

Appendix A – Site Information

Vicinity Map

Rainfall Isopluvials

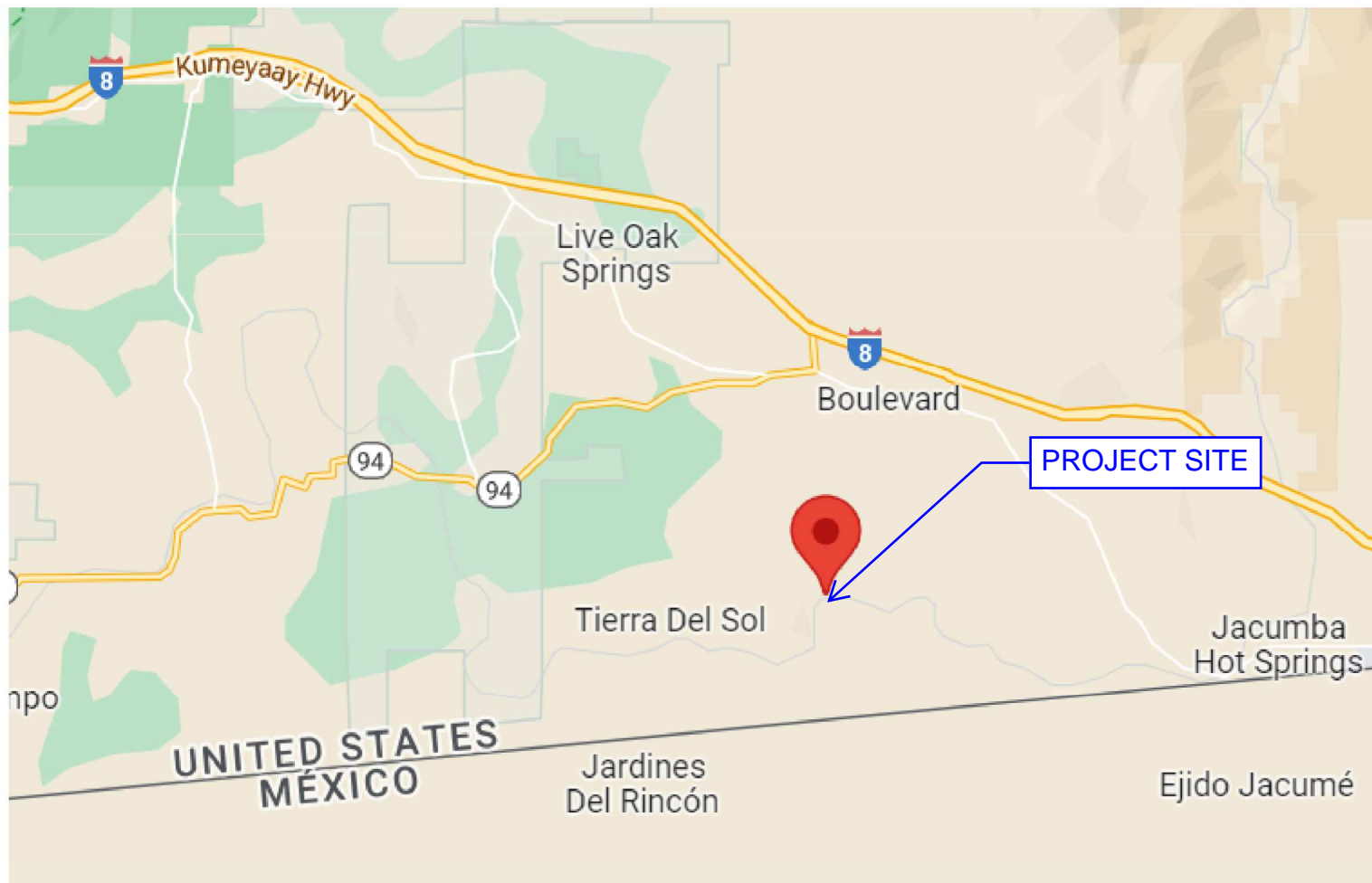
San Diego County Hydrology Manual Excerpts

San Diego County Hydraulic Manual Excerpts

FEMA FIRM

NRCS WebSoil Survey

Weighted Runoff Coefficients (EX and PR)



County of San Diego Hydrology Manual

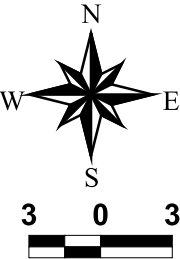
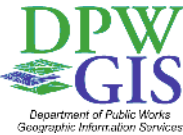


Rainfall Isopluvials

100 Year Rainfall Event - 6 Hours

— Isopluvial (inches)

Project Site:
P6(100YR) = 3.0"



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County of San Diego
Hydrology Manual

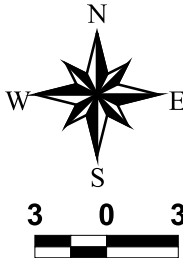
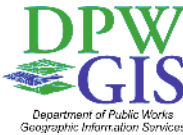


Rainfall Isophuvials

100 Year Rainfall Event - 24 Hours

Isopluvial (inches)

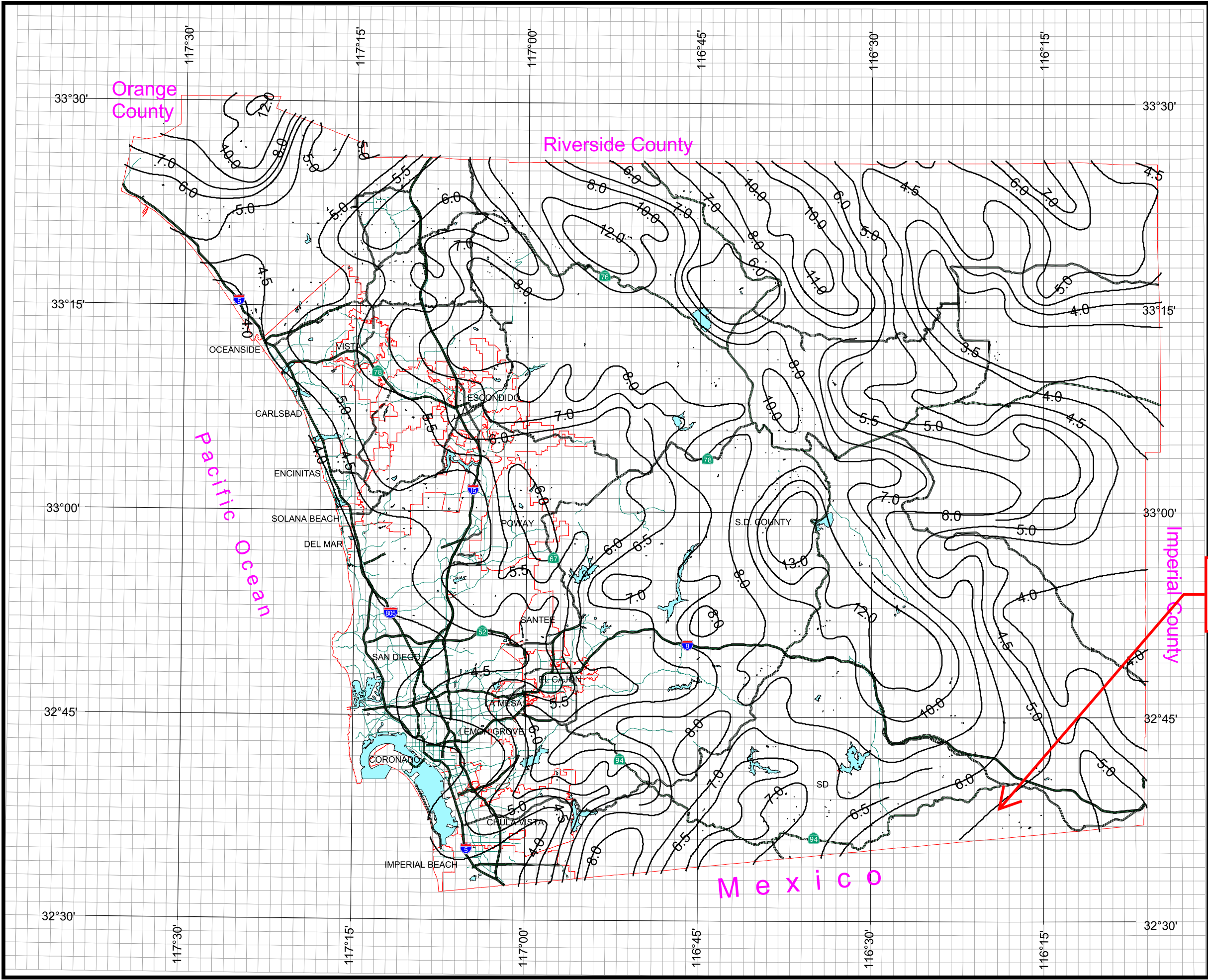
Project Site:
P24(100YR) = 5.0"



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**Table 3-1
RUNOFF COEFFICIENTS FOR URBAN AREAS**

Land Use		Runoff Coefficient "C"				
NRCS Elements	County Elements	% IMPER.	Soil Type			
			A	B	C	D
Undisturbed Natural Terrain (Natural)	Permanent Open Space	0*	0.20	0.25	0.30	0.35
Low Density Residential (LDR)	Residential, 1.0 DU/A or less	10	0.27	0.32	0.36	0.41
Low Density Residential (LDR)	Residential, 2.0 DU/A or less	20	0.34	0.38	0.42	0.46
Low Density Residential (LDR)	Residential, 2.9 DU/A or less	25	0.38	0.41	0.45	0.49
Medium Density Residential (MDR)	Residential, 4.3 DU/A or less	30	0.41	0.45	0.48	0.52
Medium Density Residential (MDR)	Residential, 7.3 DU/A or less	40	0.48	0.51	0.54	0.57
Medium Density Residential (MDR)	Residential, 10.9 DU/A or less	45	0.52	0.54	0.57	0.60
Medium Density Residential (MDR)	Residential, 14.5 DU/A or less	50	0.55	0.58	0.60	0.63
High Density Residential (HDR)	Residential, 24.0 DU/A or less	65	0.66	0.67	0.69	0.71
High Density Residential (HDR)	Residential, 43.0 DU/A or less	80	0.76	0.77	0.78	0.79
Commercial/Industrial (N. Com)	Neighborhood Commercial	80	0.76	0.77	0.78	0.79
Commercial/Industrial (G. Com)	General Commercial	85	0.80	0.80	0.81	0.82
Commercial/Industrial (O.P. Com)	Office Professional/Commercial	90	0.83	0.84	0.84	0.85
Commercial/Industrial (Limited I.)	Limited Industrial	90	0.83	0.84	0.84	0.85
Commercial/Industrial (General I.)	General Industrial	95	0.87	0.87	0.87	0.87

*The values associated with 0% impervious may be used for direct calculation of the runoff coefficient as described in Section 3.1.2 (representing the pervious runoff coefficient, C_p , for the soil type), or for areas that will remain undisturbed in perpetuity. Justification must be given that the area will remain natural forever (e.g., the area is located in Cleveland National Forest).

DU/A = dwelling units per acre

NRCS = National Resources Conservation Service

Table A-1 Average Manning Roughness Coefficients for Pavement and Gutters¹

Concrete Gutter ²	0.015
Concrete Pavement	
Float Finish.....	0.014
Broom Finish	0.016
Concrete Gutter with Asphalt Pavement	
Smooth Finish	0.013
Rough Texture	0.015
Asphalt Pavement	
Smooth Finish	0.013
Rough Texture	0.016

Based on FHWA HEC-22.

¹ Based on materials and workmanship required by standard specifications.

² Increase roughness coefficient in gutters with mild slopes where sediment might accumulate by 0.020.

Table A-2 Average Manning Roughness Coefficients for Closed Conduits³

Reinforced Concrete Pipe (RCP)	0.013
Corrugated Metal Pipe and Pipe Arch	
2-3/8 x 1/2 inch Corrugations	
Unlined	0.024
Half Lined	
Full Flow	0.018
$d/D \geq 0.60$	0.016
$d/D < 0.60$	0.013
Fully Lined	0.013
3 x 1 inch Corrugations	0.027
6 x 2 inch Corrugations	0.032
Spiral Rib Pipe	0.013
Helically Wound Pipe	
18-inch	0.015
24-inch	0.017
30-inch	0.019
36-inch	0.021
42-inch	0.022
48-inch	0.023
Plastic Pipe (HPDE and PVC)	
Smooth	0.013
Corrugated	0.024
Vitrified Clay Pipe	0.014
Cast-Iron Pipe (Uncoated)	0.013
Steel Pipe	0.011
Brick	0.017
Cast-In-Place Concrete Pipe	
Rough Wood Forms	0.017
Smooth Wood or Steel Forms	0.014

³ Based on materials and workmanship required by standard specifications.

Table A-3 Average Manning Roughness Coefficients for Small Open Channels Conveying Less than 50 cfs⁴

Lining Type	Design Flow Depth		
	0 – 0.5 ft	0.5 – 2.0 ft	> 2.0 ft
Concrete (Poured)	0.015	0.013	0.013
Air Blown Concrete	0.023	0.019	0.016
Grouted Riprap	0.040	0.030	0.028
Stone Masonry	0.042	0.032	0.030
Soil Cement	0.025	0.022	0.020
Bare Soil	0.023	0.020	0.020
Rock Cut	0.045	0.035	0.025
Rock Riprap	Based on Rock Size (See Section 5.7.2)		

Table A-4

Table A-4 Average Manning Roughness Coefficients for Larger Open Channels

Unlined Channels

Clay Loam	0.023
Sand	0.020

Lined Channels

Grass Lined (Well-Maintained)	0.035
Grass Lined (Not Maintained)	0.045
Wetland-Bottom Channels (New Channel)	0.023
Wetland-Bottom Channels (Mature Channel)	See Table A-5
Riprap-Lined Channels	See Section 5.7.2
Concrete (Poured)	0.014
Air Blown Mortar (Gunitite or Shotcrete) ⁵	0.016
Asphaltic Concrete or Bituminous Plant Mix	0.018

For channels with revetments or multiple lining types, use composite Manning roughness coefficient based on component lining materials.

⁴ Based on materials and workmanship required by standard specifications.

⁵ For air-blown concrete, use $n=0.012$ (if troweled) and $n=0.025$ if purposely roughened.

Table A-5

Table A-5 Average Manning Roughness Coefficients for Natural Channels

Minor Streams (Surface Width at Flood Stage < 100 ft)

Fairly Regular Section

(A) Some Grass and Weeds, Little or No Brush	0.030
(B) Dense Growth of Weeds, Depth of Flow Materially Greater Than Weed Height.....	0.040
(C) Some Weeds, Light Brush on Banks	0.040
(D) Some Weeds, Heavy Brush on Banks	0.060
(E) For Trees within Channel with Branches Submerged at High Stage, Increase All Above Values By.....	0.015

Irregular Section, with Pools, Slight Channel Meander

Channels (A) to (E) Above, Increase All Values By	0.015
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Mountain Streams; No Vegetation in Channel, Banks Usually Steep, Trees and Brush along Banks Submerged at High Stage

(A) Bottom, Gravel, Cobbles and Few Boulders	0.050
(B) Bottom, Cobbles with Large Boulders	0.060

Flood Plains (Adjacent To Natural Streams)

Pasture, No Brush

(A) Short Grass.....	0.030
(B) High Grass	0.040

Cultivated Areas

(A) No Crop.....	0.040
(B) Mature Row Crops.....	0.040
(C) Mature Field Crops	0.050

Heavy Weeds, Scattered Brush.....0.050

Light Brush and Trees.....0.060

Medium To Dense Brush 0.090 |

Dense Willows.....0.170

Cleared Land with Tree Stumps, 100-150 Per Acre 0.060 |

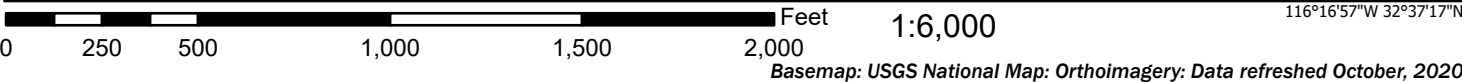
Heavy Stand of Timber, Little Undergrowth

(A) Flood Depth below Branches	0.110
(B) Flood Depth Reaches Branches.....	0.140

National Flood Hazard Layer FIRMMette



116°17'35"W 32°37'48"N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i>
		With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i>
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i>
		Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i>
		Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i>
		Area with Flood Risk due to Levee <i>Zone D</i>
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i>
		Effective LOMRs
		Area of Undetermined Flood Hazard <i>Zone D</i>
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

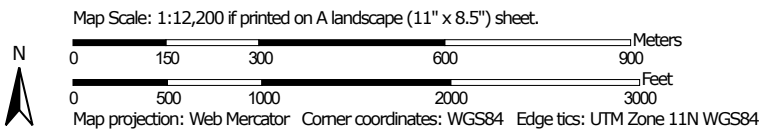
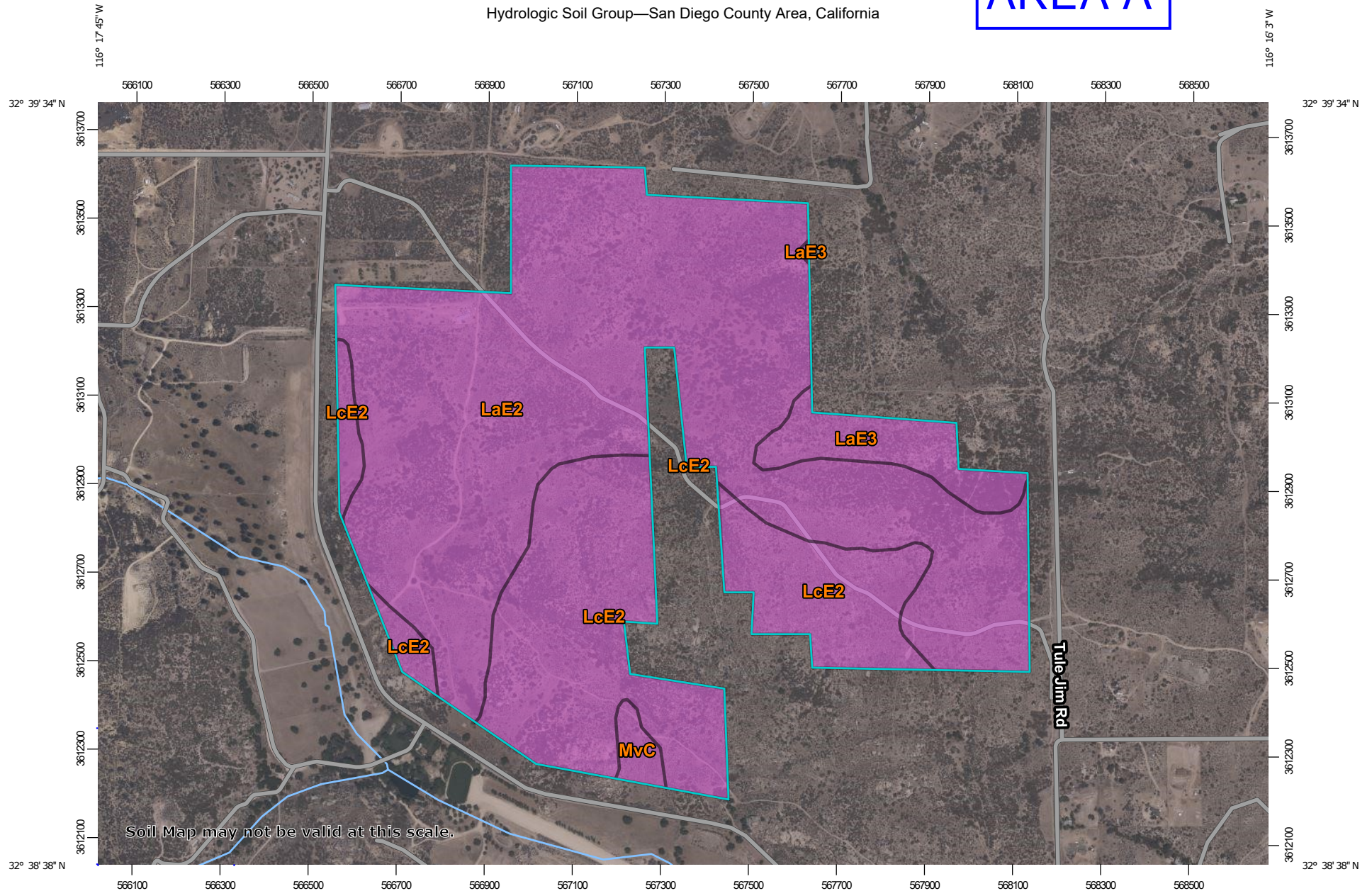
This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **6/1/2022 at 11:14 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

AREA A

Hydrologic Soil Group—San Diego County Area, California



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

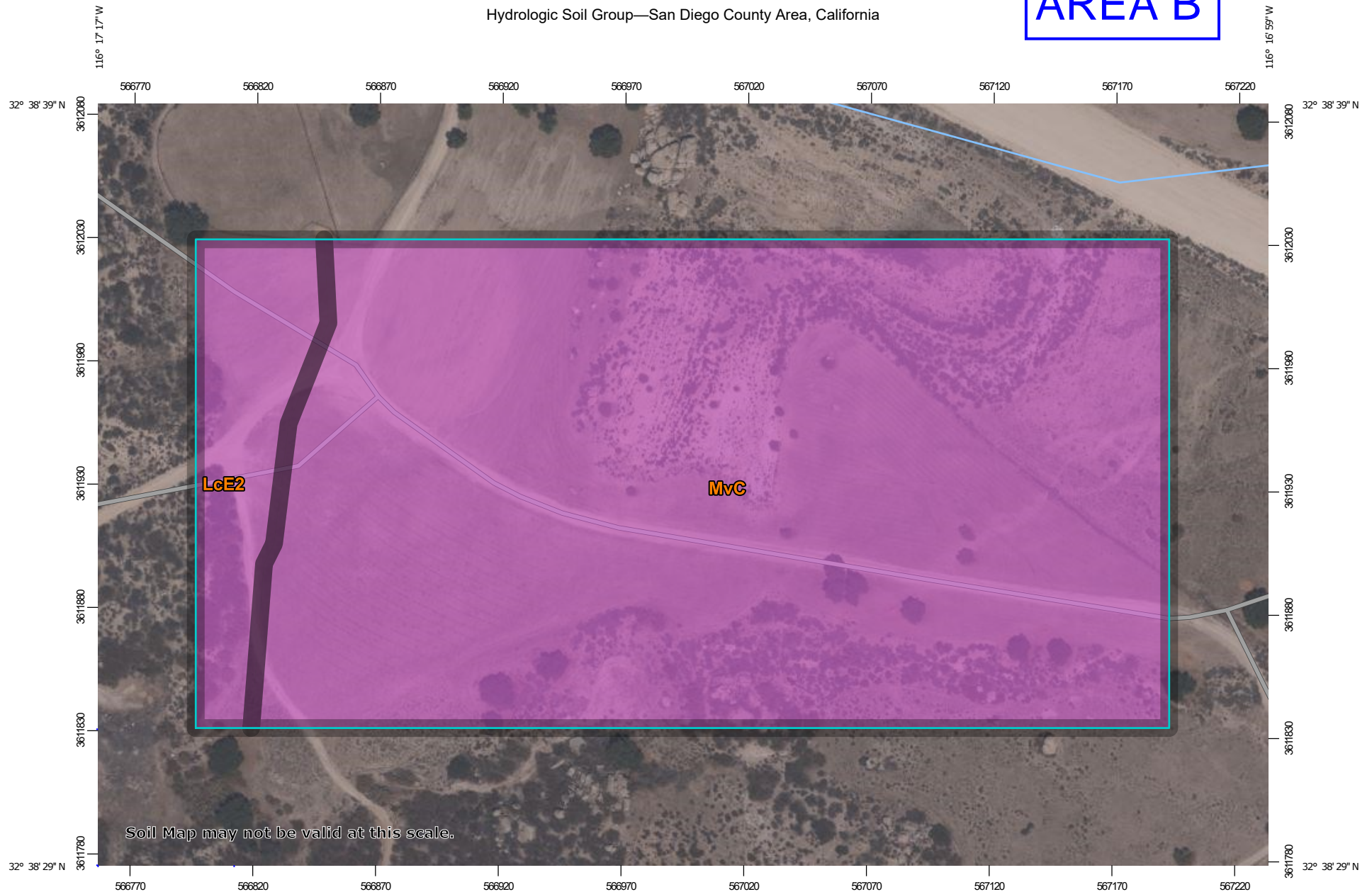
5/31/2022
Page 1 of 4

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LaE2	La Posta loamy coarse sand, 5 to 30 percent slopes, eroded	A	214.6	64.6%
LaE3	La Posta loamy coarse sand, 5 to 30 percent slopes, severely eroded	A	16.2	4.9%
LcE2	La Posta rocky loamy coarse sand, 5 to 30 percent slopes, eroded	A	97.8	29.4%
MvC	Mottsville loamy coarse sand, 2 to 9 percent slopes	A	3.6	1.1%
Totals for Area of Interest			332.2	100.0%

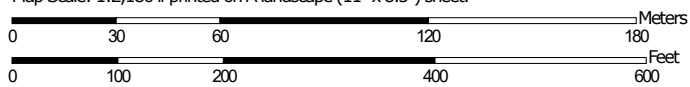
Hydrologic Soil Group—San Diego County Area, California

AREA B



Soil Map may not be valid at this scale.

Map Scale: 1:2,180 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 11N WGS84



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

5/31/2022
Page 1 of 4

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LcE2	La Posta rocky loamy coarse sand, 5 to 30 percent slopes, eroded	A	1.8	9.3%
MvC	Mottsville loamy coarse sand, 2 to 9 percent slopes	A	17.7	90.7%
Totals for Area of Interest			19.5	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

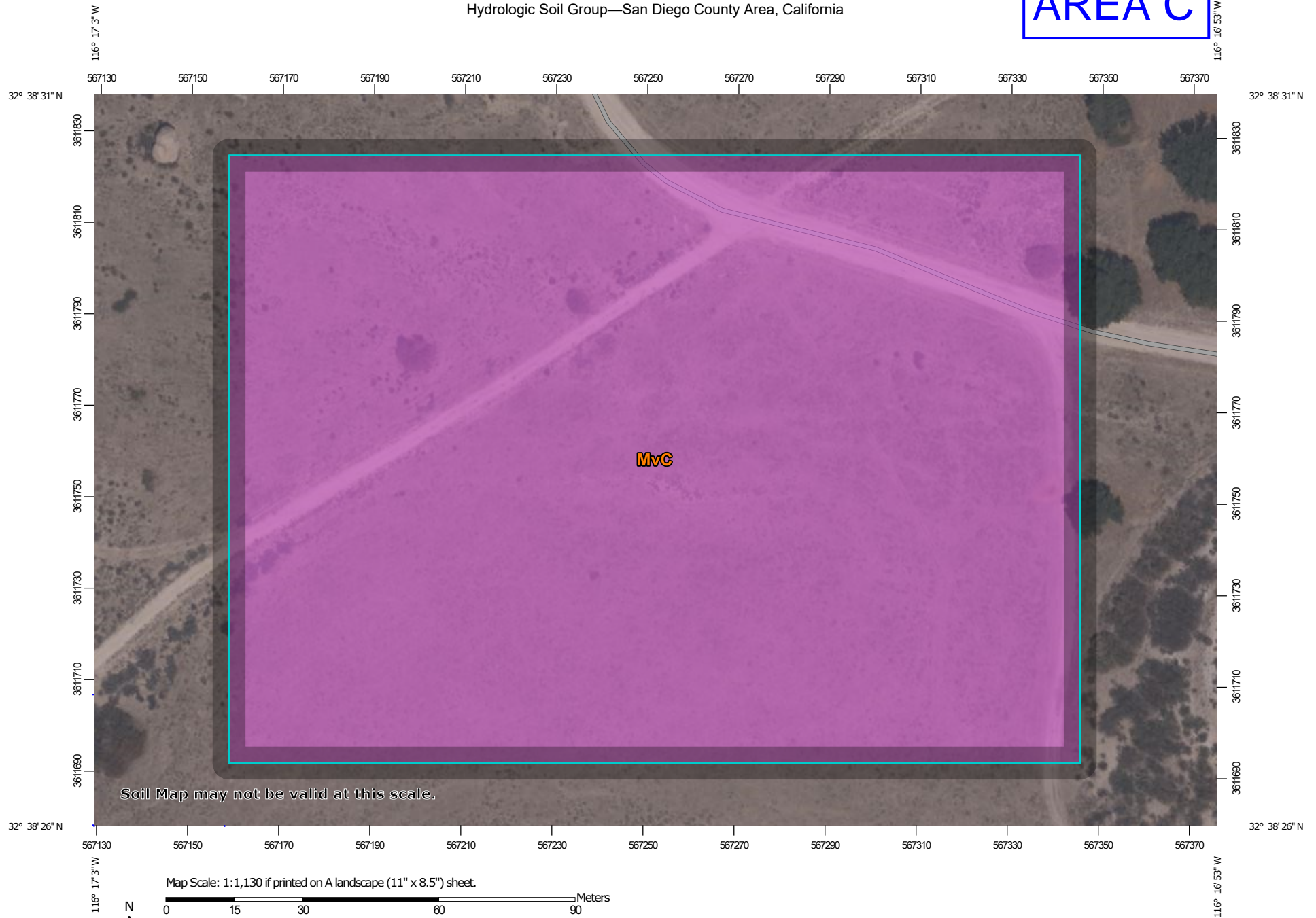
Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Hydrologic Soil Group—San Diego County Area, California

AREA C



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

5/31/2022
Page 1 of 4

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
MvC	Mottsville loamy coarse sand, 2 to 9 percent slopes	A	6.2	100.0%
Totals for Area of Interest			6.2	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

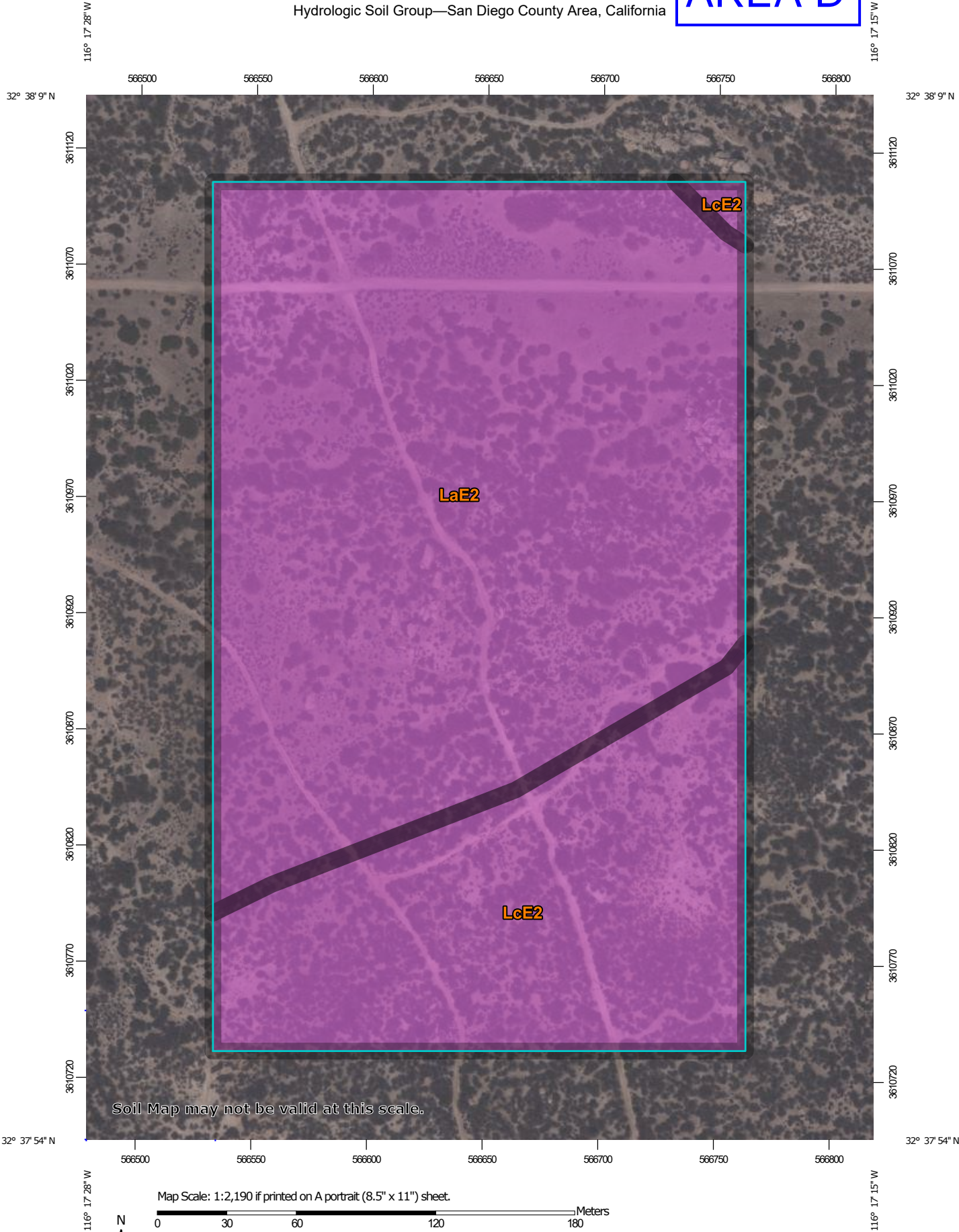
If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

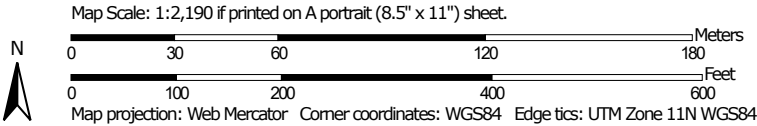
Aggregation Method: Dominant Condition

AREA D

Hydrologic Soil Group—San Diego County Area, California



Soil Map may not be valid at this scale.



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LaE2	La Posta loamy coarse sand, 5 to 30 percent slopes, eroded	A	14.9	70.0%
LcE2	La Posta rocky loamy coarse sand, 5 to 30 percent slopes, eroded	A	6.4	30.0%
Totals for Area of Interest			21.4	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

AREA E

Hydrologic Soil Group—San Diego County Area, California



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LcE2	La Posta rocky loamy coarse sand, 5 to 30 percent slopes, eroded	A	18.5	63.8%
ToE2	Tollhouse rocky coarse sandy loam, 5 to 30 percent slopes, eroded	D	10.5	36.2%
Totals for Area of Interest			29.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

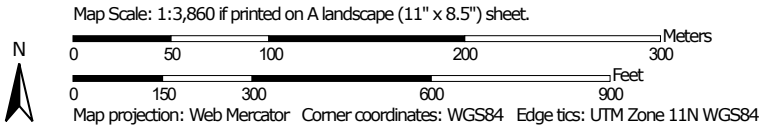
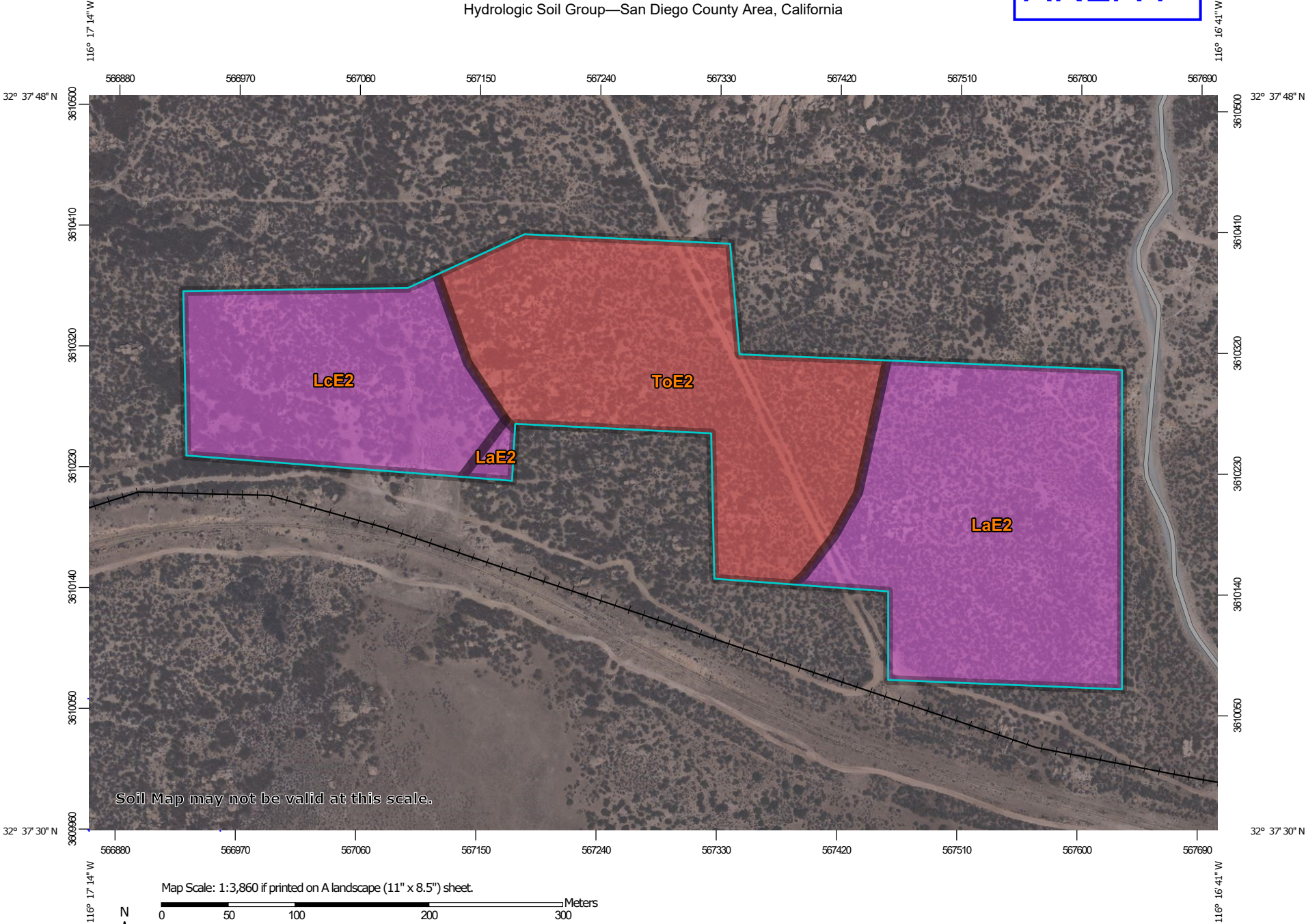
Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

AREA F

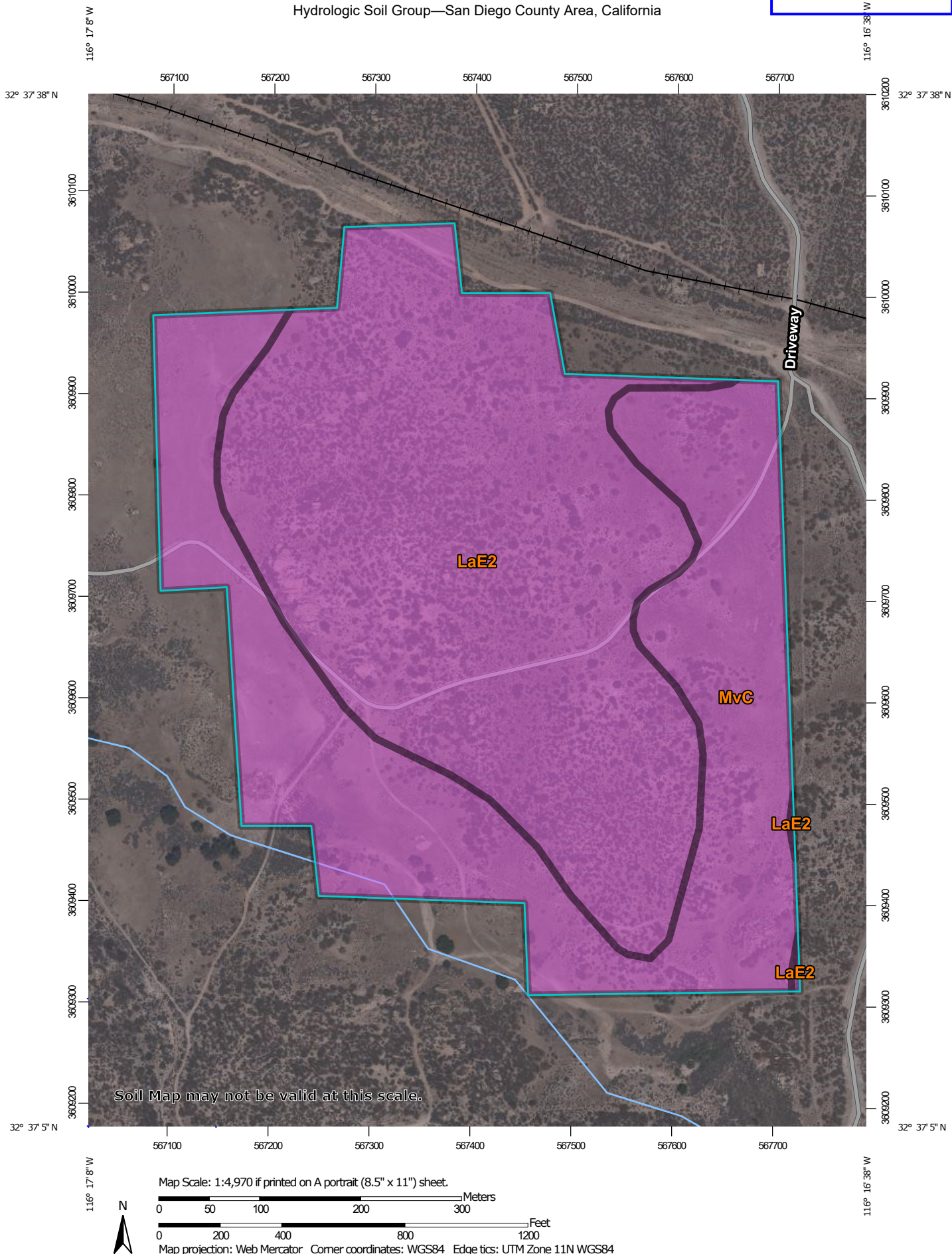
Hydrologic Soil Group—San Diego County Area, California



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LaE2	La Posta loamy coarse sand, 5 to 30 percent slopes, eroded	A	11.6	38.9%
LcE2	La Posta rocky loamy coarse sand, 5 to 30 percent slopes, eroded	A	7.1	23.7%
ToE2	Tollhouse rocky coarse sandy loam, 5 to 30 percent slopes, eroded	D	11.2	37.4%
Totals for Area of Interest			30.0	100.0%

Hydrologic Soil Group—San Diego County Area, California



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LaE2	La Posta loamy coarse sand, 5 to 30 percent slopes, eroded	A	50.9	57.8%
MvC	Mottsville loamy coarse sand, 2 to 9 percent slopes	A	37.2	42.2%
Totals for Area of Interest			88.1	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.


Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.









Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

MAP LEGEND**Area of Interest (AOI)**
 Area of Interest (AOI)
Soils**Soil Rating Polygons**





-  A
-  A/D
-  B
-  B/D
-  C
-  C/D
-  D
-  Not rated or not available

Soil Rating Lines

-  A
-  A/D
-  B
-  B/D
-  C
-  C/D
-  D
-  Not rated or not available

Soil Rating Points

-  A
-  A/D
-  B
-  B/D

-  C
-  C/D
-  D
-  Not rated or not available

Water Features
 Streams and Canals
Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background
 Aerial Photography
MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: San Diego County Area, California
Survey Area Data: Version 16, Sep 13, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 18, 2018—Aug 22, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Total Project Area = 558.00 ac
Total Project EX. Impervious = 0.00 ac
Total Project PR. Impervious = 10.488 ac

Weighted Runoff Coefficients: On-Site

EXISTING Condition

Land Use	Area A (North)	
	Area (ac)	C
Type A Natural	184.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	184.00	

EX. Area A Weighted C = 0.20

PROPOSED Condition

Land Use	Area A (North)	
	Area (ac)	C
Type A Natural	176.48	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.103	0.90
Impervious Battery Storage	5.076	0.90
Impervious Substation	0.025	0.90
Impervious Storage Building	0.009	0.90
Impervious Control Enclosure	0.010	0.90
Impervious Water Tank	0.008	0.90
Impervious Posts	0.060	0.90
Impervious Concrete Crossings	2.225	0.90
<i>Sub-Total Impervious</i>	<i>7.516</i>	<i>-</i>
Total Area =	184.00	

PR. Area A Weighted C = 0.23

Land Use	Area A (South)	
	Area (ac)	C
Type A Natural	126.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	126.00	

EX. Area A Weighted C = 0.20

Land Use	Area A (South)	
	Area (ac)	C
Type A Natural	125.10	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.138	0.90
Impervious Battery Storage	0.000	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.005	0.90
Impervious Posts	0.041	0.90
Impervious Concrete Crossings	0.716	0.90
<i>Sub-Total Impervious</i>	<i>0.900</i>	<i>-</i>
Total Area =	126.00	

PR. Area A Weighted C = 0.20

Land Use	Area B	
	Area (ac)	C
Type A Natural	17.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	17.00	

EX. Area B Weighted C = 0.20

Land Use	Area B	
	Area (ac)	C
Type A Natural	16.88	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.017	0.90
Impervious Battery Storage	0.000	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.000	0.90
Impervious Posts	0.006	0.90
Impervious Concrete Crossings	0.098	0.90
<i>Sub-Total Impervious</i>	<i>0.121</i>	<i>-</i>
Total Area =	17.00	

PR. Area B Weighted C = 0.20

Weighted Runoff Coefficients: On-Site

EXISTING Condition

Land Use	Area C	
	Area (ac)	C
Type A Natural	12.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	12.00	

EX. Area C Weighted C = 0.20

PROPOSED Condition

Land Use	Area C	
	Area (ac)	C
Type A Natural	11.91	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.017	0.90
Impervious Battery Storage	0.000	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.000	0.90
Impervious Posts	0.004	0.90
Impervious Concrete Crossings	0.064	0.90
<i>Sub-Total Impervious</i>	<i>0.085</i>	-
Total Area =	12.00	

PR. Area C Weighted C = 0.20

Land Use	Area D EAST	
	Area (ac)	C
Type A Natural	20.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	20.00	

EX. Area D Weighted C = 0.20

Land Use	Area D EAST	
	Area (ac)	C
Type A Natural	19.86	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.017	0.90
Impervious Battery Storage	0.000	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.000	0.90
Impervious Posts	0.010	0.90
Impervious Concrete Crossings	0.115	0.90
<i>Sub-Total Impervious</i>	<i>0.142</i>	-
Total Area =	20.00	

PR. Area D Weighted C = 0.20

Land Use	Area D SOUTH	
	Area (ac)	C
Type A Natural	11.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	11.00	

EX. Area D Weighted C = 0.20

Land Use	Area D SOUTH	
	Area (ac)	C
Type A Natural	10.92	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.017	0.90
Impervious Battery Storage	0.000	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.000	0.90
Impervious Posts	0.005	0.90
Impervious Concrete Crossings	0.056	0.90
<i>Sub-Total Impervious</i>	<i>0.078</i>	-
Total Area =	11.00	

PR. Area D Weighted C = 0.20

Weighted Runoff Coefficients: On-Site

Land Use	Area E	
	Area (ac)	C
Type A Natural	10.88	0.2
Type B Natural	0	0.25
Type C Natural	0.00	0.30
Type D Natural	5.12	0.35
Impervious	0.00	0.90
Total Area =	16.00	

(64% Soil A and 36% Soil D per NRCS)

Land Use	Area E	
	Area (ac)	C
Type A Natural	10.73	0.2
Type B Natural	0	0.25
Type C Natural	0.00	0.30
Type D Natural	5.11	0.35
Impervious Invertor Skids	0.0172	0.90
Impervious Battery Storage	0.0000	0.90
Impervious Substation	0.0000	0.90
Impervious Storage Building	0.0000	0.90
Impervious Control Enclosure	0.0000	0.90
Impervious Water Tank	0.0000	0.90
Impervious Posts	0.0052	0.90
Impervious Concrete Crossings	0.139	0.90
<i>Sub-Total Impervious</i>	<i>0.1614</i>	<i>-</i>
Total Area =	16.00	

EX. Area E Weighted C = 0.25

PR. Area E Weighted C = 0.25

EXISTING Condition

Land Use	Area F	
	Area (ac)	C
Type A Natural	20.20	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	11.80	0.35
Impervious	0.00	0.90
Total Area =	32.00	

(63% Soil A and 37% Soil D per NRCS)

PROPOSED Condition

Land Use	Area F	
	Area (ac)	C
Type A Natural	19.91	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	11.600	0.35
Impervious Invertor Skids	0.034	0.90
Impervious Battery Storage	0.002	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.000	0.90
Impervious Posts	0.010	0.90
Impervious Concrete Crossings	0.439	0.90
<i>Sub-Total Impervious</i>	<i>0.486</i>	<i>-</i>
Total Area =	32.00	

EX. Area F Weighted C = 0.26

PR. Area F Weighted C = 0.26

Land Use	Area G & H	
	Area (ac)	C
Type A Natural	140.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious	0.00	0.90
Total Area =	140.00	

EX. Area G Weighted C = 0.20

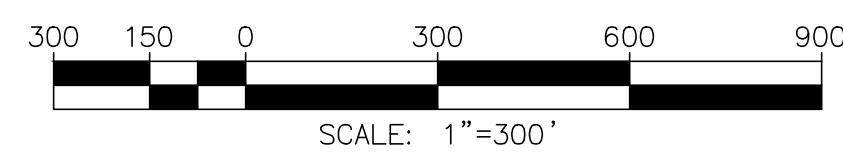
Land Use	Area G & H	
	Area (ac)	C
Type A Natural	139.00	0.20
Type B Natural	0.00	0.25
Type C Natural	0.00	0.30
Type D Natural	0.00	0.35
Impervious Invertor Skids	0.000	0.90
Impervious Battery Storage	0.000	0.90
Impervious Substation	0.000	0.90
Impervious Storage Building	0.000	0.90
Impervious Control Enclosure	0.000	0.90
Impervious Water Tank	0.000	0.90
Impervious Posts	0.000	0.90
Impervious Concrete Crossings	0.999	0.90
<i>Sub-Total Impervious</i>	<i>1.00</i>	<i>-</i>
Total Area =	140.00	

PR. Area G Weighted C = 0.20

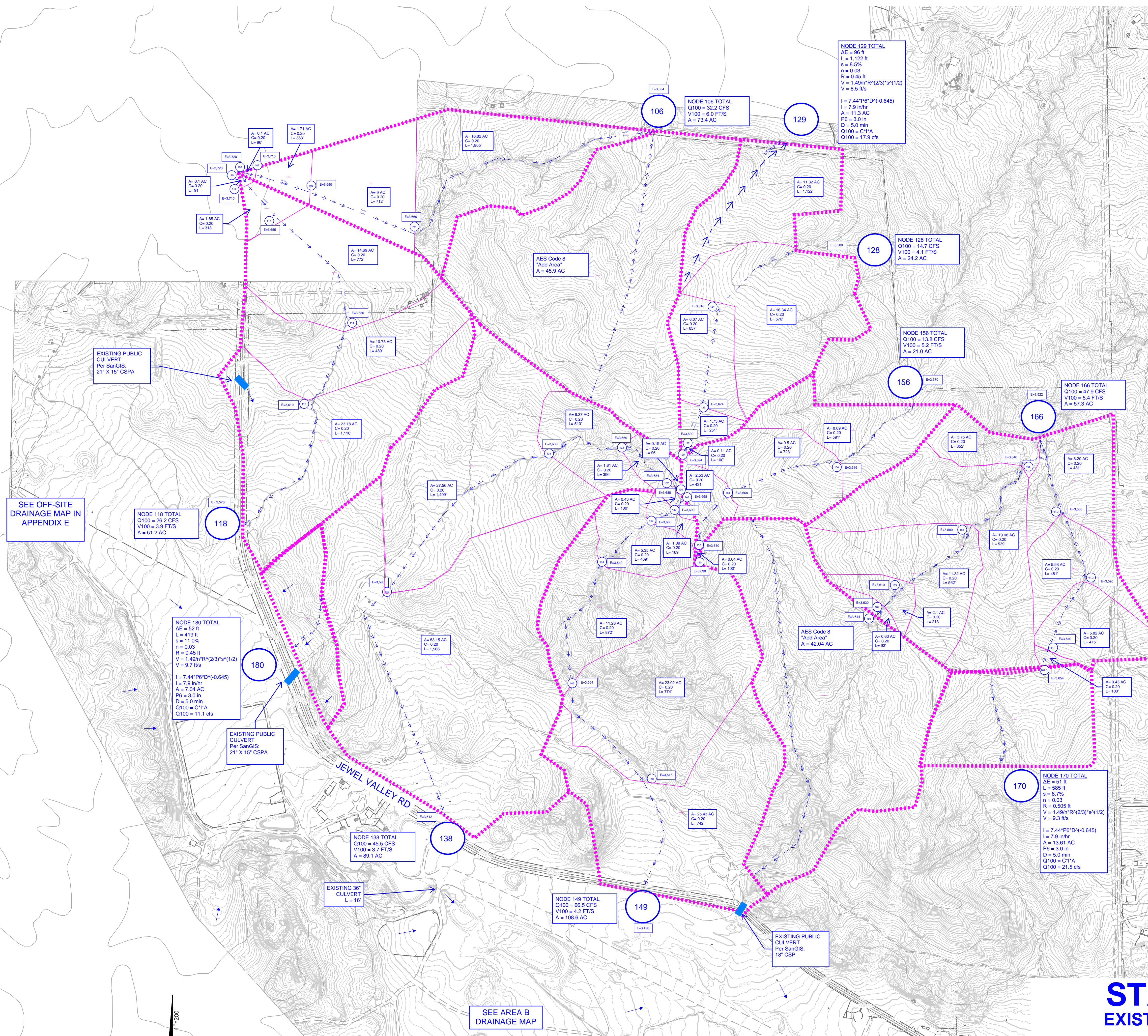
Appendix B – Existing Hydrology

Hydrologic Work Maps
AES Output

C:\USERS\PUBLIC\MBI\PROJECTS\185614_STARLIGHT\185614_EX_MAP_V2_SG.DWG
6/23/2022 10:11 AM



LEGEND
DRAINAGE BOUNDARY
FLOW PATH
DRAINAGE NODE



STARLIGHT SOLAR

EXISTING HYDROLOGIC WORK MAP

AREA A

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2016 Advanced Engineering Software (aes)
Ver. 23.0 Release Date: 07/01/2016 License ID 1264

Analysis prepared by:

***** DESCRIPTION OF STUDY *****

* 185614 STARLIGHT SOLAR *
* EX Q100 ON-SITE AREA A *
* 2024-03-01 *

FILE NAME: 614EXA.DAT
TIME/DATE OF STUDY: 07:39 03/02/2024

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

+-----+
| BEGIN AREA DRAINING TO NODE 106 |
| |
+-----+

FLOW PROCESS FROM NODE 100.00 TO NODE 102.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

INITIAL SUBAREA FLOW-LENGTH(FEET) = 96.00

UPSTREAM ELEVATION(FEET) = 3720.00

DOWNSTREAM ELEVATION(FEET) = 3710.00

ELEVATION DIFFERENCE(FEET) = 10.00

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.368

WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.155

SUBAREA RUNOFF(CFS) = 0.12

TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.12

FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3710.00 DOWNSTREAM(FEET) = 3690.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 363.00 CHANNEL SLOPE = 0.0551

CHANNEL BASE(FEET) = 60.00 "Z" FACTOR = 99.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.800

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.83

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.74

AVERAGE FLOW DEPTH(FEET) = 0.02 TRAVEL TIME(MIN.) = 8.19

Tc(MIN.) = 15.56

SUBAREA AREA(ACRES) = 1.71 SUBAREA RUNOFF(CFS) = 1.30

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 1.8 PEAK FLOW RATE(CFS) = 1.38

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.03 FLOW VELOCITY(FEET/SEC.) = 0.85

LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 459.00 FEET.

FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3690.00 DOWNSTREAM(FEET) = 3670.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 1099.00 CHANNEL SLOPE = 0.0182

CHANNEL BASE(FEET) = 50.00 "Z" FACTOR = 99.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.361

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.93
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.08
AVERAGE FLOW DEPTH(FEET) = 0.08 TRAVEL TIME(MIN.) = 17.00
Tc(MIN.) = 32.56
SUBAREA AREA(ACRES) = 14.45 SUBAREA RUNOFF(CFS) = 6.82
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 16.3 PEAK FLOW RATE(CFS) = 7.68

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.10 FLOW VELOCITY(FEET/SEC.) = 1.25
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 1558.00 FEET.

FLOW PROCESS FROM NODE 104.00 TO NODE 106.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3670.00	DOWNSTREAM(FEET) =	3554.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1371.00	CHANNEL SLOPE =	0.0846
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	7.000
MANNING'S FACTOR =	0.030	MAXIMUM DEPTH(FEET) =	2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	2.189		

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.15
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.65
AVERAGE FLOW DEPTH(FEET) = 0.51 TRAVEL TIME(MIN.) = 4.04
Tc(MIN.) = 36.60
SUBAREA AREA(ACRES) = 11.27 SUBAREA RUNOFF(CFS) = 4.93
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 27.5 PEAK FLOW RATE(CFS) = 12.05

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.54 FLOW VELOCITY(FEET/SEC.) = 5.97
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 106.00 = 2929.00 FEET.

FLOW PROCESS FROM NODE 106.00 TO NODE 106.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	2.189
--	-------

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.2000

SUBAREA AREA(ACRES) =	45.90	SUBAREA RUNOFF(CFS) =	20.09
TOTAL AREA(ACRES) =	73.4	TOTAL RUNOFF(CFS) =	32.15
TC(MIN.) =	36.60		

NODE 106

```
+-----+
| BEGIN AREA DRAINING TO NODE 118 |
+-----+
```

FLOW PROCESS FROM NODE 110.00 TO NODE 112.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

INITIAL SUBAREA FLOW-LENGTH(FEET) = 91.00

UPSTREAM ELEVATION(FEET) = 3720.00

DOWNSTREAM ELEVATION(FEET) = 3710.00

ELEVATION DIFFERENCE(FEET) = 10.00

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.174

WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.262

SUBAREA RUNOFF(CFS) = 0.13

TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.13

FLOW PROCESS FROM NODE 112.00 TO NODE 113.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3710.00 DOWNSTREAM(FEET) = 3655.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 313.00 CHANNEL SLOPE = 0.1757

CHANNEL BASE(FEET) = 50.00 "Z" FACTOR = 99.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.483

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.01

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.07

AVERAGE FLOW DEPTH(FEET) = 0.02 TRAVEL TIME(MIN.) = 4.87

Tc(MIN.) = 12.04

SUBAREA AREA(ACRES) = 1.85 SUBAREA RUNOFF(CFS) = 1.66

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 2.0 PEAK FLOW RATE(CFS) = 1.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.02 FLOW VELOCITY(FEET/SEC.) = 1.68

LONGEST FLOWPATH FROM NODE 110.00 TO NODE 113.00 = 404.00 FEET.

FLOW PROCESS FROM NODE 113.00 TO NODE 114.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 3655.00 DOWNSTREAM(FEET) = 3650.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 772.00 CHANNEL SLOPE = 0.0065
CHANNEL BASE(FEET) = 23.00 "Z" FACTOR = 15.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.011
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.28
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.25
AVERAGE FLOW DEPTH(FEET) = 0.19 TRAVEL TIME(MIN.) = 10.28
Tc(MIN.) = 22.32
SUBAREA AREA(ACRES) = 14.69 SUBAREA RUNOFF(CFS) = 8.85
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 16.6 PEAK FLOW RATE(CFS) = 10.02

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.25 FLOW VELOCITY(FEET/SEC.) = 1.47
LONGEST FLOWPATH FROM NODE 110.00 TO NODE 114.00 = 1176.00 FEET.

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FLOW PROCESS FROM NODE 114.00 TO NODE 116.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 3650.00 DOWNSTREAM(FEET) = 3610.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 489.00 CHANNEL SLOPE = 0.0818
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 14.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.879
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 13.13
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.07
AVERAGE FLOW DEPTH(FEET) = 0.43 TRAVEL TIME(MIN.) = 1.61
Tc(MIN.) = 23.93
SUBAREA AREA(ACRES) = 10.78 SUBAREA RUNOFF(CFS) = 6.21
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 27.4 PEAK FLOW RATE(CFS) = 15.79

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.46 FLOW VELOCITY(FEET/SEC.) = 5.32
LONGEST FLOWPATH FROM NODE 110.00 TO NODE 116.00 = 1665.00 FEET.

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*****
FLOW PROCESS FROM NODE 116.00 TO NODE 118.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 3610.00 DOWNSTREAM(FEET) = 3570.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1110.00 CHANNEL SLOPE = 0.0360

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CHANNEL BASE(Feet) = 0.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(Feet) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.561
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 21.89
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 3.88
AVERAGE FLOW DEPTH(Feet) = 0.53 TRAVEL TIME(Min.) = 4.77
Tc(Min.) = 28.70

SUBAREA AREA(ACRES) = 23.78 SUBAREA RUNOFF(CFS) = 12.18
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 51.2 PEAK FLOW RATE(CFS) = 26.22

NODE 118

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(Feet) = 0.57 FLOW VELOCITY(Feet/Sec.) = 4.07
LONGEST FLOWPATH FROM NODE 110.00 TO NODE 118.00 = 2775.00 Feet.

BEGIN AREA DRAINING TO NODE 128

FLOW PROCESS FROM NODE 120.00 TO NODE 122.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(Feet) = 100.00
UPSTREAM ELEVATION(Feet) = 3694.00
DOWNSTREAM ELEVATION(Feet) = 3690.00
ELEVATION DIFFERENCE(Feet) = 4.00
SUBAREA OVERLAND TIME OF FLOW(Min.) = 10.206
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.989
SUBAREA RUNOFF(CFS) = 0.11
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.11

FLOW PROCESS FROM NODE 122.00 TO NODE 123.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(Feet) = 3690.00 DOWNSTREAM(Feet) = 3674.00
CHANNEL LENGTH THRU SUBAREA(Feet) = 251.00 CHANNEL SLOPE = 0.0637
CHANNEL BASE(Feet) = 30.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(Feet) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.035
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.82
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.05
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 3.97
Tc(MIN.) = 14.18
SUBAREA AREA(ACRES) = 1.73 SUBAREA RUNOFF(CFS) = 1.40
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 1.8 PEAK FLOW RATE(CFS) = 1.48

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.39
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 123.00 = 351.00 FEET.

FLOW PROCESS FROM NODE 123.00 TO NODE 124.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3674.00 DOWNSTREAM(FEET) = 3616.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 657.00 CHANNEL SLOPE = 0.0883
CHANNEL BASE(FEET) = 35.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.271

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.46

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.01

AVERAGE FLOW DEPTH(FEET) = 0.05 TRAVEL TIME(MIN.) = 5.46

Tc(MIN.) = 19.64

SUBAREA AREA(ACRES) = 6.07 SUBAREA RUNOFF(CFS) = 3.97

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 7.9 PEAK FLOW RATE(CFS) = 5.17

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 2.27
LONGEST FLOWPATH FROM NODE 120.00 TO NODE 124.00 = 1008.00 FEET.

FLOW PROCESS FROM NODE 124.00 TO NODE 128.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3616.00 DOWNSTREAM(FEET) = 3560.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 576.00 CHANNEL SLOPE = 0.0972
CHANNEL BASE(FEET) = 13.00 "Z" FACTOR = 14.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.040

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.15

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.07

AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 2.36

Tc(MIN.) = 21.99

SUBAREA AREA(ACRES) =	16.34	SUBAREA RUNOFF(CFS) =	9.94
AREA-AVERAGE RUNOFF COEFFICIENT =	0.200		
TOTAL AREA(ACRES) =	24.2	PEAK FLOW RATE(CFS) =	14.74

NODE 128

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.20 FLOW VELOCITY(FEET/SEC.) = 4.79

LONGEST FLOWPATH FROM NODE 120.00 TO NODE 128.00 = 1584.00 FEET.

BEGIN AREA DRAINING TO NODE 138

FLOW PROCESS FROM NODE 130.00 TO NODE 132.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

INITIAL SUBAREA FLOW-LENGTH(FEET) = 96.00

UPSTREAM ELEVATION(FEET) = 3688.00

DOWNSTREAM ELEVATION(FEET) = 3684.00

ELEVATION DIFFERENCE(FEET) = 4.00

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.865

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.099

SUBAREA RUNOFF(CFS) = 0.19

TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.19

FLOW PROCESS FROM NODE 132.00 TO NODE 133.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3684.00 DOWNSTREAM(FEET) = 3660.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 396.00 CHANNEL SLOPE = 0.0606

CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 6.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.464

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.00

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.92

AVERAGE FLOW DEPTH(FEET) = 0.24 TRAVEL TIME(MIN.) = 2.26

Tc(MIN.) = 12.12

SUBAREA AREA(ACRES) = 1.81 SUBAREA RUNOFF(CFS) = 1.62

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 2.0 PEAK FLOW RATE(CFS) = 1.79

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.29 FLOW VELOCITY(FEET/SEC.) = 3.43
LONGEST FLOWPATH FROM NODE 130.00 TO NODE 133.00 = 492.00 FEET.

FLOW PROCESS FROM NODE 133.00 TO NODE 134.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3660.00 DOWNSTREAM(FEET) = 3638.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 510.00 CHANNEL SLOPE = 0.0431

CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 14.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.904

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.28

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.03

AVERAGE FLOW DEPTH(FEET) = 0.32 TRAVEL TIME(MIN.) = 2.80

Tc(MIN.) = 14.93

SUBAREA AREA(ACRES) = 6.37 SUBAREA RUNOFF(CFS) = 4.97

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 8.4 PEAK FLOW RATE(CFS) = 6.54

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.37 FLOW VELOCITY(FEET/SEC.) = 3.39

LONGEST FLOWPATH FROM NODE 130.00 TO NODE 134.00 = 1002.00 FEET.

FLOW PROCESS FROM NODE 134.00 TO NODE 136.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3638.00 DOWNSTREAM(FEET) = 3590.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 1409.00 CHANNEL SLOPE = 0.0341

CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 20.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.054

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 15.01

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.40

AVERAGE FLOW DEPTH(FEET) = 0.36 TRAVEL TIME(MIN.) = 6.91

Tc(MIN.) = 21.84

SUBAREA AREA(ACRES) = 27.56 SUBAREA RUNOFF(CFS) = 16.83

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 35.9 PEAK FLOW RATE(CFS) = 21.95

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.43 FLOW VELOCITY(FEET/SEC.) = 3.75

LONGEST FLOWPATH FROM NODE 130.00 TO NODE 136.00 = 2411.00 FEET.

FLOW PROCESS FROM NODE 136.00 TO NODE 138.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3590.00 DOWNSTREAM(FEET) = 3512.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 1556.00 CHANNEL SLOPE = 0.0501

CHANNEL BASE(FEET) = 40.00 "Z" FACTOR = 25.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.555

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 35.53

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.72

AVERAGE FLOW DEPTH(FEET) = 0.21 TRAVEL TIME(MIN.) = 6.96

Tc(MIN.) = 28.80

SUBAREA AREA(ACRES) = 53.16 SUBAREA RUNOFF(CFS) = 27.16

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 89.1 PEAK FLOW RATE(CFS) = 45.52

NODE 138

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.25 FLOW VELOCITY(FEET/SEC.) = 3.99

LONGEST FLOWPATH FROM NODE 130.00 TO NODE 138.00 = 3967.00 FEET.

BEGIN AREA DRAINING TO NODE 149

FLOW PROCESS FROM NODE 140.00 TO NODE 142.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00

UPSTREAM ELEVATION(FEET) = 3698.00

DOWNSTREAM ELEVATION(FEET) = 3690.00

ELEVATION DIFFERENCE(FEET) = 8.00

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.101

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.790

SUBAREA RUNOFF(CFS) = 0.50

TOTAL AREA(ACRES) = 0.43 TOTAL RUNOFF(CFS) = 0.50

FLOW PROCESS FROM NODE 142.00 TO NODE 143.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3690.00 DOWNSTREAM(FEET) = 3680.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 169.00 CHANNEL SLOPE = 0.0592
 CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 7.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.932
 *USER SPECIFIED(SUBAREA):
 USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.03
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.23
 AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 2.29
 Tc(MIN.) = 10.39
 SUBAREA AREA(ACRES) = 1.09 SUBAREA RUNOFF(CFS) = 1.08
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
 TOTAL AREA(ACRES) = 1.5 PEAK FLOW RATE(CFS) = 1.50

 END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.40
 LONGEST FLOWPATH FROM NODE 140.00 TO NODE 143.00 = 269.00 FEET.

FLOW PROCESS FROM NODE 143.00 TO NODE 144.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3680.00 DOWNSTREAM(FEET) = 3640.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 409.00 CHANNEL SLOPE = 0.0978
 CHANNEL BASE(FEET) = 12.00 "Z" FACTOR = 6.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.368
 *USER SPECIFIED(SUBAREA):
 USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.84
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.17
 AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 2.15
 Tc(MIN.) = 12.54
 SUBAREA AREA(ACRES) = 5.35 SUBAREA RUNOFF(CFS) = 4.67
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
 TOTAL AREA(ACRES) = 6.9 PEAK FLOW RATE(CFS) = 6.00

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.12 FLOW VELOCITY(FEET/SEC.) = 3.77
 LONGEST FLOWPATH FROM NODE 140.00 TO NODE 144.00 = 678.00 FEET.

FLOW PROCESS FROM NODE 144.00 TO NODE 146.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3640.00 DOWNSTREAM(FEET) = 3564.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 872.00 CHANNEL SLOPE = 0.0872
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 14.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.746
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.23
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.31
AVERAGE FLOW DEPTH(FEET) = 0.19 TRAVEL TIME(MIN.) = 3.37
Tc(MIN.) = 15.91
SUBAREA AREA(ACRES) = 11.26 SUBAREA RUNOFF(CFS) = 8.44
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 18.1 PEAK FLOW RATE(CFS) = 13.58

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.22 FLOW VELOCITY(FEET/SEC.) = 4.61
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 146.00 = 1550.00 FEET.

FLOW PROCESS FROM NODE 146.00 TO NODE 148.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3564.00 DOWNSTREAM(FEET) = 3516.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 774.00 CHANNEL SLOPE = 0.0620
CHANNEL BASE(FEET) = 18.00 "Z" FACTOR = 9.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.361
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 21.33
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.42
AVERAGE FLOW DEPTH(FEET) = 0.24 TRAVEL TIME(MIN.) = 2.92
Tc(MIN.) = 18.83
SUBAREA AREA(ACRES) = 23.02 SUBAREA RUNOFF(CFS) = 15.47
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 41.2 PEAK FLOW RATE(CFS) = 27.66

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.28 FLOW VELOCITY(FEET/SEC.) = 4.86
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 148.00 = 2324.00 FEET.

FLOW PROCESS FROM NODE 148.00 TO NODE 149.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3516.00 DOWNSTREAM(FEET) = 3490.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 742.00 CHANNEL SLOPE = 0.0350
CHANNEL BASE(FEET) = 9.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.062
*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 35.44
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.22
AVERAGE FLOW DEPTH(FEET) = 0.46 TRAVEL TIME(MIN.) = 2.93
Tc(MIN.) = 21.76
SUBAREA AREA(ACRES) = 25.43 SUBAREA RUNOFF(CFS) = 15.57
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 66.6 PEAK FLOW RATE(CFS) = 40.77

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.49 FLOW VELOCITY(FEET/SEC.) = 4.38
LONGEST FLOWPATH FROM NODE 140.00 TO NODE 149.00 = 3066.00 FEET.

FLOW PROCESS FROM NODE 149.00 TO NODE 149.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.062

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

AREA-AVERAGE RUNOFF COEFFICIENT = 0.2000

SUBAREA AREA(ACRES) = 42.04 SUBAREA RUNOFF(CFS) = 25.74

TOTAL AREA(ACRES) = 108.6 TOTAL RUNOFF(CFS) = 66.51

TC(MIN.) = 21.76

NODE 149

+-----+
| BEGIN AREA DRAINING TO NODE 156 |
| |
+-----+

FLOW PROCESS FROM NODE 150.00 TO NODE 152.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00

UPSTREAM ELEVATION(FEET) = 3690.00

DOWNSTREAM ELEVATION(FEET) = 3680.00

ELEVATION DIFFERENCE(FEET) = 10.00

SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.520

WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.075

SUBAREA RUNOFF(CFS) = 1.21

TOTAL AREA(ACRES) = 1.00 TOTAL RUNOFF(CFS) = 1.21

FLOW PROCESS FROM NODE 152.00 TO NODE 153.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3680.00 DOWNSTREAM(FEET) = 3668.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 431.00 CHANNEL SLOPE = 0.0278
CHANNEL BASE(FEET) = 14.00 "Z" FACTOR = 9.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.422

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.93

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.50

AVERAGE FLOW DEPTH(FEET) = 0.09 TRAVEL TIME(MIN.) = 4.79

Tc(MIN.) = 12.31

SUBAREA AREA(ACRES) = 1.57 SUBAREA RUNOFF(CFS) = 1.39

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 2.6 PEAK FLOW RATE(CFS) = 2.27

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.09 FLOW VELOCITY(FEET/SEC.) = 1.62

LONGEST FLOWPATH FROM NODE 150.00 TO NODE 153.00 = 531.00 FEET.

FLOW PROCESS FROM NODE 153.00 TO NODE 154.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3668.00 DOWNSTREAM(FEET) = 3616.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 723.00 CHANNEL SLOPE = 0.0719
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 15.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.518

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.68

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.30

AVERAGE FLOW DEPTH(FEET) = 0.08 TRAVEL TIME(MIN.) = 5.24

Tc(MIN.) = 17.54

SUBAREA AREA(ACRES) = 9.50 SUBAREA RUNOFF(CFS) = 6.68

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 12.1 PEAK FLOW RATE(CFS) = 8.49

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.10 FLOW VELOCITY(FEET/SEC.) = 2.80

LONGEST FLOWPATH FROM NODE 150.00 TO NODE 154.00 = 1254.00 FEET.

FLOW PROCESS FROM NODE 154.00 TO NODE 156.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3616.00 DOWNSTREAM(FEET) = 3570.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 591.00 CHANNEL SLOPE = 0.0778
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.293
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.42
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.21
AVERAGE FLOW DEPTH(FEET) = 0.47 TRAVEL TIME(MIN.) = 1.89
Tc(MIN.) = 19.43

SUBAREA AREA(ACRES) = 8.89	SUBAREA RUNOFF(CFS) = 5.85
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200	
TOTAL AREA(ACRES) = 21.0	PEAK FLOW RATE(CFS) = 13.80

NODE 156

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.50 FLOW VELOCITY(FEET/SEC.) = 5.51
LONGEST FLOWPATH FROM NODE 150.00 TO NODE 156.00 = 1845.00 FEET.

+-----+
| BEGIN AREA DRAINING TO NODE 166 |
| |
+-----+

FLOW PROCESS FROM NODE 160.00 TO NODE 162.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 93.00
UPSTREAM ELEVATION(FEET) = 3644.00
DOWNSTREAM ELEVATION(FEET) = 3630.00
ELEVATION DIFFERENCE(FEET) = 14.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.252
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.219
SUBAREA RUNOFF(CFS) = 0.78
TOTAL AREA(ACRES) = 0.63 TOTAL RUNOFF(CFS) = 0.78

FLOW PROCESS FROM NODE 162.00 TO NODE 163.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

ELEVATION DATA: UPSTREAM(FEET) = 3630.00 DOWNSTREAM(FEET) = 3610.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 213.00 CHANNEL SLOPE = 0.0939
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.733

```

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.99
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.64
AVERAGE FLOW DEPTH(FEET) = 0.23 TRAVEL TIME(MIN.) = 0.98
Tc(MIN.) = 8.23
SUBAREA AREA(ACRES) = 2.10 SUBAREA RUNOFF(CFS) = 2.41
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 2.7 PEAK FLOW RATE(CFS) = 3.13

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END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.28 FLOW VELOCITY(FEET/SEC.) = 4.06
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 163.00 = 306.00 FEET.

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FLOW PROCESS FROM NODE 163.00 TO NODE 164.00 IS CODE = 51

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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 3610.00 DOWNSTREAM(FEET) = 3590.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 562.00 CHANNEL SLOPE = 0.0356
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.811

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*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.59
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.65
AVERAGE FLOW DEPTH(FEET) = 0.49 TRAVEL TIME(MIN.) = 2.57
Tc(MIN.) = 10.80
SUBAREA AREA(ACRES) = 11.32 SUBAREA RUNOFF(CFS) = 10.89
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 14.0 PEAK FLOW RATE(CFS) = 13.52

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END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.58 FLOW VELOCITY(FEET/SEC.) = 4.06
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 164.00 = 868.00 FEET.

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FLOW PROCESS FROM NODE 164.00 TO NODE 165.00 IS CODE = 51

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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 3590.00 DOWNSTREAM(FEET) = 3540.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 539.00 CHANNEL SLOPE = 0.0928
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 17.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.406

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*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0

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TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 21.94
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.70
AVERAGE FLOW DEPTH(FEET) = 0.48 TRAVEL TIME(MIN.) = 1.58
Tc(MIN.) = 12.37
SUBAREA AREA(ACRES) = 19.08 SUBAREA RUNOFF(CFS) = 16.81
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 33.1 PEAK FLOW RATE(CFS) = 29.20

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.53 FLOW VELOCITY(FEET/SEC.) = 6.13
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 165.00 = 1407.00 FEET.

FLOW PROCESS FROM NODE 165.00 TO NODE 166.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3540.00	DOWNSTREAM(FEET) =	3522.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	352.00	CHANNEL SLOPE =	0.0511
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	10.000
MANNING'S FACTOR =	0.030	MAXIMUM DEPTH(FEET) =	2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.185		

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 30.77

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.69

AVERAGE FLOW DEPTH(FEET) = 0.74 TRAVEL TIME(MIN.) = 1.03

Tc(MIN.) = 13.40

SUBAREA AREA(ACRES) = 3.75 SUBAREA RUNOFF(CFS) = 3.14

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 36.9 PEAK FLOW RATE(CFS) = 30.87

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.74 FLOW VELOCITY(FEET/SEC.) = 5.71
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 166.00 = 1759.00 FEET.

FLOW PROCESS FROM NODE 166.00 TO NODE 166.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS =	2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:	
TIME OF CONCENTRATION(MIN.) =	13.40
RAINFALL INTENSITY(INCH/HR) =	4.18
TOTAL STREAM AREA(ACRES) =	36.88
PEAK FLOW RATE(CFS) AT CONFLUENCE =	30.87

FLOW PROCESS FROM NODE 161.00 TO NODE 161.10 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3654.00
DOWNSTREAM ELEVATION(FEET) = 3640.00
ELEVATION DIFFERENCE(FEET) = 14.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.520
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.075
SUBAREA RUNOFF(CFS) = 0.52
TOTAL AREA(ACRES) = 0.43 TOTAL RUNOFF(CFS) = 0.52

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FLOW PROCESS FROM NODE 161.10 TO NODE 161.20 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
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ELEVATION DATA: UPSTREAM(FEET) = 3640.00 DOWNSTREAM(FEET) = 3590.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 475.00 CHANNEL SLOPE = 0.1053
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.287
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.61
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.38
AVERAGE FLOW DEPTH(FEET) = 0.29 TRAVEL TIME(MIN.) = 1.81
Tc(MIN.) = 9.33
SUBAREA AREA(ACRES) = 5.82 SUBAREA RUNOFF(CFS) = 6.15
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 6.2 PEAK FLOW RATE(CFS) = 6.61

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END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.36 FLOW VELOCITY(FEET/SEC.) = 5.06
LONGEST FLOWPATH FROM NODE 161.00 TO NODE 161.20 = 575.00 FEET.

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FLOW PROCESS FROM NODE 161.20 TO NODE 161.30 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====

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```

ELEVATION DATA: UPSTREAM(FEET) = 3590.00 DOWNSTREAM(FEET) = 3556.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 481.00 CHANNEL SLOPE = 0.0707
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.750
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.43
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.76

```


AVERAGE FLOW DEPTH(FEET) = 0.45 TRAVEL TIME(MIN.) = 1.69
Tc(MIN.) = 11.01
SUBAREA AREA(ACRES) = 5.93 SUBAREA RUNOFF(CFS) = 5.63
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 12.2 PEAK FLOW RATE(CFS) = 11.57

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.48 FLOW VELOCITY(FEET/SEC.) = 5.07
LONGEST FLOWPATH FROM NODE 161.00 TO NODE 161.30 = 1056.00 FEET.

FLOW PROCESS FROM NODE 161.30 TO NODE 166.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3556.00 DOWNSTREAM(FEET) = 3522.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 481.00 CHANNEL SLOPE = 0.0707
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.378

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 15.16

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.41

AVERAGE FLOW DEPTH(FEET) = 0.53 TRAVEL TIME(MIN.) = 1.48

Tc(MIN.) = 12.49

SUBAREA AREA(ACRES) = 8.20 SUBAREA RUNOFF(CFS) = 7.18

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 20.4 PEAK FLOW RATE(CFS) = 17.85

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.56 FLOW VELOCITY(FEET/SEC.) = 5.66
LONGEST FLOWPATH FROM NODE 161.00 TO NODE 166.00 = 1537.00 FEET.

FLOW PROCESS FROM NODE 166.00 TO NODE 166.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.49
RAINFALL INTENSITY(INCH/HR) = 4.38
TOTAL STREAM AREA(ACRES) = 20.38
PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.85

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	30.87	13.40	4.185	36.88
2	17.85	12.49	4.378	20.38

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	46.62	12.49	4.378
2	47.92	13.40	4.185

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 47.92 Tc(MIN.) = 13.40

TOTAL AREA(ACRES) = 57.3

LONGEST FLOWPATH FROM NODE 160.00 TO NODE 166.00 = 1759.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 57.3 TC(MIN.) = 13.40

PEAK FLOW RATE(CFS) = 47.92

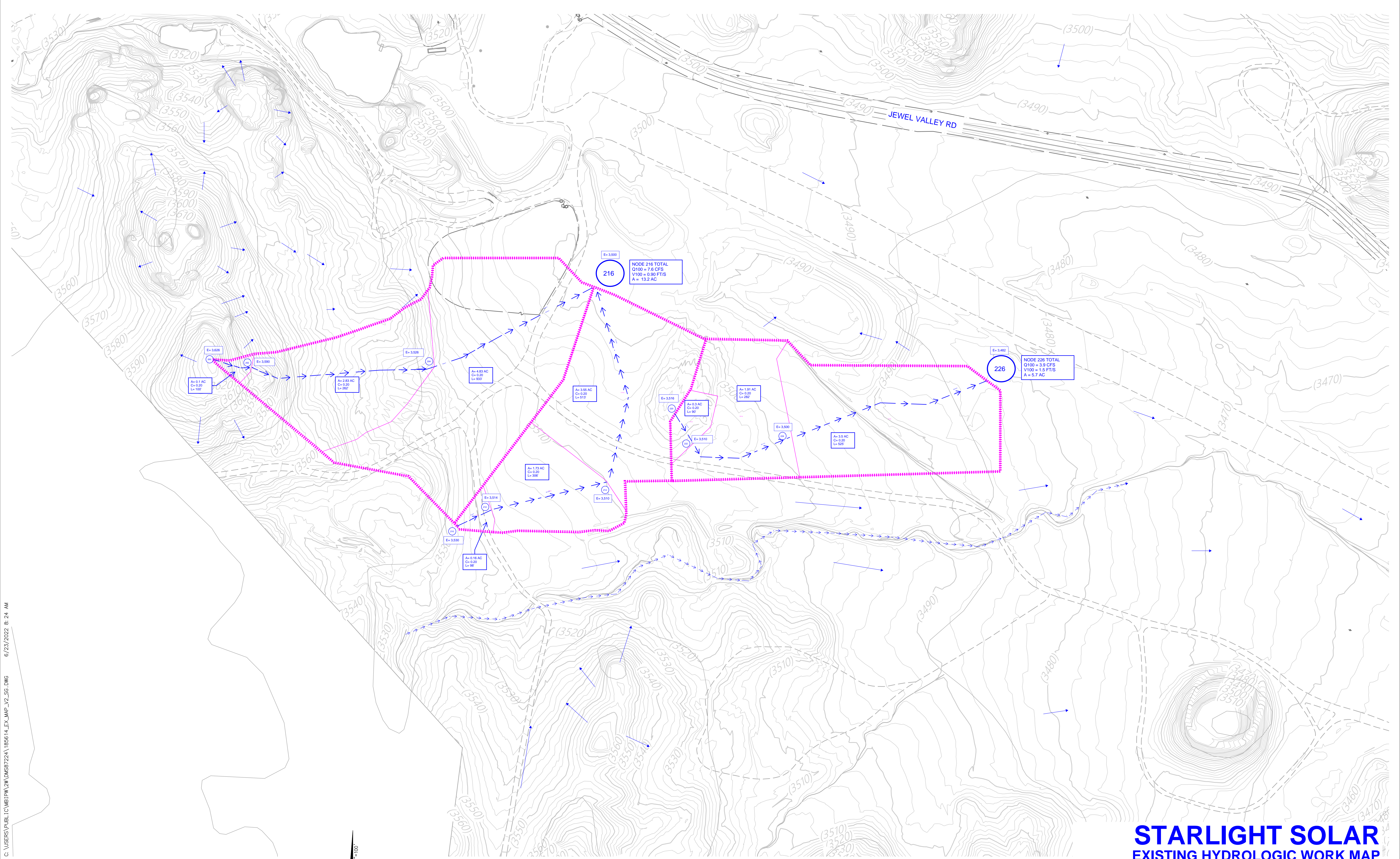
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NODE 166

=====

END OF RATIONAL METHOD ANALYSIS



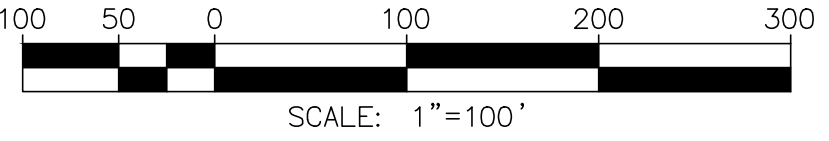


STARLIGHT SOLAR

EXISTING HYDROLOGIC WORK MAP

AREA B

- LEGEND**
- DRAINAGE BOUNDARY
 - FLOW PATH
 - DRAINAGE NODE



C:\USERS\PUBLIC\W61PW\2\DWG\87224\185614_EX_MAP_V2_SG.DWG 6/23/2022 8:24 AM

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2016 Advanced Engineering Software (aes)
Ver. 23.0 Release Date: 07/01/2016 License ID 1264

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 185614 STARLIGHT SOLAR *
* EX Q100 ON-SITE AREA B *
* 2024-03-01 *

FILE NAME: 614EXB.DAT
TIME/DATE OF STUDY: 07:59 03/02/2024

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

FLOW PROCESS FROM NODE 200.00 TO NODE 202.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 INITIAL SUBAREA FLOW-LENGTH(Feet) = 100.00
 UPSTREAM ELEVATION(Feet) = 3626.00
 DOWNSTREAM ELEVATION(Feet) = 3590.00
 ELEVATION DIFFERENCE(Feet) = 36.00
 SUBAREA OVERLAND TIME OF FLOW(Min.) = 7.520
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.075
 SUBAREA RUNOFF(CFS) = 0.12
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.12

FLOW PROCESS FROM NODE 202.00 TO NODE 204.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(Feet) = 3590.00 DOWNSTREAM(Feet) = 3526.00
 CHANNEL LENGTH THRU SUBAREA(Feet) = 262.00 CHANNEL SLOPE = 0.2443
 CHANNEL BASE(Feet) = 20.00 "Z" FACTOR = 8.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(Feet) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.285

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.63
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 2.41
 AVERAGE FLOW DEPTH(Feet) = 0.03 TRAVEL TIME(Min.) = 1.81
 Tc(Min.) = 9.33
 SUBAREA AREA(ACRES) = 2.83 SUBAREA RUNOFF(CFS) = 2.99
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
 TOTAL AREA(ACRES) = 2.9 PEAK FLOW RATE(CFS) = 3.10

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(Feet) = 0.05 FLOW VELOCITY(Feet/Sec.) = 3.12
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 204.00 = 362.00 FEET.

FLOW PROCESS FROM NODE 204.00 TO NODE 216.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(Feet) = 3526.00 DOWNSTREAM(Feet) = 3500.00
 CHANNEL LENGTH THRU SUBAREA(Feet) = 933.00 CHANNEL SLOPE = 0.0279
 CHANNEL BASE(Feet) = 25.00 "Z" FACTOR = 15.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(Feet) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.425

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.80
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 1.74
 AVERAGE FLOW DEPTH(Feet) = 0.10 TRAVEL TIME(Min.) = 8.95

Tc(MIN.) = 18.28
SUBAREA AREA(ACRES) = 4.83 SUBAREA RUNOFF(CFS) = 3.31
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 7.8 PEAK FLOW RATE(CFS) = 5.32

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.11 FLOW VELOCITY(FEET/SEC.) = 1.82
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 216.00 = 1295.00 FEET.

FLOW PROCESS FROM NODE 216.00 TO NODE 216.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 18.28
RAINFALL INTENSITY(INCH/HR) = 3.43
TOTAL STREAM AREA(ACRES) = 7.76
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.32

FLOW PROCESS FROM NODE 210.00 TO NODE 212.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 98.00
UPSTREAM ELEVATION(FEET) = 3530.00
DOWNSTREAM ELEVATION(FEET) = 3514.00
ELEVATION DIFFERENCE(FEET) = 16.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.444
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.115
SUBAREA RUNOFF(CFS) = 0.20
TOTAL AREA(ACRES) = 0.16 TOTAL RUNOFF(CFS) = 0.20

FLOW PROCESS FROM NODE 212.00 TO NODE 214.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3514.00 DOWNSTREAM(FEET) = 3510.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 306.00 CHANNEL SLOPE = 0.0131
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 45.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.852
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.90
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.65

AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 7.79
Tc(MIN.) = 15.24
SUBAREA AREA(ACRES) = 1.73 SUBAREA RUNOFF(CFS) = 1.33
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 1.9 PEAK FLOW RATE(CFS) = 1.46

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 0.80
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 214.00 = 404.00 FEET.

FLOW PROCESS FROM NODE 214.00 TO NODE 216.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3510.00 DOWNSTREAM(FEET) = 3500.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 513.00 CHANNEL SLOPE = 0.0195
CHANNEL BASE(FEET) = 55.00 "Z" FACTOR = 35.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.817

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.48

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.90

AVERAGE FLOW DEPTH(FEET) = 0.05 TRAVEL TIME(MIN.) = 9.51

Tc(MIN.) = 24.75

SUBAREA AREA(ACRES) = 3.55 SUBAREA RUNOFF(CFS) = 2.00

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 5.4 PEAK FLOW RATE(CFS) = 3.07

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 0.96
LONGEST FLOWPATH FROM NODE 210.00 TO NODE 216.00 = 917.00 FEET.

FLOW PROCESS FROM NODE 216.00 TO NODE 216.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 24.75

RAINFALL INTENSITY(INCH/HR) = 2.82

TOTAL STREAM AREA(ACRES) = 5.44

PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.07

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	5.32	18.28	3.425	7.76
2	3.07	24.75	2.817	5.44

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.58	18.28	3.425
2	7.44	24.75	2.817

NODE 216

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 7.58 Tc(MIN.) = 18.28
TOTAL AREA(ACRES) = 13.2

LONGEST FLOWPATH FROM NODE 200.00 TO NODE 216.00 = 1295.00 FEET.

END NODE 216
BEGIN NODE 220

FLOW PROCESS FROM NODE 220.00 TO NODE 222.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 90.00
UPSTREAM ELEVATION(FEET) = 3516.00
DOWNSTREAM ELEVATION(FEET) = 3510.00
ELEVATION DIFFERENCE(FEET) = 6.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 8.166
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.760
SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 0.30 TOTAL RUNOFF(CFS) = 0.35

FLOW PROCESS FROM NODE 222.00 TO NODE 224.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3510.00 DOWNSTREAM(FEET) = 3500.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 282.00 CHANNEL SLOPE = 0.0355
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 30.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.370
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.18
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.08
AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 4.36
Tc(MIN.) = 12.53

SUBAREA AREA(ACRES) = 1.91 SUBAREA RUNOFF(CFS) = 1.67
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 2.2 PEAK FLOW RATE(CFS) = 1.93

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 1.21
LONGEST FLOWPATH FROM NODE 220.00 TO NODE 224.00 = 372.00 FEET.

FLOW PROCESS FROM NODE 224.00 TO NODE 226.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3500.00 DOWNSTREAM(FEET) = 3482.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 525.00 CHANNEL SLOPE = 0.0343

CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 30.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.409

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.13

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.49

AVERAGE FLOW DEPTH(FEET) = 0.07 TRAVEL TIME(MIN.) = 5.89

Tc(MIN.) = 18.42

SUBAREA AREA(ACRES) = 3.50 SUBAREA RUNOFF(CFS) = 2.39

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 5.7 PEAK FLOW RATE(CFS) = 3.89

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.08 FLOW VELOCITY(FEET/SEC.) = 1.52

LONGEST FLOWPATH FROM NODE 220.00 TO NODE 226.00 = 897.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 5.7 TC(MIN.) = 18.42
PEAK FLOW RATE(CFS) = 3.89

NODE 226

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END OF RATIONAL METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2016 Advanced Engineering Software (aes)
Ver. 23.0 Release Date: 07/01/2016 License ID 1264

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 185614 STARLIGHT SOLAR *
* EX Q100 ONSITE & OFFSITE AREA C *
* 2024-03-01 *

FILE NAME: 614EXC.DAT
TIME/DATE OF STUDY: 08:06 03/02/2024

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

FLOW PROCESS FROM NODE 300.00 TO NODE 302.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 83.00
UPSTREAM ELEVATION(FEET) = 3560.00
DOWNSTREAM ELEVATION(FEET) = 3540.00
ELEVATION DIFFERENCE(FEET) = 20.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.851
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.451
SUBAREA RUNOFF(CFS) = 0.12
TOTAL AREA(ACRES) = 0.09 TOTAL RUNOFF(CFS) = 0.12

FLOW PROCESS FROM NODE 302.00 TO NODE 304.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3540.00	DOWNSTREAM(FEET) =	3528.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	201.00	CHANNEL SLOPE =	0.0597
CHANNEL BASE(FEET) =	18.00	"Z" FACTOR =	6.000
MANNING'S FACTOR =	0.030	MAXIMUM DEPTH(FEET) =	2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.005		

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.47
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.01
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 3.30
Tc(MIN.) = 10.15
SUBAREA AREA(ACRES) = 0.70 SUBAREA RUNOFF(CFS) = 0.70
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 0.8 PEAK FLOW RATE(CFS) = 0.79

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.03 FLOW VELOCITY(FEET/SEC.) = 1.30
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 304.00 = 284.00 FEET.

FLOW PROCESS FROM NODE 304.00 TO NODE 312.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3528.00	DOWNSTREAM(FEET) =	3510.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	250.00	CHANNEL SLOPE =	0.0720
CHANNEL BASE(FEET) =	17.00	"Z" FACTOR =	24.000
MANNING'S FACTOR =	0.030	MAXIMUM DEPTH(FEET) =	2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.413		

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.04
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.90
AVERAGE FLOW DEPTH(FEET) = 0.06 TRAVEL TIME(MIN.) = 2.19

Tc(MIN.) = 12.34
SUBAREA AREA(ACRES) = 2.80 SUBAREA RUNOFF(CFS) = 2.47
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 3.6 PEAK FLOW RATE(CFS) = 3.17

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.07 FLOW VELOCITY(FEET/SEC.) = 2.30
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 312.00 = 534.00 FEET.

FLOW PROCESS FROM NODE 304.00 TO NODE 312.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.34
RAINFALL INTENSITY(INCH/HR) = 4.41
TOTAL STREAM AREA(ACRES) = 3.59
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.17

FLOW PROCESS FROM NODE 306.00 TO NODE 308.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 97.00
UPSTREAM ELEVATION(FEET) = 3630.00
DOWNSTREAM ELEVATION(FEET) = 3610.00
ELEVATION DIFFERENCE(FEET) = 20.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.406
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.135
SUBAREA RUNOFF(CFS) = 0.15
TOTAL AREA(ACRES) = 0.12 TOTAL RUNOFF(CFS) = 0.15

FLOW PROCESS FROM NODE 308.00 TO NODE 310.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3610.00 DOWNSTREAM(FEET) = 3584.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 595.00 CHANNEL SLOPE = 0.0437
CHANNEL BASE(FEET) = 9.00 "Z" FACTOR = 5.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.934
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .0300
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.63
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.35

AVERAGE FLOW DEPTH(FEET) = 0.05 TRAVEL TIME(MIN.) = 7.34
Tc(MIN.) = 14.75
SUBAREA AREA(ACRES) = 6.76 SUBAREA RUNOFF(CFS) = 0.80
AREA-AVERAGE RUNOFF COEFFICIENT = 0.033
TOTAL AREA(ACRES) = 6.9 PEAK FLOW RATE(CFS) = 0.89

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 1.50
LONGEST FLOWPATH FROM NODE 306.00 TO NODE 310.00 = 692.00 FEET.

FLOW PROCESS FROM NODE 310.00 TO NODE 311.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3584.00 DOWNSTREAM(FEET) = 3530.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 619.00 CHANNEL SLOPE = 0.0872
CHANNEL BASE(FEET) = 35.00 "Z" FACTOR = 4.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.181

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.07

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.79

AVERAGE FLOW DEPTH(FEET) = 0.05 TRAVEL TIME(MIN.) = 5.75

Tc(MIN.) = 20.50

SUBAREA AREA(ACRES) = 7.19 SUBAREA RUNOFF(CFS) = 4.57

AREA-AVERAGE RUNOFF COEFFICIENT = 0.118

TOTAL AREA(ACRES) = 14.1 PEAK FLOW RATE(CFS) = 5.30

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 2.35
LONGEST FLOWPATH FROM NODE 306.00 TO NODE 311.00 = 1311.00 FEET.

FLOW PROCESS FROM NODE 311.00 TO NODE 312.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3530.00 DOWNSTREAM(FEET) = 3510.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 501.00 CHANNEL SLOPE = 0.0399
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 4.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.008

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.10

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.49

AVERAGE FLOW DEPTH(FEET) = 0.63 TRAVEL TIME(MIN.) = 1.86

Tc(MIN.) = 22.36

SUBAREA AREA(ACRES) = 6.00 SUBAREA RUNOFF(CFS) = 3.61

AREA-AVERAGE RUNOFF COEFFICIENT = 0.143
TOTAL AREA(ACRES) = 20.1 PEAK FLOW RATE(CFS) = 8.62

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.68 FLOW VELOCITY(FEET/SEC.) = 4.71
LONGEST FLOWPATH FROM NODE 306.00 TO NODE 312.00 = 1812.00 FEET.

FLOW PROCESS FROM NODE 311.00 TO NODE 312.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 22.36
RAINFALL INTENSITY(INCH/HR) = 3.01
TOTAL STREAM AREA(ACRES) = 20.07
PEAK FLOW RATE(CFS) AT CONFLUENCE = 8.62

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	3.17	12.34	4.413	3.59
2	8.62	22.36	3.008	20.07

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.93	12.34	4.413
2	10.78	22.36	3.008

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 10.78 Tc(MIN.) = 22.36
TOTAL AREA(ACRES) = 23.7
LONGEST FLOWPATH FROM NODE 306.00 TO NODE 312.00 = 1812.00 FEET.

FLOW PROCESS FROM NODE 312.00 TO NODE 314.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3510.00 DOWNSTREAM(FEET) = 3488.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 645.00 CHANNEL SLOPE = 0.0341
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 25.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.744
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.51

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.13
AVERAGE FLOW DEPTH(FEET) = 0.40 TRAVEL TIME(MIN.) = 3.43
Tc(MIN.) = 25.79
SUBAREA AREA(ACRES) = 6.32 SUBAREA RUNOFF(CFS) = 3.47
AREA-AVERAGE RUNOFF COEFFICIENT = 0.162
TOTAL AREA(ACRES) = 30.0 PEAK FLOW RATE(CFS) = 13.30

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.41 FLOW VELOCITY(FEET/SEC.) = 3.18
LONGEST FLOWPATH FROM NODE 306.00 TO NODE 314.00 = 2457.00 FEET.

FLOW PROCESS FROM NODE 314.00 TO NODE 330.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3488.00 DOWNSTREAM(FEET) = 3472.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 476.00 CHANNEL SLOPE = 0.0336
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 36.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.541

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 14.25

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.44

AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 3.26

Tc(MIN.) = 29.05

SUBAREA AREA(ACRES) = 3.77 SUBAREA RUNOFF(CFS) = 1.92

AREA-AVERAGE RUNOFF COEFFICIENT = 0.166

TOTAL AREA(ACRES) = 33.8 PEAK FLOW RATE(CFS) = 14.23

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.16 FLOW VELOCITY(FEET/SEC.) = 2.43
LONGEST FLOWPATH FROM NODE 306.00 TO NODE 330.00 = 2933.00 FEET.

FLOW PROCESS FROM NODE 314.00 TO NODE 330.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 29.05
RAINFALL INTENSITY(INCH/HR) = 2.54
TOTAL STREAM AREA(ACRES) = 33.75
PEAK FLOW RATE(CFS) AT CONFLUENCE = 14.23

FLOW PROCESS FROM NODE 316.00 TO NODE 318.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 99.00
 UPSTREAM ELEVATION(FEET) = 3580.00
 DOWNSTREAM ELEVATION(FEET) = 3578.00
 ELEVATION DIFFERENCE(FEET) = 2.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 11.836
 WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
 THE MAXIMUM OVERLAND FLOW LENGTH = 85.30
 (Reference: Table 3-1B of Hydrology Manual)
 THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN T_c CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.534
 SUBAREA RUNOFF(CFS) = 0.64
 TOTAL AREA(ACRES) = 0.71 TOTAL RUNOFF(CFS) = 0.64

FLOW PROCESS FROM NODE 318.00 TO NODE 320.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3578.00 DOWNSTREAM(FEET) = 3540.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 363.00 CHANNEL SLOPE = 0.1047
 CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 99.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.770
 *USER SPECIFIED(SUBAREA):
 USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.72
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.54
 AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 3.92
 T_c(MIN.) = 15.76
 SUBAREA AREA(ACRES) = 2.82 SUBAREA RUNOFF(CFS) = 2.13
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
 TOTAL AREA(ACRES) = 3.5 PEAK FLOW RATE(CFS) = 2.66

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.81
 LONGEST FLOWPATH FROM NODE 316.00 TO NODE 320.00 = 462.00 FEET.

FLOW PROCESS FROM NODE 320.00 TO NODE 322.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3540.00 DOWNSTREAM(FEET) = 3496.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 674.00 CHANNEL SLOPE = 0.0653
 CHANNEL BASE(FEET) = 7.00 "Z" FACTOR = 5.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.351
 *USER SPECIFIED(SUBAREA):
 USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.59
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.56
 AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 3.16
 Tc(MIN.) = 18.91
 SUBAREA AREA(ACRES) = 5.75 SUBAREA RUNOFF(CFS) = 3.85
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
 TOTAL AREA(ACRES) = 9.3 PEAK FLOW RATE(CFS) = 6.22

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.20 FLOW VELOCITY(FEET/SEC.) = 3.99
 LONGEST FLOWPATH FROM NODE 316.00 TO NODE 322.00 = 1136.00 FEET.

FLOW PROCESS FROM NODE 322.00 TO NODE 330.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3496.00 DOWNSTREAM(FEET) = 3472.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 686.00 CHANNEL SLOPE = 0.0350
 CHANNEL BASE(FEET) = 15.00 "Z" FACTOR = 22.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.896

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.92

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.38

AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 4.80

Tc(MIN.) = 23.71

SUBAREA AREA(ACRES) = 2.43 SUBAREA RUNOFF(CFS) = 1.41

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 11.7 PEAK FLOW RATE(CFS) = 6.78

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.16 FLOW VELOCITY(FEET/SEC.) = 2.37
 LONGEST FLOWPATH FROM NODE 316.00 TO NODE 330.00 = 1822.00 FEET.

FLOW PROCESS FROM NODE 322.00 TO NODE 330.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 23.71
 RAINFALL INTENSITY(INCH/HR) = 2.90
 TOTAL STREAM AREA(ACRES) = 11.71
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.78

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	14.23	29.05	2.541	33.75

2 6.78 23.71 2.896 11.71

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	19.27	23.71	2.896
2	20.18	29.05	2.541

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 20.18 Tc(MIN.) = 29.05

TOTAL AREA(ACRES) = 45.5

LONGEST FLOWPATH FROM NODE 306.00 TO NODE 330.00 = 2933.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 45.5 TC(MIN.) = 29.05

PEAK FLOW RATE(CFS) = 20.18

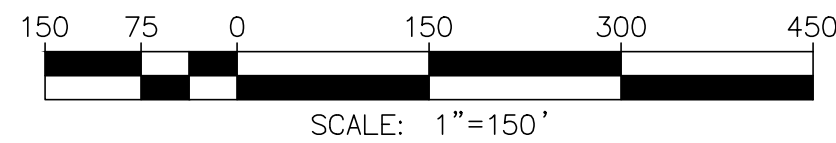
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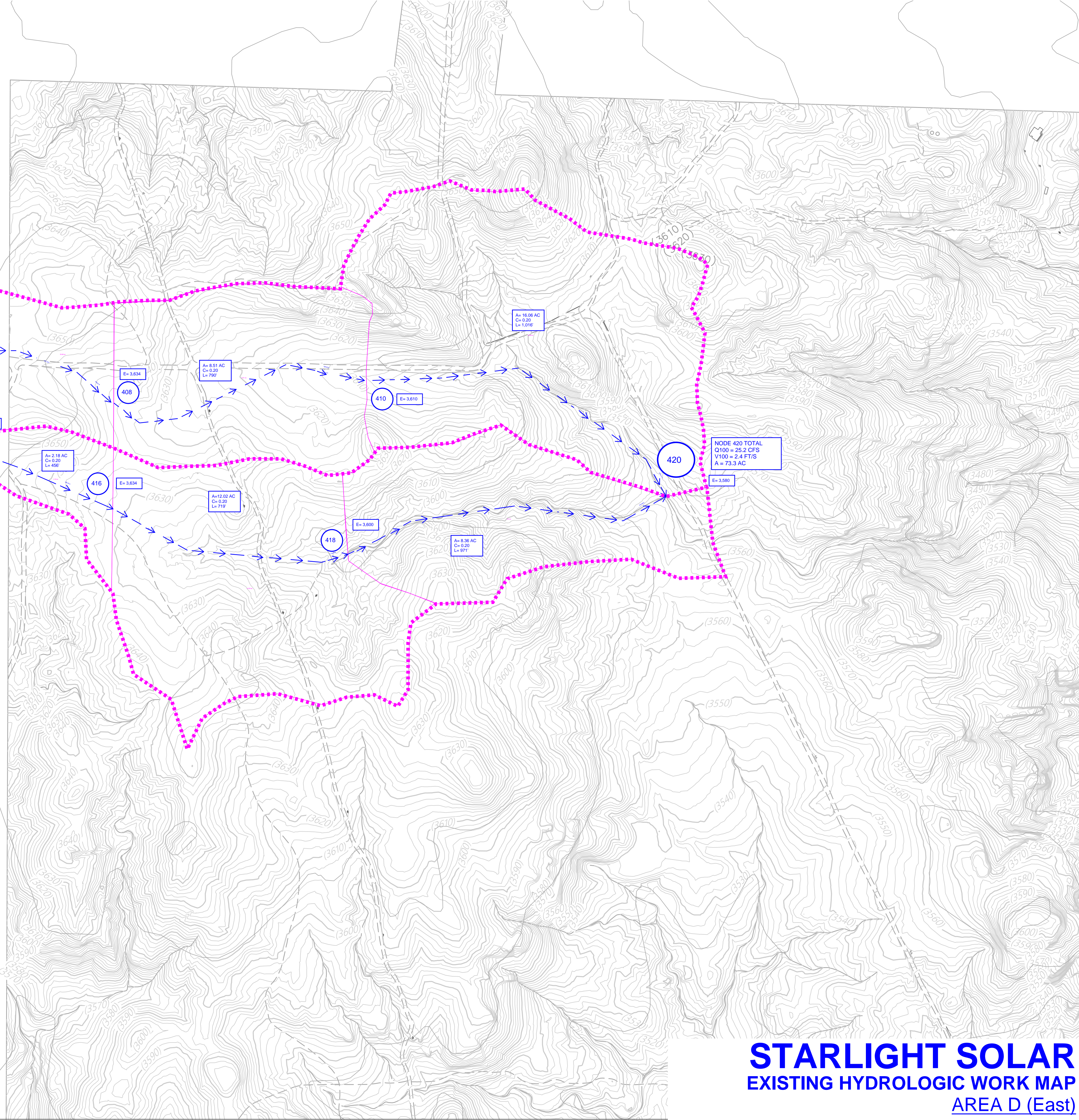
END OF RATIONAL METHOD ANALYSIS

↑

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LEGEND
DRAINAGE BOUNDARY
FLOW PATH
DRAINAGE NODE



STARLIGHT SOLAR
EXISTING HYDROLOGIC WORK MAP
AREA D (East)

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2016 Advanced Engineering Software (aes)
Ver. 23.0 Release Date: 07/01/2016 License ID 1264

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 185614 STARLIGHT SOLAR *
* EX Q100 AREA D (EASTERLY FLOW) *
* 2024-03-01 *

FILE NAME: 614EXD.DAT
TIME/DATE OF STUDY: 08:14 03/02/2024

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: CONSIDER ALL CONFLUENCE STREAM COMBINATIONS
FOR ALL DOWNSTREAM ANALYSES

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL								
	HALF- WIDTH	CROWN TO CROSSFALL	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT	GUTTER-GEOMETRIES: WIDTH	LIP	HIKE	MANNING FACTOR
NO.	(FT)	(FT)		(FT)	(FT)	(FT)	(FT)	(n)
===	=====	=====	=====	=====	=====	=====	=====	=====
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

FLOW PROCESS FROM NODE 400.00 TO NODE 401.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

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*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 96.00
UPSTREAM ELEVATION(FEET) = 3692.00
DOWNSTREAM ELEVATION(FEET) = 3690.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 11.780
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 86.25
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.548
SUBAREA RUNOFF(CFS) = 0.17
TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.17

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FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
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ELEVATION DATA: UPSTREAM(FEET) = 3690.00 DOWNSTREAM(FEET) = 3684.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 265.00 CHANNEL SLOPE = 0.0226
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 99.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.430
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.76
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.68
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 6.46
Tc(MIN.) = 18.24
SUBAREA AREA(ACRES) = 1.66 SUBAREA RUNOFF(CFS) = 1.14
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 1.8 PEAK FLOW RATE(CFS) = 1.27

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END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 0.86
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 402.00 = 361.00 FEET.

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FLOW PROCESS FROM NODE 402.00 TO NODE 404.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====

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ELEVATION DATA: UPSTREAM(FEET) = 3684.00 DOWNSTREAM(FEET) = 3680.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 383.00 CHANNEL SLOPE = 0.0104
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 99.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.735
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000

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S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.79
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.83
AVERAGE FLOW DEPTH(FEET) = 0.09 TRAVEL TIME(MIN.) = 7.68
Tc(MIN.) = 25.92
SUBAREA AREA(ACRES) = 5.45 SUBAREA RUNOFF(CFS) = 2.98
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 7.3 PEAK FLOW RATE(CFS) = 3.99

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.10 FLOW VELOCITY(FEET/SEC.) = 0.95
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 404.00 = 744.00 FEET.

FLOW PROCESS FROM NODE 404.00 TO NODE 406.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3680.00 DOWNSTREAM(FEET) = 3660.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 983.00 CHANNEL SLOPE = 0.0203
CHANNEL BASE(FEET) = 43.00 "Z" FACTOR = 99.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.119

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.45

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.30

AVERAGE FLOW DEPTH(FEET) = 0.09 TRAVEL TIME(MIN.) = 12.56

Tc(MIN.) = 38.48

SUBAREA AREA(ACRES) = 11.55 SUBAREA RUNOFF(CFS) = 4.90

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 18.9 PEAK FLOW RATE(CFS) = 7.99

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.10 FLOW VELOCITY(FEET/SEC.) = 1.44

LONGEST FLOWPATH FROM NODE 400.00 TO NODE 406.00 = 1727.00 FEET.

FLOW PROCESS FROM NODE 406.00 TO NODE 408.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3660.00 DOWNSTREAM(FEET) = 3634.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 660.00 CHANNEL SLOPE = 0.0394
CHANNEL BASE(FEET) = 17.00 "Z" FACTOR = 16.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.991

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.43

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.80

AVERAGE FLOW DEPTH(FEET) = 0.17 TRAVEL TIME(MIN.) = 3.93
Tc(MIN.) = 42.41
SUBAREA AREA(ACRES) = 7.24 SUBAREA RUNOFF(CFS) = 2.88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 26.1 PEAK FLOW RATE(CFS) = 10.39

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.18 FLOW VELOCITY(FEET/SEC.) = 2.90
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 408.00 = 2387.00 FEET.

FLOW PROCESS FROM NODE 408.00 TO NODE 410.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3634.00 DOWNSTREAM(FEET) = 3610.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 790.00 CHANNEL SLOPE = 0.0304
CHANNEL BASE(FEET) = 15.00 "Z" FACTOR = 30.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.850

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.96

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.57

AVERAGE FLOW DEPTH(FEET) = 0.22 TRAVEL TIME(MIN.) = 5.12

Tc(MIN.) = 47.53

SUBAREA AREA(ACRES) = 8.51 SUBAREA RUNOFF(CFS) = 3.15

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 34.6 PEAK FLOW RATE(CFS) = 12.80

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.22 FLOW VELOCITY(FEET/SEC.) = 2.63

LONGEST FLOWPATH FROM NODE 400.00 TO NODE 410.00 = 3177.00 FEET.

FLOW PROCESS FROM NODE 410.00 TO NODE 420.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3610.00 DOWNSTREAM(FEET) = 3580.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1016.00 CHANNEL SLOPE = 0.0295
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 25.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 1.719

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 15.56

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.96

AVERAGE FLOW DEPTH(FEET) = 0.30 TRAVEL TIME(MIN.) = 5.72

Tc(MIN.) = 53.25

SUBAREA AREA(ACRES) = 16.06 SUBAREA RUNOFF(CFS) = 5.52

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 50.7 PEAK FLOW RATE(CFS) = 17.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.32 FLOW VELOCITY(FEET/SEC.) = 3.08
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 420.00 = 4193.00 FEET.

FLOW PROCESS FROM NODE 410.00 TO NODE 420.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 53.25
RAINFALL INTENSITY(INCH/HR) = 1.72
TOTAL STREAM AREA(ACRES) = 50.66
PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.41

FLOW PROCESS FROM NODE 412.00 TO NODE 414.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 90.00
UPSTREAM ELEVATION(FEET) = 3660.00
DOWNSTREAM ELEVATION(FEET) = 3658.00
ELEVATION DIFFERENCE(FEET) = 2.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 11.668
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 88.33
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.576
SUBAREA RUNOFF(CFS) = 0.18
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.18

FLOW PROCESS FROM NODE 414.00 TO NODE 416.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3658.00 DOWNSTREAM(FEET) = 3634.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 456.00 CHANNEL SLOPE = 0.0526
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.350
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.89

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.05
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 7.25
Tc(MIN.) = 18.92
SUBAREA AREA(ACRES) = 2.07 SUBAREA RUNOFF(CFS) = 1.39
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 2.3 PEAK FLOW RATE(CFS) = 1.52

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.04 FLOW VELOCITY(FEET/SEC.) = 1.20
LONGEST FLOWPATH FROM NODE 412.00 TO NODE 416.00 = 546.00 FEET.

FLOW PROCESS FROM NODE 416.00 TO NODE 418.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3634.00 DOWNSTREAM(FEET) = 3600.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 719.00 CHANNEL SLOPE = 0.0473
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.847

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.96

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.20

AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 5.44

Tc(MIN.) = 24.36

SUBAREA AREA(ACRES) = 12.02 SUBAREA RUNOFF(CFS) = 6.84

AREA-AVERAGE RUNOFF COEFFICIENT = 0.200

TOTAL AREA(ACRES) = 14.3 PEAK FLOW RATE(CFS) = 8.14

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.13 FLOW VELOCITY(FEET/SEC.) = 2.67

LONGEST FLOWPATH FROM NODE 412.00 TO NODE 418.00 = 1265.00 FEET.

FLOW PROCESS FROM NODE 418.00 TO NODE 420.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3600.00 DOWNSTREAM(FEET) = 3580.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 971.00 CHANNEL SLOPE = 0.0206
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 30.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.426

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.17

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.36

AVERAGE FLOW DEPTH(FEET) = 0.38 TRAVEL TIME(MIN.) = 6.85

Tc(MIN.) = 31.20

SUBAREA AREA(ACRES) = 8.36 SUBAREA RUNOFF(CFS) = 4.06
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 22.7 PEAK FLOW RATE(CFS) = 10.99

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.39 FLOW VELOCITY(FEET/SEC.) = 2.38
LONGEST FLOWPATH FROM NODE 412.00 TO NODE 420.00 = 2236.00 FEET.

FLOW PROCESS FROM NODE 418.00 TO NODE 420.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 31.20
RAINFALL INTENSITY(INCH/HR) = 2.43
TOTAL STREAM AREA(ACRES) = 22.65
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.99

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	17.41	53.25	1.719	50.66
2	10.99	31.20	2.426	22.65

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	21.19	31.20	2.426
2	25.20	53.25	1.719

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 25.20 Tc(MIN.) = 53.25
TOTAL AREA(ACRES) = 73.3
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 420.00 = 4193.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES)	=	73.3	TC(MIN.) =	53.25
PEAK FLOW RATE(CFS)	=	25.20		

NODE 420

*** PEAK FLOW RATE TABLE ***

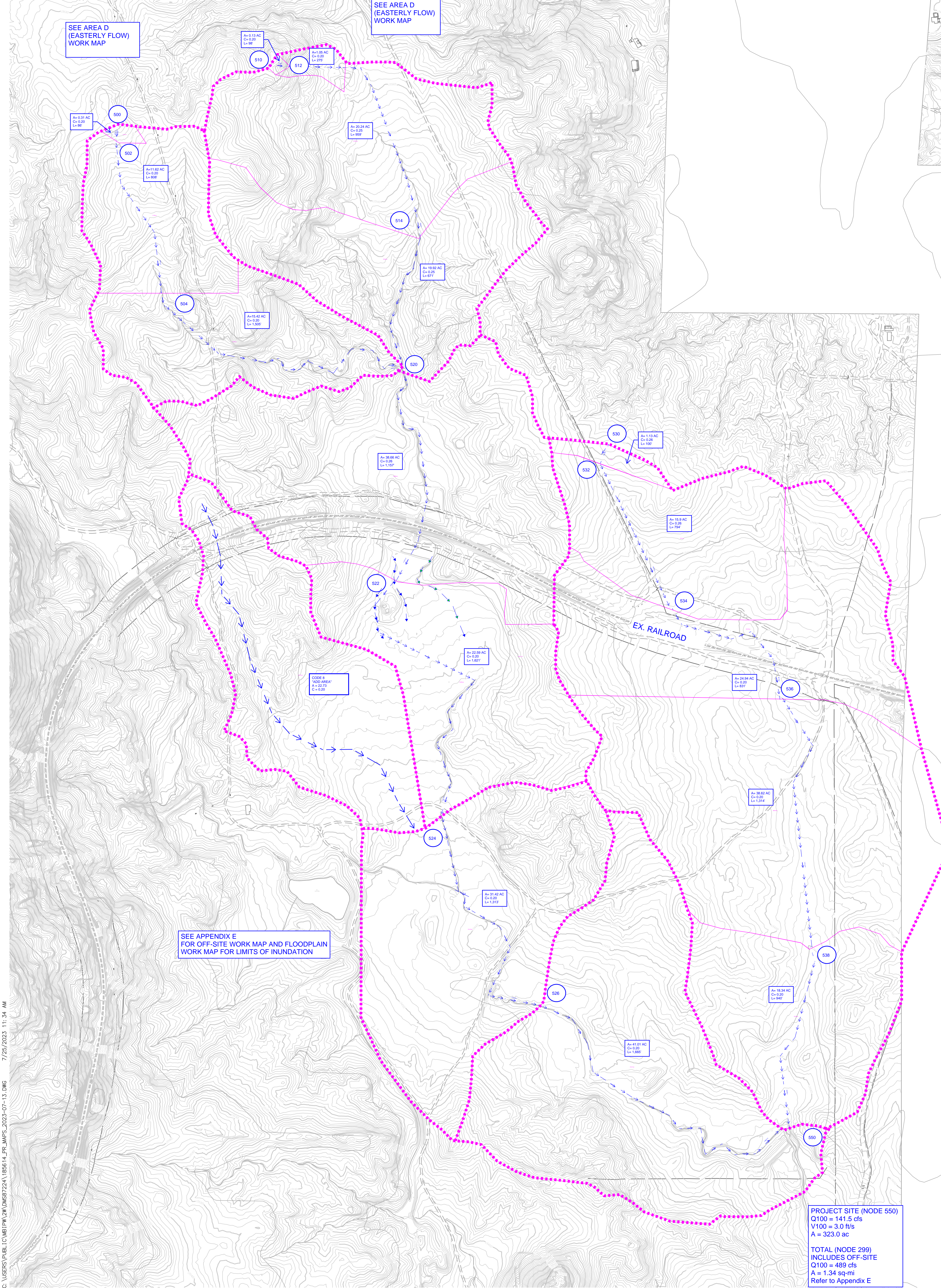
	Q(CFS)	Tc(MIN.)
1	21.19	31.20
2	25.20	53.25

=====

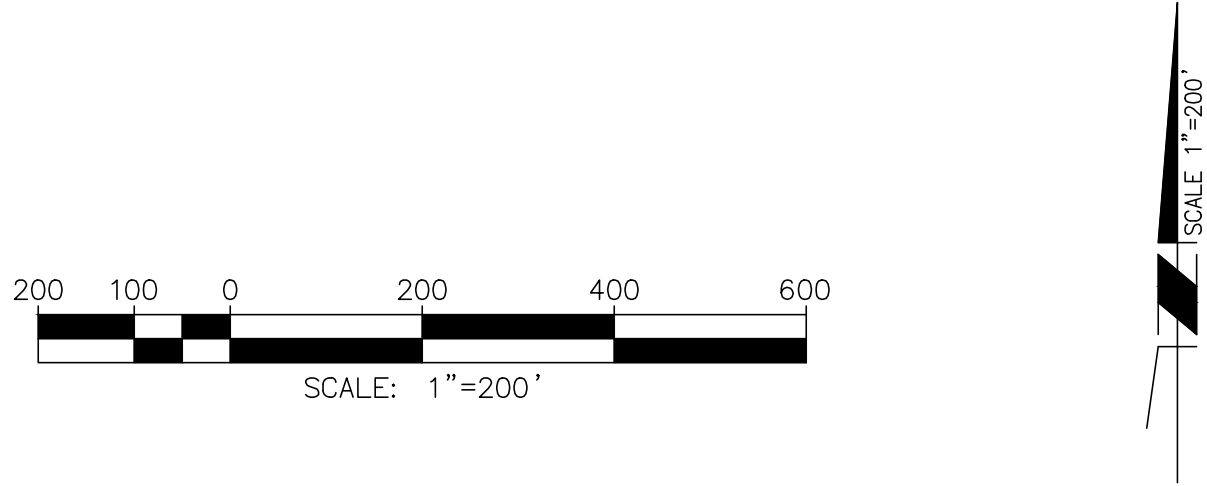
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END OF RATIONAL METHOD ANALYSIS





C:\USERS\PUBLIC\MBI\PI\2\1\DM8722A\185614_FIR_MAPS_2023-07-13.DWG 7/25/2023 11:34 AM



STARLIGHT SOLAR

EXISTING HYDROLOGIC WORK MAP

AREA D (South) and AREA'S E, F, G AND H

PROJECT SITE (NODE 550)
Q100 = 141.5 cfs
V100 = 3.0 ft/s
A = 323.0 ac

TOTAL (NODE 299)
INCLUDES OFF-SITE
Q100 = 489 cfs
A = 1.34 sq-mi
Refer to Appendix E

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003,1985,1981 HYDROLOGY MANUAL
(c) Copyright 1982-2016 Advanced Engineering Software (aes)
Ver. 23.0 Release Date: 07/01/2016 License ID 1264

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* 185614 STARLIGHT SOLAR *
* EX Q100 ONSITE AREAS D (SOUTHERLY FLOW) - H *
* 2024-03-01 *

FILE NAME: 614EXDH.DAT
TIME/DATE OF STUDY: 08:22 03/02/2024

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
=== =====
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

FLOW PROCESS FROM NODE 500.00 TO NODE 502.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 86.00
UPSTREAM ELEVATION(FEET) = 3646.00
DOWNSTREAM ELEVATION(FEET) = 3640.00
ELEVATION DIFFERENCE(FEET) = 6.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.863
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.903
SUBAREA RUNOFF(CFS) = 0.37
TOTAL AREA(ACRES) = 0.31 TOTAL RUNOFF(CFS) = 0.37

FLOW PROCESS FROM NODE 502.00 TO NODE 504.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3640.00	DOWNSTREAM(FEET) =	3590.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	808.00	CHANNEL SLOPE =	0.0619
CHANNEL BASE(FEET) =	8.00	"Z" FACTOR =	15.000
MANNING'S FACTOR = 0.030	MAXIMUM DEPTH(FEET) =	2.00	
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.505		

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.69
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.29
AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 4.09
Tc(MIN.) = 11.95
SUBAREA AREA(ACRES) = 11.62 SUBAREA RUNOFF(CFS) = 10.47
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 11.9 PEAK FLOW RATE(CFS) = 10.75

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.23 FLOW VELOCITY(FEET/SEC.) = 4.00
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 504.00 = 894.00 FEET.

FLOW PROCESS FROM NODE 504.00 TO NODE 520.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3590.00	DOWNSTREAM(FEET) =	3500.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1505.00	CHANNEL SLOPE =	0.0598
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	10.000
MANNING'S FACTOR = 0.030	MAXIMUM DEPTH(FEET) =	2.00	
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	3.618		

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 16.35
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.18
AVERAGE FLOW DEPTH(FEET) = 0.56 TRAVEL TIME(MIN.) = 4.84
Tc(MIN.) = 16.79

SUBAREA AREA(ACRES) = 15.42 SUBAREA RUNOFF(CFS) = 11.16
AREA-AVERAGE RUNOFF COEFFICIENT = 0.200
TOTAL AREA(ACRES) = 27.4 PEAK FLOW RATE(CFS) = 19.79

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.61 FLOW VELOCITY(FEET/SEC.) = 5.40
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 520.00 = 2399.00 FEET.

FLOW PROCESS FROM NODE 504.00 TO NODE 520.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 16.79
RAINFALL INTENSITY(INCH/HR) = 3.62
TOTAL STREAM AREA(ACRES) = 27.35
PEAK FLOW RATE(CFS) AT CONFLUENCE = 19.79

FLOW PROCESS FROM NODE 510.00 TO NODE 512.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 98.00
UPSTREAM ELEVATION(FEET) = 3624.00
DOWNSTREAM ELEVATION(FEET) = 3606.00
ELEVATION DIFFERENCE(FEET) = 18.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.444
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.115
SUBAREA RUNOFF(CFS) = 0.16
TOTAL AREA(ACRES) = 0.13 TOTAL RUNOFF(CFS) = 0.16

FLOW PROCESS FROM NODE 512.00 TO NODE 514.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3606.00 DOWNSTREAM(FEET) = 3524.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 959.00 CHANNEL SLOPE = 0.0855
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.784
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2500
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.24
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.64
AVERAGE FLOW DEPTH(FEET) = 0.36 TRAVEL TIME(MIN.) = 3.45

Tc(MIN.) = 10.89
SUBAREA AREA(ACRES) = 20.24 SUBAREA RUNOFF(CFS) = 24.21
AREA-AVERAGE RUNOFF COEFFICIENT = 0.250
TOTAL AREA(ACRES) = 20.4 PEAK FLOW RATE(CFS) = 24.33

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.47 FLOW VELOCITY(FEET/SEC.) = 5.50
LONGEST FLOWPATH FROM NODE 510.00 TO NODE 514.00 = 1057.00 FEET.

FLOW PROCESS FROM NODE 514.00 TO NODE 520.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3524.00 DOWNSTREAM(FEET) = 3500.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 671.00 CHANNEL SLOPE = 0.0358
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 40.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.079

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2500
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 34.51
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.66
AVERAGE FLOW DEPTH(FEET) = 0.49 TRAVEL TIME(MIN.) = 3.05
Tc(MIN.) = 13.95
SUBAREA AREA(ACRES) = 19.92 SUBAREA RUNOFF(CFS) = 20.31
AREA-AVERAGE RUNOFF COEFFICIENT = 0.250
TOTAL AREA(ACRES) = 40.3 PEAK FLOW RATE(CFS) = 41.06

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.52 FLOW VELOCITY(FEET/SEC.) = 3.77
LONGEST FLOWPATH FROM NODE 510.00 TO NODE 520.00 = 1728.00 FEET.

FLOW PROCESS FROM NODE 514.00 TO NODE 520.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 13.95
RAINFALL INTENSITY(INCH/HR) = 4.08
TOTAL STREAM AREA(ACRES) = 40.29
PEAK FLOW RATE(CFS) AT CONFLUENCE = 41.06

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	19.79	16.79	3.618	27.35
2	41.06	13.95	4.079	40.29

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	57.49	13.95	4.079
2	56.21	16.79	3.618

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 57.49 Tc(MIN.) = 13.95

TOTAL AREA(ACRES) = 67.6

LONGEST FLOWPATH FROM NODE 500.00 TO NODE 520.00 = 2399.00 FEET.

FLOW PROCESS FROM NODE 520.00 TO NODE 522.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3500.00 DOWNSTREAM(FEET) = 3484.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 1157.00 CHANNEL SLOPE = 0.0138

CHANNEL BASE(FEET) = 13.00 "Z" FACTOR = 10.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.386

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2600

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 74.51

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.13

AVERAGE FLOW DEPTH(FEET) = 0.84 TRAVEL TIME(MIN.) = 4.67

Tc(MIN.) = 18.61

SUBAREA AREA(ACRES) = 38.66 SUBAREA RUNOFF(CFS) = 34.03

AREA-AVERAGE RUNOFF COEFFICIENT = 0.241

TOTAL AREA(ACRES) = 106.3 PEAK FLOW RATE(CFS) = 86.63

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.91 FLOW VELOCITY(FEET/SEC.) = 4.34

LONGEST FLOWPATH FROM NODE 500.00 TO NODE 522.00 = 3556.00 FEET.

FLOW PROCESS FROM NODE 522.00 TO NODE 524.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3484.00 DOWNSTREAM(FEET) = 3450.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 1621.00 CHANNEL SLOPE = 0.0210

CHANNEL BASE(FEET) = 11.00 "Z" FACTOR = 16.000

MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.846

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 93.06

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.70

AVERAGE FLOW DEPTH(FEET) = 0.82 TRAVEL TIME(MIN.) = 5.75
Tc(MIN.) = 24.37
SUBAREA AREA(ACRES) = 22.59 SUBAREA RUNOFF(CFS) = 12.86
AREA-AVERAGE RUNOFF COEFFICIENT = 0.234
TOTAL AREA(ACRES) = 128.9 PEAK FLOW RATE(CFS) = 86.63

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.79 FLOW VELOCITY(FEET/SEC.) = 4.63
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 524.00 = 5177.00 FEET.

FLOW PROCESS FROM NODE 524.00 TO NODE 524.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.846
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.2285
SUBAREA AREA(ACRES) = 22.73 SUBAREA RUNOFF(CFS) = 12.94
TOTAL AREA(ACRES) = 151.6 TOTAL RUNOFF(CFS) = 98.61
TC(MIN.) = 24.37

FLOW PROCESS FROM NODE 524.00 TO NODE 526.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3450.00 DOWNSTREAM(FEET) = 3430.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1313.00 CHANNEL SLOPE = 0.0152
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.440
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 106.28
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.34
AVERAGE FLOW DEPTH(FEET) = 0.80 TRAVEL TIME(MIN.) = 6.56
Tc(MIN.) = 30.93
SUBAREA AREA(ACRES) = 31.42 SUBAREA RUNOFF(CFS) = 15.33
AREA-AVERAGE RUNOFF COEFFICIENT = 0.224
TOTAL AREA(ACRES) = 183.0 PEAK FLOW RATE(CFS) = 99.89

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.78 FLOW VELOCITY(FEET/SEC.) = 3.27
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 526.00 = 6490.00 FEET.

FLOW PROCESS FROM NODE 526.00 TO NODE 550.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 3430.00 DOWNSTREAM(FEET) = 3420.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1665.00 CHANNEL SLOPE = 0.0060
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 40.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.000
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2000
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 108.10
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.49
AVERAGE FLOW DEPTH(FEET) = 1.04 TRAVEL TIME(MIN.) = 11.16
Tc(MIN.) = 42.08
SUBAREA AREA(ACRES) = 41.01 SUBAREA RUNOFF(CFS) = 16.41
AREA-AVERAGE RUNOFF COEFFICIENT = 0.219
TOTAL AREA(ACRES) = 224.0 PEAK FLOW RATE(CFS) = 99.89

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 1.01 FLOW VELOCITY(FEET/SEC.) = 2.44
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 550.00 = 8155.00 FEET.

*****
FLOW PROCESS FROM NODE 526.00 TO NODE 550.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 42.08
RAINFALL INTENSITY(INCH/HR) = 2.00
TOTAL STREAM AREA(ACRES) = 224.05
PEAK FLOW RATE(CFS) AT CONFLUENCE = 99.89

*****
FLOW PROCESS FROM NODE 530.00 TO NODE 532.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
USER-SPECIFIED RUNOFF COEFFICIENT = .2600
S.C.S. CURVE NUMBER (AMC II) = 0
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3552.00
DOWNSTREAM ELEVATION(FEET) = 3518.00
ELEVATION DIFFERENCE(FEET) = 34.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.019
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.351
SUBAREA RUNOFF(CFS) = 1.87
TOTAL AREA(ACRES) = 1.13 TOTAL RUNOFF(CFS) = 1.87

*****
FLOW PROCESS FROM NODE 532.00 TO NODE 534.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

```


>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3518.00 DOWNSTREAM(FEET) = 3480.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 794.00 CHANNEL SLOPE = 0.0479
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.805

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2600

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.94

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.48

AVERAGE FLOW DEPTH(FEET) = 0.23 TRAVEL TIME(MIN.) = 3.80

Tc(MIN.) = 10.82

SUBAREA AREA(ACRES) = 15.90 SUBAREA RUNOFF(CFS) = 19.86

AREA-AVERAGE RUNOFF COEFFICIENT = 0.260

TOTAL AREA(ACRES) = 17.0 PEAK FLOW RATE(CFS) = 21.27

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.32 FLOW VELOCITY(FEET/SEC.) = 4.10

LONGEST FLOWPATH FROM NODE 530.00 TO NODE 534.00 = 894.00 FEET.

FLOW PROCESS FROM NODE 534.00 TO NODE 536.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3480.00 DOWNSTREAM(FEET) = 3456.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 831.00 CHANNEL SLOPE = 0.0289
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 35.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.909

*USER SPECIFIED(SUBAREA):

USER-SPECIFIED RUNOFF COEFFICIENT = .2000

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 31.08

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.40

AVERAGE FLOW DEPTH(FEET) = 0.47 TRAVEL TIME(MIN.) = 4.08

Tc(MIN.) = 14.89

SUBAREA AREA(ACRES) = 24.94 SUBAREA RUNOFF(CFS) = 19.50

AREA-AVERAGE RUNOFF COEFFICIENT = 0.224

TOTAL AREA(ACRES) = 42.0 PEAK FLOW RATE(CFS) = 36.81

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.51 FLOW VELOCITY(FEET/SEC.) = 3.51

LONGEST FLOWPATH FROM NODE 530.00 TO NODE 536.00 = 1725.00 FEET.

FLOW PROCESS FROM NODE 536.00 TO NODE 538.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 3456.00 DOWNSTREAM(FEET) = 3440.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 1314.00 CHANNEL SLOPE = 0.0122
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 80.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.814
 *USER SPECIFIED(SUBAREA):
 USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 47.76
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.21
 AVERAGE FLOW DEPTH(FEET) = 0.46 TRAVEL TIME(MIN.) = 9.89
 Tc(MIN.) = 24.79
 SUBAREA AREA(ACRES) = 38.62 SUBAREA RUNOFF(CFS) = 21.74
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.213
 TOTAL AREA(ACRES) = 80.6 PEAK FLOW RATE(CFS) = 48.24

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.46 FLOW VELOCITY(FEET/SEC.) = 2.22
 LONGEST FLOWPATH FROM NODE 530.00 TO NODE 538.00 = 3039.00 FEET.

FLOW PROCESS FROM NODE 538.00 TO NODE 550.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 3440.00 DOWNSTREAM(FEET) = 3420.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 940.00 CHANNEL SLOPE = 0.0213
 CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 60.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.490
 *USER SPECIFIED(SUBAREA):
 USER-SPECIFIED RUNOFF COEFFICIENT = .2000
 S.C.S. CURVE NUMBER (AMC II) = 0
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 52.80
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.02
 AVERAGE FLOW DEPTH(FEET) = 0.46 TRAVEL TIME(MIN.) = 5.18
 Tc(MIN.) = 29.97
 SUBAREA AREA(ACRES) = 18.34 SUBAREA RUNOFF(CFS) = 9.13
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.210
 TOTAL AREA(ACRES) = 98.9 PEAK FLOW RATE(CFS) = 51.82

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.46 FLOW VELOCITY(FEET/SEC.) = 2.99
 LONGEST FLOWPATH FROM NODE 530.00 TO NODE 550.00 = 3979.00 FEET.

FLOW PROCESS FROM NODE 538.00 TO NODE 550.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

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TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 29.97
 RAINFALL INTENSITY(INCH/HR) = 2.49

TOTAL STREAM AREA(ACRES) = 98.93
PEAK FLOW RATE(CFS) AT CONFLUENCE = 51.82

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	99.89	42.08	2.000	224.05
2	51.82	29.97	2.490	98.93

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	132.06	29.97	2.490
2	141.52	42.08	2.000

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 141.52 Tc(MIN.) = 42.08

TOTAL AREA(ACRES) = 323.0

LONGEST FLOWPATH FROM NODE 500.00 TO NODE 550.00 = 8155.00 FEET.

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 323.0 TC(MIN.) = 42.08
PEAK FLOW RATE(CFS) = 141.52

NODE 550

END OF RATIONAL METHOD ANALYSIS

