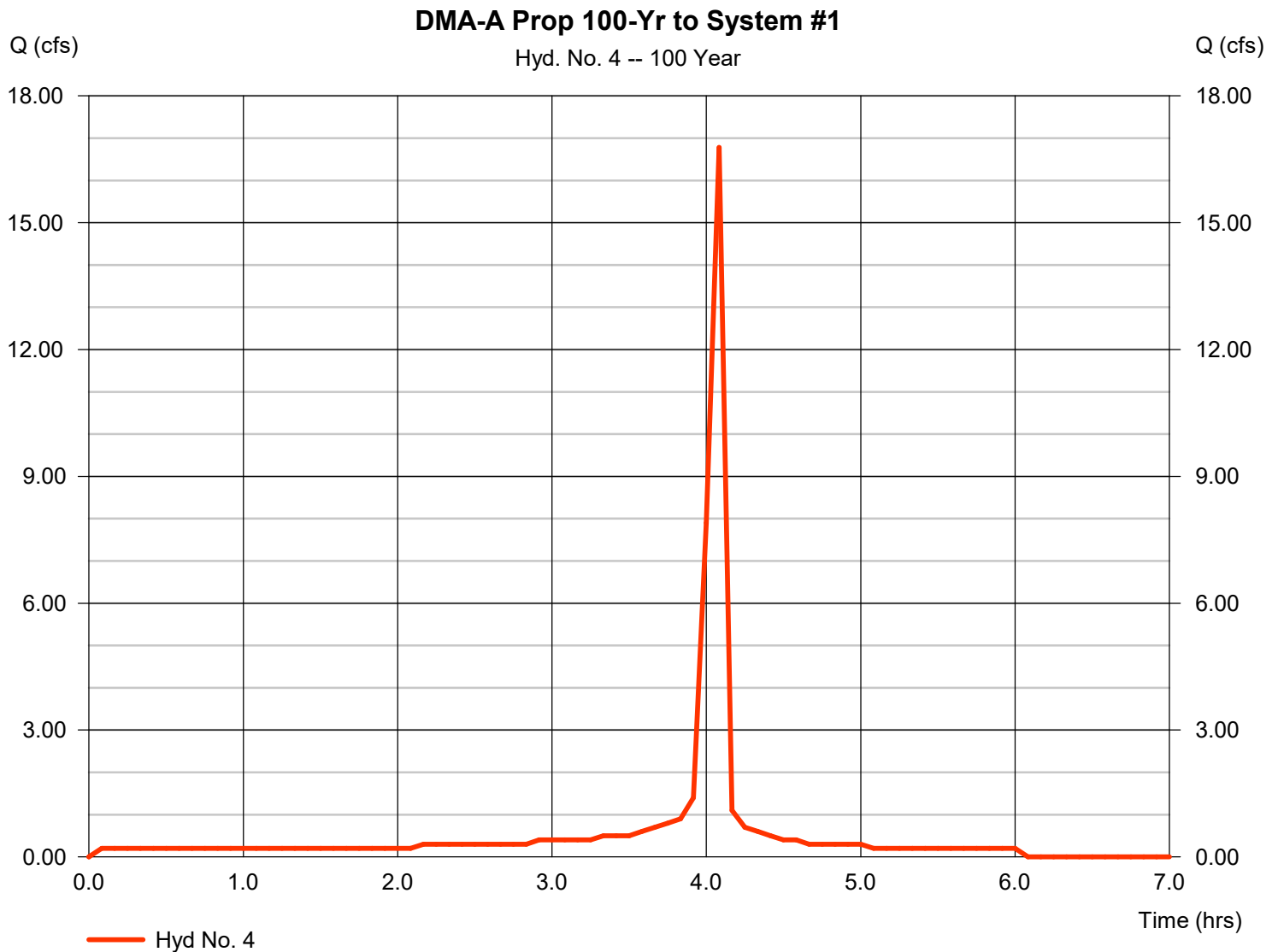
Wednesday, 05 / 19 / 2021

Hyd. No. 4

DMA-A Prop 100-Yr to System #1

Hydrograph type = Manual
Storm frequency = 100 yrs
Time interval = 5 min

Peak discharge = 16.78 cfs
Time to peak = 4.08 hrs
Hyd. volume = 14,364 cuft

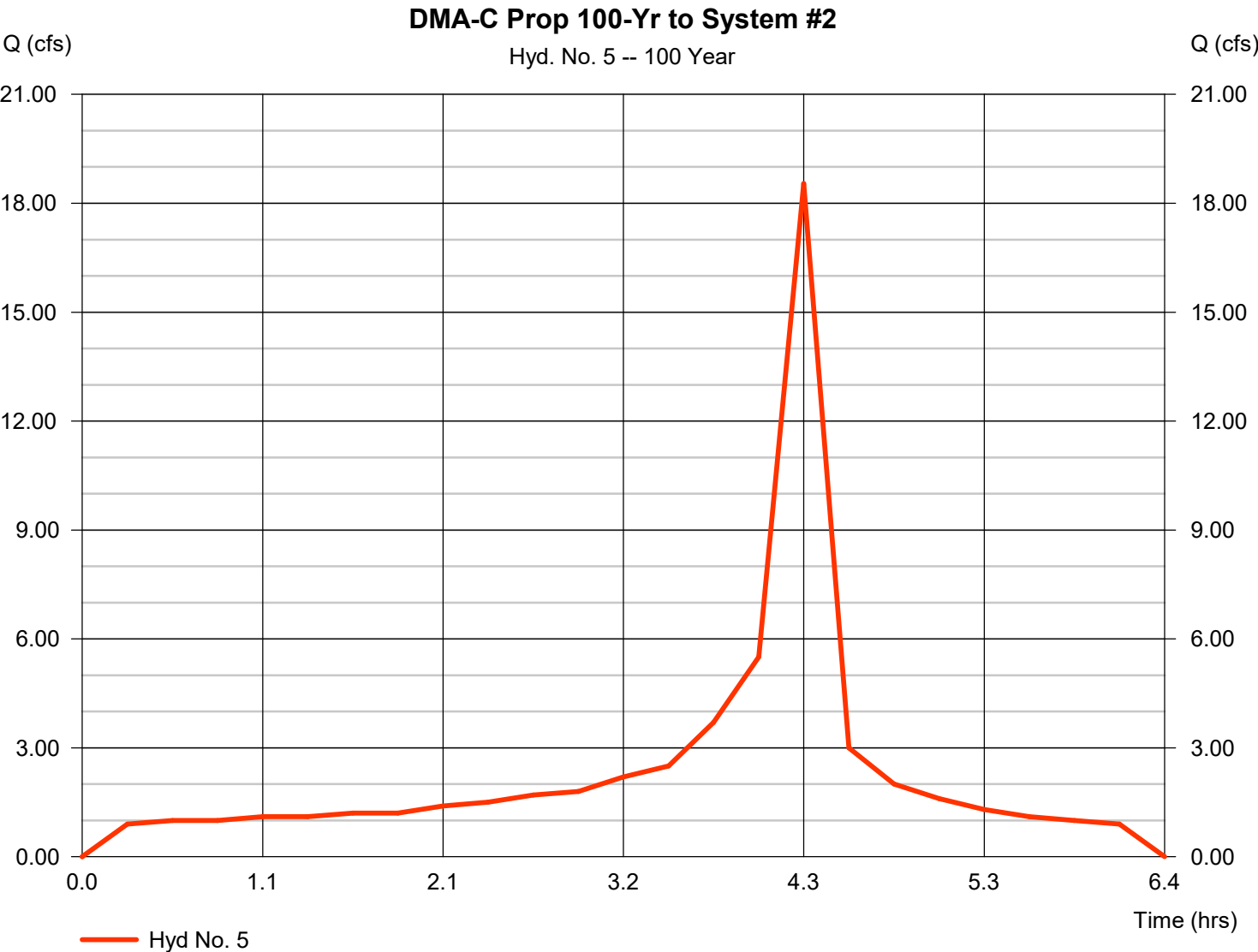


Hydrograph Report

Hyd. No. 5

DMA-C Prop 100-Yr to System #2

Hydrograph type	= Manual	Peak discharge	= 18.54 cfs
Storm frequency	= 100 yrs	Time to peak	= 4.27 hrs
Time interval	= 16 min	Hyd. volume	= 54,950 cuft



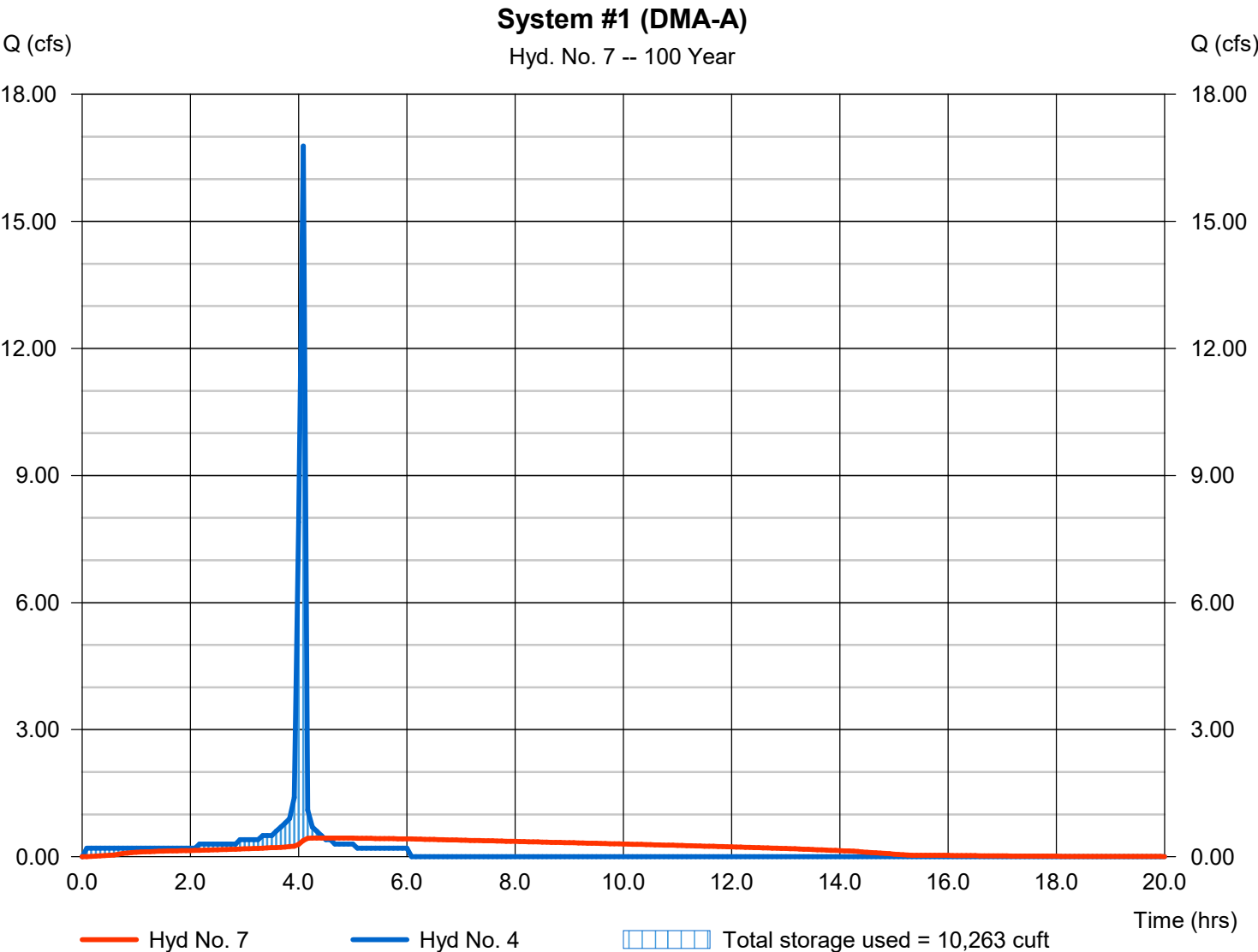
Hydrograph Report

Hyd. No. 7

System #1 (DMA-A)

Hydrograph type	= Reservoir	Peak discharge	= 0.442 cfs
Storm frequency	= 100 yrs	Time to peak	= 4.50 hrs
Time interval	= 5 min	Hyd. volume	= 14,348 cuft
Inflow hyd. No.	= 4 - DMA-A Prop 100-Yr to System #1	Max. Elevation	= 103.63 ft
Reservoir name	= System 1 (DMA-A)	Max. Storage	= 10,263 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Tuesday, 05 / 25 / 2021

Pond No. 1 - System 1 (DMA-A)

Pond Data

UG Chambers -Invert elev. = 100.00 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 154.50 ft, No. Barrels = 4, Slope = 0.00%, Headers = Yes

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	688	688
1.00	101.00	n/a	1,195	1,883
1.50	101.50	n/a	1,453	3,336
2.00	102.00	n/a	1,601	4,937
2.50	102.50	n/a	1,673	6,610
3.00	103.00	n/a	1,672	8,282
3.50	103.50	n/a	1,601	9,883
4.00	104.00	n/a	1,453	11,336
4.50	104.50	n/a	1,194	12,530
5.00	105.00	n/a	687	13,217

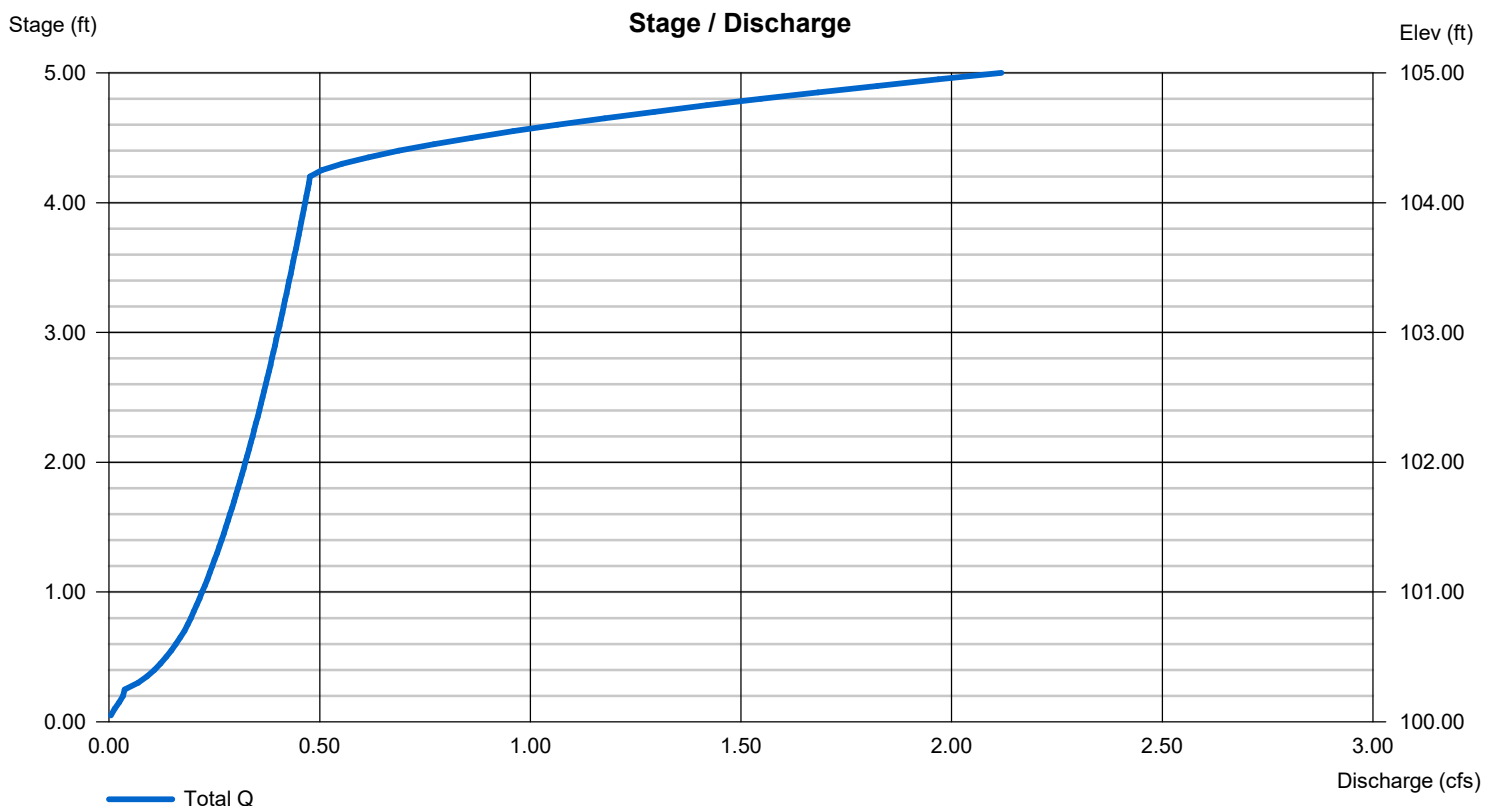
Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 3.00	Inactive	Inactive	Inactive
Span (in)	= 3.00	0.00	0.00	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 100.00	0.00	0.00	0.00
Length (ft)	= 4.00	0.00	0.00	0.00
Slope (%)	= 0.50	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 0.67	0.00	0.00	0.00
Crest El. (ft)	= 104.20	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Pond Report

4

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Tuesday, 05 / 25 / 2021

Pond No. 1 - System 1 (DMA-A)

Pond Data

UG Chambers -Invert elev. = 100.00 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 154.50 ft, No. Barrels = 4, Slope = 0.00%, Headers = Yes

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	688	688
1.00	101.00	n/a	1,195	1,883
1.50	101.50	n/a	1,453	3,336
2.00	102.00	n/a	1,601	4,937
2.50	102.50	n/a	1,673	6,610
3.00	103.00	n/a	1,672	8,282
3.50	103.50	n/a	1,601	9,883
4.00	104.00	n/a	1,453	11,336
4.50	104.50	n/a	1,194	12,530
5.00	105.00	n/a	687	13,217

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 3.00	Inactive	Inactive	Inactive
Span (in)	= 3.00	0.00	0.00	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 100.00	0.00	0.00	0.00
Length (ft)	= 4.00	0.00	0.00	0.00
Slope (%)	= 0.50	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 0.67	0.00	0.00	0.00
Crest El. (ft)	= 104.20	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	100.00	0.00	---	---	---	0.00	---	---	---	---	---	0.000
0.50	688	100.50	0.14 oc	---	---	---	0.00	---	---	---	---	---	0.135
1.00	1,883	101.00	0.22 ic	---	---	---	0.00	---	---	---	---	---	0.221
1.50	3,336	101.50	0.28 ic	---	---	---	0.00	---	---	---	---	---	0.277
2.00	4,937	102.00	0.32 ic	---	---	---	0.00	---	---	---	---	---	0.324
2.50	6,610	102.50	0.36 ic	---	---	---	0.00	---	---	---	---	---	0.364
3.00	8,282	103.00	0.40 ic	---	---	---	0.00	---	---	---	---	---	0.401
3.50	9,883	103.50	0.43 ic	---	---	---	0.00	---	---	---	---	---	0.434
4.00	11,336	104.00	0.47 ic	---	---	---	0.00	---	---	---	---	---	0.465
4.50	12,530	104.50	0.49 ic	---	---	---	0.37	---	---	---	---	---	0.861
5.00	13,217	105.00	0.52 ic	---	---	---	1.60	---	---	---	---	---	2.118

Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

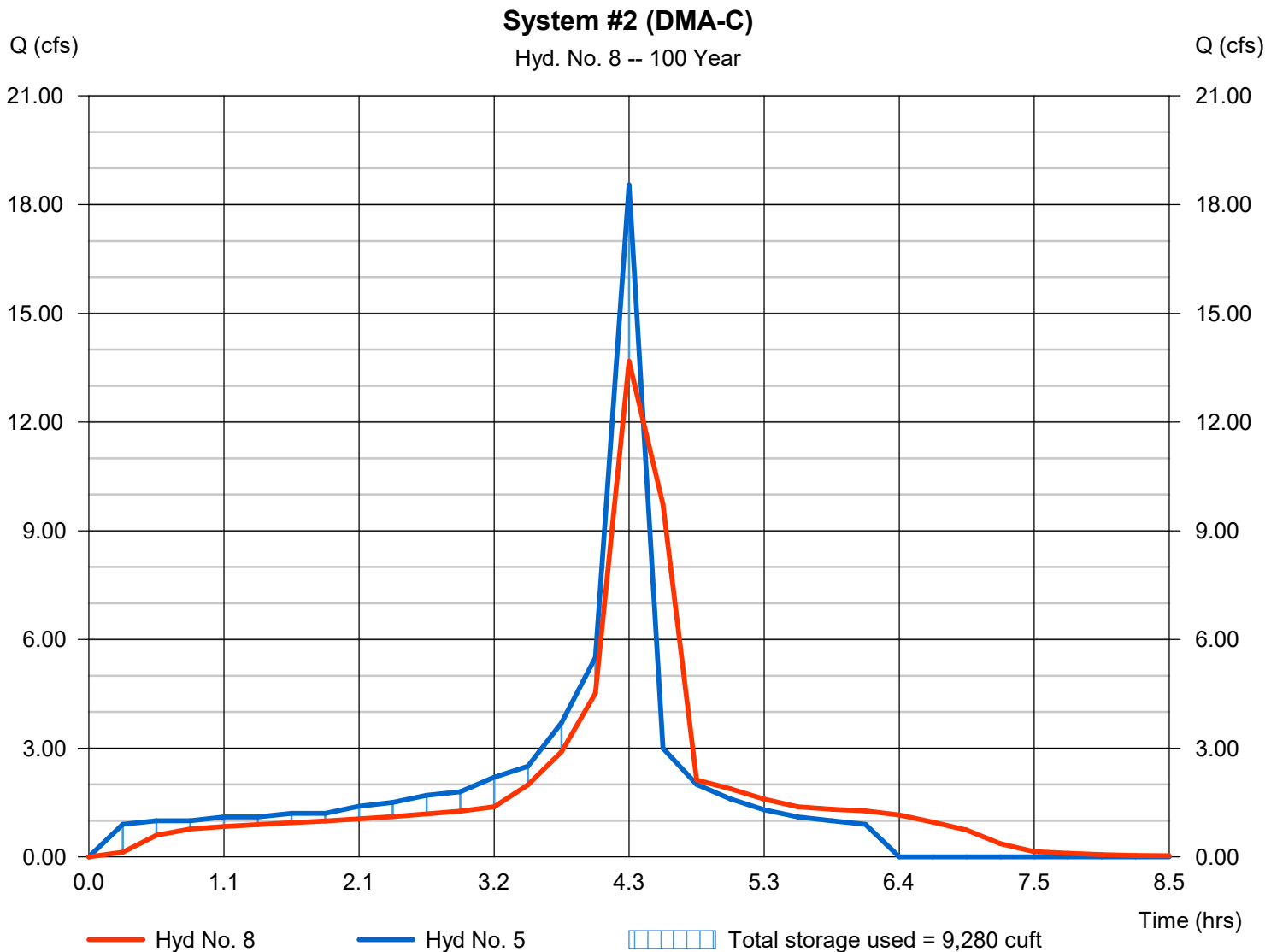
Wednesday, 05 / 19 / 2021

Hyd. No. 8

System #2 (DMA-C)

Hydrograph type	= Reservoir	Peak discharge	= 13.68 cfs
Storm frequency	= 100 yrs	Time to peak	= 4.27 hrs
Time interval	= 16 min	Hyd. volume	= 54,942 cuft
Inflow hyd. No.	= 5 - DMA-C Prop 100-Yr to System #2	Max. Elevation	= 105.00 ft
Reservoir name	= System 2 (DMA-C)	Max. Storage	= 9,280 cuft

Storage Indication method used.



Pond Report

8

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 05 / 19 / 2021

Pond No. 2 - System 2 (DMA-C)

Pond Data

UG Chambers -Invert elev. = 100.00 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 110.00 ft, No. Barrels = 4, Slope = 0.00%, Headers = Yes

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	506	506
1.00	101.00	n/a	879	1,385
1.50	101.50	n/a	1,069	2,454
2.00	102.00	n/a	1,177	3,631
2.50	102.50	n/a	1,230	4,862
3.00	103.00	n/a	1,230	6,092
3.50	103.50	n/a	1,177	7,269
4.00	104.00	n/a	1,069	8,337
4.50	104.50	n/a	878	9,216
5.00	105.00	n/a	505	9,721

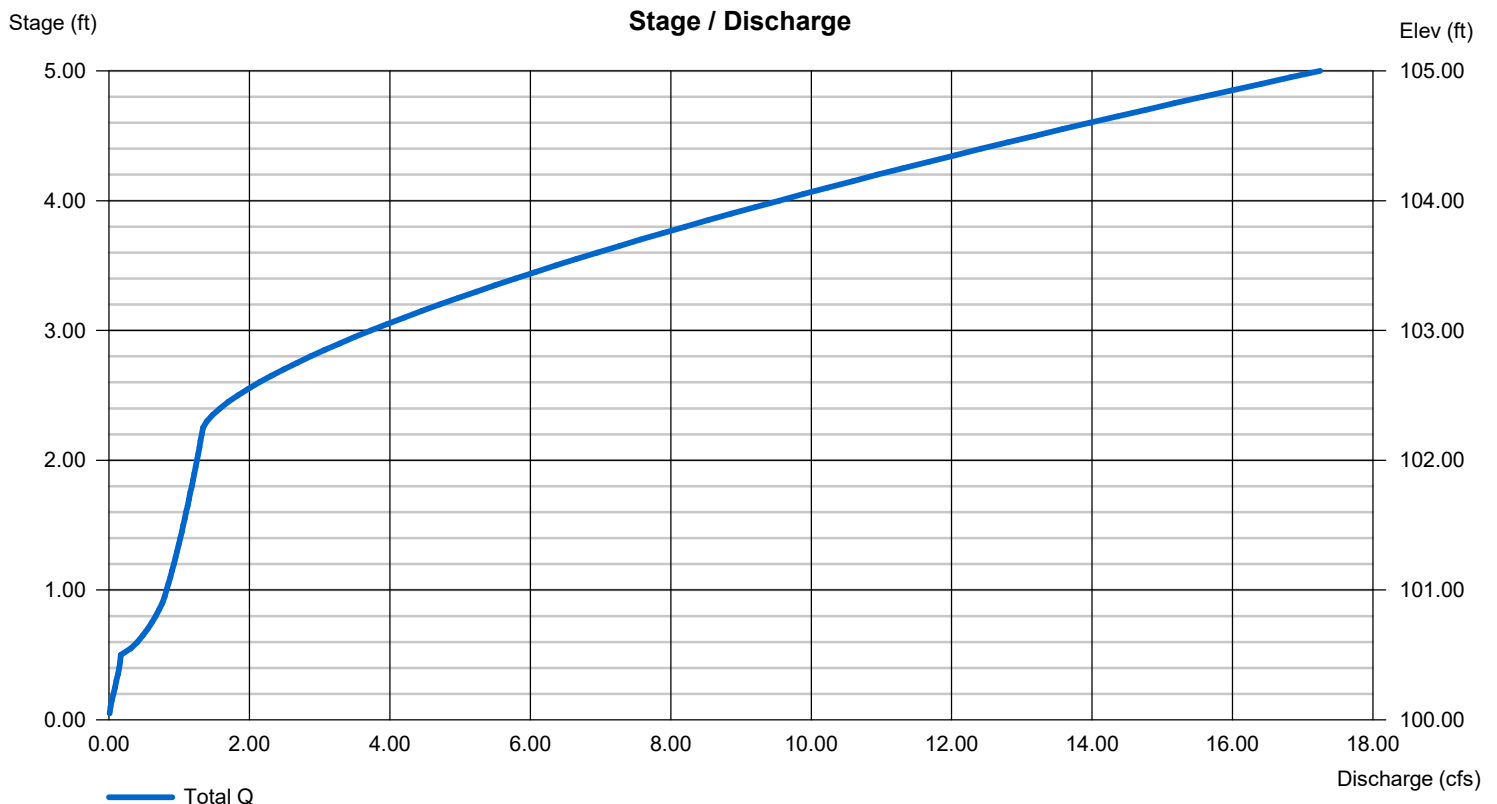
Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 6.00	Inactive	0.00	0.00
Span (in)	= 6.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0
Invert El. (ft)	= 100.00	0.00	0.00	0.00
Length (ft)	= 4.00	0.00	0.00	0.00
Slope (%)	= 0.50	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 1.00	0.00	0.00	0.00
Crest El. (ft)	= 102.25	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 05 / 19 / 2021

Pond No. 2 - System 2 (DMA-C)

Pond Data

UG Chambers -Invert elev. = 100.00 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 110.00 ft, No. Barrels = 4, Slope = 0.00%, Headers = Yes

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	506	506
1.00	101.00	n/a	879	1,385
1.50	101.50	n/a	1,069	2,454
2.00	102.00	n/a	1,177	3,631
2.50	102.50	n/a	1,230	4,862
3.00	103.00	n/a	1,230	6,092
3.50	103.50	n/a	1,177	7,269
4.00	104.00	n/a	1,069	8,337
4.50	104.50	n/a	878	9,216
5.00	105.00	n/a	505	9,721

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 6.00	Inactive	0.00	0.00
Span (in)	= 6.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0
Invert El. (ft)	= 100.00	0.00	0.00	0.00
Length (ft)	= 4.00	0.00	0.00	0.00
Slope (%)	= 0.50	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 1.00	0.00	0.00	0.00
Crest El. (ft)	= 102.25	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	100.00	0.00	---	---	---	0.00	---	---	---	---	---	0.000
0.50	506	100.50	0.17 oc	---	---	---	0.00	---	---	---	---	---	0.166
1.00	1,385	101.00	0.82 ic	---	---	---	0.00	---	---	---	---	---	0.819
1.50	2,454	101.50	1.06 ic	---	---	---	0.00	---	---	---	---	---	1.057
2.00	3,631	102.00	1.25 ic	---	---	---	0.00	---	---	---	---	---	1.251
2.50	4,862	102.50	1.42 ic	---	---	---	0.42	---	---	---	---	---	1.834
3.00	6,092	103.00	1.57 ic	---	---	---	2.16	---	---	---	---	---	3.731
3.50	7,269	103.50	1.70 ic	---	---	---	4.65	---	---	---	---	---	6.358
4.00	8,337	104.00	1.83 ic	---	---	---	7.71	---	---	---	---	---	9.540
4.50	9,216	104.50	1.95 ic	---	---	---	11.24	---	---	---	---	---	13.19
5.00	9,721	105.00	2.06 ic	---	---	---	15.19	---	---	---	---	---	17.25

APPENDIX F

National Flood Hazard Layer Map

National Flood Hazard Layer FIRMette



117°1'36"W 33°14'4"N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard Zone D
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **11/23/2020 at 11:38 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

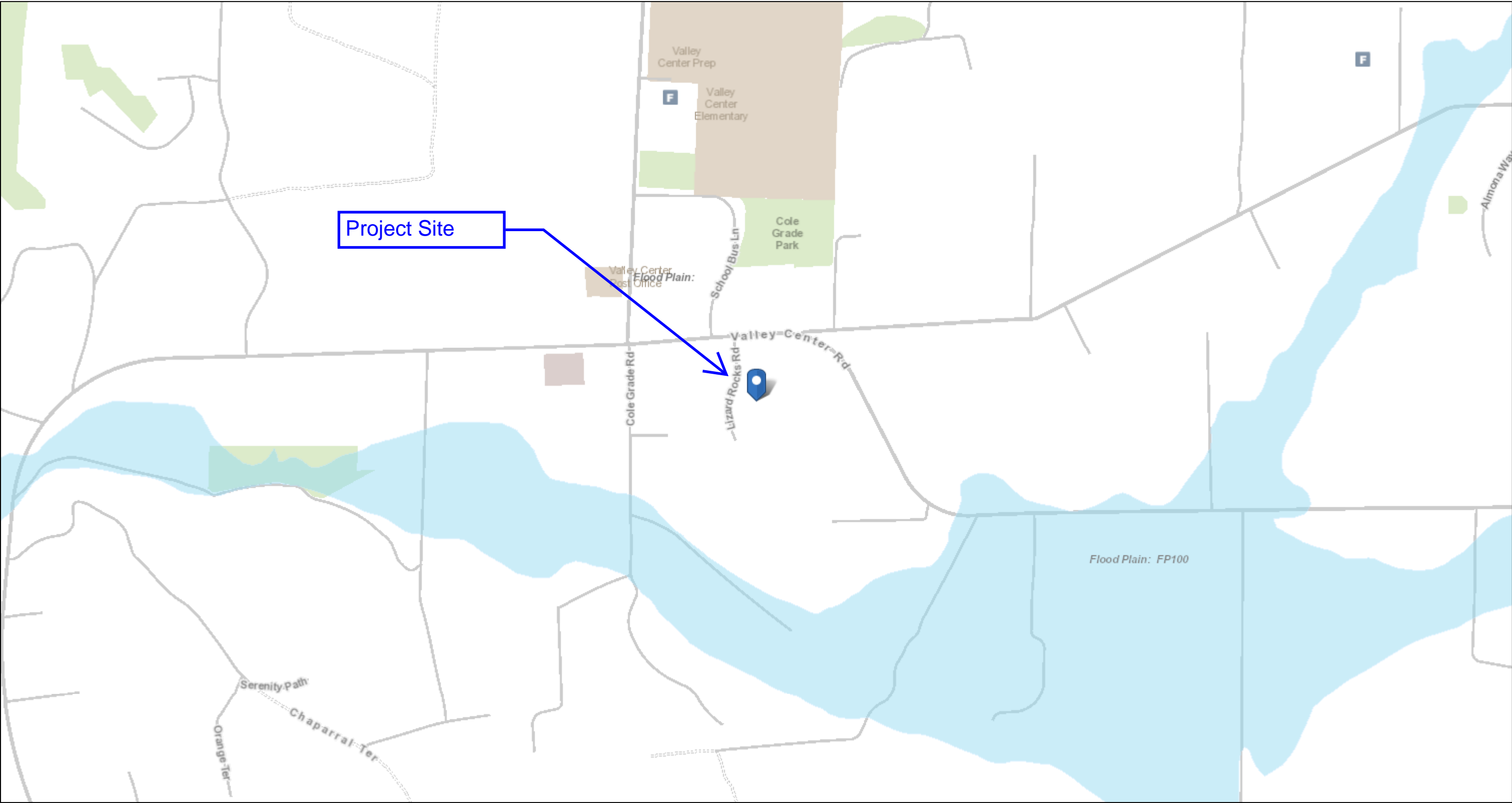
This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

USGS The National Map: Orthoimagery. Data refreshed October, 2020.

0 250 500 1,000 1,500 2,000 Feet 1:6,000

117°0'59"W 33°13'33"N

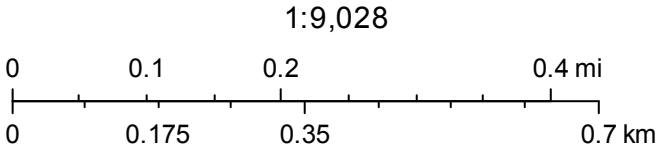
(San Diego County) Parcel Lookup Tool Map



November 23, 2020

Flood Plain

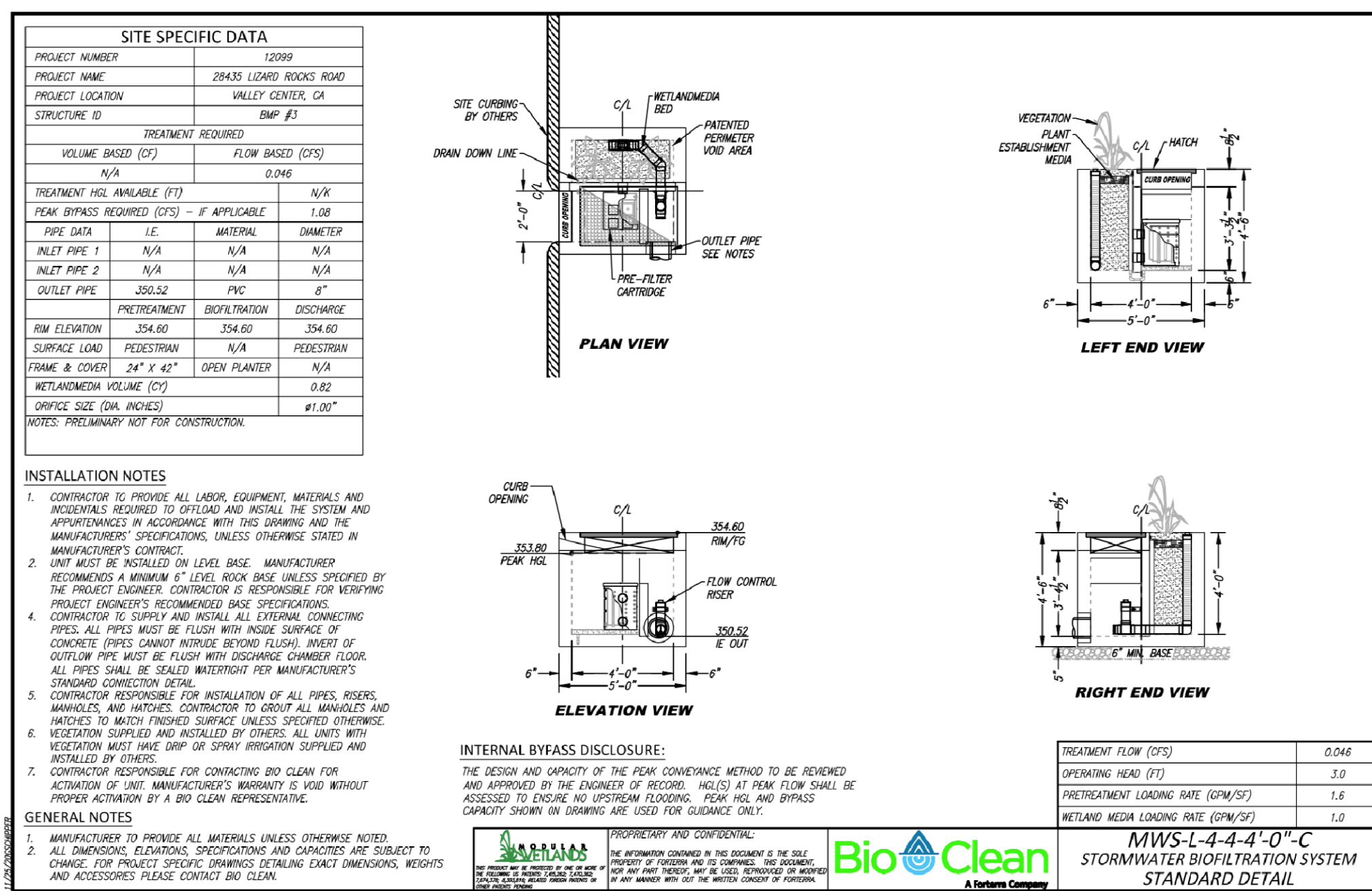
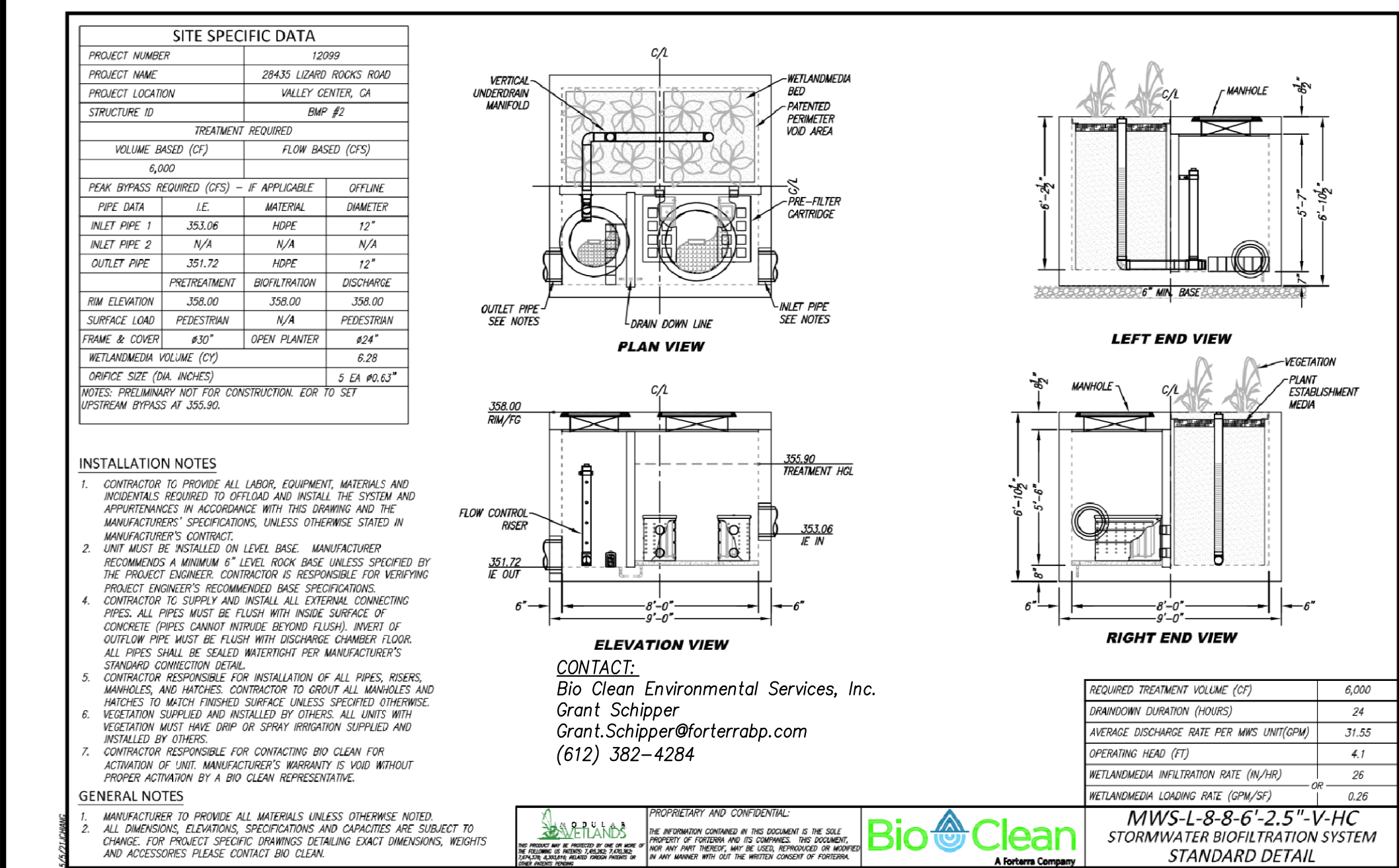
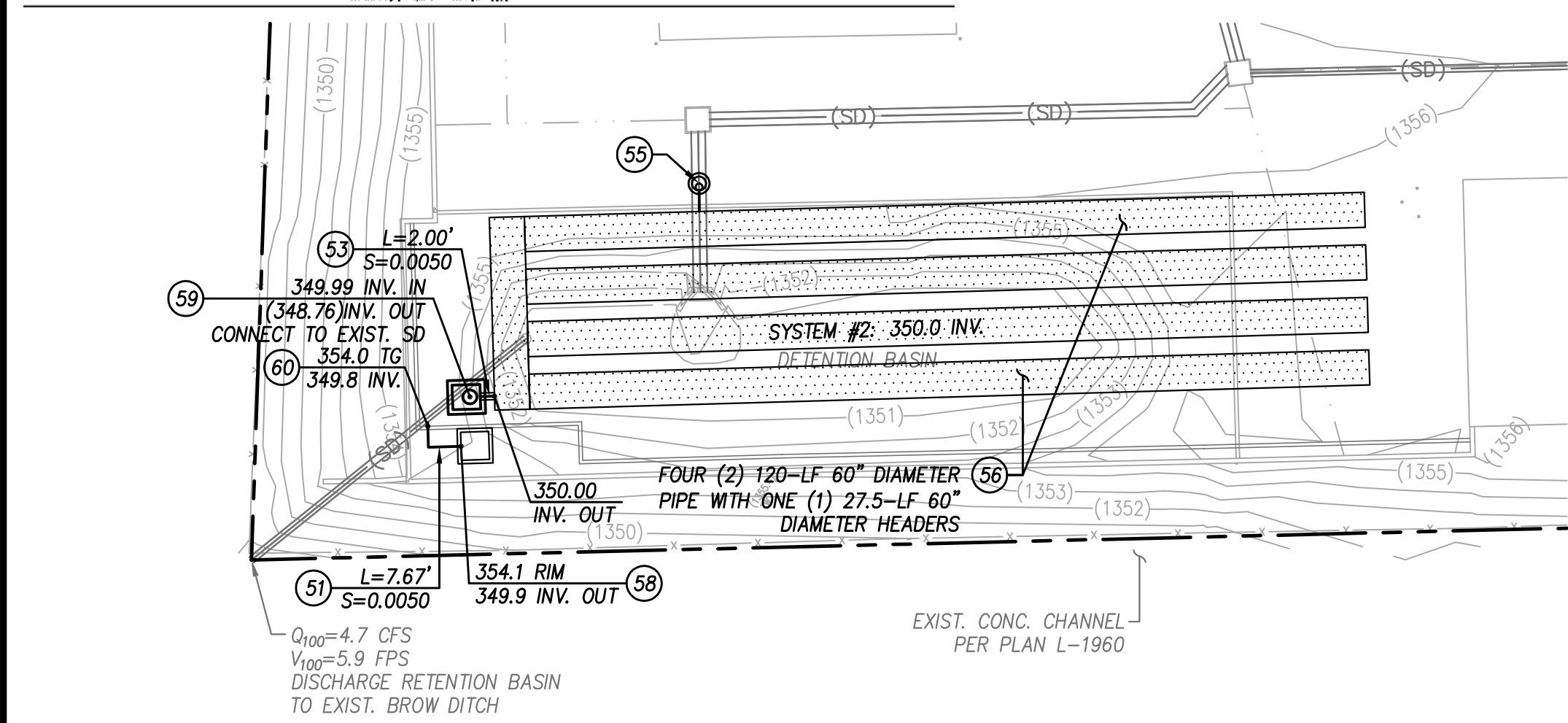
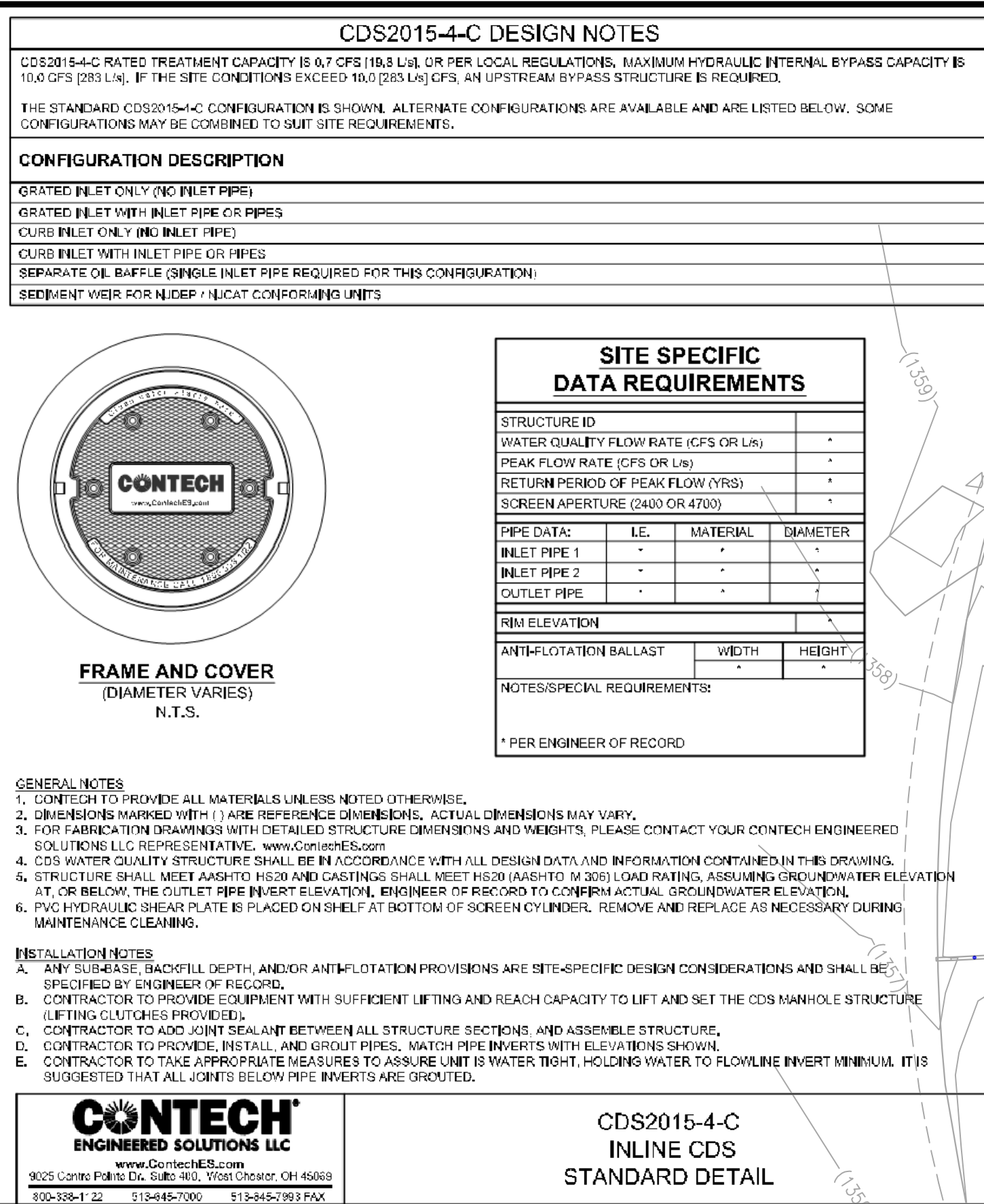
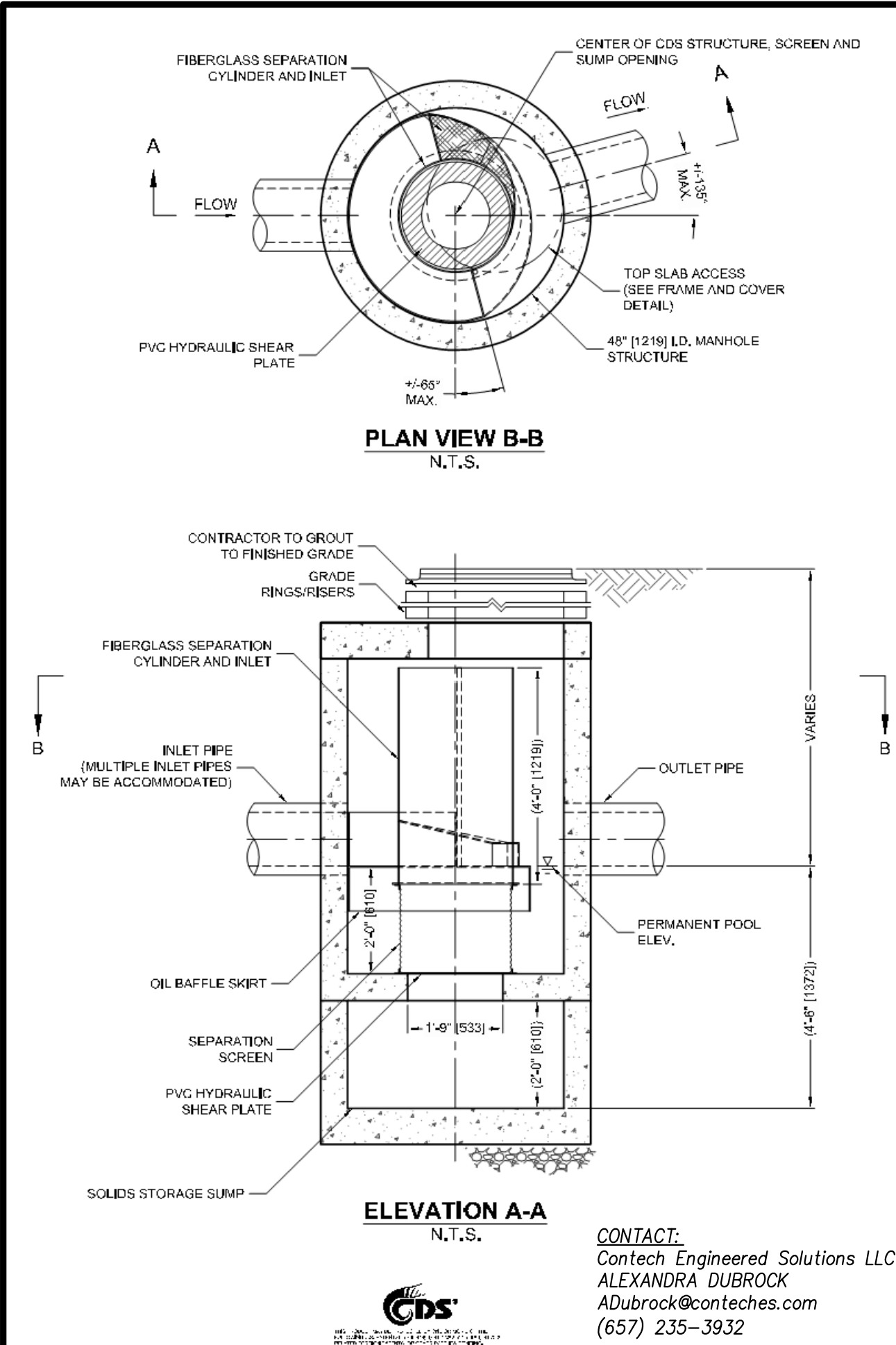
- ZONE X
- FP100
- FP500
- FW100



SanGIS

APPENDIX G

Conceptual Utility Plan



- STORM DRAIN CONSTRUCTION NOTES:**
- CONSTRUCT 8" PVC STORM DRAIN
 - CONSTRUCT 12" PVC STORM DRAIN
 - CONSTRUCT 18" HDPE STORM DRAIN
 - CONSTRUCT 24" HDPE STORM DRAIN
 - INSTALL CONTECH CDS UNIT, CDS2015-4-C, FOR TREATMENT WITH OVERFLOW BYPASS
 - INSTALL CONTECH CMP DETENTION SYSTEM
 - INSTALL MODULAR WETLAND SYSTEM (MWS), MWS-L-8-8-V, FOR STORMWATER TREATMENT
 - INSTALL MODULAR WETLAND SYSTEM (MWS), MWS-L-4-4-C, FOR STORMWATER TREATMENT
 - CONSTRUCT WEIR OUTLET STRUCTURE MANHOLE PER DETAIL ON SHEET 3
 - CONSTRUCT 18"x18" GRATE INLET
 - CONSTRUCT CHANNEL DRAIN

LEGEND:

← DIRECTION OF PIPE FLOW

—SD— PROPOSED STORM DRAIN SYSTEM

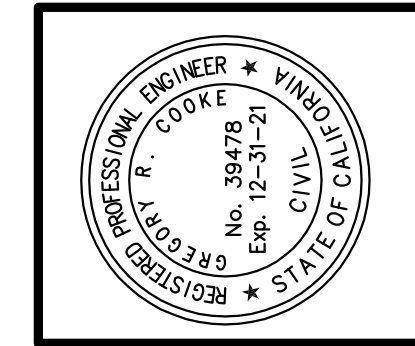
(CB) EXISTING CATCH BASIN/DRAINAGE INLET



PREPARED BY: **dlbrc** Engineering, Inc.
Civil Engineering/Land Surveying/Land Planning
180 S. Old Springs Road
Suite 210
Anheim Hills, CA 92808
714-685-6860

DATE: _____

GREGORY R. COOKE
R.C.E. 39478



NO.	REVISION:	DATE:

GS VALLEY EXPANSION
24835 LIZARD ROCKS ROAD
VALLEY CENTER, CA 92082

PROJECT: _____

DRAWING NAME: **PRELIMINARY STORM DRAIN PLAN**

ISSUE: **CONCEPTUAL**

DATE: **05/25/2021**

CHECKED: **GRC** DRAWN: **MS**

DRAWING FILE: **19132CG**

PROJECT NO.: **19-132**

SHEET NUMBER: **4**

OF 6 SHEETS

SCALE: **AS SHOWN**

APPENDIX H

As-Built Reference Plans

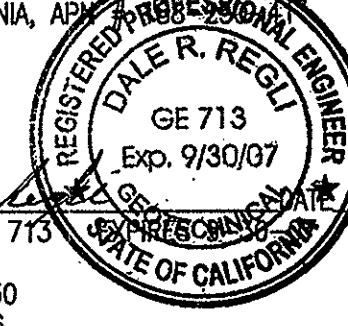
GENERAL NOTES

1. A PERMIT SHALL BE OBTAINED FROM THE COUNTY DEPARTMENT OF PUBLIC WORKS FOR ANY WORK WITHIN THE COUNTY RIGHT-OF-WAY.
2. THE STRUCTURAL SECTION SHALL BE IN ACCORDANCE WITH SAN DIEGO COUNTY STANDARDS FOR PRIVATE ROADS AND AS APPROVED BY THE MATERIALS LABORATORY.
3. APPROVAL OF THESE IMPROVEMENT PLANS AS SHOWN DOES NOT CONSTITUTE APPROVAL OF ANY CONSTRUCTION OUTSIDE THE PROJECT BOUNDARY.
4. ALL UNDERGROUND UTILITIES WITHIN THE STREET RIGHT-OF-WAY SHALL BE CONSTRUCTED, CONNECTED AND TESTED PRIOR TO CONSTRUCTION OF DIKE, CURB, CROSS GUTTER AND PAVING.
5. THE EXISTENCE AND LOCATION OF EXISTING UNDERGROUND FACILITIES SHOWN ON THESE PLANS WERE OBTAINED BY A SEARCH OF THE AVAILABLE RECORDS TO THE BEST OF OUR KNOWLEDGE THERE ARE NO OTHER EXISTING FACILITIES EXCEPT AS SHOWN ON THESE PLANS. HOWEVER THE CONTRACTOR IS REQUIRED TO TAKE PRECAUTIONARY MEASURES TO PROTECT ANY EXISTING FACILITY SHOWN HEREON AND ANY OTHER WHICH IS NOT OF RECORD OR NOT SHOWN ON THESE PLANS.
6. LOCATION AND ELEVATION OF IMPROVEMENTS TO BE MET BY WORK TO BE DONE SHALL BE CONFIRMED BY FIELD MEASUREMENTS PRIOR TO CONSTRUCTION OF NEW WORK. CONTRACTOR WILL MAKE EXPLORATORY EXCAVATIONS AND LOCATE EXISTING UNDERGROUND FACILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS TO PLANS IF REVISIONS ARE NECESSARY BECAUSE OF ACTUAL LOCATION OF EXISTING FACILITIES.
7. THE CONTRACTOR SHALL NOTIFY THE SAN DIEGO GAS AND ELECTRIC COMPANY PRIOR TO STARTING WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK WITH COMPANY REPRESENTATIVES.
NOTICE: ALL ELECTRICAL AND GAS SERVICES WITHIN THIS PROJECT ARE "UNDERGROUND INSTALLATIONS." FOR LOCATIONS OF ELECTRICAL CABLES AND GAS PIPING AND APPURTENANCES CONTACT THE SAN DIEGO GAS AND ELECTRIC COMPANY.
TELEPHONE: 619-232-4252
8. THE CONTRACTOR SHALL NOTIFY PACIFIC BELL TELEPHONE COMPANY PRIOR TO STARTING WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK WITH COMPANY REPRESENTATIVES.
NOTICE: ALL TELEPHONE SERVICES WITHIN THIS PROJECT BOUNDARY ARE "UNDERGROUND INSTALLATIONS." FOR LOCATION OF CABLES AND APPURTENANCES CONTACT PACIFIC BELL TELEPHONE COMPANY
TELEPHONE: 619-293-0595
9. IT SHALL BE THE RESPONSIBILITY OF THE DEVELOPER TO CONTACT THE UTILITY AGENCIES, ADVISE THEM OF THE PROPOSED IMPROVEMENTS AND BEAR THE COST OF RELOCATIONS, IF NEEDED.
10. ALL TELEVISION SERVICES WITHIN THIS PROJECT ARE "UNDERGROUND INSTALLATIONS." FOR LOCATION OF CABLES AND APPURTENANCES CONTACT ADELPHIA CABLE COMMUNICATIONS, TELEPHONE: 760-728-5002
11. NO PAVING SHALL BE DONE UNTIL EXISTING POWER POLES ARE RELOCATED OUTSIDE THE AREAS TO BE PAVED.
12. PRIVATE ROAD IMPROVEMENTS SHOWN HEREON ARE FOR INFORMATION ONLY. COUNTY OFFICIALS SIGNATURE HEREON DOES NOT CONSTITUTE APPROVAL OR RESPONSIBILITY OF ANY KIND FOR THE DESIGN OR CONSTRUCTION OF THESE PRIVATE IMPROVEMENTS.
13. ALL CUT AND FILL SLOPES OVER THREE (3) FEET CREATED BY GRADING FOR STREETS AND DRIVEWAYS SHALL BE HYDROSEEDING WITH SAN DIEGO COUNTY APPROVED HYDROSEED MIXTURE. HYDROSEEDING SLOPES SHALL BE IRRIGATED BY WATER TRUCK UNTIL THE MIXTURE GERMINATES AND GROWTH IS ESTABLISHED.
14. ALL INTERSECTING ROADS AND DRIVEWAYS SHALL BE GRADED AND/ OR RECONSTRUCTED, AS REQUIRED TO MATCH NEW IMPROVEMENTS AND PROVIDE SMOOTH TRANSITION
15. ALL TREES WITHIN THE RIGHT-OF-WAY SHALL BE REMOVED.

SOILS ENGINEER CERTIFICATION

THIS GRADING PLAN HAS BEEN REVIEWED BY THE UNDERSIGNED AND FOUND TO BE CONFORMANCE WITH THE RECOMMENDATIONS AS OUTLINED IN OUR SOILS REPORT FOR THIS PROJECT. THE SOILS REPORT SHALL BE CONSIDERED AS A PART OF THIS PLAN, AND ALL GRADING WORK SHALL BE DONE IN ACCORDANCE WITH THE SPECIFICATIONS AND RECOMMENDATIONS OF SAID REPORT DATED JANUARY 27, 2004
PREPARED BY: NORTH COUNTY COMPACTION ENGINEERING INC., PROJECT NO. CE-7090
TITLED: "PRELIMINARY SOILS INVESTIGATION AND STORAGE FACILITY, LIZARD ROCKS ROAD, VALLEY CENTER, CALIFORNIA, 92080"

BY: DALE R. REGI GE 713
P.O. BOX 302002
ESCONDIDO, CA 92030
TELE: 760-480-1116



WORK TO BE DONE

IMPROVEMENTS CONSIST OF THE FOLLOWING WORK TO BE DONE ACCORDING TO THESE PLANS, THE CURRENT SAN DIEGO COUNTY DEPARTMENT OF PUBLIC WORKS STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS FOR IMPROVEMENT OF SUBDIVISION STREETS AND STANDARD REFERENCE DRAWINGS.

ITEM	REGIONAL STANDARD DRAWING NO.	SYMBOL
CONCRETE PAVEMENT (5.5" CONC. / 4" CLASS ID)		
ASPHALT CONCRETE DIKE	S.D.R.S.D. C-5 (TYPE A)	
RIP RAP ENERGY DISSIPATER	S.D.R.S.D. D-40 (TYPE 2)	
BROW DITCH	S.D.R.S.D. D-75 (TYPE D)	
MASONRY RETAINING WALL PER S.D.R.S.D. C-3		
6" CONCRETE CURB PER S.D.R.S.D. G- (6")		
TYPE "A" CURB INLET PER S.D.R.S.D. D-1		
STORM DRAIN PIPE		
CUT SLOPE (2:1 MAX)		
FILL SLOPE (2:1 MAX)		
CUT/FILL DAYLIGHT LINE		
WING TYPE HEADWALL PER S.D.R.S.D. D-34		
U TYPE HEADWALL PER S.D.R.S.D. D-34		

EARTHWORK QUANTITIES

	CUT	FILL
EXCAVATION (RAW):	7959 C.Y.	
COMPACTION (RAW):		8644 C.Y.
CLEAR & GRUB:	<0 C.Y.>	
PAVEMENT SECTION:	-0- C.Y.	
TOTALS:	7959 C.Y.	8644 C.Y.
IMPORT:		685 C.Y.

THE EARTHWORK QUANTITIES SHOWN WERE CALCULATED BASED ON THE PRISMOIDAL (CONTOUR SLICE) METHOD. THE QUANTITIES INCLUDE STREET PAVEMENT AND BASE (RAW) VOLUMES.
THE EARTHWORK QUANTITIES SHOWN DO NOT ACCOUNT FOR THE IMPACTS OF THE FINAL EARTH MOVING QUANTITIES OF THE FOLLOWING ITEMS:

- 1) SHRINK OR SWELL OF THE NATIVE OR IMPORTED MATERIAL
- 2) EXCAVATION OF BUILDING AND COLUMN FOOTINGS
- 3) SPOIL MATERIAL CREATED FROM PIPE TRENCHING AND BACKFILL
- 4) PLACEMENT OF LANDSCAPING TOP SOILS
- 5) RETAINING WALL EXCAVATION AND BACKFILL

THE CONTRACTOR SHALL PREPARE SEPARATE EARTHWORK QUANTITY CALCULATIONS PRIOR TO BIDDING AND SHALL BASE HIS/HER BASE BID EXCLUSIVELY ON HIS/HER OWN COMPUTATION.

OWNER'S CERTIFICATE

IT IS AGREED THAT FIELD CONDITIONS MAY REQUIRE CHANGES TO THESE PLANS.

IT IS FURTHER AGREED THAT THE OWNER (DEVELOPER) SHALL HAVE A REGISTERED CIVIL ENGINEER MAKE SUCH CHANGES, ALTERATIONS OR ADDITIONS TO THESE PLANS WHICH THE DIRECTOR OF PUBLIC WORKS DETERMINES ARE NECESSARY AND DESIRABLE FOR THE PROPER COMPLETION OF THE IMPROVEMENTS.

I FURTHER AGREE TO COMMENCE WORK ON ANY IMPROVEMENTS SHOWN IN THESE PLANS WITHIN EXISTING COUNTY RIGHT-OF-WAY WITHIN 60 DAYS AFTER ISSUANCE OF THE CONSTRUCTION PERMIT AND TO PURSUE SUCH WORK ACTIVELY ON EVERY NORMAL WORKING DAY UNTIL COMPLETED, IRRESPECTIVE AND INDEPENDENT OF OTHER WORK ASSOCIATED WITH THIS PROJECT UNDER MY CONTROL.

OWNER/PERMITTEE:

BY: JOSEPH S. LAGHNER DATE: 2/20/12
LIZARD ROCK, LLC
BY: MIKE FARLEY DATE: 2/20/12
CML-CA LIZARD ROCK, LLC

SPECIAL GRADING NOTE

RESTRICT ALL BRUSHING, CLEARING AND/ OR GRADING SUCH THAT NONE WILL BE ALLOWED DURING THE AVIAN BREEDING SEASON. THIS IS DEFINED AS OCCURRING BETWEEN FEBRUARY 1 AND AUGUST 1. THE DIRECTOR OF PLANNING AND LAND USE, MAY WAIVE THIS CONDITION, THROUGH WRITTEN CONFORMANCE FROM THE UNITED STATES FISH AND WILDLIFE SERVICE AND THE CALIFORNIA DEPARTMENT OF FISH AND GAME, THAT NO NESTING BIRDS ARE PRESENT IN THE VICINITY OF THE BRUSHING, CLEARING OR GRADING.

FIRE DEPARTMENT NOTE

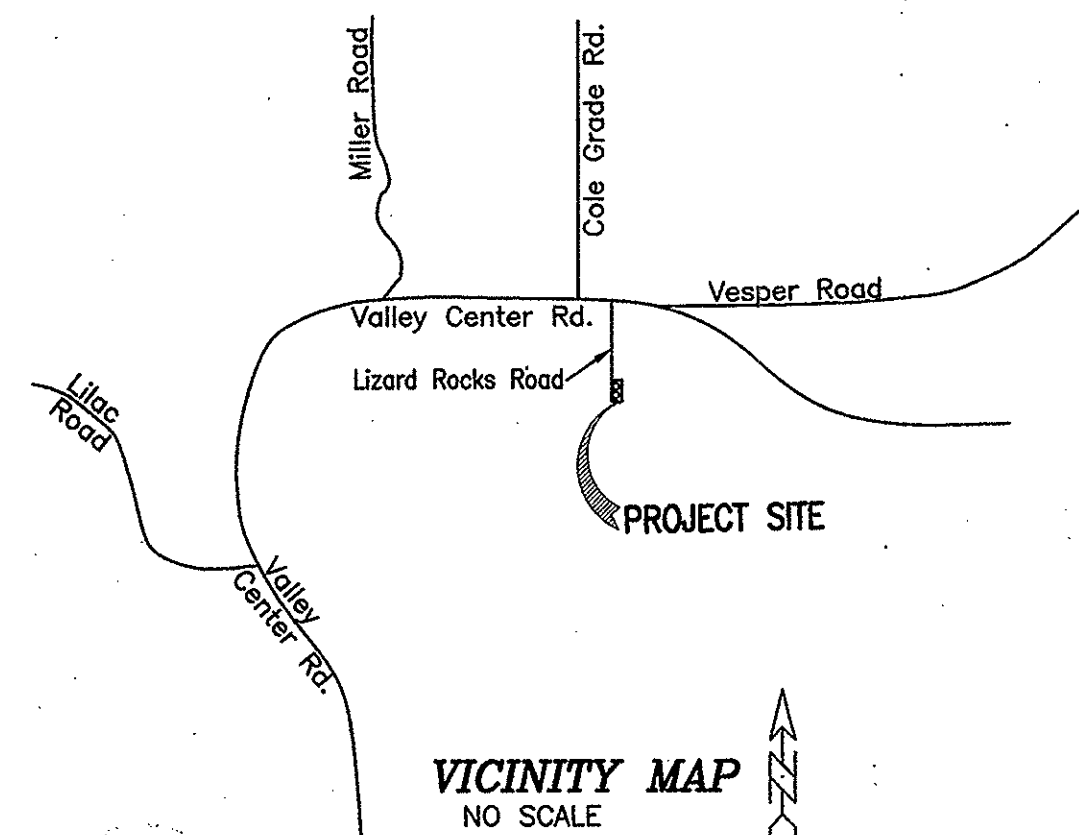
ALL ONSITE FIRE HYDRANTS MUST BE COMMERCIAL HYDRANTS AND HAVE A MINIMUM FIRE FLOW OF 2000 G.P.M. AT 25 P.S.I. ALL COMPONENTS MUST MEET THE APPROVAL OF THE VALLEY CENTER FIRE PROTECTION DISTRICT AND THE VALLEY CENTER MUNICIPAL WATER DISTRICT. DESIGN OF THE WATER SUPPLY, TYPE AND LOCATION OF THE FIRE HYDRANTS MUST BE SUBMITTED TO THE FIRE MARSHALL FOR APPROVAL PRIOR TO ANY BUILDING MATERIALS BEING PLACED ON SITE.

GENERAL NOTES

1. APPROVAL OF THIS GRADING PLAN DOES NOT CONSTITUTE APPROVAL OF VERTICAL OR HORIZONTAL ALIGNMENT OF ANY PRIVATE ROAD SHOWN HEREON FOR COUNTY ROAD PURPOSES.
2. FINAL APPROVAL OF THESE GRADING PLANS SUBJECT TO FINAL APPROVAL OF THE ASSOCIATED IMPROVEMENT PLANS WHERE APPLICABLE. FINAL CURB ELEVATIONS MAY REQUIRE CHANGES IN THESE PLANS.
3. IMPORT MATERIAL SHALL BE OBTAINED FROM A LEGAL SITE.
4. A CONSTRUCTION, EXCAVATION OR ENCROACHMENT PERMIT FROM THE DEPARTMENT OF PUBLIC WORKS WILL BE REQUIRED FOR ANY WORK IN THE COUNTY RIGHT-OF-WAY.
5. ALL SLOPES OVER THREE FEET IN HEIGHT WILL BE PLANTED IN ACCORDANCE WITH SAN DIEGO COUNTY SPECIFICATIONS.
6. THE CONTRACTOR SHALL VERIFY THE EXISTENCE AND LOCATION OF ALL UTILITIES BEFORE COMMENCING WORK. NOTICE OF PROPOSED WORK SHALL BE GIVEN TO THE FOLLOWING AGENCIES:
SAN DIEGO GAS & ELECTRIC: TELEPHONE NO. 619-232-4252, EXT. 1658
S.B.C. TELEPHONE NO. 619-298-0595
CATV: ADELPHIA CABLE COMMUNICATIONS, TELEPHONE: 760-728-5002
WATER: FALLBROOK PUBLIC UTILITY DIST. TELEPHONE NO: 760-749-1600
DIGALERT: TELEPHONE NO: 1-800-422-4133
7. A SOILS REPORT MAY BE REQUIRED PRIOR TO THE ISSUANCE OF A BUILDING PERMIT.
8. APPROVAL OF THESE PLANS BY THE DIRECTOR OF PUBLIC WORKS DOES NOT AUTHORIZE ANY WORK OR GRADING TO BE PERFORMED UNTIL THE PROPERTY OWNER'S PERMISSION HAS BEEN OBTAINED AND VALID GRADING PERMIT HAS BEEN ISSUED.
9. THE DIRECTOR OF PUBLIC WORKS' APPROVAL OF THESE PLANS DOES NOT CONSTITUTE COUNTY BUILDING OFFICIAL APPROVAL OF ANY FOUNDATION FOR STRUCTURES TO BE PLACED ON THE AREA COVERED BY THESE PLANS. NO WAIVER OF THE GRADING ORDINANCE REQUIREMENTS CONCERNING MINIMUM COVER OVER EXPANSIVE SOIL IS MADE OR IMPLIED (SECTIONS 87.403 & 87.410). ANY SUCH WAIVER MUST BE OBTAINED FROM THE DIRECTOR OF PLANNING AND LAND USE.
10. ALL OPERATIONS CONDUCTED ON THE PREMISES, INCLUDING THE WARMING UP, REPAIR, ARRIVAL, DEPARTURE OR RUNNING OF TRUCKS, EARTH MOVING EQUIPMENT, CONSTRUCTION EQUIPMENT AND ANY OTHER ASSOCIATED GRADING EQUIPMENT SHALL BE LIMITED TO THE PERIOD BETWEEN 7:00 AM AND 6:00 PM EACH DAY. MONDAY THRU SATURDAY, AND NO EARTH MOVING OR GRADING OPERATIONS SHALL BE CONDUCTED ON THE PREMISES ON SUNDAYS OR HOLIDAYS.
11. ALL MAJOR SLOPES SHALL BE ROUNDED INTO EXISTING TERRAIN TO PRODUCE A CONTINUED TRANSITION FROM CUT OR FILL FACES TO NATURAL GROUND AND ABUTTING CUT OR FILL SURFACES.
12. NOTWITHSTANDING THE MINIMUM STANDARDS SET FORTH IN THE GRADING ORDINANCE AND NOTWITHSTANDING THE APPROVAL OF THESE GRADING PLANS, THE PERMITTEE IS RESPONSIBLE FOR THE PREVENTION OF DAMAGE TO ADJACENT PROPERTY. NO PERSON SHALL EXCAVATE ON LAND SO CLOSE TO THE PROPERTY LINE AS TO ENDANGER ANY ADJOINING PUBLIC STREET, SIDEWALK, ALLEY, FUNCTION OF ANY SEWAGE DISPOSAL SYSTEM, OR ANY OTHER PUBLIC OR PRIVATE PROPERTY WITHOUT SUPPORTING AND PROTECTING SUCH PROPERTY FROM SETTLING, CRACKING, EROSION, SILTING, SCOUR OR OTHER DAMAGE WHICH MIGHT RESULT FROM THE GRADING DESCRIBED ON THIS PLAN. THE COUNTY WILL HOLD THE PERMITTEE RESPONSIBLE FOR CORRECTION OF NON-DEDICATED IMPROVEMENTS WHICH DAMAGE ADJACENT PROPERTY.
13. SLOPE RATIOS:
CUT-2:1
FILL-2:1
EXCAVATION: 7959 C.Y.
FILL: 8644 C.Y.
IMPORT: 685 C.Y.
(NOTE: A SEPARATE VALID PERMIT MUST EXIST FOR EITHER WASTE OR IMPORT AREAS)
14. SPECIAL CONDITION: IF ANY ARCHEOLOGICAL RESOURCES ARE DISCOVERED ON THE SITE OF THIS GRADING DURING GRADING OPERATIONS, SUCH OPERATIONS WILL CEASE IMMEDIATELY, AND THE PERMITTEE WILL NOTIFY THE DIRECTOR OF PUBLIC WORKS OF THE DISCOVERY. GRADING OPERATIONS WILL NOT RESUME UNTIL THE PERMITTEE HAS RECEIVED WRITTEN AUTHORITY FROM THE DIRECTOR OF PUBLIC WORKS TO DO SO.
15. FINISHED GRADING SHALL BE CERTIFIED BY A REGISTERED CIVIL ENGINEER AND INSPECTED BY THE COUNTY ENGINEER FOR DRAINAGE CLEARANCE. (APPROVAL OF ROUGH GRADING DOES NOT CERTIFY FINISH BECAUSE OF POTENTIAL SURFACE DRAINAGE PROBLEMS THAT MAY BE CREATED BY LANDSCAPING ACCOMPLISHED AFTER ROUGH GRADING CERTIFICATION.

SHEET LEGEND

NO.	DESCRIPTION	NO.	DESCRIPTION
1	TITLE SHEET	9	LANDSCAPE TITLE SHEET AND NOTES
2	NOTES	10	IRRIGATION PLAN
3	GRADING PLAN	11	IRRIGATION DETAILS
4	GRADING PLAN	12	IRRIGATION SPECIFICATIONS
5	GRADING PLAN	13	PLANTING PLAN
6	PROFILES AND SECTION PROFILES	14	PLANTING NOTES AND DETAILS
7	EROSION CONTROL PLAN	15	PLANTING SPECIFICATIONS
8	EROSION CONTROL PLAN AND NOTES	16	DATA AND TREATMENT CONTROL



VICINITY MAP
NO SCALE

THOMAS BROS. PAGE 1090, F2

STORMWATER TREATMENT BMP TABLE			
DESCRIPTION / TYPE	SHEET NO.	MAINTENANCE CATEGORY	REVISIONS
CATCH BASIN INSERTS	3 & 4	ONE	

BMPs APPROVED AS PART OF THE STORMWATER MANAGEMENT PLAN DATED 1/31/12 ON FILE WITH DPW. ANY CHANGES TO THE ABOVE BMPs WILL REQUIRE SWMP REVISION AND PLAN CHANGE APPROVALS.

ENVIRONMENTAL SERVICES UNIT

Approved for Compliance With Environmental Review

Approved By: SUS WATERS

Date: JANUARY 25, 2012

FIRE AGENCY VALLEY CENTER FIRE PROTECTION DISTRICT

APPROVAL: Joe G. Janki

DATE: 7/19/06

RECORD PLAN

BY: Joe G. Janki DATE: 10/9/12

R.C.E. 723080

EXPIRES: 12/31/13

DECLARATION OF RESPONSIBLE CHARGE

I HEREBY DECLARE THAT I AM THE ENGINEER OF WORK FOR THIS PROJECT, THAT I HAVE EXERCISED RESPONSIBLE CHARGE OVER THE DESIGN OF THE PROJECT AS DEFINED IN SECTION 6703 OF THE BUSINESS AND PROFESSIONS CODE, AND THAT THE DESIGN IS CONSISTENT WITH CURRENT STANDARDS.

I UNDERSTAND THAT THE CHECK OF PROJECT DRAWINGS AND SPECIFICATIONS BY THE COUNTY OF SAN DIEGO IS CONFINED TO REVIEW ONLY AND DOES NOT RELIEVE ME AS ENGINEER OF WORK, OF MY RESPONSIBILITIES FOR PROJECT DESIGN

BY: GARY LIPKA DATE: 7/17/06

RCE NO: 23080

EXPIRES: 12/31/09

COUNTY APPROVED CHANGES

NO.	DESCRIPTION:	APPROVED BY:	DATE:
1	PERMIT EXTENSION	<u>CH</u>	<u>1/12/08</u>
2	ADD: ESH BLOCK	<u>CH</u>	<u>3/12/2012</u>
3	BMP TABLE NEW	<u>CH</u>	<u>3/12/2012</u>
4	OWNER'S FILTER	<u>CH</u>	<u>3/12/2012</u>
5	WATER RES. SHS 30.0	<u>CH</u>	<u>3/12/2012</u>
6	NEW SHEET 16	<u>CH</u>	<u>3/12/2012</u>
7	NEW PERMIT	<u>CH</u>	<u>3/12/2012</u>
8	VOIDSHEET 13, ADDSHEET 14	<u>CH</u>	<u>3/12/2012</u>
9	VOIDSHEET 15, ADDSHEET 16	<u>CH</u>	<u>3/12/2012</u>
10	RECORD PLAN	<u>CH</u>	<u>3/12/2012</u>

PERMITS

REZONE PERMIT NO.	N/A
SITE PLAN REVIEW NO.	S-03-026
STREET IMPROVEMENT PLANS	CG 4696
NOTICE OF INTENT(WDID)	9 37C339875
BENCH MARK	
DESCRIPTION:	MONUMENT NO. 1031 PER R.O.S 14889
LOCATION:	2" IRON PIPE W/BRASS DISC MARKED "GPS KE31"
RECORD FROM:	NGVD 29
ELEVATION:	1349.30
DATUM:	M.S.L.

PRIVATE CONTRACT

SHEET 1	COUNTY OF SAN DIEGO DEPARTMENT OF PUBLIC WORKS	1576 SHEETS
GRADING PLAN FOR: LIZARD ROCKS STORAGE PARCEL 3, PARCEL MAP NO. 7078 CALIFORNIA COORDINATE INDEX <u>386-1761</u>		
APPROVED FOR: MORRIS FARRINGTON COUNTY ENGINEER <u>[Signature]</u>		ENGINEER OF WORK <u>[Signature]</u> DATE: <u>7/17/06</u> GARY LEIPA RUC# 22000 EXPIRES 12/31/06
CHECKED BY: <u>[Signature]</u> DATE: <u>11/9/06</u>		DRAWING PERMIT NO. APPROVAL DATE: <u>L-14940</u>



OWNER'S / PERMITTEE'S
NAME: LIZARD ROCK LLC CML-CA LIZARD ROCK LLC
ADDRESS: 1728 YUCCA ROAD 700 NORTHWEST 107TH AVENUE
OCEANSIDE, CA 92054 MIAMI, FL 33172

TELEPHONE NO: 760-754-3130

SHORT LEGAL DESCRIPTION: PARCEL 3, PARCEL MAP NO. 7078

A.P.N. NO: 188-250-41

SITE ADDRESS: 28407 LIZARD ROCKS ROAD, VALLEY CENTER

PDS2012-4701-14940

PLOT DATE: 7/17/06

ENGINEER: AQUATERRA ENGINEERING INC. TELEPHONE: 760-439-2802

SPECIAL NOTES

THE FOLLOWING NOTES ARE PROVIDED TO GIVE DIRECTIONS TO THE CONTRACTOR BY THE ENGINEER OF WORK. THE COUNTY ENGINEER'S SIGNATURE ON THESE PLANS DOES NOT CONSTITUTE APPROVAL OF THESE PLANS DOES NOT CONSTITUTE APPROVAL OF THESE NOTES AND THE COUNTY WILL NOT BE RESPONSIBLE FOR THEIR ENFORCEMENT.

1) NEITHER THE OWNER NOR THE ENGINEER OF WORK WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT, AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY HEALTH STANDARDS, LAWS AND REGULATIONS.

2) CONTRACTOR WILL MAKE EXPLORATION EXCAVATIONS AND LOCATE EXISTING UNDERGROUND FACILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS TO PLANS IF REVISION IS NECESSARY BECAUSE OF NEW WORK.

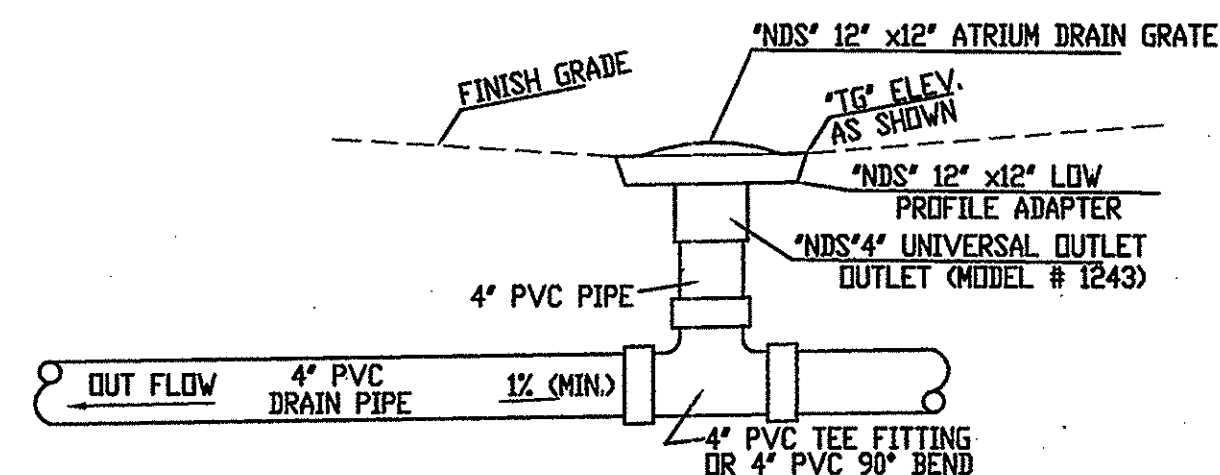
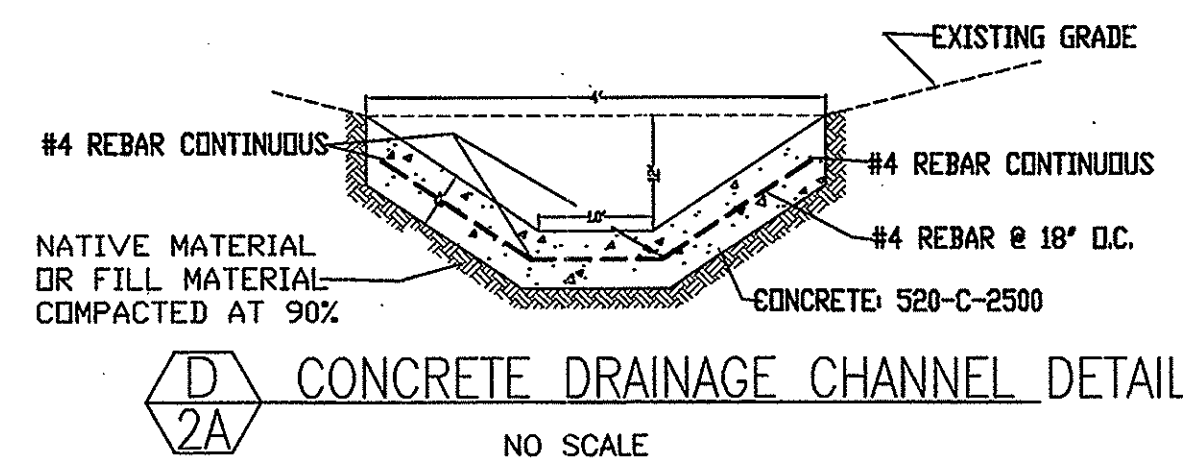
3) CONTRACTOR IS REQUIRED TO TAKE PRECAUTIONARY MEASURES TO PROTECT THE UTILITY LINES SHOWN HEREON AND ANY OTHER EXISTING LINES NOT OF RECORD OR NOT SHOWN ON THESE PLANS.

4) CONTRACTOR SHALL NOTIFY SAN DIEGO GAS & ELECTRIC CO. PRIOR TO STARTING WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK WITH COMPANY REPRESENTATIVES. FOR LOCATION OF ELECTRICAL CABLES, GAS PIPING AND APPURTENANCES, CONTACT THE SAN DIEGO GAS & ELECTRIC COMPANY, TELEPHONE 232-4252, EXT. 1658.

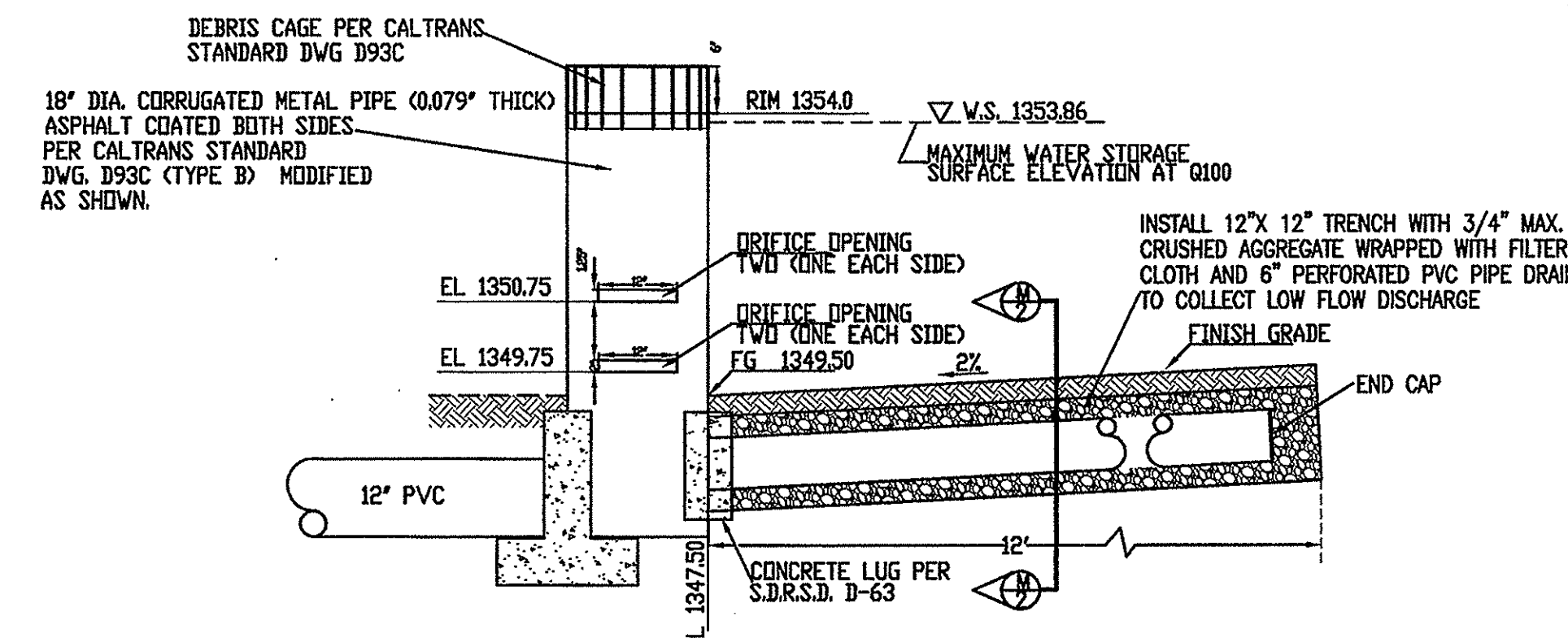
5) CONTRACTOR SHALL NOTIFY THE PACIFIC TELEPHONE CO. PRIOR TO STARTING WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK WITH COMPANY REPRESENTATIVES. FOR LOCATION OF CABLES AND APPURTENANCES, CONTACT THE PACIFIC TELEPHONE COMPANY, TELEPHONE 298-0595.

6) CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY, AND THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS, AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT EXCEPTING LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.

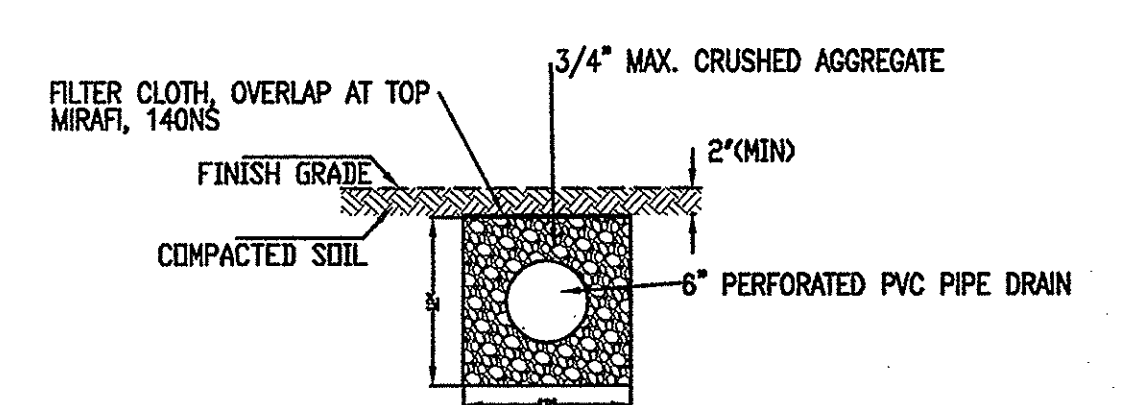
7) THE CONTRACTOR SHALL BE RESPONSIBLE TO INSURE THAT ALL WATER UTILITIES AND STORM DRAINS ARE BUILT IN ACCORDANCE WITH THESE PLANS. IF THERE IS ANY QUESTION REGARDING THESE PLANS OR FIELD STAKES, THE CONTRACTOR SHALL REQUEST AN INTERPRETATION BEFORE DOING ANY WORK BY CALLING THE ENGINEER OF WORK AT 780-439-2802. THE CONTRACTOR SHALL ALSO TAKE THE NECESSARY STEPS TO PROTECT ANY ADJACENT PROPERTY FROM HIS OPERATIONS BY APPROPRIATE MEANS (SAND BAGS, HAY BALES, TEMPORARY DESILTING BASIN, DIKES, SHORING, ETC.) UNTIL SUCH TIME THAT THE PROJECT IS COMPLETED AND ACCEPTED FOR MAINTENANCE BY WHATEVER OWNER, AGENCY OR ASSOCIATION IS TO BE ULTIMATELY RESPONSIBLE FOR MAINTENANCE.



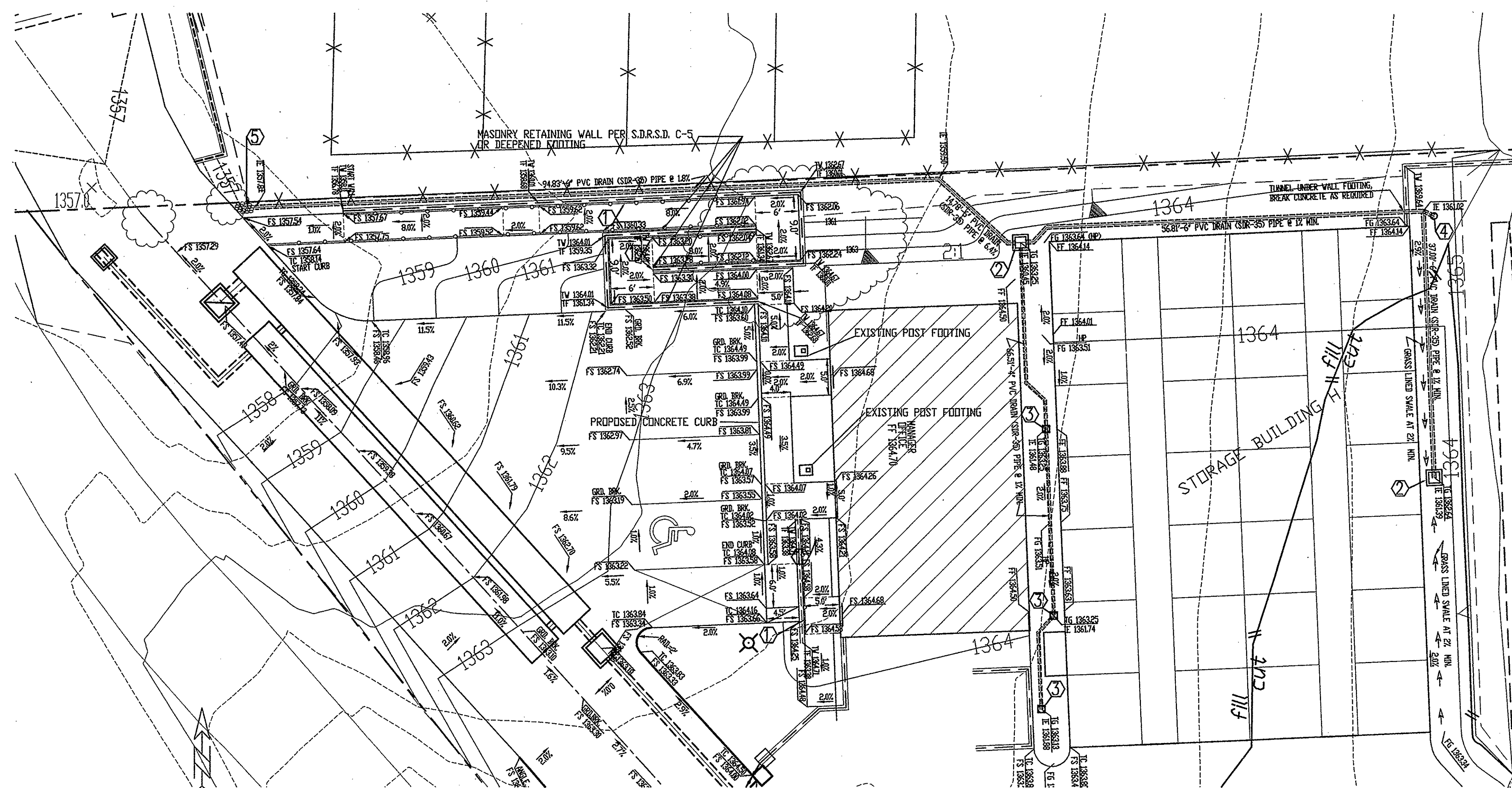
DETAIL: 12" X 12" DRAIN INLET
NO SCALE



BASIN DISCHARGE STANDPIPE DETAIL
NO SCALE



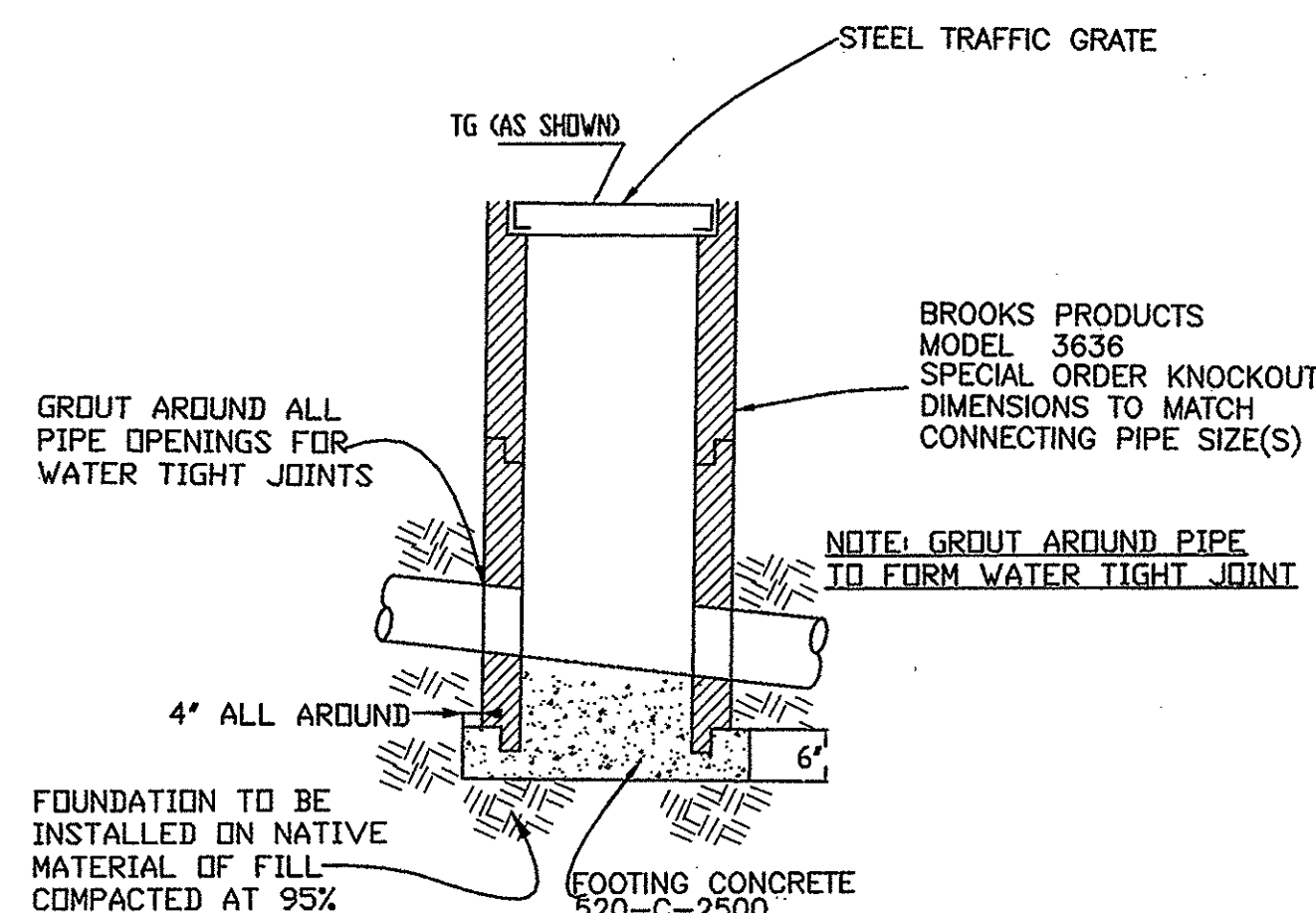
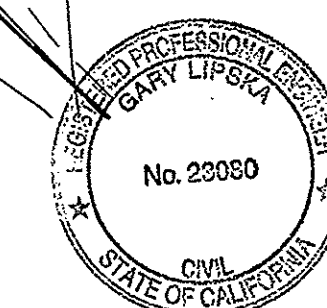
SECTION "M"-"M" SUBDRAIN TRENCH
NO SCALE



ENGINEER'S NOTES

- 1) ALL PVC DRAINAGE PIPES SHALL CONFORM TO SDR-35 PER ASTM D3034. OR SURE-LOK P477 AS MANUFACTURED BY HANCO INC. (ASTM M252, TYPE S)
- 2) LENGTHS ON DRAINAGE PIPE ARE FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.
- 3) CONTRACTOR TO VERIFY HORIZONTAL AND VERTICAL LOCATION OF EXISTING STORM DRAINAGE SYSTEMS PRIOR TO TRENCHING FOR ADJACENT OR CONNECTING FACILITIES. IN THE EVENT ACTUAL LOCATION VARIES SIGNIFICANTLY FROM THAT SHOWN ON PLAN, THE ENGINEER OF WORK SHALL BE NOTIFIED PRIOR TO PROCEEDING.

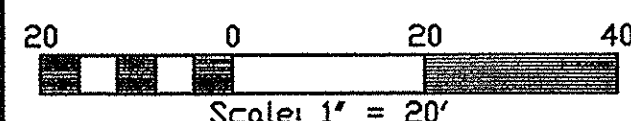
DESCRIPTION	SYMBOL
ELEVATION, TOP OF FOOTING	TF
ELEVATION, TOP OF WALL	TW
ELEVATION, TOP OF CURB	TC
ELEVATION, FINISH SURFACE	FS
ELEVATION, FINISH GRADE	FG
ELEVATION, TOP OF GRADE	TG
ELEVATION, BOTTOM OF FOOTING	BF
ELEVATION, INVERT ELEVATION	IE
ELEVATION LABEL, EXISTING	(596.00)
ELEVATION LABEL, PROPOSED	596.00
ELEVATION FINISH FLOOR	FF
DETAIL REFERENCE	2X



CATCH BASIN
NO SCALE

CONSTRUCTION LEGEND

- 1) HANDRAIL OR GUARDRAIL
- 2) "NDS" 18" X 18" CATCH BASIN WITH PLASTIC GRATE. (MODEL # 1800)
- 3) DRAIN INLET PER DETAIL A ON THIS SHEET
- 4) MODIFIED 6" CLEANDUT PER SDRSD SC-1, TYPE A C MODIFIED TO HAVE CAP AT FINISH GRADE, NO AC PAVEMENT
- 5) RIPRAP BLANKET

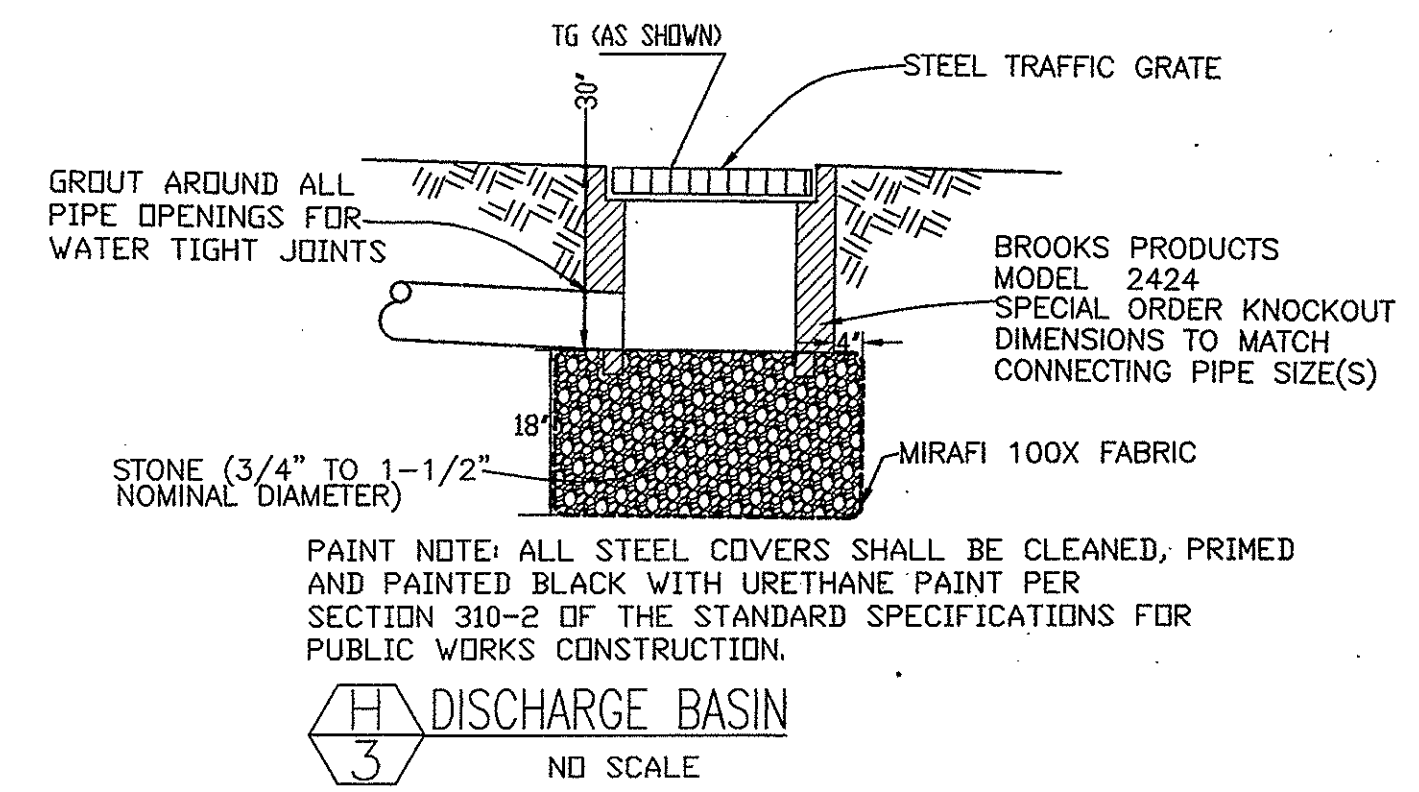
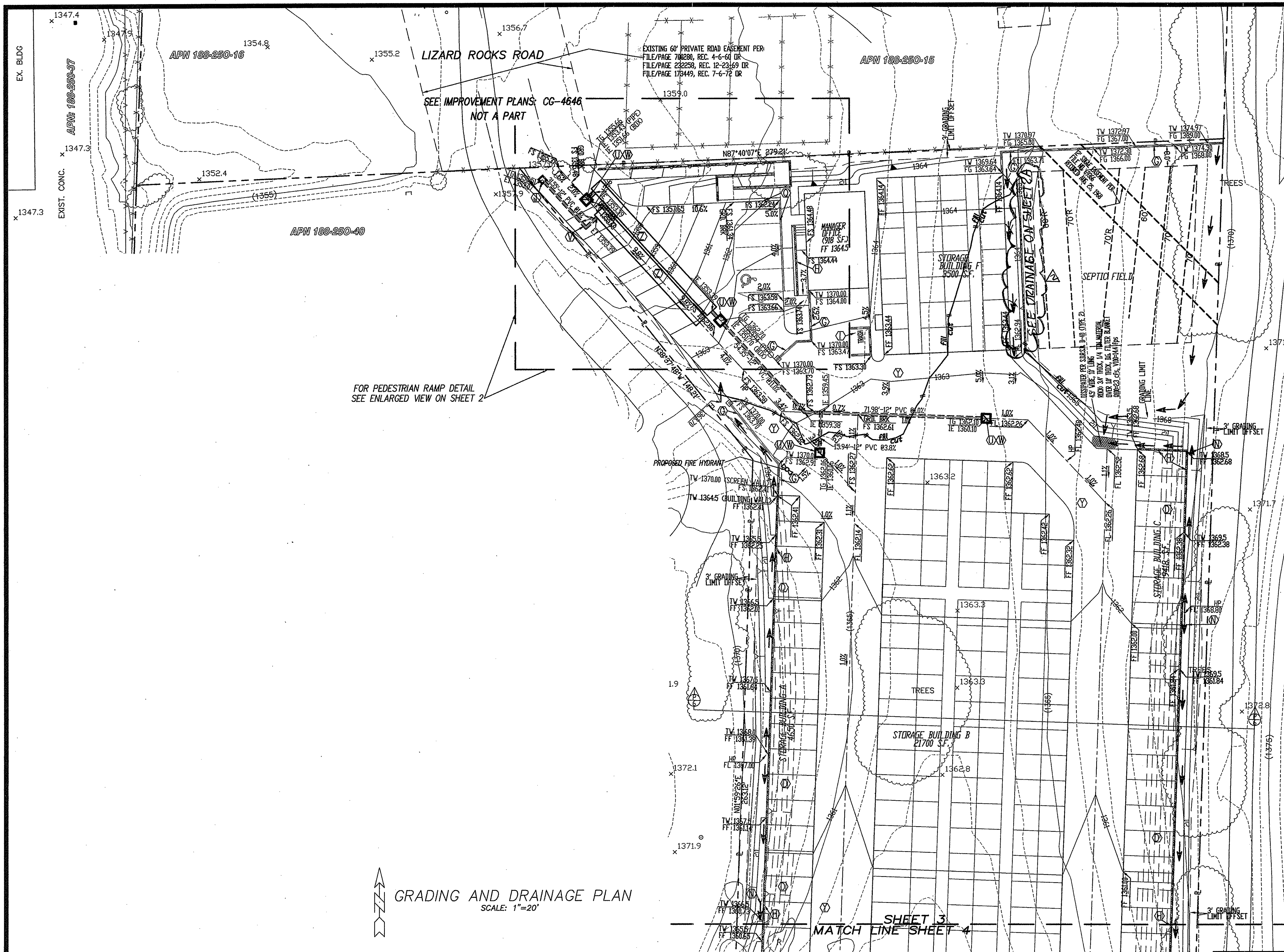


Aquatera Engineering Inc.
CIVIL ENGINEERING • LAND PLANNING
1843 Camperino Place
Oceanside, CA 92054
Tel: (760) 438-2802
Fax: (760) 438-2886

RECORD PLAN
BY: *Ng* DATE: 1/07/13
R.C.E. 125080
EXPIRES: 12/31/13

COUNTY APPROVED CHANGES			
NO.	DESCRIPTION	APPROVED BY	DATE
1	NEW SHEET, RECORD PLAN	KGB	10/30/13

PERMITS	PRIVATE CONTRACT
REZONE PERMIT NO. N/A SITE PLAN REVIEW NO. S-03-026 STREET IMPROVEMENT PLANS CG 4646 NOTICE OF INTENT(WDID): 9-37C339975 BENCH MARK DESCRIPTION: MONUMENT NO. 1031 PER R.O.S 14689 2" IRON PIPE W/BRASS DISC MARKED "GPS KE31" LOCATION: COLE GRADE ROAD, 68' W/LY OF VALLEY CENTER ROAD INTERSECTION RECORD FROM: NGVD 29 ELEVATION: 1349.30 DATUM: M.S.L.	SHEET 2A COUNTY OF SAN DIEGO DEPARTMENT OF PUBLIC WORKS 16 SHEETS GRADING PLAN FOR: LIZARD ROCKS STORAGE PARCEL 3, PARCEL MAP NO. 7078 CALIFORNIA COORDINATE INDEX 386-1761 APPROVED FOR: MORIANO ENGINEERING Kenneth S. Beagell DATE: 1/13 APPROVED BY: N/A DATE: 10-30-13 DRAWING PERMIT NO. L-14940



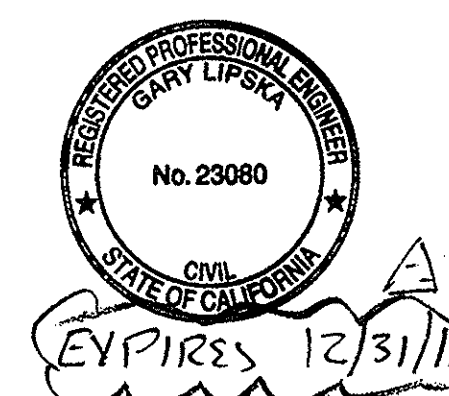
- CONSTRUCTION LEGEND**
- ① TEMPORARY GRADED SLOPES PRIOR TO CONSTRUCTION OF BUILDING WALL
 - ② CONCRETE DRIVEWAY PER S.D.R.S.D. G-14A
 - ③ MASONRY RETAINING WALL PER S.D.R.S.D. C-3
 - ④ 6' HIGH MASONRY SLUMP BLOCK SCREEN WALL (PER SEPARATE PERMIT)
 - ⑤ MASONRY RETAINING WALL PER ARCHITECT'S PLAN (PER SEPARATE PERMIT)
 - ⑥ TRASH ENCLOSURE WITH DECORATIVE MASONRY SCREEN WALL (PER SEPARATE PERMIT)
 - ⑦ **DISCHARGE BASIN PER DETAIL 'H' ON THIS SHEET**
 - ⑧ DETENTION BASIN STAND PIPE OUTLET PER DETAIL 'C' ON SHEET 2
 - ⑨ TYPE 'F' CATCH BASIN PER S.D.R.S.D. D-7
 - ⑩ CONCRETE BROW DITCH PER S.D.R.S.D. D-75, TYPE B
 - ⑪ CONCRETE HEADWALL PER S.D.R.S.D. D-34
 - ⑫ 6' CONCRETE CURB PER S.D.R.S.D. G- (6')
 - ⑬ NOT USED
 - ⑭ NO. 3636 BROOKS PRODUCTS WITH TRAFFIC GRATE PER DETAIL 'B' ON SHEET 2
 - ⑮ **INSTALL CATCH BASIN INSERTS WITH FILTER MEDIA PER DETAIL 'E' ON SHEET 16**
 - ⑯ 1/2" MESH FILTER - FLO GARD CATCH BASIN INSERT AS MANUFACTURED BY KRISTAR ENTERPRISES, INC., SANTA ROSA, CA - 800-579-8819
 - ⑰ CONCRETE PAVEMENT
 - ⑱ STORM CHAMBER, AS MANUFACTURED BY HYDROLOGIC SOLUTIONS (877-426-9128)

ENGINEER'S NOTES

- 1) ALL PVC DRAINAGE PIPES SHALL CONFORM TO SDR-35 PER ASTM D3034 OR "SURE-LOK F477" AS MANUFACTURED BY HANCOX INC (ASTM M252, TYPE S)
- 2) LENGTHS ON DRAINAGE PIPE ARE FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE
- 3) CONTRACTOR TO VERIFY HORIZONTAL AND VERTICAL LOCATION OF EXISTING UNDERGROUND SEWER AND STORM DRAINAGE SYSTEMS PRIOR TO TRENCHING FOR NEW CONNECTING FACILITIES. IN THE EVENT ACTUAL LOCATION VARIES SIGNIFICANTLY FROM THAT SHOWN ON PLAN, THE ENGINEER OF WORK SHALL BE NOTIFIED PRIOR TO PROCEEDING

SYMBOL LEGEND

DESCRIPTION	SYMBOL
ELEVATION, TOP OF FOOTING	TF
ELEVATION, TOP OF WALL	TW
ELEVATION, TOP OF CURB	TC
ELEVATION, FINISH SURFACE	FS
ELEVATION, FINISH GRADE	FG
ELEVATION, TOP OF GRATE	TG
ELEVATION, BOTTOM OF FOOTING	BF
ELEVATION, FLOW LINE	FL
ELEVATION, INVERT ELEVATION	IE
ELEVATION LABEL, EXISTING	(596.00)
ELEVATION LABEL, PROPOSED	596.00
ELEVATION FINISH FLOOR	FF
DETAIL REFERENCE	CX
DETAIL LABEL	2X
SHEET NO.	



GRADING AND DRAINAGE PLAN
SCALE: 1"=20'

CIVIL ENGINEERING • LAND PLANNING
Aquatera Engineering Inc.
1843 Campesino Place
Oceanside, CA 92054
Tel: (760) 439-2802
Fax: (760) 439-2888

RECORD PLAN

BY: *[Signature]* DATE: 10/9/12
R.C.E. 23080
EXPIRES: 12/31/13

COUNTY APPROVED CHANGES

NO.	DESCRIPTION	APPROVED BY	DATE
1	REVISE LEGEND + SHEET COUNT	<i>[Signature]</i>	3/14/2012
2	ADD DRAINAGE NOTES	<i>[Signature]</i>	10/30/13
3	RECORD PLAN	<i>[Signature]</i>	

PERMITS

REZONE PERMIT NO. N/A
SITE PLAN REVIEW NO. S-03-026
STREET IMPROVEMENT PLANS CG 4646
NOTICE OF INTENT(WDID): 9 37C339975

BENCH MARK

DESCRIPTION: MONUMENT NO. 1031 PER R.O.S. 14689
2" IRON PIPE W/BRASS DISC MARKED "GPS KE31"
LOCATION: COLE GRADE ROAD, 68' W/Y OF VALLEY CENTER ROAD INTERSECTION
RECORD FROM: NGVD 29
ELEVATION: 1349.30 DATUM: M.S.L.

PRIVATE CONTRACT

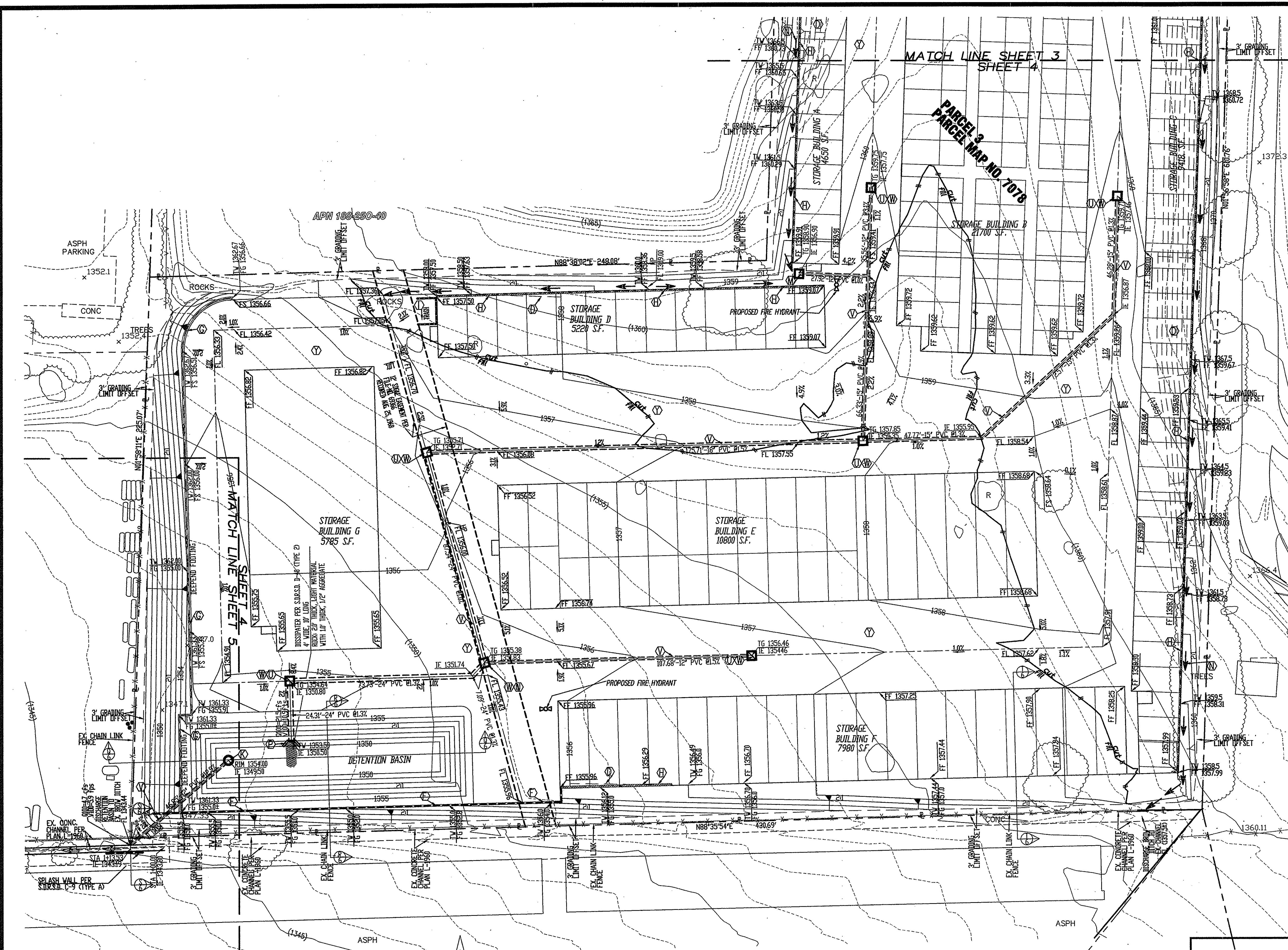
SHEET 3
COUNTY OF SAN DIEGO
DEPARTMENT OF PUBLIC WORKS

GRADING PLAN FOR:
LIZARD ROCKS STORAGE
PARCEL 3, PARCEL MAP NO. 7078
CALIFORNIA COORDINATE INDEX 386-1761

APPROVED FOR:
MUNICIPAL ENGINEER
COUNTY ENGINEER

ENGINEER OF WORK
[Signature]
DATE 11/4/12
RECORD NO. L-14940

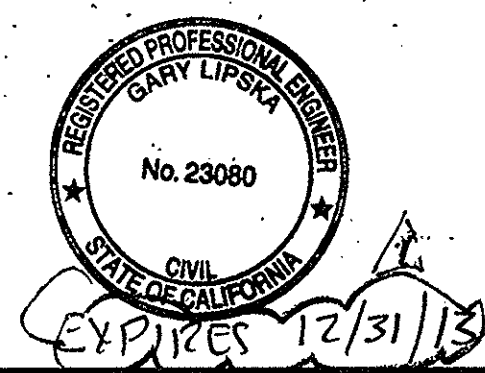
ENGINEER: AQUATERA ENGINEERING INC.
TELEPHONE: 760-439-2802



- CONSTRUCTION LEGEND**
- ① TEMPORARY GRADED SLOPES PRIOR TO CONSTRUCTION OF BUILDING WALL
 - ② CONCRETE DRIVEWAY PER S.D.R.S.D. G-14A
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 - ④ 6' HIGH MASONRY SLUMP BLOCK SCREEN WALL (PER SEPARATE PERMIT)
 - ⑤ MASONRY RETAINING WALL PER ARCHITECTS PLAN (PER SEPARATE PERMIT)
 - ⑥ TRASH ENCLOSURE WITH DECORATIVE MASONRY SCREEN WALL (PER SEPARATE PERMIT)
 - ⑦ **DISCHARGE BASIN PER DETAIL 'H' ON SHEET 3**
 - ⑧ DETENTION BASIN STAND PIPE OUTLET PER DETAIL 'C' ON SHEET 4
 - ⑨ TYPE 'F' CATCH BASIN PER S.D.R.S.D. D-7
 - ⑩ CONCRETE BROW DITCH PER S.D.R.S.D. D-75, TYPE B
 - ⑪ CONCRETE HEADWALL PER S.D.R.S.D. D-34
 - ⑫ 6" CONCRETE CURB PER S.D.R.S.D. G- (6')
 - ⑬ NOT USED
 - ⑭ NO. 3636 BROOKS PRODUCTS WITH TRAFFIC GRATE PER DETAIL 'B' ON SHEET 2
 - ⑮ PVC DRAINAGE PIPE, SIZE AS SHOWN ON PLAN
 - ⑯ **INSTALL CATCH BASIN INSERTS WITH FILTER MEDIA PER DETAIL 'Z' ON SHEET 16**
 - ⑰ **INSTALL 18" DIAMETER FILTER-FLAT CATCH BASIN INSERT AS MANUFACTURED BY KRISTAR ENTERPRISES, INC., SANTA ROSA, CA - 800-579-8819**
 - ⑱ CONCRETE PAVEMENT
 - ⑲ STORM CHAMBER, AS MANUFACTURED BY HYDROLOGIC SOLUTIONS (877-426-9128)

SYMBOL LEGEND

DESCRIPTION	SYMBOL
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ELEVATION, TOP OF GRATE	TG
ELEVATION, BOTTOM OF FOOTING	BF
ELEVATION, FLOW LINE	FL
ELEVATION, INVERT ELEVATION	IE
ELEVATION LABEL, EXISTING	(596.00)
ELEVATION LABEL, PROPOSED	(596.00)
ELEVATION FINISH FLOOR	FF
DETAIL REFERENCE	CA
DETAIL LABEL	2
SHEET NO.	16



20 0 20 40
Scale: 1" = 20'

Aquatera Engineering Inc.
CIVIL ENGINEERING • LAND PLANNING
1843 Camperino Place
Oceanside, CA 92054
Tel: (760) 439-2802
Fax: (760) 439-2886

GRADING AND DRAINAGE PLAN
SCALE: 1"=20'

RECORD PLAN
BY: *[Signature]* DATE: 10/9/12
R.C.E. **23080**
EXPIRES: **12/31/13**

COUNTY APPROVED CHANGES

NO.	DESCRIPTION	APPROVED BY:	DATE:
1	REVISE LEGEND + SHEET COUNT	<i>[Signature]</i>	3/14/2012
2	ADD DRAINAGE NOTES	<i>[Signature]</i>	10/30/13
3	RECORD PLAN	<i>[Signature]</i>	

PERMITS

REZONE PERMIT NO. N/A
SITE PLAN REVIEW NO. S-03-026
STREET IMPROVEMENT PLANS CG 4646
NOTICE OF INTENT(WID): 9_37C339975

BENCH MARK

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LOCATION: COLE GRADE ROAD, 68' WLY OF VALLEY CENTER ROAD INTERSECTION
RECORD FROM: NGVD 29
ELEVATION: 1349.30 DATUM: M.S.L.

PRIVATE CONTRACT

SHEET 4
COUNTY OF SAN DIEGO
DEPARTMENT OF PUBLIC WORKS

GRADING PLAN FOR:
LIZARD ROCKS STORAGE
PARCEL 3, PARCEL MAP NO. 7078
CALIFORNIA COORDINATE INDEX 386-1781

APPROVED FOR:
MAYOR AND ENGINEER
COUNTY ENGINEER
[Signature]
DATE: 11/9/12

ENGINEER: AQUATERRA ENGINEERING INC.
TELEPHONE: 760-439-2802



GENERAL NOTES

1. A PERMIT SHALL BE OBTAINED FROM THE COUNTY DEPARTMENT OF PUBLIC WORKS FOR ANY WORK WITHIN THE STREET RIGHT-OF-WAY.
2. THE STRUCTURAL SECTION SHALL BE IN ACCORDANCE WITH SAN DIEGO COUNTY STANDARDS AND AS APPROVED BY THE MATERIALS LABORATORY.
3. APPROVAL OF THESE IMPROVEMENT PLANS AS SHOWN DOES NOT CONSTITUTE APPROVAL OF ANY CONSTRUCTION OUTSIDE THE PROJECT BOUNDARY.
4. ALL UNDERGROUND UTILITIES WITHIN THE STREET RIGHT-OF-WAY SHALL BE CONSTRUCTED, CONNECTED AND TESTED PRIOR TO CONSTRUCTION OF BERM, CURB, CROSS GUTTER AND PAVING.
5. THE EXISTANCE AND LOCATION OF EXISTING UNDERGROUND FACILITIES SHOWN ON THESE PLANS WERE OBTAINED BY A SEARCH OF THE AVAILABLE RECORDS TO THE BEST OF OUR KNOWLEDGE. THERE ARE NO OTHER EXISTING FACILITIES EXCEPT AS SHOWN ON THESE PLANS. HOWEVER THE CONTRACTOR IS REQUIRED TO TAKE PRECAUTIONARY MEASURES TO PROTECT ANY EXISTING FACILITY SHOWN HEREON AND ANY OTHER WHICH IS NOT OF RECORD OR NOT SHOWN ON THESE PLANS.
6. LOCATION AND ELEVATION OF IMPROVEMENTS TO BE MET BY WORK TO BE DONE SHALL BE CONFIRMED BY FIELD MEASUREMENTS PRIOR TO CONSTRUCTION OF NEW WORK. CONTRACTOR WILL MAKE EXPLORATORY EXCAVATIONS AND LOCATE EXISTING UNDERGROUND FACILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS TO PLANS IF REVISIONS ARE NECESSARY BECAUSE OF ACTUAL LOCATION OF EXISTING FACILITIES.
7. THE CONTRACTOR SHALL NOTIFY THE SAN DIEGO GAS AND ELECTRIC COMPANY PRIOR TO STARTING WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK WITH COMPANY REPRESENTATIVES. NOTICE: ALL ELECTRICAL AND GAS SERVICES WITHIN THIS PROJECT ARE "UNDERGROUND INSTALLATIONS." FOR LOCATIONS OF ELECTRICAL CABLES AND GAS PIPING AND APPURTENANCES CONTACT THE SAN DIEGO GAS AND ELECTRIC COMPANY, TELEPHONE: 619-293-0595.
8. THE CONTRACTOR SHALL NOTIFY AMERICAN TELEPHONE AND TELEGRAPH PRIOR TO STARTING WORK NEAR COMPANY FACILITIES AND SHALL COORDINATE HIS WORK WITH COMPANY REPRESENTATIVES. NOTICE: ALL TELEPHONE SERVICES WITHIN THIS PROJECT BOUNDARY ARE "UNDERGROUND INSTALLATIONS." FOR LOCATION OF CABLES AND APPURTENANCES CONTACT AMERICAN TELEPHONE AND TELEGRAPH, TELEPHONE: 1-800-422-4133.
9. IT SHALL BE THE RESPONSIBILITY OF THE DEVELOPER TO CONTACT THE UTILITY AGENCIES, ADVISE THEM OF THE PROPOSED IMPROVEMENTS AND BEAR THE COST OF RELOCATIONS, IF NEEDED.
10. ALL TELEVISION SERVICES WITHIN THIS PROJECT ARE "UNDERGROUND INSTALLATIONS." FOR LOCATION OF CABLES AND APPURTENANCES CONTACT ADAPTA CABLE COMMUNICATIONS, TELEPHONE: 760-728-5002.
11. POWER SOURCES AND RUNS SERVING STREET LIGHTS SHALL BE SHOWN ON THE "AS-BUILT" IMPROVEMENT DRAWINGS. ALL SOURCES SHALL BE LOCATED WITHIN THE DEDICATED RIGHT-OF-WAY, OR WITHIN EASEMENTS DEDICATED TO THE COUNTY OF SAN DIEGO.
12. NO PAVING SHALL BE DONE UNTIL EXISTING POWER POLES ARE RELOCATED OUTSIDE THE AREAS TO BE PAVED.
13. PRIVATE ROAD IMPROVEMENTS SHOWN HEREON ARE FOR INFORMATION ONLY. COUNTY OFFICIALS SIGNATURE HEREON DOES NOT CONSTITUTE APPROVAL OR RESPONSIBILITY OF ANY KIND FOR THE DESIGN OR CONSTRUCTION OF THESE PRIVATE IMPROVEMENTS.
14. ALL CUT AND FILL SLOPES OVER THREE (3) FEET CREATED BY GRADING FOR STREETS AND DRIVEWAYS SHALL BE HYDROSEED WITH SAN DIEGO COUNTY APPROVED HYDROSEED MIXTURE. HYDROSEED SLOPES SHALL BE IRRIGATED BY WATER TRUCK UNTIL THE MIXTURE GERMINATES AND GROWTH IS ESTABLISHED.
15. ALL INTERSECTING ROADS AND DRIVEWAYS SHALL BE GRADED AND/OR RECONSTRUCTED, AS REQUIRED TO MATCH NEW IMPROVEMENTS AND PROVIDE SMOOTH TRANSITION.
16. ALL TREES WITHIN THE RIGHT-OF-WAY SHALL BE REMOVED.

GENERAL NOTES (CONTINUED)

17. ALL SIGNS TO BE ALUMINUM WITH 3M HIGH INTENSITY TYPE REFLECTIVE FACE OR EQUIVALENT.
18. CONTRACTOR WILL BE RESPONSIBLE FOR THE REPLACEMENT OF ANY STRIPING, PAVEMENT MARKERS OR LEGENDS OBLITERATED BY THE CONSTRUCTION OF THIS PROJECT.
19. ALL NEW STRIPING AND SANDBLASTING OF REDUNDANT STRIPING SHALL BE DONE BY THE CONTRACTOR.
20. ALL CUT AND FILL SLOPES THREE FEET (3') CREATED BY GRADING FOR STREETS AND DRIVEWAYS SHALL BE HYDROSEED WITH A SAN DIEGO COUNTY APPROVED HYDROSEED MIXTURE. HYDROSEED SLOPES SHALL BE IRRIGATED BY A WATER TRUCK UNTIL THE MIXTURE GERMINATES AND GROWTH IS ESTABLISHED.
21. ASPHALT CONCRETE SURFACING MATERIAL SHALL BE HAND-RAKED AND COMPACTED TO FORM SMOOTH TAPERED CONNECTIONS ALONG ALL EDGES INCLUDING THOSE EDGES ADJACENT TO SOIL. THE EDGES OF ASPHALT CONCRETE SHALL BE HAND-RAKED AT 45 DEGREES OR FLATTER, SO AS TO PROVIDE A SMOOTH TRANSITION NEXT TO THE EXISTING SOIL, INCLUDING THOSE AREAS SCHEDULED FOR SHOULDER BACKING. THE ABOVE SHALL BE DONE TO THE SATISFACTION OF THE DIRECTOR OF PUBLIC WORKS.
22. CONTRACTOR SHALL APPLY FOR A TRAFFIC CONTROL PERMIT TWO WEEKS PRIOR TO COMMENCEMENT OF WORK. NO WORK SHALL COMMENCE UNTIL ISSUANCE OF SAID PERMIT. FOR INFORMATION CALL (858) 874-4025. THE TRAFFIC CONTROL PERMIT SHALL REQUIRE SPECIFICATIONS FOR THE INFORMATIONAL SIGNS.

BMP STENCIL PLACEMENT NOTES

- A) ALL STORM DRAIN INLETS AND CATCH BASINS WITHIN THE PROJECT SHALL HAVE A STENCIL OR TILE PLACED WITH PROHIBITIVE LANGUAGE, SUCH AS: "NO DUMPING- I LIVE IN THE PACIFIC OCEAN" AND/OR GRAPHICAL ICONS TO DISCOURAGE ILLEGAL DUMPING.
- B) SIGNS AND PROHIBITIVE LANGUAGE AND/OR GRAPHICAL ICONS, WHICH PROHIBIT ILLEGAL DUMPING, MUST BE POSTED AT PUBLIC ACCESS POINTS ALONG CHANNELS AND CREEKS WITHIN THE PROJECT AREA.
- C) LEGIBILITY OF STENCILS, TILES AND SIGNS MUST BE MAINTAINED AND THE TILES MUST BE PLACED FLUSH WITH THE TOP OF CONCRETE TO REDUCE TRIPPING BY PEDESTRIANS.

NOTICE: THE ISSUANCE OF THIS PERMIT/ APPROVAL BY THE COUNTY OF SAN DIEGO DOES NOT AUTHORIZE THE APPLICANT FOR SAID PERMIT / APPROVAL TO VIOLATE ANY FEDERAL, STATE OR COUNTY LAWS, ORDINANCES, REGULATIONS OR POLICIES INCLUDING BUT NOT LIMITED TO THE FEDERAL ENDANGERED SPECIES ACT AND ANY AMENDMENTS THERETO.

EXISTING UTILITY FACILITY NOTE

THE CONTRACTOR SHALL VERIFY THE EXISTENCE AND LOCATION OF ALL UTILITIES BEFORE COMMENCING WORK. NOTICE OF PROPOSED WORK SHALL BE GIVEN TO THE FOLLOWING AGENCIES:

SAN DIEGO GAS & ELECTRIC: TELEPHONE NO. 619-232-4252, EXT. 1658
A.T. & T. TELEPHONE NO. 619-296-0595
CITY OF ADELPHIA CABLE COMMUNICATIONS, TELEPHONE: 760-728-5002
WATER: VALLEY CENTER MUNICIPAL WATER DISTRICT
DIG ALERT: TELEPHONE NO: 760-735-4500
TELEPHONE NO: 1-800-422-4133

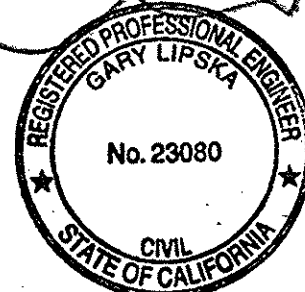
DIG ALERT NOTICE

2 DAYS PRIOR TO EXCAVATION, THE CONTRACTOR SHALL CONTACT UNDERGROUND SERVICE ALERT AT 1-800-422-4133 TO MARK OUT EXISTING UNDERGROUND FACILITIES WITHIN THE WORK AREA.

STORMWATER TREATMENT BMP TABLE

DESCRIPTION/TYPE	SHEET NO.	MAINTENANCE CATEGORY	REVISIONS
BIORETENTION SWALE	2A+3A	FOUR	

BMP'S APPROVED AS PART OF THE STORMWATER MANAGEMENT PLAN DATED 7/26/12 ON FILE WITH DPW. ANY CHANGES TO THE ABOVE BMP'S WILL REQUIRE SWMP REVISION AND PLAN CHANGE APPROVAL



DECLARATION OF RESPONSIBLE CHARGE

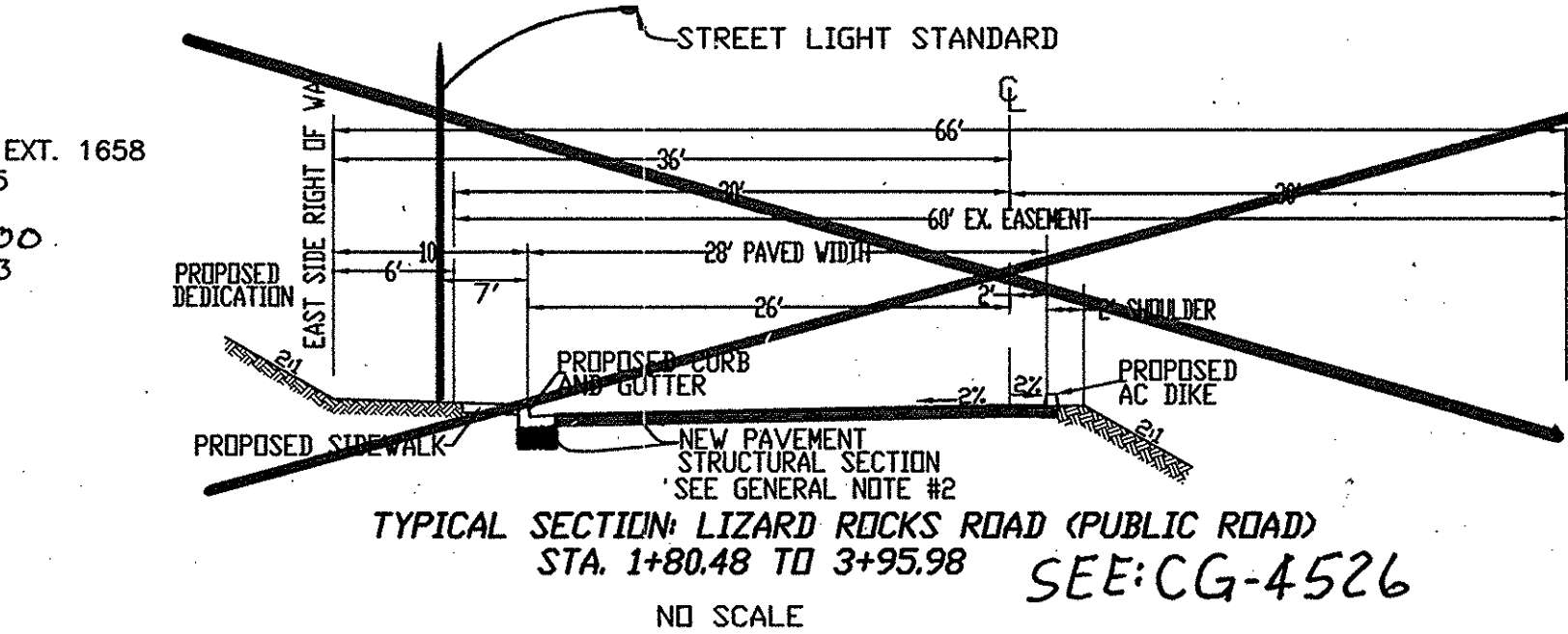
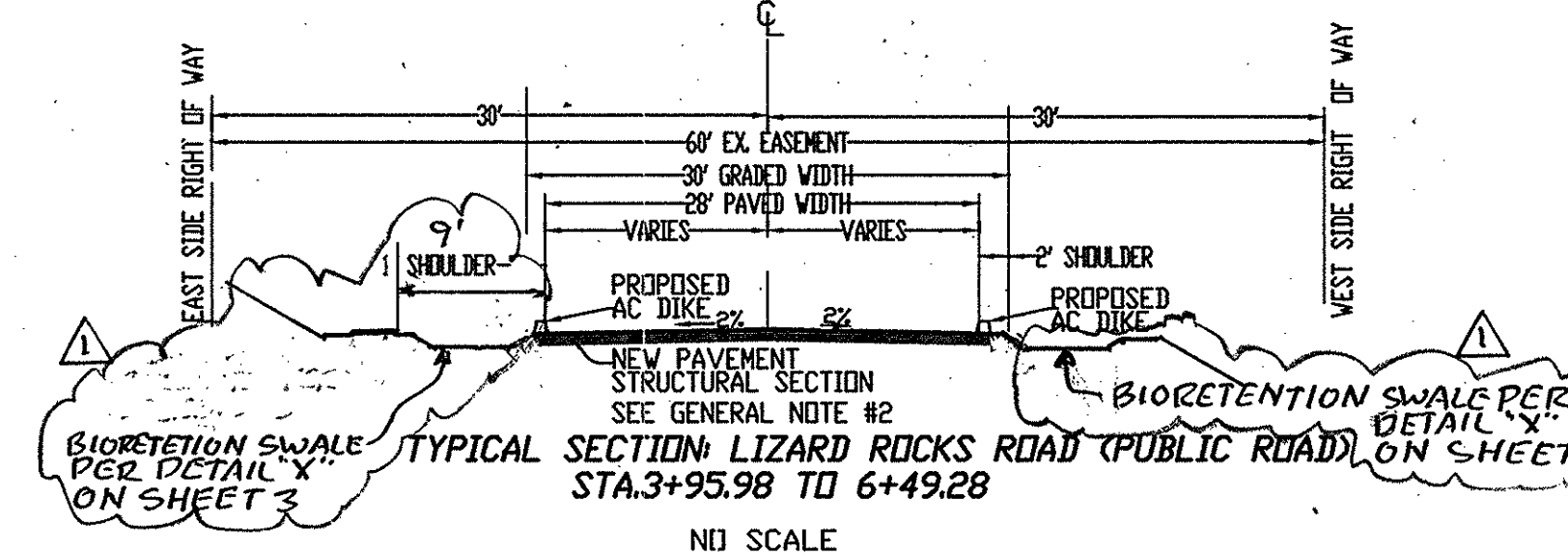
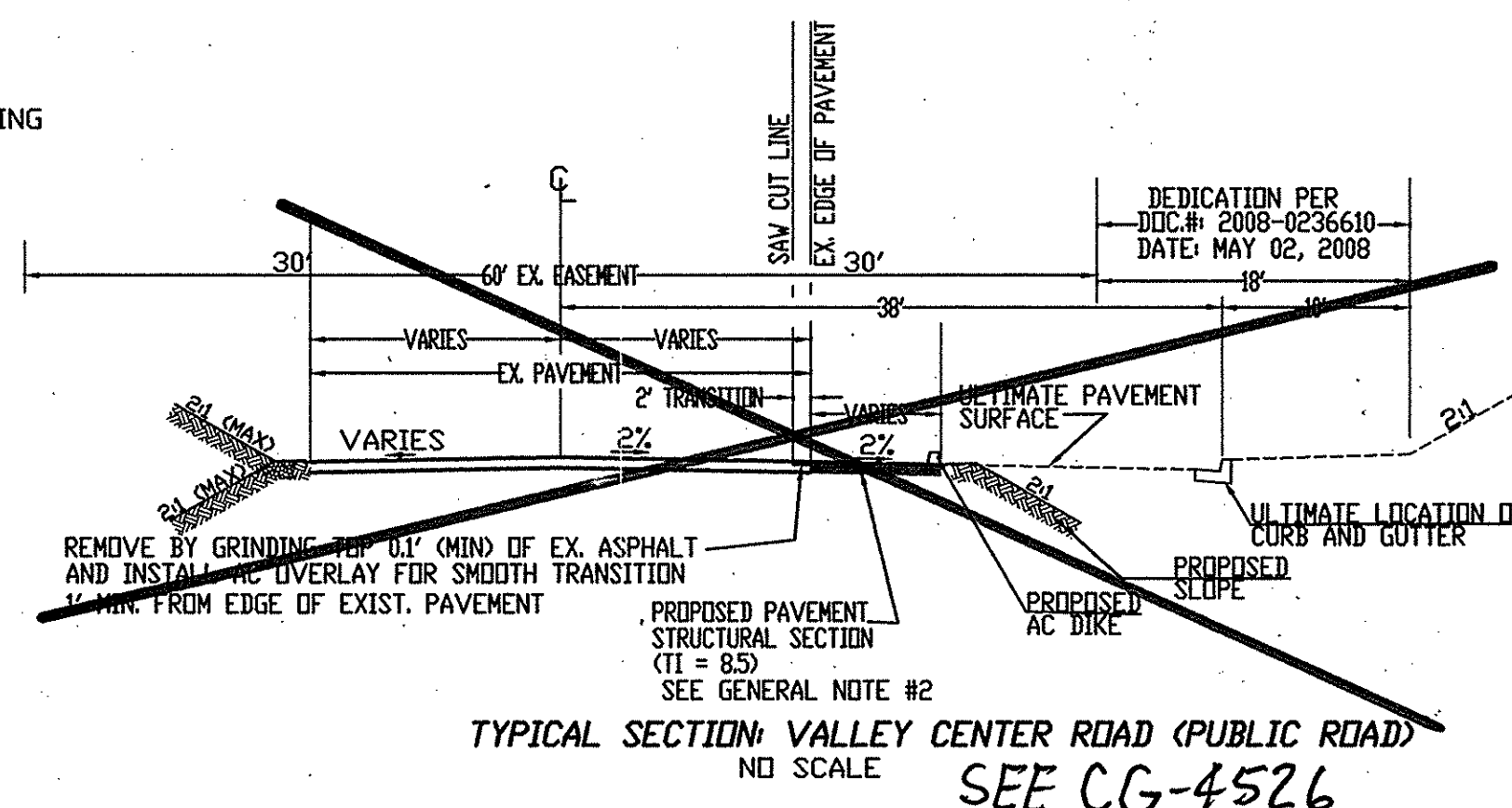
I HEREBY DECLARE THAT I AM THE ENGINEER OF WORK FOR THIS PROJECT, THAT I HAVE EXERCISED RESPONSIBLE CHARGE OVER THE DESIGN OF THE PROJECT AS DEFINED IN SECTION 6703 OF THE BUSINESS AND PROFESSIONS CODE, AND THAT THE DESIGN IS CONSISTENT WITH CURRENT STANDARDS.

I UNDERSTAND THAT THE CHECK OF PROJECT DRAWINGS AND SPECIFICATIONS BY THE COUNTY OF SAN DIEGO IS CONFINED TO REVIEW ONLY AND DOES NOT RELIEVE ME, AS ENGINEER OF WORK, OF MY RESPONSIBILITIES FOR PROJECT DESIGN.

BY: GARY LIPSKA DATE: 5/14/13
RCE NO: 23080 EXPIRES: 12/31/13

RECORD PLAN

NAME: GARY LIPSKA
R.C.E. 23080 EXPIRES 12/31/13
DATE: 10/14/13



WORK TO BE DONE

IMPROVEMENTS CONSIST OF THE FOLLOWING WORK TO BE DONE ACCORDING TO THESE PLANS, THE CURRENT SAN DIEGO COUNTY DEPARTMENT OF PUBLIC WORKS STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS FOR IMPROVEMENT OF SUB-DIVISION STREETS AND STANDARD REFERENCE DRAWINGS.

LEGEND

DESCRIPTION	DWG. NO.	SYMBOL
AC PAVEMENT - AS APPROVED BY MATERIALS LABORATORY		
CONCRETE CURB	S.D.R.S.D. NO. G-1 (G)	
CONCRETE CURB AND GUTTER	S.D.R.S.D. G-2 (TYPE G)	
CONCRETE SIDEWALK	S.D.R.S.D. G-7, G-9, G-10	
PEDESTRIAN RAMP	S.D.R.S.D. G-27 (TYPE A), G-32	
CONCRETE DRIVEWAY	S.D.R.S.D. G-14B	
6" DIKES- ASPHALT CONCRETE	S.D.R.S.D. G-5 (TYPE A)	
STORM DRAIN PIPE (SIZE AS SHOWN)		
SIDEWALK UNDERDRAIN	S.D.R.S.D. D-25	
STREET LIGHT (250 WATTS, H.P.S.)	S.D.R.S.D. E-1, E-2	
CONCRETE BROW DITCH PER S.D.R.S.D. D-75 (TYPE "D")	S.D.R.S.D. D-75 (TYPE "D")	
GRASS LINED SWALE	DETAIL "W" ON SHEET 3	
WATER SERVICE CONNECTION	V.C.M.W.D. DWG. W-2	
FIRE HYDRANT	V.C.M.W.D. DWG. W-8	
STREET SURVEY MONUMENT	S.D.R.S.D. M-10	
STREET SIGNS		
DOUBLE STREET NAME SIGNS		

CUT SLOPE (2:1 MAX)

FILL SLOPE (2:1 MAX)

CUT/FILL DAYLIGHT LINE

AC OVERLAY FOR SMOOTH TRANSITION

REMOVE TOP 0.1' OF EX. ASPHALT AND INSTALL AC OVERLAY FOR SMOOTH TRANSITION

CONCRETE PAVEMENT

BIORETENTION SWALE

CATCH BASIN, TYPE "F"

CATCH BASIN, TYPE "G"

INFILTRATION BASIN

DETAIL "X", SHEET 3

S.D.R.S.D. D-7

S.D.R.S.D. D-8

DETAIL "Y", SHEET 3

INDEX OF SHEETS

- 1 - TITLE SHEET
- 2 - LIZARD ROCKS ROAD PLAN AND PROFILE
- 3 - VALLEY CENTER ROAD PLAN AND DETAILS (SEE CG 4526)
- 4 - EROSION CONTROL PLAN
- 5 - EROSION CONTROL NOTES
- 6 - LIZARD ROCKS ROAD SIGNING AND STRIPING (SEE CG 4526)
- 7 - VALLEY CENTER ROAD SIGNING AND STRIPING (SEE CG 4526)
- 8 - WATER FEATURES IMPROVEMENT PLAN
- 9 - DMA AND TREATMENT CONTROL PLAN
- 10 - WATER PLAN
- 11 - WATER PLAN

OWNER/PERMITTEE

PERMITTEE: CML-CA LIZARD ROCK LLC PHONE: 949-389-1410
ADDRESS: 700 NORTH WEST 107TH AVE, MIAMI, FL 33172
BY: M. J. Jolly DATE: 5/18/12 APN: 188-250-41
MIKE FARLEY

OWNER'S/PERMITTEE'S CERTIFICATE

IT IS AGREED THAT FIELD CONDITIONS MAY REQUIRE CHANGES TO THESE PLANS.
IT IS FURTHER AGREED THAT THE PERMITTEE (DEVELOPER) SHALL HAVE A REGISTERED CIVIL ENGINEER MAKE SUCH CHANGES ALTERATIONS OR ADDITIONS TO THESE PLANS WHICH THE DIRECTOR OF PUBLIC WORKS DETERMINES ARE NECESSARY AND DESIRABLE FOR THE PROPER COMPLETION OF THE IMPROVEMENTS.
I FURTHER AGREE TO COMMENCE WORK ON ANY IMPROVEMENTS SHOWN ON THESE PLANS WITHIN EXISTING COUNTY RIGHT-OF-WAY WITHIN 60 DAYS AFTER ISSUANCE OF THE CONSTRUCTION PERMIT AND TO PURSUE SUCH WORK ACTIVELY ON EVERY NORMAL WORKING DAY UNTIL COMPLETED.

OWNER/PERMITTEE: PERMITTEE: LIZARD ROCK LLC SEE ABOVE PHONE NO: 760-749-8060
ADDRESS: 27606 COBB LANE, VALLEY CENTER, CA 92082
BY: JOE GALLAGHER PRESIDENT OF MSK MANAGEMENT INC., MANAGER DATE: 6-3-09 APN: 188-250-41

PERMITS

AEIS NO. 03-08-029
SITE PLAN REVIEW NO. S-03-026
GRADING PLAN L-14940
NOTICE OF INTENT 9-37C353626
BENCH MARK
DESCRIPTION: MONUMENT NO. 1031 PER R.O.S. 14689
2" IRON PIPE W/BRASS DISC MARKED "GPS KE31"
LOCATION: COLE GRADE ROAD, 68' WLY OF VALLEY CENTER ROAD INTERSECTION
RECORD FROM: NGVD 29
ELEVATION: 1349.30 DATUM: M.S.L.

PRIVATE CONTRACT

SHEET 1 COUNTY OF SAN DIEGO DEPARTMENT OF PUBLIC WORKS
IMPROVEMENT PLAN FOR: LIZARD ROCKS ROAD
PARCEL 3, PARCEL MAP NO. 7078
CALIFORNIA COORDINATE INDEX 386-1761
APPROVED FOR: MOHAMMAD FAKHREDDINE COUNTY ENGINEER
ENGINEER OF WORK: GARY LIPSKA DATE: 5/14/13
REVIEWED BY: GARY LIPSKA DATE: 5/14/13
APPROVED DATE: CG 4646

VCMWD APPROVED CHANGES

NO.	DESCRIPTION	APPROVED BY	DATE
1	UPDATE SHEETS 10 AND 11	<u>W. J. Jolly</u>	4/2/12

VALLEY CENTER FIRE PROTECTION DISTRICT

APPROVED BY: W. J. Jolly DATE: 3.23.09

COUNTY APPROVED CHANGES

NO.	DESCRIPTION	APPROVED BY	DATE
1	NEW OWNER, REPLACE SHEET 2+3, REVISE SHEETS 4-6, ADD BMP TABLES 9, 10+11, REVISE SHEET COUNT, UPDATE LICENSE DATE, DELETE SHEETS 7 & 8, VOID SHEET 11, ADD SHEET 11A	<u>K. J. Jolly</u>	8/2/12
2		<u>K. J. Jolly</u>	11/6/12

VALLEY CENTER MUNICIPAL WATER DISTRICT

VALID FOR 1 YEAR FROM THE DATE OF SIGNATURE

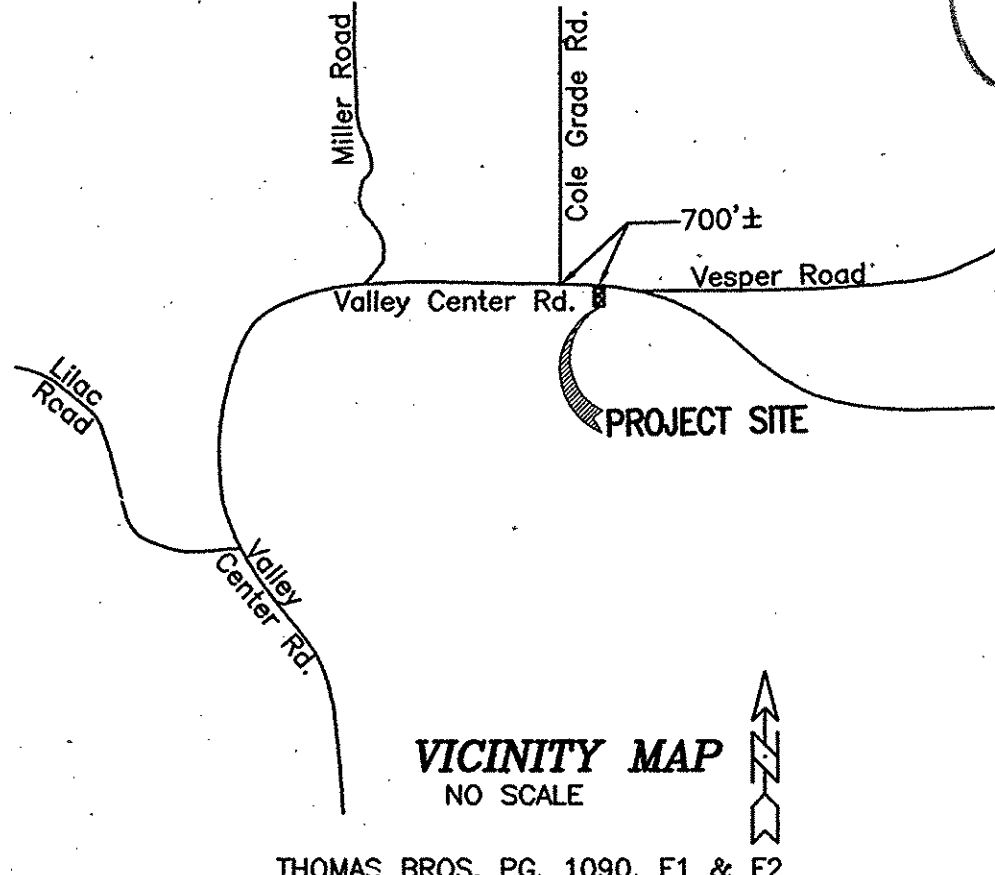
GENERAL MANAGER: A. L. Jolly DATE: 3/13/09
DIRECTOR OF OPERATIONS FACILITIES: W. J. Jolly DATE: 3-12-09
DISTRICT ENGINEER: W. J. Jolly DATE: 3-12-09

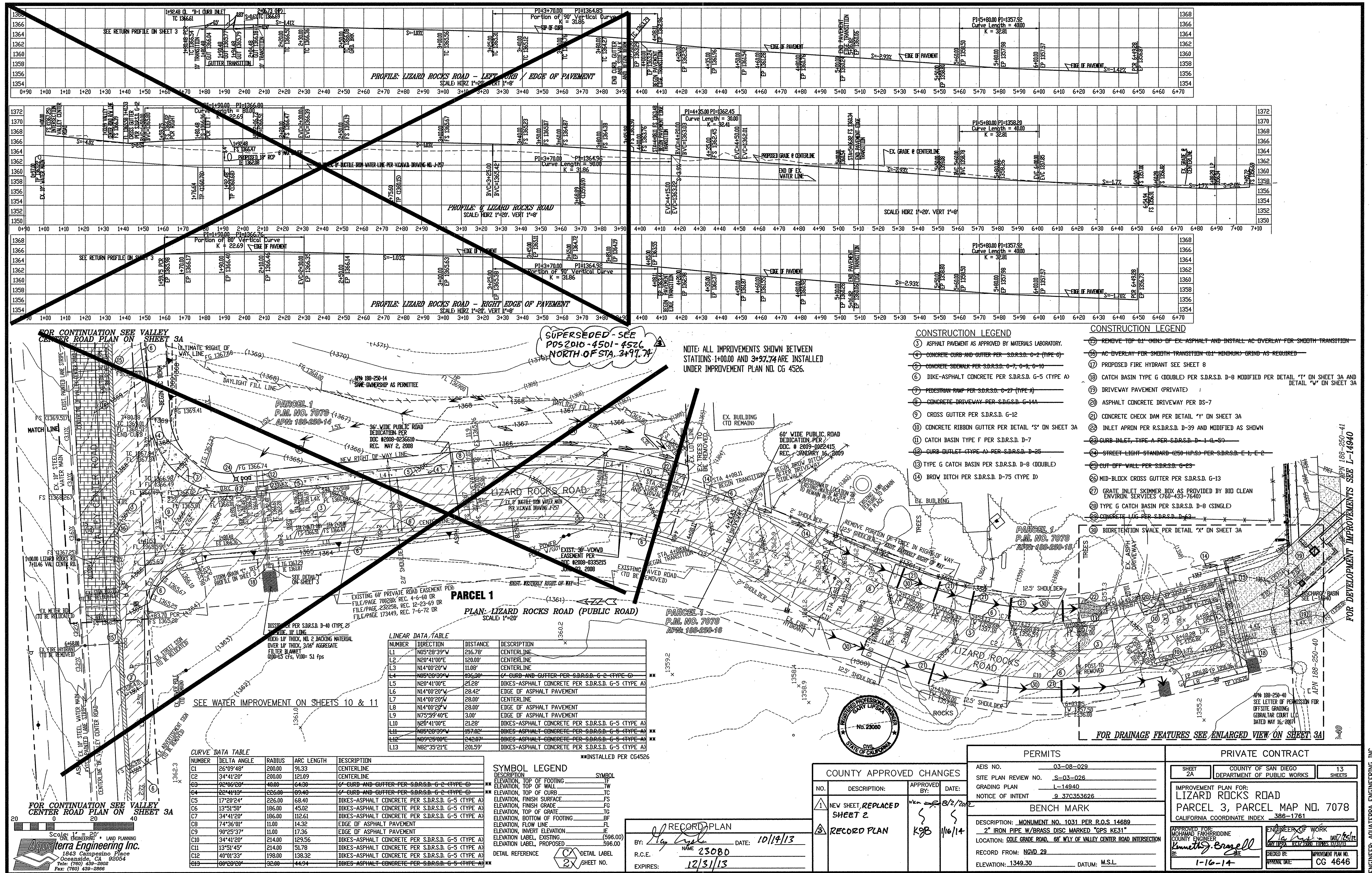
ENGINEER OF WORK

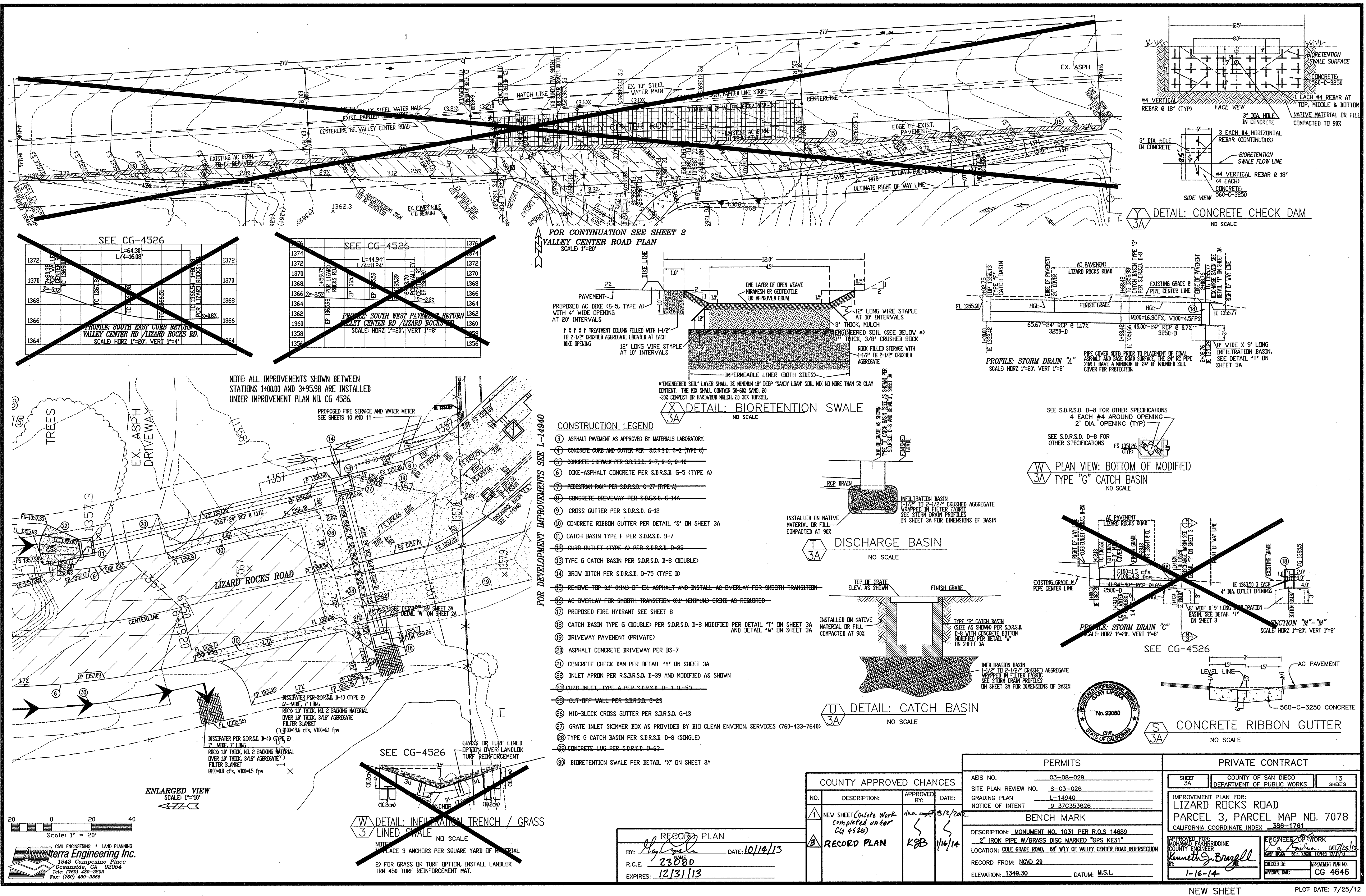
NAME: AQUATERRA ENGINEERING INC.
PHONE NO: 760-439-2802, FAX: 760-430-2866
ADDRESS: 1843 CAMPESTRO PLACE OCEANSIDE, CA 92054

LEGAL DESCRIPTION

PARCEL 3, PARCEL MAP NO. 7078
ASSESSORS PARCEL NO. 188-250-41







CIVIL ENGINEERING • LAND PLANNING
Aquerra Engineering Inc.
1843 Compasino Place
Oceanside, CA 92054
Tel: (760) 439-2802
Fax: (760) 439-2806

RECORD PLAN
BY: *[Signature]* DATE: 10/14/13
R.C.E. 23080
EXPIRES: 12/31/13

COUNTY APPROVED CHANGES			
NO.	DESCRIPTION	APPROVED BY:	DATE:
1	NEW SHEET (Delete Work completed under CG 4526)	<i>[Signature]</i>	8/1/2013
2	RECORD PLAN	KGB	1/16/14

PERMITS	
AEIS NO.	03-08-029
SITE PLAN REVIEW NO.	S-03-026
GRADING PLAN	L-14940
NOTICE OF INTENT	9-37C353626
BENCH MARK	
DESCRIPTION: MONUMENT NO. 1031 PER R.O.S. 14689	
2" IRON PIPE W/BRASS DISC MARKED "GPS KE31"	
LOCATION: COLE GRADE ROAD, 68' WLY OF VALLEY CENTER ROAD INTERSECTION	
RECORD FROM: NGVD 29	
ELEVATION: 1349.30 DATUM: M.S.L.	

PRIVATE CONTRACT		
SHEET 3A	COUNTY OF SAN DIEGO DEPARTMENT OF PUBLIC WORKS	13 SHEETS
IMPROVEMENT PLAN FOR: LIZARD ROCKS ROAD		
PARCEL 3, PARCEL MAP NO. 7078		
CALIFORNIA COORDINATE INDEX 386-1781		
APPROVED FOR: MOHAMMAD FAKHRUDDINE COUNTY ENGINEER		
BY: <i>[Signature]</i> DATE: 7/25/13		
CHECKED BY: <i>[Signature]</i> APPROVAL DATE: 1-16-14		
IMPROVEMENT PLAN NO. CG 4646		

ENGINEER: AQUATERRA ENGINEERING INC. TELEPHONE: 760-439-2802

plot date: 7-25-12