

ESCONDIDO ESTATES

SAN DIEGO COUNTY, CALIFORNIA

PDS2020-TM-5639

PRELIMINARY HYDROLOGY REPORT

PREPARED FOR:

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Date Prepared: 12.2020

Date Revised: 06/2021

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Project Number: 108-001



ENGINEERING & CONSULTING

This report was prepared by or under the supervision of the undersigned registered civil engineer who attests to the technical information contained herein. The registered civil engineer has also judged the qualifications of any technical specialists providing engineering data upon which recommendations, conclusions, and decisions are based.

I hereby declare that I am the Engineer of Work for this project, that I have exercised responsible charge over the design of the project as defined in Section 6703 of the Business and Professions code, and that the design is consistent with current standards.

I understand that the check of project drawings and specifications by the County of San Diego is confined to a review only and does not relieve me, as engineer of work, of my responsibilities for project design.



Eric Lissner, P.E.

6/29/2021

Date

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I. INTRODUCTION

a. PROJECT SITE DESCRIPTION

The subject site is located at the southeast corner of the intersection of San Pasqual Valley Road and Idaho Avenue in an unincorporated area southeast of the City of Escondido in the County of San Diego, California. Refer to section 1.c for the vicinity map. The site measures approximately 700' along the frontage of San Pasqual Valley Road and extends approximately 910' along the frontage of Idaho Avenue, consisting of 10.10 acres of undeveloped, vacant land covered with vegetation. The trapezoid-shaped site is bounded by Idaho Avenue to the north, San Pasqual Valley Road to the west, a nursery to the south, and developed residential properties to the east. An ephemeral drainage course, which is overlain by a County of San Diego Walnut Woodlands biological designation, runs along San Pasqual Valley Road within the property boundary. The area that comprises the drainage course and the walnut woodlands is proposed to be a lettered lot and is referred to hereon as Lot 'B.'

The land developer is proposing a residential community consisting of 20 detached single-family lots with associated infrastructure over approximately 9.84 acres within an existing parcel (APN 234-231-0100). This report is intended to accompany the entitlement document submittals for the proposed project.

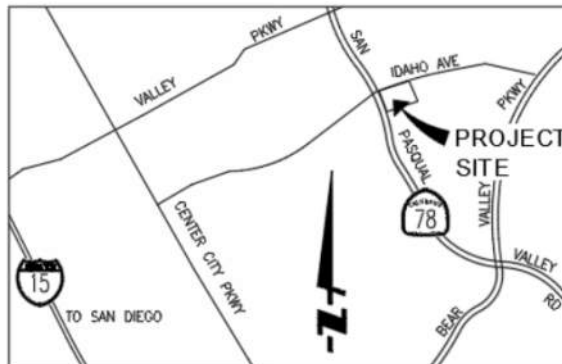
b. PURPOSE AND SCOPE

The purpose of this preliminary study is to analyze the pre-and post-development drainage conditions in order to provide adequate drainage facilities for the proposed development project and minimize impact to downstream flood control facilities and properties.

The scope of this study includes:

1. Determination of offsite and onsite points of flow concentration
2. Existing condition 100-year storm event rational method analysis
3. Proposed condition 100-year storm event rational method analysis
4. Layout and preliminary design of storm drain facilities
5. Peak storm detention/flood attenuation calculations

c. PROJECT LOCATION MAP



II. EXISTING TOPOGRAPHIC AND HYDROLOGIC CONDITIONS

a. EXISTING TOPOGRAPHY

The project site is located on fallow, vacant land with existing vegetation that can be categorized per San Diego County Resource Protection Ordinance as agricultural, walnut woodlands, and eucalyptus woodlands. The site is characterized by modest hillside terrain, sloping downhill in a westerly direction. Elevations within the project boundary range from approximately 687' to approximately 750' above MSL. Existing onsite vegetation consists primarily of grass, nonnative wild plants, and shrubs covering the majority of the 10.10 acre site; however, an ephemeral drainage course is marked by large, mature trees. Based on Google Earth aerial imagery dating back to October 1995, it appears that the vegetation is maintained and cleared periodically, leaving fallow land as the primary ground cover. For the purposes of this hydrologic analysis, it is assumed that the ground cover is primarily unvegetated in order to assess the storm event which produces the most runoff under parameters outlined in Section III.b.

According to the NRCS Web Soil Survey, approximately 90% of the site consists of Fallbrook-Vista sandy loams and the remaining 10% is made up of Ramona sandy loam, both of which are classified as hydrologic soil group C.

The subject site falls in the Hodges hydrological area, located within the San Dieguito River watershed. Runoff generated in the existing condition exits the site via a 8'W x 2'H box culvert at the eastern project boundary, crossing San Pasqual Valley Road to an unnamed natural drainage course that discharges into Lake Hodges, approximately 4 miles south of the project, where it joins the San Dieguito River, ultimately discharging into the Pacific Ocean north of Del Mar and south of Solana Beach.

b. EXISTING DRAINAGE PATTERN

Approximately 57.33 acres of offsite area is tributary to the proposed site, as shown in the existing condition hydrology maps, Figures 1 & 2 in Appendix A. Two primary offsite tributary areas are identified; one north of Idaho Avenue as shown in Figure 2 in Appendix A, and

the other along the eastern property boundary, as shown in Figure 1 in Appendix A. The former larger offsite tributary area is bound by San Pasqual Valley Road to the west, residential properties north of Birch Avenue to the north, and Quartz Hill Lane to the east. This area is comprised primarily of fallow, undeveloped land in a similar condition to that of the subject property, as well as single-family detached lots along the western edge of the watershed fronting San Pasqual Valley Road and the northern edge fronting Birch Avenue, totalling approximately 57.33 acres. Runoff from this offsite tributary area sheetflows in a southerly direction to an existing 30" corrugated metal pipe (CMP) just north of Idaho Avenue, where it outlets into the drainage course onsite. The CMP meters the peak flow from this tributary area.

The latter offsite tributary is made up of approximately 4.76 acres east of the property boundary. The runoff from this area emanates from the residential development bordering the property to the east, sheetflowing onto the property, joining the onsite existing drainage pattern which ultimately joins the ephemeral drainage course, discharging to a 8W' x 2H' box culvert west of the property boundary.

Onsite storm runoff tends to surface flow in a westerly direction toward the ephemeral drainage course, exiting the site to a 8'W x 2'H box culvert just west of the property boundary.

c. EXISTING STORM DRAIN FACILITIES

As discussed in section II.a, the subject site is located on fallow, undeveloped land. There are no known private existing storm drain facilities except as discussed below.

Offsite, as mentioned in section II.b, the 30" CMP north of Idaho Avenue accepts drainage from approximately 57.33 acres of offsite tributary area and outlets into the ephemeral drainage course, discharging to a box culvert west of the project boundary. Per the City of Escondido Drainage Master Plan (November 1995), the box culvert (KC-23 #883) has dimensions of 10'W x 4'H and a Q_c of 662cfs. However, field measurements show the box culvert is 8'W x 2'H, indicating the Q_c from the Drainage Master Plan is likely inaccurate.

III. HYDROLOGIC ANALYSIS

a. STORM FREQUENCY

This report will analyze the 100-year storm event in the existing and proposed conditions for drainage purposes per the San Diego County Hydrology Manual requiring 100-year storm analysis.

b. METHODOLOGY

The study site falls under the jurisdiction of the County of San Diego, which accepts methodologies and practices as described in the San Diego County Hydrology Manual dated June, 2003.

The computer program used for this report to perform hydrologic calculations is the CivilCadd/Civil Design (CivilD) Engineering Software, Version 9.0 program packaged in 2014 by Bondiman and Associates, Inc. CivilD's Rational Method Program was used to determine all runoff in this report. This software has been identified as acceptable software within the County of San Diego. Calculations for the analyses are provided in Appendix C.

c. EXISTING CONDITION

In the existing condition, approximately 40.9 cfs of offsite runoff enters the project site at the eastern boundary due to metered flow provided by a 30" CMP in Idaho Avenue. Storm flows continue to flow in a westerly direction, as discussed in section II.b to the ephemeral drainage course, confluent with the offsite flows from the tributary area north of Idaho Avenue, exiting the site via the 8'W x 2'H box culvert west of the project. The runoff quantities generated from the 100-year storm event in the existing condition utilizing county 6-hour and 24-hour isopleth maps are tabulated in Table III.1 below. Refer to Figure 1 in Appendix A.

Table III.1 Existing Condition Runoff

Existing Condition 100-Year Storm		
Node	Q ₁₀₀ (cfs)	Area (AC)
104	45.7	60.87
107	70.1	63.10
110	0.5	0.24
114	81.7	74.01

Table III.2 Existing Condition Initial Areas

Existing Condition 100-Year Storm				
Subarea Designation	Q ₁₀₀ (cfs)	Area (AC)	Initial Time of Concentration (min)	Maximum Overland Flow Distance (ft)
A-1	0.3	0.14	6.19	90
A-6	0.1	0.05	8.78	95
A-9	0.2	0.07	5.85	95
A-12	0.2	0.09	7.23	100

Table III.3 Existing Condition Point of Discharge

Existing Condition 100-Year Storm					
Node	Q ₁₀₀ (cfs)	Total Area (AC)	Velocity (ft/s)	Intensity (in/hr)	Time of Concentration (min)
114	81.7	74.01	2.3	4.1	19.57

Table III.4 Existing Condition Runoff Coefficients

Existing Condition 100-Year Storm		
Subarea Designation	Area (AC)	Runoff Coefficient
A-1	0.14	0.300
A-2	0.75	0.300
A-3	2.38	0.300
A-4	0.27	0.339
A-5	1.60	0.302
A-6	0.05	0.300
A-7	0.58	0.300
A-8	0.21	0.302
A-9	0.07	0.300
A-10	0.17	0.300
A-11	5.46	0.300
A-12	0.09	0.300
A-13	3.37	0.300
A-14	1.54	0.300

Tory-Walker Engineering performed a HEC-RAS analysis in order to assess the existing condition floodplain limits of the Walnut Woodlands drainage course in support of this hydrology report. Their analysis was performed using larger peak storm flows than what would be generated in the existing condition presented in this report in Appendix C. Given their analysis, no structures would be placed within the 100-year flood hazard area.

d. PROPOSED CONDITION

The proposed development is comprised of 20 single family units, required public and private roadway improvements, homeowner septic fields, and drainage/water quality facilities.

The proposed condition overall drainage patterns will generally mimic the existing drainage patterns, with all onsite storm runoff emanating from the east entering storm drain facilities via street flow. A multipurpose basin is proposed in Lot 'C', providing for water quality treatment, hydromodification mitigation, and peak flow attenuation. All onsite flows will be routed to the proposed basin, outletting to the existing 8'W x 2'H culvert as in the existing condition. Offsite runoff to the project from the property to the south and east will enter a proposed brow ditch along the perimeter walls of the project, discharging to the proposed basin. Flows from a significant portion of Idaho Avenue fronting the project are proposed to enter an inlet and discharge to the proposed basin via a privately maintained storm drain system onsite. Impervious area within the proposed condition is generally comprised of rooftops, driveways, sidewalks, and minimal residential hardscaping such as patios. The remaining area within the subject property is anticipated to be pervious, including landscaped residential yards, septic fields, the proposed basin, and Lot 'B.'

The proposed site development was analyzed using the county's rational method, which was then converted to a hydrograph using a function in the CivilD software. Outputs were then routed through the project's water quality, detention, and hydromodification facility using CivilD software and a defined stage-storage-discharge table.

Storm runoff and tributary watershed area summaries can be found in section IV below.

IV. PROPOSED ONSITE DRAINAGE FACILITIES

a. UNMITIGATED PEAK STORM FLOWS

Table IV.1 below summarizes unmitigated storm flows from the overall watershed ultimately discharging to the existing 8'W x 2'H culvert based upon rational method analysis.

Because the unmitigated proposed discharge to the box culvert would exceed the existing discharge, peak storm mitigation is provided as part of this project. Peak outflow to the existing culvert was determined by adding the peak flows tributary to the proposed basin to the peak flows not tributary to the basin. This approach results in a higher, more conservative total project peak flow rate as compared to confluencing the flows from the proposed basin with the flows not tributary to the basin.

Table IV.1 Proposed Unmitigated Peak Storm Flows

Proposed Unmitigated Condition 100-Year Storm		
	Q ₁₀₀ (cfs)	Area (AC)
Areas Tributary to Proposed Basin	36.5	13.3
Areas Not Tributary to Proposed Basin	66.9	60.7
Total Project	103.4	74.0

Table IV.2 Proposed Condition Initial Areas

Proposed Condition 100-Year Storm				
Subarea Designation	Q ₁₀₀ (cfs)	Area (AC)	Initial Time of Concentration (min)	Maximum Overland Flow Distance (ft)
C-1	0.3	0.14	6.19	100
C-4	1.0	0.25	5.10	100
C-6	0.2	0.04	5*	100
C-8	1.0	0.24	5*	100
C-12	0.2	0.06	6.32**	100
C-14	0.2	0.09	6.69	100
C-19	0.2	0.03	5*	85
C-22	0.4	0.06	5*	95

* Note: If the initial time of concentration calculated is less than 5 minutes, the time of concentration used for intensity calculations is reset to 5 minutes.

** Note: If the maximum overland flow distance is exceeded, the initial time of concentration is calculated by the CivilD program in accordance with Figures 3-3 and 3-4 of the San Diego County Hydrology Manual.

Table IV.3 Proposed Condition Point of Discharge (Unmitigated)

Proposed Condition 100-Year Storm					
Existing Condition Node	Proposed Condition Node	Q ₁₀₀ (cfs)	Total Area (AC)	Intensity (in/hr)	Time of Concentration (min)
114	403	103.4	74.0	4.6	6.89

Table IV.4 Proposed Condition Runoff Coefficients

Proposed Condition 100-Year Storm		
Subarea Designation	Area (AC)	Runoff Coefficient
C-1	0.14	0.300
C-2	0.75	0.267
C-3	1.37	0.547
C-4	0.25	0.450
C-5	2.89	0.478
C-6	0.04	0.480
C-7	0.27	0.480
C-8	0.24	0.480
C-9	1.31	0.480
C-10	0.51	0.480
C-11	0.99	0.497
C-12	0.06	0.480
C-13	0.53	0.480
C-14	0.09	0.360
C-15	3.36	0.360
C-16	0.07	0.300
C-17	0.17	0.343
C-18	0.23	0.355
C-19	0.03	0.780
C-20	0.27	0.780
C-21	1.65	0.302
C-22	0.06	0.780
C-23	0.67	0.780
C-24	0.70	0.308

b. MITIGATED PEAK STORM FLOWS

Based upon increased impervious ratios and decreased times of concentration, peak storm flows would increase from the existing to proposed conditions without mitigation. A multi-purpose basin is proposed in the southwest corner of the site, providing for water quality treatment, hydromodification mitigation, and peak flow attenuation. The proposed basin is designed in accordance with the County of San Diego's Conjunctive Use Facilities for Storm Water Management and Flood Control memorandum (January 21, 2020), with the storage volumes for the peak storm and hydromodification mitigation overlapping. Storage volume for pollutant control is provided separately from peak storm and hydromodification mitigation. Basin routing calculations are provided in Appendix B. Peak outflow to the existing culvert was determined by adding the routed peak flows tributary to the proposed basin to the peak flows not tributary to the basin. Stage-Storage-Discharge

tables for both the existing Idaho Avenua 30" culvert and the proposed detention basin are provided in Appendix B.

Table IV.5 Proposed Condition Basin Routing Summary

Proposed Condition 100-Year Storm		
	Q ₁₀₀ (cfs)	Area (AC)
Unmitigated Basin Inflow	36.5	13.27
Mitigated Basin Outflow	16.0	13.27
Delta	-20.5	0

By utilizing the distinct time of concentrations from the basin outflow and the drainage course, a confluent peak discharge and time of concentration of those combined discharges was calculated. By confluenting the peak storm flows, the peak discharge flow rates have been effectively reduced to below predevelopment levels. See summary Table IV.8 below.

Table IV.6 Proposed Condition Summary

Proposed Condition 100-Year Storm		
	Q ₁₀₀ (cfs)	Area (AC)
Existing Condition	81.7	74.0
Proposed Condition (Unmitigated)	103.4	74.0
Proposed Condition (Mitigated)	74.8	74.0
Delta	-6.9	0.0

A separate project specific Storm Water Quality Management Plan (SWQMP) has been prepared for the overall project, which contains the required water quality calculations and corresponding hydromodification mitigation calculations.

V. CONCLUSION

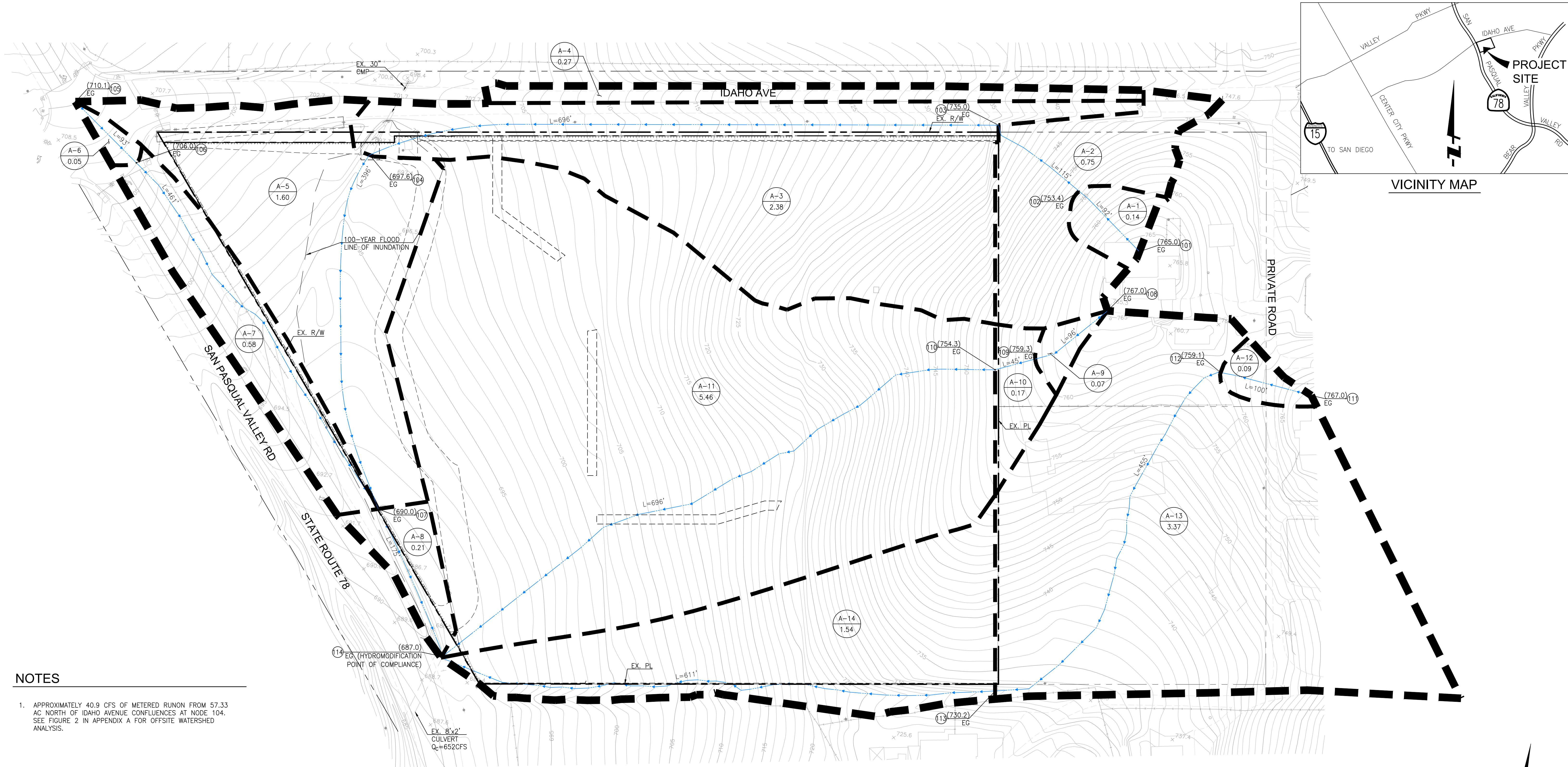
Based upon analyses provided in this report, the proposed development will not adversely impact the existing drainage conditions. As described previously, the existing drainage patterns will generally be preserved. The drainage course running through the project remains generally in its pre-project state, and is not significantly altered by the proposed development. Proposed onsite storm drain facilities will adequately convey peak storm flows through the project site to the proposed drainage basin and to San Pasqual Valley Road while maintaining 100-year storm flood protection and decreasing the peak flow from the area. Due to the proposed drainage basin, it is unlikely substantial erosion, flooding, or exceedance of existing storm water drainage systems would occur onsite or offsite.

The development does not propose any housing or structures within a 100-year flood hazard area. The project does not propose any levees or dams, therefore exposure of persons or structures to significant risk of loss, injury or death involving flooding as a result of failure of a levee or dam is highly unlikely.

VI. REFERENCES

1. San Diego County Hydrology Manual (June, 2003)
2. City of Escondido Drainage Master Plan (November, 1995)
3. Tory-Walker Flood Limits Study
4. San Diego County Conjunctive Use Facilities for Storm Water Management and Flood Control Memorandum (January 21, 2020)

APPENDIX A



NOTES

1. APPROXIMATELY 40.9 CFS OF METERED RUNON FROM 57.33 AC NORTH OF IDAHO AVENUE CONFLUENCES AT NODE 104. SEE FIGURE 2 IN APPENDIX A FOR OFFSITE WATERSHED ANALYSIS.

LEGEND

- EXISTING PROPERTY BOUNDARY
- PROPOSED PROPERTY BOUNDARY
- EXISTING RIGHT OF WAY
- PROPOSED MAJOR AREA BOUNDARY
- PROPOSED MINOR AREA BOUNDARY
- SURFACE FLOW PATH
- SURFACE FLOW DIRECTION
- (XXX.XX) EXISTING ELEVATION
- (10) PROPOSED NODE
- L=XXXX' SURFACE FLOW PATH LENGTH
- A DRAINAGE AREA DESIGNATION
- 0.83 ACREAGE

EXISTING CONDITION
HYDROLOGY SUMMARY

NODE	Q ₁₀₀ (CFS)	Σ AREA (AC)	TC (MIN)
104 ¹	45.7	60.87	17.02
107	70.1	63.10	18.97
110	0.5	0.24	6.31
114	81.7	74.01	19.57

FIGURE 1: EXISTING CONDITION HYDROLOGY MAP
ESCONDIDO ESTATES



LEGEND

- EXISTING PARCEL BOUNDARY
- PROPOSED MAJOR AREA BOUNDARY
- PROPOSED MINOR AREA BOUNDARY
- SURFACE FLOW PATH
- SURFACE FLOW DIRECTION
- (XXX.XX) EXISTING ELEVATION
- 20 PROPOSED NODE
- L=XXXX' SURFACE FLOW PATH LENGTH
- A DRAINAGE AREA DESIGNATION
- 0.83 ACREAGE

EXISTING CONDITION HYDROLOGY SUMMARY

NODE	Q ₁₀₀ (CFS)	Σ AREA (AC)
202	0.3	0.14
203	22.1	13.60
204	20.4	19.78
205	22.5	23.81
TOTAL	65.4	57.33

- NOTES:
- TOPOGRAPHY OBTAINED FROM NEARMAP ON 04/21/2021.
 - PARCEL BOUNDARIES ARE FROM COUNTY OF SAN DIEGO GIS.
 - EXISTING 30" CMP CULVERT METERS DISCHARGE FROM THE WATERSHED TO A PEAK FLOW OF 37.9 CFS.

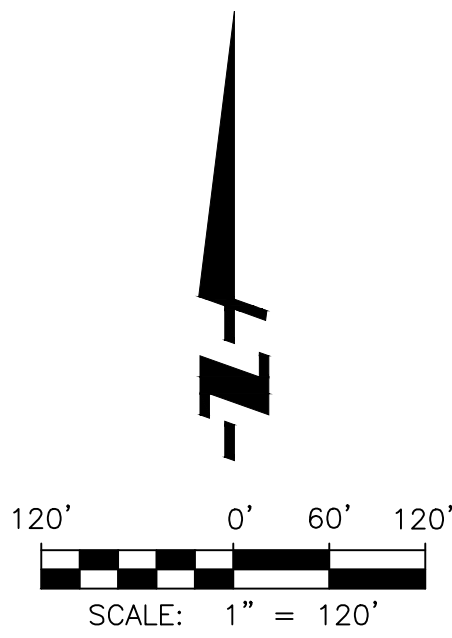
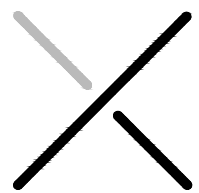
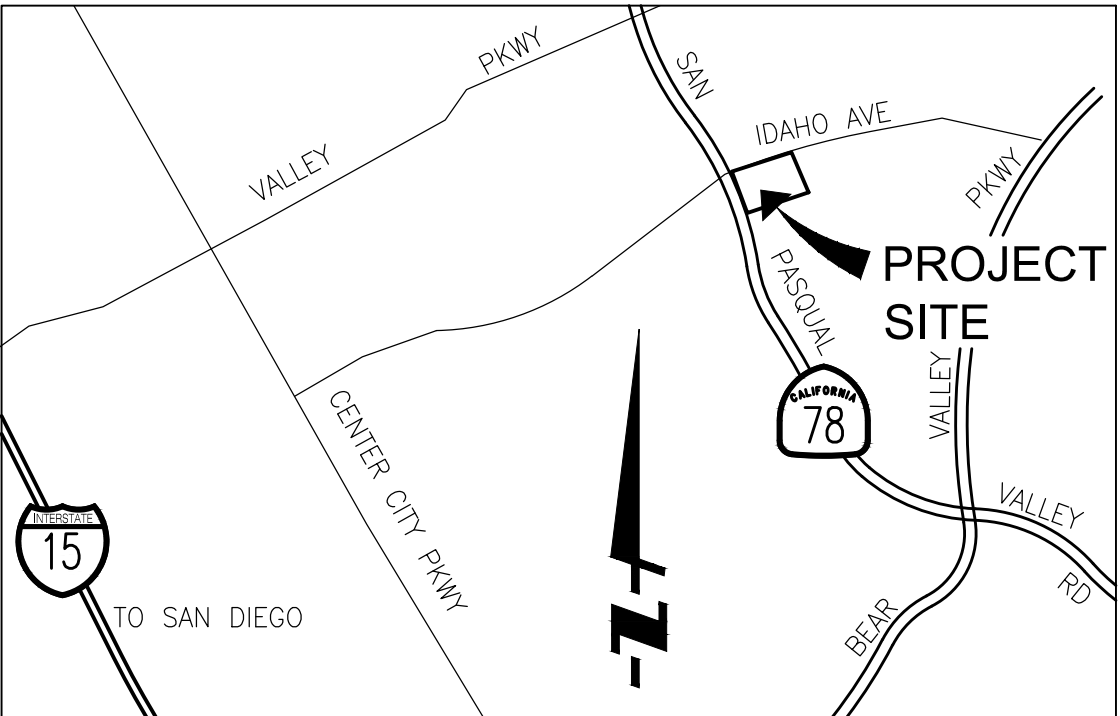


FIGURE 2: OFFSITE WATERSHED ANALYSIS
ESCONDIDO ESTATES



X ENGINEERING & CONSULTING, INC.
6 Hutton Centre Drive, Suite 650
Santa Ana, California 92707
949.522.7100 | xengineeringinc.com



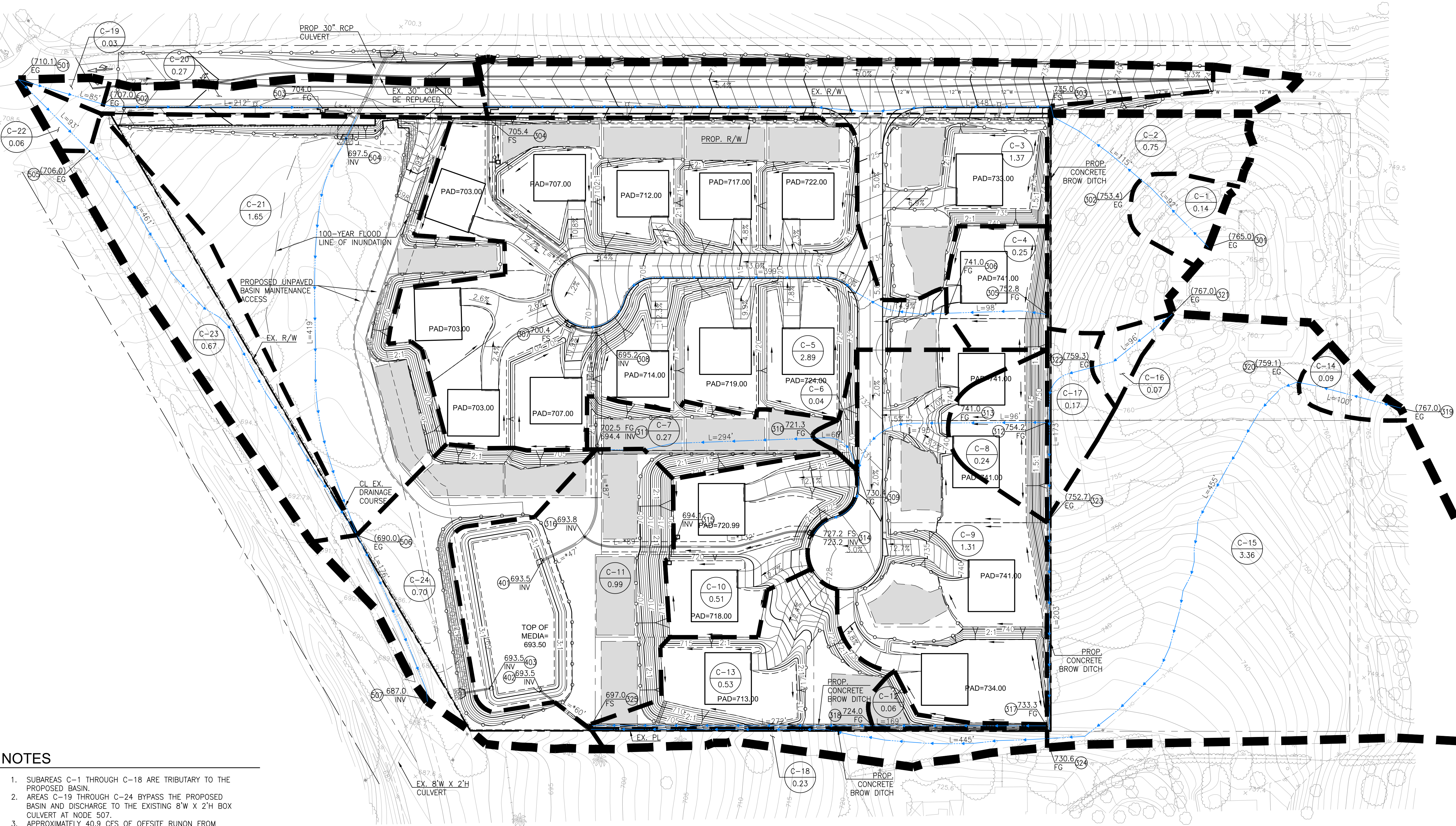
VICINITY MAP

PROPOSED CONDITION
HYDROLOGY SUMMARY

	Q ₁₀₀ (CFS)	AREA (AC)
EXISTING CONDITION	81.7	74.0
PROPOSED, UNMITIGATED CONDITION	103.4	74.0
PROPOSED, MITIGATED CONDITION	74.8	74.0
DELTA	-6.9	0

LEGEND

- EXISTING PROPERTY BOUNDARY
- PROPOSED PROPERTY BOUNDARY
- EXISTING RIGHT OF WAY
- PROPOSED MAJOR AREA BOUNDARY
- PROPOSED MINOR AREA BOUNDARY
- SURFACE FLOW PATH
- SURFACE FLOW DIRECTION
- PROPOSED SEPTIC FIELD
- EXISTING ELEVATION (XXX.XX)
- PROPOSED ELEVATION (XXX.XX)
- PROPOSED NODE (10)
- SURFACE FLOW PATH LENGTH (L=XXXX')
- PIPE FLOW PATH LENGTH (L = *XXXX')
- DRAINAGE AREA DESIGNATION (C)
- ACREAGE (0.83)



NOTES

- SUBAREAS C-1 THROUGH C-18 ARE TRIBUTARY TO THE PROPOSED BASIN.
- AREAS C-19 THROUGH C-24 BYPASS THE PROPOSED BASIN AND DISCHARGE TO THE EXISTING 8'W X 2'H BOX CULVERT AT NODE 507.
- APPROXIMATELY 40.9 CFS OF OFFSITE RUNON FROM 57.33 AC NORTH OF IDAHO AVENUE CONFLUENCES AT NODE 504. SEE FIGURE 2 IN APPENDIX A FOR OFFSITE WATERSHED ANALYSIS.

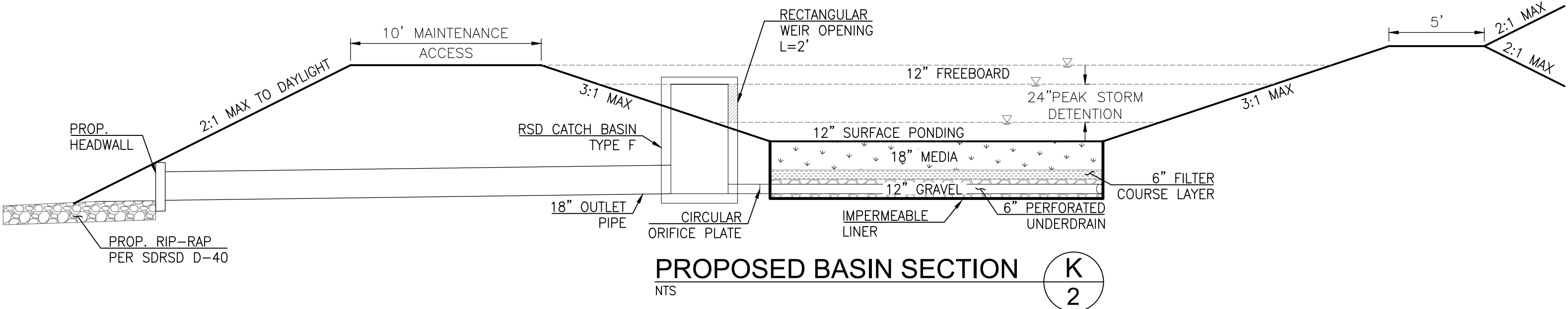


FIGURE 3: PROPOSED CONDITION HYDROLOGY MAP
ESCONDIDO ESTATES

APPENDIX B

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 06/21/21

108-001 Idaho (Escondido Estates)
Offsite Northern Watershed
Surface Flow to Existing Idaho Culvert
xtrapolated storm from County Isopluvials - No Routing

***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:
6 hour, precipitation(inches) = 3.300
24 hour precipitation(inches) = 7.300
P6/P24 = 45.2%
San Diego hydrology manual 'C' values used

+++++
Process from Point/Station 201.000 to Point/Station 202.000
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[UNDISTURBED NATURAL TERRAIN]
(Permanent Open Space)
Impervious value, Ai = 0.000
Sub-Area C Value = 0.300
Initial subarea total flow distance = 100.000(Ft.)
Highest elevation = 823.600(Ft.)
Lowest elevation = 813.800(Ft.)
Elevation difference = 9.800(Ft.) Slope = 9.800 %
INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
The maximum overland flow distance is 100.00 (Ft)

for the top area slope value of 9.80 %, in a development type of
 Permanent Open Space
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 6.73 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (9.800^{(1/3)})] = 6.73$
 Rainfall intensity (I) = 7.179(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.300
 Subarea runoff = 0.302(CFS)
 Total initial stream area = 0.140(Ac.)

+++++
 Process from Point/Station 202.000 to Point/Station 203.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

 Estimated mean flow rate at midpoint of channel = 11.848(CFS)
 Depth of flow = 0.516(Ft.), Average velocity = 4.456(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	2.00
2	20.00	0.00
3	40.00	2.00

 Manning's 'N' friction factor = 0.040

Sub-Channel flow = 11.848(CFS)

'	'	flow top width = 10.312(Ft.)
'	'	velocity = 4.456(Ft/s)
'	'	area = 2.659(Sq.Ft)
'	'	Froude number = 1.547

Upstream point elevation = 813.800(Ft.)
 Downstream point elevation = 743.000(Ft.)
 Flow length = 802.000(Ft.)
 Travel time = 3.00 min.
 Time of concentration = 9.73 min.
 Depth of flow = 0.516(Ft.)
 Average velocity = 4.456(Ft/s)
 Total irregular channel flow = 11.848(CFS)
 Irregular channel normal depth above invert elev. = 0.516(Ft.)
 Average velocity of channel(s) = 4.456(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 5.660(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]

(Permanent Open Space)
 Impervious value, $A_i = 0.000$
 Sub-Area C Value = 0.300
 Rainfall intensity = 5.660(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 ($Q=KCIA$) is $C = 0.300$ $CA = 4.122$
 Subarea runoff = 23.028(CFS) for 13.600(Ac.)
 Total runoff = 23.329(CFS) Total area = 13.740(Ac.)
 Depth of flow = 0.665(Ft.), Average velocity = 5.279(Ft/s)

+++++
 Process from Point/Station 203.000 to Point/Station 204.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

 Estimated mean flow rate at midpoint of channel = 34.018(CFS)
 Depth of flow = 0.484(Ft.), Average velocity = 2.909(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

 Manning's 'N' friction factor = 0.040

Sub-Channel flow = 34.018(CFS)
 ' ' flow top width = 48.359(Ft.)
 ' ' velocity = 2.909(Ft/s)
 ' ' area = 11.693(Sq.Ft)
 ' ' Froude number = 1.043

Upstream point elevation = 743.000(Ft.)
 Downstream point elevation = 711.400(Ft.)
 Flow length = 776.000(Ft.)
 Travel time = 4.45 min.
 Time of concentration = 14.17 min.
 Depth of flow = 0.484(Ft.)
 Average velocity = 2.909(Ft/s)
 Total irregular channel flow = 34.018(CFS)
 Irregular channel normal depth above invert elev. = 0.484(Ft.)
 Average velocity of channel(s) = 2.909(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 4.440(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)

Impervious value, $A_i = 0.000$
 Sub-Area C Value = 0.300
 Rainfall intensity = 4.440(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 ($Q=KCIA$) is $C = 0.300$ $CA = 10.056$
 Subarea runoff = 21.318(CFS) for 19.780(Ac.)
 Total runoff = 44.647(CFS) Total area = 33.520(Ac.)
 Depth of flow = 0.536(Ft.), Average velocity = 3.114(Ft/s)

++++++
 Process from Point/Station 204.000 to Point/Station 205.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

 Estimated mean flow rate at midpoint of channel = 56.424(CFS)
 Depth of flow = 0.661(Ft.), Average velocity = 2.584(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

 Manning's 'N' friction factor = 0.040

 Sub-Channel flow = 56.424(CFS)
 ' ' flow top width = 66.090(Ft.)
 ' ' velocity = 2.584(Ft/s)
 ' ' area = 21.839(Sq.Ft)
 ' ' Froude number = 0.792

Upstream point elevation = 711.400(Ft.)
 Downstream point elevation = 702.400(Ft.)
 Flow length = 425.000(Ft.)
 Travel time = 2.74 min.
 Time of concentration = 16.92 min.
 Depth of flow = 0.661(Ft.)
 Average velocity = 2.584(Ft/s)
 Total irregular channel flow = 56.424(CFS)
 Irregular channel normal depth above invert elev. = 0.661(Ft.)
 Average velocity of channel(s) = 2.584(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 3.961(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, $A_i = 0.000$

Sub-Area C Value = 0.300
 Rainfall intensity = 3.961(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.300 CA = 17.199
 Subarea runoff = 23.483(CFS) for 23.810(Ac.)
 Total runoff = 68.131(CFS) Total area = 57.330(Ac.)
 Depth of flow = 0.709(Ft.), Average velocity = 2.708(Ft/s)

++++++
 Process from Point/Station 204.000 to Point/Station 205.000
 **** 6 HOUR HYDROGRAPH ****

++++++
 Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 16.92
 Basin Area = 57.33 Acres
 6 Hour Rainfall = 3.300 Inches
 Runoff Coefficient = 0.300
 Peak Discharge = 68.13 CFS

Time (Min)	Discharge (CFS)
0	0.000
16	3.365
32	3.574
48	3.691
64	3.958
80	4.111
96	4.469
112	4.680
128	5.190
144	5.504
160	6.309
176	6.841
192	8.361
208	9.523
224	13.983
240	19.702
256	68.131
272	11.215
288	7.504
304	5.871
320	4.918
336	4.280
352	3.818
368	3.466

++++++
 6 - H O U R S T O R M
 R u n o f f H y d r o g r a p h

 Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	17.0	34.1	51.1	68.1
0+ 0	0.0000		0.00	Q				
0+ 1	0.0003		0.21	Q				
0+ 2	0.0009		0.42	Q				
0+ 3	0.0017		0.63	Q				
0+ 4	0.0029		0.84	Q				
0+ 5	0.0043		1.05	Q				
0+ 6	0.0061		1.26	Q				
0+ 7	0.0081		1.47	Q				
0+ 8	0.0104		1.68	Q				
0+ 9	0.0130		1.89	VQ				
0+10	0.0159		2.10	VQ				
0+11	0.0191		2.31	VQ				
0+12	0.0226		2.52	VQ				
0+13	0.0264		2.73	VQ				
0+14	0.0304		2.94	VQ				
0+15	0.0348		3.16	VQ				
0+16	0.0394		3.37	VQ				
0+17	0.0441		3.38	VQ				
0+18	0.0487		3.39	VQ				
0+19	0.0534		3.40	VQ				
0+20	0.0581		3.42	V Q				
0+21	0.0628		3.43	V Q				
0+22	0.0676		3.44	V Q				
0+23	0.0724		3.46	V Q				
0+24	0.0771		3.47	V Q				
0+25	0.0819		3.48	V Q				
0+26	0.0867		3.50	V Q				
0+27	0.0916		3.51	V Q				
0+28	0.0964		3.52	V Q				
0+29	0.1013		3.53	V Q				
0+30	0.1062		3.55	V Q				
0+31	0.1111		3.56	V Q				
0+32	0.1160		3.57	V Q				
0+33	0.1209		3.58	VQ				
0+34	0.1259		3.59	VQ				
0+35	0.1308		3.60	VQ				
0+36	0.1358		3.60	VQ				
0+37	0.1408		3.61	VQ				
0+38	0.1458		3.62	VQ				
0+39	0.1508		3.63	VQ				
0+40	0.1558		3.63	VQ				
0+41	0.1608		3.64	VQ				
0+42	0.1658		3.65	VQ				
0+43	0.1708		3.65	VQ				

0+44	0.1759	3.66	VQ				
0+45	0.1809	3.67	VQ				
0+46	0.1860	3.68	VQ				
0+47	0.1911	3.68	VQ				
0+48	0.1961	3.69	VQ				
0+49	0.2013	3.71	VQ				
0+50	0.2064	3.72	VQ				
0+51	0.2115	3.74	VQ				
0+52	0.2167	3.76	VQ				
0+53	0.2219	3.77	VQ				
0+54	0.2271	3.79	VQ				
0+55	0.2324	3.81	Q				
0+56	0.2376	3.82	Q				
0+57	0.2429	3.84	Q				
0+58	0.2482	3.86	Q				
0+59	0.2536	3.87	Q				
1+ 0	0.2589	3.89	Q				
1+ 1	0.2643	3.91	Q				
1+ 2	0.2697	3.92	Q				
1+ 3	0.2752	3.94	Q				
1+ 4	0.2806	3.96	Q				
1+ 5	0.2861	3.97	Q				
1+ 6	0.2916	3.98	Q				
1+ 7	0.2970	3.99	Q				
1+ 8	0.3026	4.00	Q				
1+ 9	0.3081	4.01	Q				
1+10	0.3136	4.02	Q				
1+11	0.3191	4.02	Q				
1+12	0.3247	4.03	Q				
1+13	0.3303	4.04	Q				
1+14	0.3359	4.05	Q				
1+15	0.3415	4.06	Q				
1+16	0.3471	4.07	Q				
1+17	0.3527	4.08	QV				
1+18	0.3583	4.09	QV				
1+19	0.3640	4.10	QV				
1+20	0.3696	4.11	QV				
1+21	0.3753	4.13	QV				
1+22	0.3810	4.16	QV				
1+23	0.3868	4.18	QV				
1+24	0.3926	4.20	QV				
1+25	0.3984	4.22	QV				
1+26	0.4043	4.25	QV				
1+27	0.4101	4.27	QV				
1+28	0.4160	4.29	QV				
1+29	0.4220	4.31	QV				
1+30	0.4280	4.33	QV				
1+31	0.4340	4.36	QV				
1+32	0.4400	4.38	QV				
1+33	0.4460	4.40	QV				

1+34	0.4521	4.42	QV				
1+35	0.4583	4.45	QV				
1+36	0.4644	4.47	QV				
1+37	0.4706	4.48	Q V				
1+38	0.4768	4.49	Q V				
1+39	0.4830	4.51	Q V				
1+40	0.4892	4.52	Q V				
1+41	0.4955	4.53	Q V				
1+42	0.5017	4.55	Q V				
1+43	0.5080	4.56	Q V				
1+44	0.5143	4.57	Q V				
1+45	0.5206	4.59	Q V				
1+46	0.5270	4.60	Q V				
1+47	0.5333	4.61	Q V				
1+48	0.5397	4.63	Q V				
1+49	0.5461	4.64	Q V				
1+50	0.5525	4.65	Q V				
1+51	0.5589	4.67	Q V				
1+52	0.5654	4.68	Q V				
1+53	0.5719	4.71	Q V				
1+54	0.5784	4.74	Q V				
1+55	0.5850	4.78	Q V				
1+56	0.5916	4.81	Q V				
1+57	0.5983	4.84	Q V				
1+58	0.6050	4.87	Q V				
1+59	0.6117	4.90	Q V				
2+ 0	0.6185	4.93	Q V				
2+ 1	0.6254	4.97	Q V				
2+ 2	0.6322	5.00	Q V				
2+ 3	0.6392	5.03	Q V				
2+ 4	0.6461	5.06	Q V				
2+ 5	0.6532	5.09	Q V				
2+ 6	0.6602	5.13	Q V				
2+ 7	0.6673	5.16	Q V				
2+ 8	0.6745	5.19	Q V				
2+ 9	0.6817	5.21	Q V				
2+10	0.6889	5.23	Q V				
2+11	0.6961	5.25	Q V				
2+12	0.7033	5.27	Q V				
2+13	0.7106	5.29	Q V				
2+14	0.7179	5.31	Q V				
2+15	0.7253	5.33	Q V				
2+16	0.7326	5.35	Q V				
2+17	0.7400	5.37	Q V				
2+18	0.7474	5.39	Q V				
2+19	0.7549	5.41	Q V				
2+20	0.7624	5.43	Q V				
2+21	0.7699	5.44	Q V				
2+22	0.7774	5.46	Q V				
2+23	0.7849	5.48	Q V				

2+24	0.7925	5.50	Q	V				
2+25	0.8002	5.55	Q	V				
2+26	0.8079	5.60	Q	V				
2+27	0.8157	5.65	Q	V				
2+28	0.8235	5.71	Q	V				
2+29	0.8315	5.76	Q	V				
2+30	0.8395	5.81	Q	V				
2+31	0.8475	5.86	Q	V				
2+32	0.8557	5.91	Q	V				
2+33	0.8639	5.96	Q	V				
2+34	0.8722	6.01	Q	V				
2+35	0.8805	6.06	Q	V				
2+36	0.8889	6.11	Q	V				
2+37	0.8974	6.16	Q	V				
2+38	0.9059	6.21	Q	V				
2+39	0.9146	6.26	Q	V				
2+40	0.9233	6.31	Q	V				
2+41	0.9320	6.34	Q	V				
2+42	0.9408	6.38	Q	V				
2+43	0.9496	6.41	Q	V				
2+44	0.9585	6.44	Q	V				
2+45	0.9674	6.48	Q	V				
2+46	0.9764	6.51	Q	V				
2+47	0.9854	6.54	Q	V				
2+48	0.9944	6.57	Q	V				
2+49	1.0035	6.61	Q	V				
2+50	1.0127	6.64	Q	V				
2+51	1.0219	6.67	Q	V				
2+52	1.0311	6.71	Q	V				
2+53	1.0404	6.74	Q	V				
2+54	1.0497	6.77	Q	V				
2+55	1.0591	6.81	Q	V				
2+56	1.0685	6.84	Q	V				
2+57	1.0781	6.94	Q	V				
2+58	1.0878	7.03	Q	V				
2+59	1.0976	7.13	Q	V				
3+ 0	1.1075	7.22	Q	V				
3+ 1	1.1176	7.32	Q	V				
3+ 2	1.1278	7.41	Q	V				
3+ 3	1.1381	7.51	Q	V				
3+ 4	1.1486	7.60	Q	V				
3+ 5	1.1592	7.70	Q	V				
3+ 6	1.1699	7.79	Q	V				
3+ 7	1.1808	7.89	Q	V				
3+ 8	1.1918	7.98	Q	V				
3+ 9	1.2029	8.08	Q	V				
3+10	1.2142	8.17	Q	V				
3+11	1.2256	8.27	Q	V				
3+12	1.2371	8.36	Q	V				
3+13	1.2487	8.43	Q	V				

3+14	1.2604	8.51	Q	V			
3+15	1.2722	8.58	Q	V			
3+16	1.2841	8.65	Q	V			
3+17	1.2962	8.72	Q	V			
3+18	1.3083	8.80	Q	V			
3+19	1.3205	8.87	Q	V			
3+20	1.3328	8.94	Q	V			
3+21	1.3452	9.01	Q	V			
3+22	1.3577	9.09	Q	V			
3+23	1.3704	9.16	Q	V			
3+24	1.3831	9.23	Q	V			
3+25	1.3959	9.31	Q	V			
3+26	1.4088	9.38	Q	V			
3+27	1.4218	9.45	Q	V			
3+28	1.4349	9.52	Q	V			
3+29	1.4484	9.80	Q	V			
3+30	1.4623	10.08	Q	V			
3+31	1.4766	10.36	Q	V			
3+32	1.4913	10.64	Q	V			
3+33	1.5063	10.92	Q	V			
3+34	1.5217	11.20	Q	V			
3+35	1.5375	11.47	Q	V			
3+36	1.5537	11.75	Q	V			
3+37	1.5703	12.03	Q	V			
3+38	1.5872	12.31	Q	V			
3+39	1.6046	12.59	Q	V			
3+40	1.6223	12.87	Q	V			
3+41	1.6404	13.15	Q	V			
3+42	1.6589	13.43	Q	V			
3+43	1.6778	13.70	Q	V			
3+44	1.6970	13.98	Q	V			
3+45	1.7168	14.34	Q	V			
3+46	1.7370	14.70	Q	V			
3+47	1.7578	15.06	Q	V			
3+48	1.7790	15.41	Q	V			
3+49	1.8007	15.77	Q	V			
3+50	1.8230	16.13	Q	V			
3+51	1.8457	16.49	Q	V			
3+52	1.8689	16.84	Q	V			
3+53	1.8925	17.20	Q	V			
3+54	1.9167	17.56	Q	V			
3+55	1.9414	17.92	Q	V			
3+56	1.9666	18.27	Q	V			
3+57	1.9922	18.63	Q	V			
3+58	2.0184	18.99	Q	V			
3+59	2.0450	19.34	Q	V			
4+ 0	2.0722	19.70	Q	V			
4+ 1	2.1035	22.73		Q	V		
4+ 2	2.1390	25.76			Q	V	
4+ 3	2.1786	28.78				Q	V

4+ 4	2.2224	31.81			QV			
4+ 5	2.2704	34.84			VQ			
4+ 6	2.3226	37.86			V	Q		
4+ 7	2.3789	40.89			V	Q		
4+ 8	2.4394	43.92			V	Q		
4+ 9	2.5040	46.94			V	Q		
4+10	2.5729	49.97			V	Q		
4+11	2.6459	53.00			V	Q		
4+12	2.7230	56.02			V	Q		
4+13	2.8044	59.05			V	Q		
4+14	2.8899	62.08			V	Q		
4+15	2.9795	65.10			V	Q		
4+16	3.0734	68.13			V	Q		
4+17	3.1623	64.57			V	Q		
4+18	3.2464	61.02			V	Q		
4+19	3.3255	57.46			V	Q		
4+20	3.3998	53.90			V	Q		
4+21	3.4691	50.34			V	Q		
4+22	3.5336	46.79			V	Q		
4+23	3.5931	43.23			V	Q		
4+24	3.6477	39.67			V	Q		
4+25	3.6975	36.12			V	Q		
4+26	3.7423	32.56			V	Q		
4+27	3.7823	29.00			V	Q		
4+28	3.8173	25.44			V	Q		
4+29	3.8475	21.89			V	Q		
4+30	3.8727	18.33			V	Q		
4+31	3.8931	14.77			V	Q		
4+32	3.9085	11.22			V	Q		
4+33	3.9236	10.98			V	Q		
4+34	3.9385	10.75			V	Q		
4+35	3.9529	10.52			V	Q		
4+36	3.9671	10.29			V	Q		
4+37	3.9810	10.06			V	Q		
4+38	3.9945	9.82			V	Q		
4+39	4.0077	9.59			V	Q		
4+40	4.0206	9.36			V	Q		
4+41	4.0332	9.13			V	Q		
4+42	4.0454	8.90			V	Q		
4+43	4.0574	8.66			V	Q		
4+44	4.0690	8.43			V	Q		
4+45	4.0803	8.20			V	Q		
4+46	4.0912	7.97			V	Q		
4+47	4.1019	7.74			V	Q		
4+48	4.1122	7.50			V	Q		
4+49	4.1224	7.40			V	Q		
4+50	4.1325	7.30			V	Q		
4+51	4.1424	7.20			V	Q		
4+52	4.1522	7.10			V	Q		
4+53	4.1618	6.99			V	Q		

4+54	4.1713	6.89	Q			V
4+55	4.1807	6.79	Q			V
4+56	4.1899	6.69	Q			V
4+57	4.1989	6.59	Q			V
4+58	4.2079	6.48	Q			V
4+59	4.2167	6.38	Q			V
5+ 0	4.2253	6.28	Q			V
5+ 1	4.2338	6.18	Q			V
5+ 2	4.2422	6.08	Q			V
5+ 3	4.2504	5.97	Q			V
5+ 4	4.2585	5.87	Q			V
5+ 5	4.2665	5.81	Q			V
5+ 6	4.2744	5.75	Q			V
5+ 7	4.2823	5.69	Q			V
5+ 8	4.2900	5.63	Q			V
5+ 9	4.2977	5.57	Q			V
5+10	4.3053	5.51	Q			V
5+11	4.3128	5.45	Q			V
5+12	4.3202	5.39	Q			V
5+13	4.3276	5.34	Q			V
5+14	4.3349	5.28	Q			V
5+15	4.3420	5.22	Q			V
5+16	4.3491	5.16	Q			V
5+17	4.3562	5.10	Q			V
5+18	4.3631	5.04	Q			V
5+19	4.3700	4.98	Q			V
5+20	4.3767	4.92	Q			V
5+21	4.3835	4.88	Q			V
5+22	4.3901	4.84	Q			V
5+23	4.3967	4.80	Q			V
5+24	4.4033	4.76	Q			V
5+25	4.4098	4.72	Q			V
5+26	4.4162	4.68	Q			V
5+27	4.4226	4.64	Q			V
5+28	4.4290	4.60	Q			V
5+29	4.4352	4.56	Q			V
5+30	4.4415	4.52	Q			V
5+31	4.4476	4.48	Q			V
5+32	4.4537	4.44	Q			V
5+33	4.4598	4.40	Q			V
5+34	4.4658	4.36	Q			V
5+35	4.4718	4.32	Q			V
5+36	4.4777	4.28	Q			V
5+37	4.4835	4.25	Q			V
5+38	4.4893	4.22	Q			V
5+39	4.4951	4.19	Q			V
5+40	4.5008	4.16	Q			V
5+41	4.5065	4.14	Q			V
5+42	4.5122	4.11	Q			V
5+43	4.5178	4.08	Q			V

5+44	4.5234	4.05	Q				V
5+45	4.5289	4.02	Q				V
5+46	4.5344	3.99	Q				V
5+47	4.5399	3.96	Q				V
5+48	4.5453	3.93	Q				V
5+49	4.5507	3.91	Q				V
5+50	4.5560	3.88	Q				V
5+51	4.5613	3.85	Q				V
5+52	4.5666	3.82	Q				V
5+53	4.5718	3.80	Q				V
5+54	4.5770	3.77	Q				V
5+55	4.5822	3.75	Q				V
5+56	4.5873	3.73	Q				V
5+57	4.5924	3.71	Q				V
5+58	4.5975	3.69	Q				V
5+59	4.6025	3.66	Q				V
6+ 0	4.6076	3.64	Q				V
6+ 1	4.6125	3.62	Q				V
6+ 2	4.6175	3.60	Q				V
6+ 3	4.6224	3.58	Q				V
6+ 4	4.6273	3.55	Q				V
6+ 5	4.6322	3.53	Q				V
6+ 6	4.6370	3.51	Q				V
6+ 7	4.6418	3.49	Q				V
6+ 8	4.6466	3.47	Q				V

End of computations, total study area = 57.330 (Ac.)

FLOOD HYDROGRAPH ROUTING PROGRAM
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 Study date: 06/21/21

 108-001 Idaho (Escondido Estates)
 Offsite Northern Watershed
 Surface Flow to Existing Culvert
 Extrapolated storm from County Isopluvials - Routed

Program License Serial Number 6440

 ***** HYDROGRAPH INFORMATION *****

From study/file name: 1080010ffsiteExNR.rte
 *****HYDROGRAPH DATA*****
 Number of intervals = 368
 Time interval = 1.0 (Min.)
 Maximum/Peak flow rate = 68.131 (CFS)
 Total volume = 4.647 (Ac.Ft)
 Status of hydrographs being held in storage

	Stream 1	Stream 2	Stream 3	Stream 4	Stream 5
Peak (CFS)	0.000	0.000	0.000	0.000	0.000
Vol (Ac.Ft)	0.000	0.000	0.000	0.000	0.000

+++++
 Process from Point/Station 104.000 to Point/Station 104.000
 **** RETARDING BASIN ROUTING ****

 User entry of depth-outflow-storage data

Total number of inflow hydrograph intervals = 368
 Hydrograph time unit = 1.000 (Min.)
 Initial depth in storage basin = 0.00(Ft.)

 Initial basin depth = 0.00 (Ft.)
 Initial basin storage = 0.00 (Ac.Ft)
 Initial basin outflow = 0.00 (CFS)

 Depth vs. Storage and Depth vs. Discharge data:

Basin Depth (Ft.)	Storage (Ac.Ft)	Outflow (CFS)	(S-O*dt/2) (Ac.Ft)	(S+O*dt/2) (Ac.Ft)
0.000	0.000	0.000	0.000	0.000
2.500	0.450	27.310	0.431	0.469
2.600	0.490	28.380	0.470	0.510
3.400	0.860	35.810	0.835	0.885
3.600	0.970	55.080	0.932	1.008
4.350	1.490	225.640	1.335	1.645
4.600	1.700	303.990	1.491	1.909

 Hydrograph Detention Basin Routing

Graph values: 'I'= unit inflow; 'O'=outflow at time shown

Time (Hours)	Inflow (CFS)	Outflow (CFS)	Storage (Ac.Ft)	.0	17.0	34.07	51.10	68.13	Depth (Ft.)
0.017	0.21	0.01	0.000	0					0.00
0.033	0.42	0.03	0.001	0					0.00
0.050	0.63	0.07	0.001	0					0.01
0.067	0.84	0.13	0.002	0					0.01
0.083	1.05	0.19	0.003	0					0.02
0.100	1.26	0.27	0.004	0					0.02
0.117	1.47	0.36	0.006	0					0.03
0.133	1.68	0.46	0.008	0					0.04
0.150	1.89	0.56	0.009	0					0.05
0.167	2.10	0.68	0.011	0					0.06
0.183	2.31	0.80	0.013	OI					0.07
0.200	2.52	0.93	0.015	OI					0.09
0.217	2.73	1.07	0.018	OI					0.10
0.233	2.94	1.21	0.020	OI					0.11
0.250	3.16	1.36	0.022	OI					0.12
0.267	3.37	1.51	0.025	OI					0.14
0.283	3.38	1.66	0.027	OI					0.15
0.300	3.39	1.80	0.030	OI					0.16
0.317	3.40	1.93	0.032	OI					0.18
0.333	3.42	2.04	0.034	OI					0.19
0.350	3.43	2.16	0.036	O					0.20
0.367	3.44	2.26	0.037	O					0.21
0.383	3.46	2.35	0.039	O					0.22
0.400	3.47	2.44	0.040	O					0.22
0.417	3.48	2.53	0.042	O					0.23
0.433	3.50	2.60	0.043	O					0.24
0.450	3.51	2.68	0.044	O					0.24
0.467	3.52	2.74	0.045	O					0.25
0.483	3.53	2.81	0.046	O					0.26

0.500	3.55	2.86	0.047	0					0.26
0.517	3.56	2.92	0.048	0					0.27
0.533	3.57	2.97	0.049	0					0.27
0.550	3.58	3.02	0.050	0					0.28
0.567	3.59	3.07	0.051	0					0.28
0.583	3.60	3.11	0.051	0					0.28
0.600	3.60	3.15	0.052	0					0.29
0.617	3.61	3.18	0.052	0					0.29
0.633	3.62	3.22	0.053	0					0.29
0.650	3.63	3.25	0.054	0					0.30
0.667	3.63	3.28	0.054	0					0.30
0.683	3.64	3.31	0.055	0					0.30
0.700	3.65	3.34	0.055	0					0.31
0.717	3.65	3.36	0.055	0					0.31
0.733	3.66	3.39	0.056	0					0.31
0.750	3.67	3.41	0.056	0					0.31
0.767	3.68	3.43	0.057	0					0.31
0.783	3.68	3.45	0.057	0					0.32
0.800	3.69	3.47	0.057	0					0.32
0.817	3.71	3.49	0.057	0					0.32
0.833	3.72	3.51	0.058	0					0.32
0.850	3.74	3.52	0.058	0					0.32
0.867	3.76	3.54	0.058	0					0.32
0.883	3.77	3.56	0.059	0					0.33
0.900	3.79	3.58	0.059	0					0.33
0.917	3.81	3.60	0.059	0					0.33
0.933	3.82	3.61	0.060	0					0.33
0.950	3.84	3.63	0.060	0					0.33
0.967	3.86	3.65	0.060	0					0.33
0.983	3.87	3.67	0.060	0					0.34
1.000	3.89	3.68	0.061	0					0.34
1.017	3.91	3.70	0.061	0					0.34
1.033	3.92	3.72	0.061	0					0.34
1.050	3.94	3.74	0.062	0					0.34
1.067	3.96	3.75	0.062	0					0.34
1.083	3.97	3.77	0.062	0					0.35
1.100	3.98	3.79	0.062	0					0.35
1.117	3.99	3.80	0.063	0					0.35
1.133	4.00	3.82	0.063	0					0.35
1.150	4.01	3.83	0.063	0					0.35
1.167	4.02	3.85	0.063	0					0.35
1.183	4.02	3.86	0.064	0					0.35
1.200	4.03	3.87	0.064	0					0.35
1.217	4.04	3.89	0.064	0					0.36
1.233	4.05	3.90	0.064	0					0.36
1.250	4.06	3.91	0.064	0					0.36
1.267	4.07	3.92	0.065	0					0.36
1.283	4.08	3.94	0.065	0					0.36
1.300	4.09	3.95	0.065	0					0.36
1.317	4.10	3.96	0.065	0					0.36

1.333	4.11	3.97	0.065	0					0.36
1.350	4.13	3.98	0.066	0					0.36
1.367	4.16	4.00	0.066	0					0.37
1.383	4.18	4.01	0.066	0					0.37
1.400	4.20	4.03	0.066	0					0.37
1.417	4.22	4.04	0.067	0					0.37
1.433	4.25	4.06	0.067	0					0.37
1.450	4.27	4.07	0.067	OI					0.37
1.467	4.29	4.09	0.067	OI					0.37
1.483	4.31	4.11	0.068	OI					0.38
1.500	4.33	4.12	0.068	OI					0.38
1.517	4.36	4.14	0.068	OI					0.38
1.533	4.38	4.16	0.069	OI					0.38
1.550	4.40	4.18	0.069	OI					0.38
1.567	4.42	4.20	0.069	OI					0.38
1.583	4.45	4.22	0.069	OI					0.39
1.600	4.47	4.24	0.070	OI					0.39
1.617	4.48	4.25	0.070	OI					0.39
1.633	4.49	4.27	0.070	0					0.39
1.650	4.51	4.29	0.071	0					0.39
1.667	4.52	4.31	0.071	0					0.39
1.683	4.53	4.33	0.071	0					0.40
1.700	4.55	4.34	0.072	0					0.40
1.717	4.56	4.36	0.072	0					0.40
1.733	4.57	4.38	0.072	0					0.40
1.750	4.59	4.39	0.072	0					0.40
1.767	4.60	4.41	0.073	0					0.40
1.783	4.61	4.43	0.073	0					0.41
1.800	4.63	4.44	0.073	0					0.41
1.817	4.64	4.46	0.073	0					0.41
1.833	4.65	4.47	0.074	0					0.41
1.850	4.67	4.49	0.074	0					0.41
1.867	4.68	4.50	0.074	0					0.41
1.883	4.71	4.52	0.074	0					0.41
1.900	4.74	4.53	0.075	0					0.42
1.917	4.78	4.55	0.075	0					0.42
1.933	4.81	4.57	0.075	0					0.42
1.950	4.84	4.59	0.076	0					0.42
1.967	4.87	4.61	0.076	0					0.42
1.983	4.90	4.63	0.076	0					0.42
2.000	4.93	4.66	0.077	0					0.43
2.017	4.97	4.68	0.077	0					0.43
2.033	5.00	4.71	0.078	0					0.43
2.050	5.03	4.73	0.078	0					0.43
2.067	5.06	4.76	0.078	0					0.44
2.083	5.09	4.78	0.079	0					0.44
2.100	5.13	4.81	0.079	0					0.44
2.117	5.16	4.83	0.080	0					0.44
2.133	5.19	4.86	0.080	0					0.45
2.150	5.21	4.89	0.081	0					0.45

2.167	5.23	4.92	0.081	0				0.45
2.183	5.25	4.94	0.081	0				0.45
2.200	5.27	4.97	0.082	0				0.45
2.217	5.29	4.99	0.082	0				0.46
2.233	5.31	5.02	0.083	0				0.46
2.250	5.33	5.04	0.083	0				0.46
2.267	5.35	5.06	0.083	0				0.46
2.283	5.37	5.09	0.084	0				0.47
2.300	5.39	5.11	0.084	0				0.47
2.317	5.41	5.13	0.085	0				0.47
2.333	5.43	5.16	0.085	0				0.47
2.350	5.44	5.18	0.085	0				0.47
2.367	5.46	5.20	0.086	0				0.48
2.383	5.48	5.22	0.086	0				0.48
2.400	5.50	5.24	0.086	0				0.48
2.417	5.55	5.27	0.087	0				0.48
2.433	5.60	5.29	0.087	0				0.48
2.450	5.65	5.32	0.088	0				0.49
2.467	5.71	5.35	0.088	0				0.49
2.483	5.76	5.38	0.089	0				0.49
2.500	5.81	5.41	0.089	0				0.50
2.517	5.86	5.44	0.090	0				0.50
2.533	5.91	5.48	0.090	0				0.50
2.550	5.96	5.52	0.091	0				0.50
2.567	6.01	5.55	0.092	0				0.51
2.583	6.06	5.59	0.092	0				0.51
2.600	6.11	5.63	0.093	0				0.52
2.617	6.16	5.67	0.093	0				0.52
2.633	6.21	5.71	0.094	0				0.52
2.650	6.26	5.75	0.095	0				0.53
2.667	6.31	5.80	0.096	0				0.53
2.683	6.34	5.84	0.096	0				0.53
2.700	6.38	5.88	0.097	0				0.54
2.717	6.41	5.92	0.098	OI				0.54
2.733	6.44	5.96	0.098	OI				0.55
2.750	6.48	6.00	0.099	OI				0.55
2.767	6.51	6.04	0.100	OI				0.55
2.783	6.54	6.08	0.100	OI				0.56
2.800	6.57	6.12	0.101	OI				0.56
2.817	6.61	6.16	0.101	OI				0.56
2.833	6.64	6.19	0.102	OI				0.57
2.850	6.67	6.23	0.103	OI				0.57
2.867	6.71	6.27	0.103	OI				0.57
2.883	6.74	6.30	0.104	OI				0.58
2.900	6.77	6.34	0.104	OI				0.58
2.917	6.81	6.38	0.105	OI				0.58
2.933	6.84	6.41	0.106	0				0.59
2.950	6.94	6.45	0.106	0				0.59
2.967	7.03	6.49	0.107	0				0.59
2.983	7.13	6.54	0.108	0				0.60

3.000	7.22	6.59	0.109	0				0.60
3.017	7.32	6.65	0.110	0				0.61
3.033	7.41	6.70	0.110	0				0.61
3.050	7.51	6.76	0.111	0				0.62
3.067	7.60	6.83	0.112	0				0.62
3.083	7.70	6.89	0.114	0				0.63
3.100	7.79	6.96	0.115	0				0.64
3.117	7.89	7.03	0.116	0				0.64
3.133	7.98	7.10	0.117	0				0.65
3.150	8.08	7.18	0.118	0				0.66
3.167	8.17	7.25	0.120	0				0.66
3.183	8.27	7.33	0.121	0				0.67
3.200	8.36	7.41	0.122	0				0.68
3.217	8.43	7.49	0.123	0				0.69
3.233	8.51	7.57	0.125	0				0.69
3.250	8.58	7.65	0.126	OI				0.70
3.267	8.65	7.72	0.127	OI				0.71
3.283	8.72	7.80	0.129	OI				0.71
3.300	8.80	7.88	0.130	OI				0.72
3.317	8.87	7.96	0.131	OI				0.73
3.333	8.94	8.03	0.132	OI				0.74
3.350	9.01	8.11	0.134	OI				0.74
3.367	9.09	8.18	0.135	OI				0.75
3.383	9.16	8.26	0.136	OI				0.76
3.400	9.23	8.33	0.137	OI				0.76
3.417	9.31	8.41	0.139	OI				0.77
3.433	9.38	8.48	0.140	OI				0.78
3.450	9.45	8.56	0.141	0				0.78
3.467	9.52	8.63	0.142	0				0.79
3.483	9.80	8.72	0.144	0				0.80
3.500	10.08	8.81	0.145	0				0.81
3.517	10.36	8.93	0.147	0				0.82
3.533	10.64	9.05	0.149	0				0.83
3.550	10.92	9.19	0.151	OI				0.84
3.567	11.20	9.34	0.154	OI				0.86
3.583	11.47	9.50	0.157	OI				0.87
3.600	11.75	9.67	0.159	OI				0.89
3.617	12.03	9.85	0.162	OI				0.90
3.633	12.31	10.04	0.165	OI				0.92
3.650	12.59	10.23	0.169	OI				0.94
3.667	12.87	10.43	0.172	0 I				0.95
3.683	13.15	10.64	0.175	0 I				0.97
3.700	13.43	10.85	0.179	OI				0.99
3.717	13.70	11.07	0.182	OI				1.01
3.733	13.98	11.29	0.186	OI				1.03
3.750	14.34	11.52	0.190	OI				1.05
3.767	14.70	11.76	0.194	OI				1.08
3.783	15.06	12.01	0.198	0 I				1.10
3.800	15.41	12.27	0.202	0 I				1.12
3.817	15.77	12.54	0.207	0 I				1.15

3.833	16.13	12.81	0.211	OI				1.17
3.850	16.49	13.09	0.216	OI				1.20
3.867	16.84	13.38	0.220	OI				1.22
3.883	17.20	13.67	0.225	O I				1.25
3.900	17.56	13.97	0.230	O I				1.28
3.917	17.92	14.27	0.235	O I				1.31
3.933	18.27	14.58	0.240	O I				1.33
3.950	18.63	14.89	0.245	O I				1.36
3.967	18.99	15.20	0.250	OI				1.39
3.983	19.34	15.52	0.256	O I				1.42
4.000	19.70	15.84	0.261	O I				1.45
4.017	22.73	16.27	0.268	O I				1.49
4.033	25.76	16.91	0.279	O I				1.55
4.050	28.78	17.74	0.292	O I				1.62
4.067	31.81	18.75	0.309	O I				1.72
4.083	34.84	19.92	0.328	O I				1.82
4.100	37.86	21.24	0.350	O I				1.94
4.117	40.89	22.69	0.374	O I				2.08
4.133	43.92	24.27	0.400	O I				2.22
4.150	46.94	25.97	0.428	O I				2.38
4.167	49.97	27.52	0.458	O I				2.52
4.183	53.00	28.39	0.490	O I				2.60
4.200	56.02	29.10	0.526	O I				2.68
4.217	59.05	29.87	0.564	O I				2.76
4.233	62.08	30.71	0.606	O I				2.85
4.250	65.10	31.61	0.651	O I				2.95
4.267	68.13	32.56	0.698	O I				3.05
4.283	64.57	33.49	0.744	O I				3.15
4.300	61.02	34.29	0.784	O I				3.24
4.317	57.46	34.97	0.818	O I				3.31
4.333	53.90	35.53	0.846	O I				3.37
4.350	50.34	37.18	0.868	O I				3.41
4.367	46.79	39.63	0.882	O I				3.44
4.383	43.23	40.79	0.888	OI				3.45
4.400	39.67	40.93	0.889	IO				3.45
4.417	36.12	40.28	0.886	I O				3.45
4.433	32.56	39.00	0.878	I O				3.43
4.450	29.00	37.23	0.868	I O				3.41
4.467	25.44	35.72	0.855	I O				3.39
4.483	21.89	35.39	0.839	I O				3.35
4.500	18.33	34.97	0.818	I O				3.31
4.517	14.77	34.47	0.793	I O				3.26
4.533	11.22	33.88	0.764	I O				3.19
4.550	10.98	33.26	0.733	I O				3.13
4.567	10.75	32.65	0.703	I O				3.06
4.583	10.52	32.05	0.673	I O				3.00
4.600	10.29	31.46	0.643	I O				2.93
4.617	10.06	30.88	0.614	I O				2.87
4.633	9.82	30.31	0.586	I O				2.81
4.650	9.59	29.74	0.558	I O				2.75

4.667	9.36	29.19	0.530	I	0		2.69
4.683	9.13	28.65	0.503	I	0		2.63
4.700	8.90	28.02	0.477	I	0		2.57
4.717	8.66	27.33	0.451	I	0		2.50
4.733	8.43	25.84	0.426	I	0		2.37
4.750	8.20	24.44	0.403	I	0		2.24
4.767	7.97	23.13	0.381	I	0		2.12
4.783	7.74	21.90	0.361	I	0		2.00
4.800	7.50	20.75	0.342	I	0		1.90
4.817	7.40	19.69	0.324	I	0		1.80
4.833	7.30	18.70	0.308	I	0		1.71
4.850	7.20	17.78	0.293	I	0		1.63
4.867	7.10	16.93	0.279	I	0		1.55
4.883	6.99	16.13	0.266	I	0		1.48
4.900	6.89	15.39	0.254	I	0		1.41
4.917	6.79	14.71	0.242	I	0		1.35
4.933	6.69	14.07	0.232	I	0		1.29
4.950	6.59	13.47	0.222	I	0		1.23
4.967	6.48	12.92	0.213	I	0		1.18
4.983	6.38	12.40	0.204	I	0		1.13
5.000	6.28	11.91	0.196	I	0		1.09
5.017	6.18	11.45	0.189	I	0		1.05
5.033	6.08	11.03	0.182	I	0		1.01
5.050	5.97	10.62	0.175	I	0		0.97
5.067	5.87	10.25	0.169	I	0		0.94
5.083	5.81	9.89	0.163	I	0		0.91
5.100	5.75	9.56	0.158	I	0		0.88
5.117	5.69	9.26	0.153	I	0		0.85
5.133	5.63	8.97	0.148	I	0		0.82
5.150	5.57	8.70	0.143	I	0		0.80
5.167	5.51	8.44	0.139	IO			0.77
5.183	5.45	8.21	0.135	IO			0.75
5.200	5.39	7.98	0.132	IO			0.73
5.217	5.34	7.77	0.128	IO			0.71
5.233	5.28	7.58	0.125	IO			0.69
5.250	5.22	7.39	0.122	IO			0.68
5.267	5.16	7.21	0.119	IO			0.66
5.283	5.10	7.04	0.116	IO			0.64
5.300	5.04	6.89	0.113	IO			0.63
5.317	4.98	6.73	0.111	IO			0.62
5.333	4.92	6.59	0.109	IO			0.60
5.350	4.88	6.46	0.106	IO			0.59
5.367	4.84	6.33	0.104	O			0.58
5.383	4.80	6.21	0.102	O			0.57
5.400	4.76	6.09	0.100	O			0.56
5.417	4.72	5.98	0.099	O			0.55
5.433	4.68	5.88	0.097	O			0.54
5.450	4.64	5.78	0.095	O			0.53
5.467	4.60	5.69	0.094	O			0.52
5.483	4.56	5.60	0.092	O			0.51

5.500	4.52	5.51	0.091	0					0.50
5.517	4.48	5.43	0.090	0					0.50
5.533	4.44	5.36	0.088	0					0.49
5.550	4.40	5.28	0.087	0					0.48
5.567	4.36	5.21	0.086	0					0.48
5.583	4.32	5.14	0.085	0					0.47
5.600	4.28	5.07	0.084	0					0.46
5.617	4.25	5.01	0.082	IO					0.46
5.633	4.22	4.94	0.081	IO					0.45
5.650	4.19	4.89	0.081	IO					0.45
5.667	4.16	4.83	0.080	IO					0.44
5.683	4.14	4.77	0.079	IO					0.44
5.700	4.11	4.72	0.078	IO					0.43
5.717	4.08	4.67	0.077	IO					0.43
5.733	4.05	4.62	0.076	IO					0.42
5.750	4.02	4.58	0.075	IO					0.42
5.767	3.99	4.53	0.075	IO					0.41
5.783	3.96	4.49	0.074	IO					0.41
5.800	3.93	4.44	0.073	IO					0.41
5.817	3.91	4.40	0.073	IO					0.40
5.833	3.88	4.36	0.072	IO					0.40
5.850	3.85	4.32	0.071	IO					0.40
5.867	3.82	4.28	0.071	IO					0.39
5.883	3.80	4.24	0.070	0					0.39
5.900	3.77	4.21	0.069	0					0.39
5.917	3.75	4.17	0.069	0					0.38
5.933	3.73	4.14	0.068	0					0.38
5.950	3.71	4.10	0.068	0					0.38
5.967	3.69	4.07	0.067	0					0.37
5.983	3.66	4.04	0.067	0					0.37
6.000	3.64	4.01	0.066	0					0.37
6.017	3.62	3.98	0.066	0					0.36
6.033	3.60	3.95	0.065	0					0.36
6.050	3.58	3.92	0.065	0					0.36
6.067	3.55	3.89	0.064	0					0.36
6.083	3.53	3.86	0.064	0					0.35
6.100	3.51	3.84	0.063	0					0.35
6.117	3.49	3.81	0.063	0					0.35
6.133	3.47	3.78	0.062	0					0.35
6.150	0.00	3.62	0.060	IO					0.33
6.167	0.00	3.33	0.055	IO					0.30
6.183	0.00	3.06	0.050	IO					0.28
6.200	0.00	2.81	0.046	IO					0.26
6.217	0.00	2.59	0.043	IO					0.24
6.233	0.00	2.38	0.039	IO					0.22
6.250	0.00	2.19	0.036	IO					0.20
6.267	0.00	2.01	0.033	0					0.18
6.283	0.00	1.85	0.031	0					0.17
6.300	0.00	1.70	0.028	0					0.16
6.317	0.00	1.57	0.026	0					0.14

6.333	0.00	1.44	0.024	0					0.13
6.350	0.00	1.33	0.022	0					0.12
6.367	0.00	1.22	0.020	0					0.11
6.383	0.00	1.12	0.018	0					0.10
6.400	0.00	1.03	0.017	0					0.09
6.417	0.00	0.95	0.016	0					0.09
6.433	0.00	0.87	0.014	0					0.08
6.450	0.00	0.80	0.013	0					0.07
6.467	0.00	0.74	0.012	0					0.07
6.483	0.00	0.68	0.011	0					0.06
6.500	0.00	0.62	0.010	0					0.06
6.517	0.00	0.57	0.009	0					0.05
6.533	0.00	0.53	0.009	0					0.05
6.550	0.00	0.49	0.008	0					0.04
6.567	0.00	0.45	0.007	0					0.04
6.583	0.00	0.41	0.007	0					0.04
6.600	0.00	0.38	0.006	0					0.03
6.617	0.00	0.35	0.006	0					0.03
6.633	0.00	0.32	0.005	0					0.03
6.650	0.00	0.29	0.005	0					0.03
6.667	0.00	0.27	0.004	0					0.02
6.683	0.00	0.25	0.004	0					0.02
6.700	0.00	0.23	0.004	0					0.02
6.717	0.00	0.21	0.003	0					0.02
6.733	0.00	0.19	0.003	0					0.02
6.750	0.00	0.18	0.003	0					0.02
6.767	0.00	0.16	0.003	0					0.01
6.783	0.00	0.15	0.002	0					0.01
6.800	0.00	0.14	0.002	0					0.01
6.817	0.00	0.13	0.002	0					0.01
6.833	0.00	0.12	0.002	0					0.01
6.850	0.00	0.11	0.002	0					0.01
6.867	0.00	0.10	0.002	0					0.01
6.883	0.00	0.09	0.002	0					0.01
6.900	0.00	0.08	0.001	0					0.01
6.917	0.00	0.08	0.001	0					0.01
6.933	0.00	0.07	0.001	0					0.01
6.950	0.00	0.07	0.001	0					0.01
6.967	0.00	0.06	0.001	0					0.01
6.983	0.00	0.06	0.001	0					0.01
7.000	0.00	0.05	0.001	0					0.00
7.017	0.00	0.05	0.001	0					0.00
7.033	0.00	0.04	0.001	0					0.00
7.050	0.00	0.04	0.001	0					0.00
7.067	0.00	0.04	0.001	0					0.00
7.083	0.00	0.03	0.001	0					0.00
7.100	0.00	0.03	0.001	0					0.00
7.117	0.00	0.03	0.000	0					0.00
7.133	0.00	0.03	0.000	0					0.00
7.150	0.00	0.02	0.000	0					0.00

7.167	0.00	0.02	0.000	0					0.00
7.183	0.00	0.02	0.000	0					0.00
7.200	0.00	0.02	0.000	0					0.00
7.217	0.00	0.02	0.000	0					0.00
7.233	0.00	0.02	0.000	0					0.00
7.250	0.00	0.01	0.000	0					0.00
7.267	0.00	0.01	0.000	0					0.00
7.283	0.00	0.01	0.000	0					0.00
7.300	0.00	0.01	0.000	0					0.00
7.317	0.00	0.01	0.000	0					0.00

*****HYDROGRAPH DATA*****

Number of intervals = 439

Time interval = 1.0 (Min.)

Maximum/Peak flow rate = 40.932 (CFS)

Total volume = 4.646 (Ac.Ft)

Status of hydrographs being held in storage

	Stream 1	Stream 2	Stream 3	Stream 4	Stream 5
Peak (CFS)	0.000	0.000	0.000	0.000	0.000
Vol (Ac.Ft)	0.000	0.000	0.000	0.000	0.000

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
 San Diego County Flood Control Division 2003 hydrology manual
 Rational Hydrology Study Date: 06/21/21

 108-001 Idaho (Escondido Estates)
 Onsite Existing Condition
 Extrapolated storm from County Isopluvials
 Routed Northern Offsite

***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
 English (in-lb) input data Units used

Map data precipitation entered:
 6 hour, precipitation(inches) = 3.300
 24 hour precipitation(inches) = 7.300
 P6/P24 = 45.2%
 San Diego hydrology manual 'C' values used

+++++
 Process from Point/Station 101.000 to Point/Station 102.000
 **** INITIAL AREA EVALUATION ****

 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Initial subarea total flow distance = 92.000(Ft.)
 Highest elevation = 765.000(Ft.)
 Lowest elevation = 753.400(Ft.)
 Elevation difference = 11.600(Ft.) Slope = 12.609 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)

for the top area slope value of 12.61 %, in a development type of
Permanent Open Space

In Accordance With Figure 3-3

Initial Area Time of Concentration = 6.19 minutes

$TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$

$TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (12.609^{(1/3)})] = 6.19$

Rainfall intensity (I) = 7.579(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.300

Subarea runoff = 0.318(CFS)

Total initial stream area = 0.140(Ac.)

+++++
Process from Point/Station 102.000 to Point/Station 103.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 1.128(CFS)

Depth of flow = 0.104(Ft.), Average velocity = 2.074(Ft/s)

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow = 1.128(CFS)

' ' flow top width = 10.429(Ft.)

' ' velocity = 2.074(Ft/s)

' ' area = 0.544(Sq.Ft)

' ' Froude number = 1.600

Upstream point elevation = 753.400(Ft.)

Downstream point elevation = 735.000(Ft.)

Flow length = 115.000(Ft.)

Travel time = 0.92 min.

Time of concentration = 7.11 min.

Depth of flow = 0.104(Ft.)

Average velocity = 2.074(Ft/s)

Total irregular channel flow = 1.128(CFS)

Irregular channel normal depth above invert elev. = 0.104(Ft.)

Average velocity of channel(s) = 2.074(Ft/s)

Adding area flow to channel

Rainfall intensity (I) = 6.928(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[UNDISTURBED NATURAL TERRAIN]

(Permanent Open Space)
 Impervious value, $A_i = 0.000$
 Sub-Area C Value = 0.300
 Rainfall intensity = 6.928(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 ($Q=KCIA$) is $C = 0.300$ $CA = 0.267$
 Subarea runoff = 1.531(CFS) for 0.750(Ac.)
 Total runoff = 1.850(CFS) Total area = 0.890(Ac.)
 Depth of flow = 0.126(Ft.), Average velocity = 2.347(Ft/s)

+++++
 Process from Point/Station 103.000 to Point/Station 104.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

 Estimated mean flow rate at midpoint of channel = 3.206(CFS)
 Depth of flow = 0.189(Ft.), Average velocity = 1.789(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

 Manning's 'N' friction factor = 0.040

Sub-Channel flow = 3.206(CFS)
 ' ' flow top width = 18.934(Ft.)
 ' ' velocity = 1.789(Ft/s)
 ' ' area = 1.792(Sq.Ft)
 ' ' Froude number = 1.024

Upstream point elevation = 735.000(Ft.)
 Downstream point elevation = 697.600(Ft.)
 Flow length = 696.000(Ft.)
 Travel time = 6.49 min.
 Time of concentration = 13.60 min.
 Depth of flow = 0.189(Ft.)
 Average velocity = 1.789(Ft/s)
 Total irregular channel flow = 3.206(CFS)
 Irregular channel normal depth above invert elev. = 0.189(Ft.)
 Average velocity of channel(s) = 1.789(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 4.561(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)

Impervious value, $A_i = 0.000$
 Sub-Area C Value = 0.300
 Rainfall intensity = 4.561(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is $C = 0.300$ $CA = 0.981$
 Subarea runoff = 2.624(CFS) for 2.380(Ac.)
 Total runoff = 4.474(CFS) Total area = 3.270(Ac.)
 Depth of flow = 0.215(Ft.), Average velocity = 1.944(Ft/s)

++++++
 Process from Point/Station 104.000 to Point/Station 104.000
 **** SUBAREA FLOW ADDITION ****

Rainfall intensity (I) = 4.561(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [COMMERCIAL area type]
 (General Commercial)
 Impervious value, $A_i = 0.850$
 Sub-Area C Value = 0.810
 Time of concentration = 13.60 min.
 Rainfall intensity = 4.561(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is $C = 0.339$ $CA = 1.200$
 Subarea runoff = 0.997(CFS) for 0.270(Ac.)
 Total runoff = 5.471(CFS) Total area = 3.540(Ac.)

++++++
 Process from Point/Station 104.000 to Point/Station 104.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 3.540(Ac.)
 Runoff from this stream = 5.471(CFS)
 Time of concentration = 13.60 min.
 Rainfall intensity = 4.561(In/Hr)
 Program is now starting with Main Stream No. 2

++++++
 Process from Point/Station 104.000 to Point/Station 104.000
 **** USER DEFINED FLOW INFORMATION AT A POINT ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Rainfall intensity (I) = 3.946(In/Hr) for a 100.0 year storm
 User specified values are as follows:
 TC = 17.02 min. Rain intensity = 3.95(In/Hr)
 Total area = 57.330(Ac.) Total runoff = 40.932(CFS)

++++++
 Process from Point/Station 104.000 to Point/Station 104.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 57.330(Ac.)
 Runoff from this stream = 40.932(CFS)
 Time of concentration = 17.02 min.
 Rainfall intensity = 3.946(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	5.471	13.60	4.561
2	40.932	17.02	3.946

Qmax(1) =

$1.000 * 1.000 * 5.471) +$
 $1.000 * 0.799 * 40.932) + = 38.171$

Qmax(2) =

$0.865 * 1.000 * 5.471) +$
 $1.000 * 1.000 * 40.932) + = 45.666$

Total of 2 main streams to confluence:

Flow rates before confluence point:

5.471 40.932

Maximum flow rates at confluence using above data:

38.171 45.666

Area of streams before confluence:

3.540 57.330

Results of confluence:

Total flow rate = 45.666(CFS)

Time of concentration = 17.020 min.

Effective stream area after confluence = 60.870(Ac.)

+++++
 Process from Point/Station 104.000 to Point/Station 107.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 57.508(CFS)
 Depth of flow = 1.065(Ft.), Average velocity = 3.377(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :
 Point number 'X' coordinate 'Y' coordinate
 1 0.00 5.00
 2 75.00 0.00
 3 150.00 5.00
 Manning's 'N' friction factor = 0.040

 Sub-Channel flow = 57.508(CFS)
 ' ' flow top width = 31.964(Ft.)
 ' ' velocity = 3.377(Ft/s)
 ' ' area = 17.029(Sq.Ft)
 ' ' Froude number = 0.815

Upstream point elevation = 697.600(Ft.)
 Downstream point elevation = 690.000(Ft.)
 Flow length = 396.000(Ft.)
 Travel time = 1.95 min.
 Time of concentration = 18.97 min.
 Depth of flow = 1.065(Ft.)
 Average velocity = 3.377(Ft/s)
 Total irregular channel flow = 57.508(CFS)
 Irregular channel normal depth above invert elev. = 1.065(Ft.)
 Average velocity of channel(s) = 3.377(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 3.678(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Rainfall intensity = 3.678(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.302 CA = 18.879
 Subarea runoff = 23.780(CFS) for 1.600(Ac.)
 Total runoff = 69.445(CFS) Total area = 62.470(Ac.)
 Depth of flow = 1.144(Ft.), Average velocity = 3.540(Ft/s)

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+++++
Process from Point/Station      107.000 to Point/Station      107.000
**** CONFLUENCE OF MAIN STREAMS ****

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The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 62.470(Ac.)
 Runoff from this stream = 69.445(CFS)
 Time of concentration = 18.97 min.
 Rainfall intensity = 3.678(In/Hr)
 Program is now starting with Main Stream No. 2

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+++++
Process from Point/Station      105.000 to Point/Station      106.000
**** INITIAL AREA EVALUATION ****

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Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Initial subarea total flow distance = 93.000(Ft.)
 Highest elevation = 710.100(Ft.)
 Lowest elevation = 706.000(Ft.)
 Elevation difference = 4.100(Ft.) Slope = 4.409 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 4.41 %, in a development type of
 Permanent Open Space
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 8.78 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (4.409^{(1/3)})] = 8.78$
 Rainfall intensity (I) = 6.046(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.300
 Subarea runoff = 0.091(CFS)
 Total initial stream area = 0.050(Ac.)

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+++++
Process from Point/Station      106.000 to Point/Station      107.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

```

Estimated mean flow rate at midpoint of channel = 0.512(CFS)
 Depth of flow = 0.216(Ft.), Average velocity = 1.561(Ft/s)

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	7.00	0.00
3	14.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow	=	0.512(CFS)
'	'	flow top width = 3.030(Ft.)
'	'	velocity = 1.561(Ft/s)
'	'	area = 0.328(Sq.Ft)
'	'	Froude number = 0.836

Upstream point elevation = 706.000(Ft.)

Downstream point elevation = 690.000(Ft.)

Flow length = 461.000(Ft.)

Travel time = 4.92 min.

Time of concentration = 13.70 min.

Depth of flow = 0.216(Ft.)

Average velocity = 1.561(Ft/s)

Total irregular channel flow = 0.512(CFS)

Irregular channel normal depth above invert elev. = 0.216(Ft.)

Average velocity of channel(s) = 1.561(Ft/s)

Adding area flow to channel

Rainfall intensity (I) = 4.538(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[UNDISTURBED NATURAL TERRAIN]

(Permanent Open Space)

Impervious value, Ai = 0.000

Sub-Area C Value = 0.300

Rainfall intensity = 4.538(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for total area

(Q=KCIA) is C = 0.300 CA = 0.189

Subarea runoff = 0.767(CFS) for 0.580(Ac.)

Total runoff = 0.858(CFS) Total area = 0.630(Ac.)

Depth of flow = 0.263(Ft.), Average velocity = 1.776(Ft/s)

+++++

Process from Point/Station 107.000 to Point/Station 107.000

**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2

Stream flow area = 0.630(Ac.)

Runoff from this stream = 0.858(CFS)

Time of concentration = 13.70 min.

Rainfall intensity = 4.538(In/Hr)

Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	69.445	18.97	3.678
---	--------	-------	-------

2	0.858	13.70	4.538
---	-------	-------	-------

Qmax(1) =

1.000 *	1.000 *	69.445)	+	
0.811 *	1.000 *	0.858)	+	70.140

Qmax(2) =

1.000 *	0.722 *	69.445)	+	
1.000 *	1.000 *	0.858)	+	51.013

Total of 2 main streams to confluence:

Flow rates before confluence point:

69.445	0.858
--------	-------

Maximum flow rates at confluence using above data:

70.140	51.013
--------	--------

Area of streams before confluence:

62.470	0.630
--------	-------

Results of confluence:

Total flow rate = 70.140(CFS)

Time of concentration = 18.974 min.

Effective stream area after confluence = 63.100(Ac.)

+++++

Process from Point/Station 107.000 to Point/Station 114.000

**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

!!Warning: Water is above left or right bank elevations

!!Warning: Water is above left or right bank elevations

!!Warning: Water is above left or right bank elevations

Estimated mean flow rate at midpoint of channel = 70.170(CFS)

Depth of flow = 1.522(Ft.), Average velocity = 4.903(Ft/s)

!!Warning: Water is above left or right bank elevations

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
--------------	----------------	----------------

1	0.00	1.00
---	------	------

2	7.00	0.00
---	------	------

3	14.00	1.00
---	-------	------

Manning's 'N' friction factor = 0.040

 Sub-Channel flow = 70.170(CFS)
 ' ' flow top width = 14.000(Ft.)
 ' ' velocity = 4.903(Ft/s)
 ' ' area = 14.312(Sq.Ft)
 ' ' Froude number = 0.855

Upstream point elevation = 690.000(Ft.)
 Downstream point elevation = 687.000(Ft.)
 Flow length = 175.000(Ft.)
 Travel time = 0.59 min.
 Time of concentration = 19.57 min.
 Depth of flow = 1.522(Ft.)
 Average velocity = 4.903(Ft/s)
 Total irregular channel flow = 70.170(CFS)
 Irregular channel normal depth above invert elev. = 1.522(Ft.)
 Average velocity of channel(s) = 4.903(Ft/s)
 !!Warning: Water is above left or right bank elevations
 Adding area flow to channel
 Rainfall intensity (I) = 3.606(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 The area added to the existing stream causes a
 a lower flow rate of Q = 68.985(CFS)
 therefore the upstream flow rate of Q = 70.140(CFS) is being used
 Rainfall intensity = 3.606(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.302 CA = 19.131
 Subarea runoff = 0.000(CFS) for 0.210(Ac.)
 Total runoff = 70.140(CFS) Total area = 63.310(Ac.)
 Depth of flow = 1.522(Ft.), Average velocity = 4.902(Ft/s)
 !!Warning: Water is above left or right bank elevations

++++
 Process from Point/Station 114.000 to Point/Station 114.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 63.310(Ac.)
 Runoff from this stream = 70.140(CFS)
 Time of concentration = 19.57 min.

Rainfall intensity = 3.606(In/Hr)
 Program is now starting with Main Stream No. 2

+++++
 Process from Point/Station 108.000 to Point/Station 109.000
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Initial subarea total flow distance = 96.000(Ft.)
 Highest elevation = 767.000(Ft.)
 Lowest elevation = 759.300(Ft.)
 Elevation difference = 7.700(Ft.) Slope = 8.021 %
 Top of Initial Area Slope adjusted by User to 14.896 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 14.90 %, in a development type of
 Permanent Open Space
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.85 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (14.896^{(1/3)})] = 5.85$
 Rainfall intensity (I) = 7.855(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.300
 Subarea runoff = 0.165(CFS)
 Total initial stream area = 0.070(Ac.)

+++++
 Process from Point/Station 109.000 to Point/Station 110.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 0.365(CFS)
 Depth of flow = 0.095(Ft.), Average velocity = 1.622(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	25.00	0.00
3	50.00	1.00

 Manning's 'N' friction factor = 0.040

Sub-Channel flow = 0.365(CFS)
 ' ' flow top width = 4.745(Ft.)
 ' ' velocity = 1.622(Ft/s)
 ' ' area = 0.225(Sq.Ft)
 ' ' Froude number = 1.312

 Upstream point elevation = 759.300(Ft.)
 Downstream point elevation = 754.300(Ft.)
 Flow length = 45.000(Ft.)
 Travel time = 0.46 min.
 Time of concentration = 6.31 min.
 Depth of flow = 0.095(Ft.)
 Average velocity = 1.622(Ft/s)
 Total irregular channel flow = 0.365(CFS)
 Irregular channel normal depth above invert elev. = 0.095(Ft.)
 Average velocity of channel(s) = 1.622(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 7.479(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Rainfall intensity = 7.479(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.300 CA = 0.072
 Subarea runoff = 0.374(CFS) for 0.170(Ac.)
 Total runoff = 0.539(CFS) Total area = 0.240(Ac.)
 Depth of flow = 0.110(Ft.), Average velocity = 1.788(Ft/s)

++++++
 Process from Point/Station 110.000 to Point/Station 114.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 4.773(CFS)
 Depth of flow = 0.197(Ft.), Average velocity = 2.463(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :
 Point number 'X' coordinate 'Y' coordinate
 1 0.00 1.00
 2 50.00 0.00
 3 100.00 1.00
 Manning's 'N' friction factor = 0.040

Sub-Channel flow = 4.774(CFS)

```

'      '      flow top width =      19.689(Ft.)
'      '      velocity=      2.463(Ft/s)
'      '      area =      1.938(Sq.Ft)
'      '      Froude number =      1.383

```

```

Upstream point elevation = 754.300(Ft.)
Downstream point elevation = 687.000(Ft.)
Flow length = 696.000(Ft.)
Travel time = 4.71 min.
Time of concentration = 11.03 min.
Depth of flow = 0.197(Ft.)
Average velocity = 2.463(Ft/s)
Total irregular channel flow = 4.773(CFS)
Irregular channel normal depth above invert elev. = 0.197(Ft.)
Average velocity of channel(s) = 2.463(Ft/s)
Adding area flow to channel
Rainfall intensity (I) = 5.221(In/Hr) for a 100.0 year storm
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[UNDISTURBED NATURAL TERRAIN ]
(Permanent Open Space )
Impervious value, Ai = 0.000
Sub-Area C Value = 0.300
Rainfall intensity = 5.221(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for total area
(Q=KCIA) is C = 0.300 CA = 1.710
Subarea runoff = 8.389(CFS) for 5.460(Ac.)
Total runoff = 8.928(CFS) Total area = 5.700(Ac.)
Depth of flow = 0.249(Ft.), Average velocity = 2.880(Ft/s)

```

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+++++
Process from Point/Station 114.000 to Point/Station 114.000
**** CONFLUENCE OF MAIN STREAMS ****

```

The following data inside Main Stream is listed:

```

In Main Stream number: 2
Stream flow area = 5.700(Ac.)
Runoff from this stream = 8.928(CFS)
Time of concentration = 11.03 min.
Rainfall intensity = 5.221(In/Hr)
Program is now starting with Main Stream No. 3

```

```

+++++
Process from Point/Station 111.000 to Point/Station 112.000
**** INITIAL AREA EVALUATION ****

```

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Initial subarea total flow distance = 100.000(Ft.)
 Highest elevation = 767.000(Ft.)
 Lowest elevation = 759.100(Ft.)
 Elevation difference = 7.900(Ft.) Slope = 7.900 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 7.90 %, in a development type of
 Permanent Open Space
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 7.23 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (7.900^{(1/3)})] = 7.23$
 Rainfall intensity (I) = 6.854(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.300
 Subarea runoff = 0.185(CFS)
 Total initial stream area = 0.090(Ac.)

++++++
 Process from Point/Station 112.000 to Point/Station 113.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 2.786(CFS)
 Depth of flow = 0.174(Ft.), Average velocity = 1.839(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

 Manning's 'N' friction factor = 0.040

Sub-Channel flow = 2.786(CFS)

'	'	flow top width =	17.407(Ft.)
'	'	velocity=	1.839(Ft/s)
'	'	area =	1.515(Sq.Ft)
'	'	Froude number =	1.098

Upstream point elevation = 759.100(Ft.)
 Downstream point elevation = 730.200(Ft.)
 Flow length = 455.000(Ft.)

Travel time = 4.12 min.
 Time of concentration = 11.35 min.
 Depth of flow = 0.174(Ft.)
 Average velocity = 1.839(Ft/s)
 Total irregular channel flow = 2.786(CFS)
 Irregular channel normal depth above invert elev. = 0.174(Ft.)
 Average velocity of channel(s) = 1.839(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 5.123(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Rainfall intensity = 5.123(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.300 CA = 1.038
 Subarea runoff = 5.132(CFS) for 3.370(Ac.)
 Total runoff = 5.317(CFS) Total area = 3.460(Ac.)
 Depth of flow = 0.222(Ft.), Average velocity = 2.161(Ft/s)

++++++
 Process from Point/Station 113.000 to Point/Station 114.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

 Estimated mean flow rate at midpoint of channel = 5.815(CFS)
 Depth of flow = 0.225(Ft.), Average velocity = 2.301(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow = 5.815(CFS)
 ' ' flow top width = 22.484(Ft.)
 ' ' velocity = 2.301(Ft/s)
 ' ' area = 2.528(Sq.Ft)
 ' ' Froude number = 1.209

Upstream point elevation = 730.200(Ft.)
 Downstream point elevation = 687.000(Ft.)
 Flow length = 611.000(Ft.)
 Travel time = 4.43 min.

Time of concentration = 15.78 min.
 Depth of flow = 0.225(Ft.)
 Average velocity = 2.301(Ft/s)
 Total irregular channel flow = 5.815(CFS)
 Irregular channel normal depth above invert elev. = 0.225(Ft.)
 Average velocity of channel(s) = 2.301(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 4.143(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Rainfall intensity = 4.143(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.300 CA = 1.500
 Subarea runoff = 0.897(CFS) for 1.540(Ac.)
 Total runoff = 6.214(CFS) Total area = 5.000(Ac.)
 Depth of flow = 0.231(Ft.), Average velocity = 2.339(Ft/s)

++++++
 Process from Point/Station 114.000 to Point/Station 114.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 3
 Stream flow area = 5.000(Ac.)
 Runoff from this stream = 6.214(CFS)
 Time of concentration = 15.78 min.
 Rainfall intensity = 4.143(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	70.140	19.57	3.606
2	8.928	11.03	5.221
3	6.214	15.78	4.143

Qmax(1) =
 1.000 * 1.000 * 70.140) +
 0.691 * 1.000 * 8.928) +
 0.870 * 1.000 * 6.214) + = 81.716

Qmax(2) =
 1.000 * 0.563 * 70.140) +
 1.000 * 1.000 * 8.928) +

$1.000 * 0.699 * 6.214) + = 52.786$
 $Q_{max}(3) =$
 $1.000 * 0.806 * 70.140) +$
 $0.793 * 1.000 * 8.928) +$
 $1.000 * 1.000 * 6.214) + = 69.862$

Total of 3 main streams to confluence:

Flow rates before confluence point:

70.140 8.928 6.214

Maximum flow rates at confluence using above data:

81.716 52.786 69.862

Area of streams before confluence:

63.310 5.700 5.000

Results of confluence:

Total flow rate = 81.716(CFS)

Time of concentration = 19.569 min.

Effective stream area after confluence = 74.010(Ac.)

++++++
 Process from Point/Station 114.000 to Point/Station 114.000
 **** 6 HOUR HYDROGRAPH ****

++++++
 Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 19.57

Basin Area = 74.01 Acres

6 Hour Rainfall = 3.300 Inches

Runoff Coefficient = 0.302

Peak Discharge = 81.72 CFS

Time (Min)	Discharge (CFS)
0	0.000
19	4.291
38	4.602
57	4.780
76	5.195
95	5.441
114	6.034
133	6.399
152	7.335
171	7.953
190	9.721
209	11.073
228	16.258
247	22.907
266	81.716

285	13.040
304	8.725
323	6.826
342	5.718
361	4.977

+++++

6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	20.4	40.9	61.3	81.7
0+ 0	0.0000		0.00	Q				
0+ 1	0.0003		0.23	Q				
0+ 2	0.0009		0.45	Q				
0+ 3	0.0019		0.68	Q				
0+ 4	0.0031		0.90	Q				
0+ 5	0.0047		1.13	Q				
0+ 6	0.0065		1.36	Q				
0+ 7	0.0087		1.58	Q				
0+ 8	0.0112		1.81	Q				
0+ 9	0.0140		2.03	Q				
0+10	0.0171		2.26	VQ				
0+11	0.0205		2.48	VQ				
0+12	0.0243		2.71	VQ				
0+13	0.0283		2.94	VQ				
0+14	0.0327		3.16	VQ				
0+15	0.0373		3.39	VQ				
0+16	0.0423		3.61	VQ				
0+17	0.0476		3.84	VQ				
0+18	0.0532		4.07	VQ				
0+19	0.0591		4.29	V Q				
0+20	0.0650		4.31	V Q				
0+21	0.0710		4.32	V Q				
0+22	0.0770		4.34	V Q				
0+23	0.0830		4.36	V Q				
0+24	0.0890		4.37	V Q				
0+25	0.0950		4.39	V Q				
0+26	0.1011		4.41	V Q				
0+27	0.1072		4.42	V Q				
0+28	0.1133		4.44	V Q				
0+29	0.1195		4.45	V Q				
0+30	0.1256		4.47	V Q				
0+31	0.1318		4.49	V Q				
0+32	0.1380		4.50	V Q				
0+33	0.1442		4.52	V Q				
0+34	0.1505		4.54	V Q				
0+35	0.1567		4.55	VQ				

0+36	0.1630	4.57	VQ				
0+37	0.1694	4.59	VQ				
0+38	0.1757	4.60	VQ				
0+39	0.1820	4.61	VQ				
0+40	0.1884	4.62	VQ				
0+41	0.1948	4.63	VQ				
0+42	0.2012	4.64	VQ				
0+43	0.2076	4.65	VQ				
0+44	0.2140	4.66	VQ				
0+45	0.2204	4.67	VQ				
0+46	0.2269	4.68	VQ				
0+47	0.2333	4.69	VQ				
0+48	0.2398	4.70	VQ				
0+49	0.2463	4.70	VQ				
0+50	0.2528	4.71	VQ				
0+51	0.2593	4.72	VQ				
0+52	0.2658	4.73	VQ				
0+53	0.2723	4.74	VQ				
0+54	0.2789	4.75	VQ				
0+55	0.2854	4.76	VQ				
0+56	0.2920	4.77	VQ				
0+57	0.2986	4.78	VQ				
0+58	0.3052	4.80	Q				
0+59	0.3118	4.82	Q				
1+ 0	0.3185	4.85	Q				
1+ 1	0.3252	4.87	Q				
1+ 2	0.3319	4.89	Q				
1+ 3	0.3387	4.91	Q				
1+ 4	0.3455	4.93	Q				
1+ 5	0.3523	4.95	Q				
1+ 6	0.3592	4.98	Q				
1+ 7	0.3661	5.00	Q				
1+ 8	0.3730	5.02	Q				
1+ 9	0.3799	5.04	Q				
1+10	0.3869	5.06	Q				
1+11	0.3939	5.09	Q				
1+12	0.4009	5.11	Q				
1+13	0.4080	5.13	Q				
1+14	0.4151	5.15	Q				
1+15	0.4222	5.17	Q				
1+16	0.4294	5.20	Q				
1+17	0.4366	5.21	Q				
1+18	0.4438	5.22	Q				
1+19	0.4510	5.23	Q				
1+20	0.4582	5.25	QV				
1+21	0.4654	5.26	QV				
1+22	0.4727	5.27	QV				
1+23	0.4800	5.29	QV				
1+24	0.4873	5.30	QV				
1+25	0.4946	5.31	QV				

1+26	0.5019	5.32	QV				
1+27	0.5093	5.34	QV				
1+28	0.5167	5.35	QV				
1+29	0.5240	5.36	QV				
1+30	0.5314	5.38	QV				
1+31	0.5389	5.39	QV				
1+32	0.5463	5.40	QV				
1+33	0.5538	5.41	QV				
1+34	0.5612	5.43	QV				
1+35	0.5687	5.44	QV				
1+36	0.5763	5.47	QV				
1+37	0.5839	5.50	QV				
1+38	0.5915	5.53	QV				
1+39	0.5991	5.57	QV				
1+40	0.6069	5.60	Q V				
1+41	0.6146	5.63	Q V				
1+42	0.6224	5.66	Q V				
1+43	0.6302	5.69	Q V				
1+44	0.6381	5.72	Q V				
1+45	0.6460	5.75	Q V				
1+46	0.6540	5.78	Q V				
1+47	0.6620	5.82	Q V				
1+48	0.6701	5.85	Q V				
1+49	0.6782	5.88	Q V				
1+50	0.6863	5.91	Q V				
1+51	0.6945	5.94	Q V				
1+52	0.7027	5.97	Q V				
1+53	0.7110	6.00	Q V				
1+54	0.7193	6.03	Q V				
1+55	0.7276	6.05	Q V				
1+56	0.7360	6.07	Q V				
1+57	0.7444	6.09	Q V				
1+58	0.7528	6.11	Q V				
1+59	0.7613	6.13	Q V				
2+ 0	0.7697	6.15	Q V				
2+ 1	0.7782	6.17	Q V				
2+ 2	0.7867	6.19	Q V				
2+ 3	0.7953	6.21	Q V				
2+ 4	0.8039	6.23	Q V				
2+ 5	0.8125	6.25	Q V				
2+ 6	0.8211	6.26	Q V				
2+ 7	0.8298	6.28	Q V				
2+ 8	0.8384	6.30	Q V				
2+ 9	0.8471	6.32	Q V				
2+10	0.8559	6.34	Q V				
2+11	0.8646	6.36	Q V				
2+12	0.8734	6.38	Q V				
2+13	0.8822	6.40	Q V				
2+14	0.8911	6.45	Q V				
2+15	0.9001	6.50	Q V				

2+16	0.9091	6.55	Q	V				
2+17	0.9182	6.60	Q	V				
2+18	0.9273	6.65	Q	V				
2+19	0.9366	6.69	Q	V				
2+20	0.9458	6.74	Q	V				
2+21	0.9552	6.79	Q	V				
2+22	0.9646	6.84	Q	V				
2+23	0.9741	6.89	Q	V				
2+24	0.9837	6.94	Q	V				
2+25	0.9933	6.99	Q	V				
2+26	1.0030	7.04	Q	V				
2+27	1.0128	7.09	Q	V				
2+28	1.0226	7.14	Q	V				
2+29	1.0325	7.19	Q	V				
2+30	1.0425	7.24	Q	V				
2+31	1.0525	7.29	Q	V				
2+32	1.0626	7.34	Q	V				
2+33	1.0728	7.37	Q	V				
2+34	1.0830	7.40	Q	V				
2+35	1.0932	7.43	Q	V				
2+36	1.1035	7.47	Q	V				
2+37	1.1138	7.50	Q	V				
2+38	1.1242	7.53	Q	V				
2+39	1.1346	7.56	Q	V				
2+40	1.1451	7.60	Q	V				
2+41	1.1556	7.63	Q	V				
2+42	1.1661	7.66	Q	V				
2+43	1.1767	7.69	Q	V				
2+44	1.1873	7.73	Q	V				
2+45	1.1980	7.76	Q	V				
2+46	1.2088	7.79	Q	V				
2+47	1.2195	7.82	Q	V				
2+48	1.2304	7.86	Q	V				
2+49	1.2412	7.89	Q	V				
2+50	1.2521	7.92	Q	V				
2+51	1.2631	7.95	Q	V				
2+52	1.2742	8.05	Q	V				
2+53	1.2854	8.14	Q	V				
2+54	1.2967	8.23	Q	V				
2+55	1.3082	8.33	Q	V				
2+56	1.3198	8.42	Q	V				
2+57	1.3315	8.51	Q	V				
2+58	1.3434	8.60	Q	V				
2+59	1.3553	8.70	Q	V				
3+ 0	1.3675	8.79	Q	V				
3+ 1	1.3797	8.88	Q	V				
3+ 2	1.3921	8.98	Q	V				
3+ 3	1.4045	9.07	Q	V				
3+ 4	1.4172	9.16	Q	V				
3+ 5	1.4299	9.26	Q	V				

3+ 6	1.4428	9.35	Q	V				
3+ 7	1.4558	9.44	Q	V				
3+ 8	1.4689	9.54	Q	V				
3+ 9	1.4822	9.63	Q	V				
3+10	1.4956	9.72	Q	V				
3+11	1.5091	9.79	Q	V				
3+12	1.5227	9.86	Q	V				
3+13	1.5363	9.93	Q	V				
3+14	1.5501	10.01	Q	V				
3+15	1.5640	10.08	Q	V				
3+16	1.5780	10.15	Q	V				
3+17	1.5921	10.22	Q	V				
3+18	1.6062	10.29	Q	V				
3+19	1.6205	10.36	Q	V				
3+20	1.6349	10.43	Q	V				
3+21	1.6493	10.50	Q	V				
3+22	1.6639	10.57	Q	V				
3+23	1.6786	10.65	Q	V				
3+24	1.6933	10.72	Q	V				
3+25	1.7082	10.79	Q	V				
3+26	1.7231	10.86	Q	V				
3+27	1.7382	10.93	Q	V				
3+28	1.7534	11.00	Q	V				
3+29	1.7686	11.07	Q	V				
3+30	1.7842	11.35	Q	V				
3+31	1.8002	11.62	Q	V				
3+32	1.8166	11.89	Q	V				
3+33	1.8334	12.16	Q	V				
3+34	1.8505	12.44	Q	V				
3+35	1.8680	12.71	Q	V				
3+36	1.8859	12.98	Q	V				
3+37	1.9042	13.26	Q	V				
3+38	1.9228	13.53	Q	V				
3+39	1.9418	13.80	Q	V				
3+40	1.9612	14.07	Q	V				
3+41	1.9809	14.35	Q	V				
3+42	2.0011	14.62	Q	V				
3+43	2.0216	14.89	Q	V				
3+44	2.0425	15.17	Q	V				
3+45	2.0638	15.44	Q	V				
3+46	2.0854	15.71	Q	V				
3+47	2.1074	15.99	Q	V				
3+48	2.1298	16.26	Q	V				
3+49	2.1527	16.61	Q	V				
3+50	2.1760	16.96	Q	V				
3+51	2.1999	17.31	Q	V				
3+52	2.2242	17.66	Q	V				
3+53	2.2490	18.01	Q	V				
3+54	2.2743	18.36	Q	V				
3+55	2.3001	18.71	Q	V				

3+56	2.3263	19.06	Q	V			
3+57	2.3530	19.41	Q	V			
3+58	2.3803	19.76	Q	V			
3+59	2.4080	20.11	Q	V			
4+ 0	2.4361	20.46	Q	V			
4+ 1	2.4648	20.81	Q	V			
4+ 2	2.4939	21.16	Q	V			
4+ 3	2.5236	21.51	Q	V			
4+ 4	2.5537	21.86	Q	V			
4+ 5	2.5843	22.21	Q	V			
4+ 6	2.6153	22.56	Q	V			
4+ 7	2.6469	22.91	Q	V			
4+ 8	2.6827	26.00	Q	V			
4+ 9	2.7228	29.10	Q	V			
4+10	2.7671	32.19	Q	V			
4+11	2.8157	35.29	Q	V			
4+12	2.8686	38.38	Q	V			
4+13	2.9257	41.48	Q	V			
4+14	2.9871	44.57	Q	V			
4+15	3.0528	47.67	Q	V			
4+16	3.1227	50.76	Q	V			
4+17	3.1969	53.86	Q	V			
4+18	3.2753	56.95	Q	V			
4+19	3.3580	60.05	Q	V			
4+20	3.4450	63.14	Q	V			
4+21	3.5363	66.24	Q	V			
4+22	3.6318	69.33	Q	V			
4+23	3.7315	72.43	Q	V			
4+24	3.8356	75.53	Q	V			
4+25	3.9439	78.62	Q	V			
4+26	4.0564	81.72	Q	V			
4+27	4.1640	78.10	Q	V			
4+28	4.2666	74.49	Q	V			
4+29	4.3642	70.87	Q	V			
4+30	4.4568	67.26	Q	V			
4+31	4.5445	63.64	Q	V			
4+32	4.6272	60.03	Q	V			
4+33	4.7049	56.41	Q	V			
4+34	4.7776	52.80	Q	V			
4+35	4.8454	49.18	Q	V			
4+36	4.9081	45.57	Q	V			
4+37	4.9659	41.96	Q	V			
4+38	5.0187	38.34	Q	V			
4+39	5.0666	34.73	Q	V			
4+40	5.1094	31.11	Q	V			
4+41	5.1473	27.50	Q	V			
4+42	5.1802	23.88	Q	V			
4+43	5.2081	20.27	Q	V			
4+44	5.2311	16.65	Q	V			
4+45	5.2490	13.04	Q	V			

4+46	5.2667	12.81	Q			V
4+47	5.2840	12.59	Q			V
4+48	5.3010	12.36	Q			V
4+49	5.3177	12.13	Q			V
4+50	5.3341	11.90	Q			V
4+51	5.3502	11.68	Q			V
4+52	5.3660	11.45	Q			V
4+53	5.3815	11.22	Q			V
4+54	5.3966	11.00	Q			V
4+55	5.4114	10.77	Q			V
4+56	5.4260	10.54	Q			V
4+57	5.4402	10.31	Q			V
4+58	5.4541	10.09	Q			V
4+59	5.4676	9.86	Q			V
5+ 0	5.4809	9.63	Q			V
5+ 1	5.4939	9.41	Q			V
5+ 2	5.5065	9.18	Q			V
5+ 3	5.5188	8.95	Q			V
5+ 4	5.5308	8.72	Q			V
5+ 5	5.5427	8.62	Q			V
5+ 6	5.5545	8.52	Q			V
5+ 7	5.5661	8.42	Q			V
5+ 8	5.5775	8.32	Q			V
5+ 9	5.5889	8.23	Q			V
5+10	5.6001	8.13	Q			V
5+11	5.6111	8.03	Q			V
5+12	5.6220	7.93	Q			V
5+13	5.6328	7.83	Q			V
5+14	5.6435	7.73	Q			V
5+15	5.6540	7.63	Q			V
5+16	5.6643	7.53	Q			V
5+17	5.6746	7.43	Q			V
5+18	5.6846	7.33	Q			V
5+19	5.6946	7.23	Q			V
5+20	5.7044	7.13	Q			V
5+21	5.7141	7.03	Q			V
5+22	5.7236	6.93	Q			V
5+23	5.7330	6.83	Q			V
5+24	5.7424	6.77	Q			V
5+25	5.7516	6.71	Q			V
5+26	5.7608	6.65	Q			V
5+27	5.7698	6.59	Q			V
5+28	5.7788	6.53	Q			V
5+29	5.7878	6.48	Q			V
5+30	5.7966	6.42	Q			V
5+31	5.8054	6.36	Q			V
5+32	5.8140	6.30	Q			V
5+33	5.8226	6.24	Q			V
5+34	5.8312	6.18	Q			V
5+35	5.8396	6.13	Q			V

5+36	5.8480	6.07	Q				V
5+37	5.8562	6.01	Q				V
5+38	5.8644	5.95	Q				V
5+39	5.8725	5.89	Q				V
5+40	5.8806	5.83	Q				V
5+41	5.8885	5.78	Q				V
5+42	5.8964	5.72	Q				V
5+43	5.9042	5.68	Q				V
5+44	5.9120	5.64	Q				V
5+45	5.9197	5.60	Q				V
5+46	5.9274	5.56	Q				V
5+47	5.9350	5.52	Q				V
5+48	5.9425	5.48	Q				V
5+49	5.9500	5.44	Q				V
5+50	5.9575	5.41	Q				V
5+51	5.9649	5.37	Q				V
5+52	5.9722	5.33	Q				V
5+53	5.9795	5.29	Q				V
5+54	5.9867	5.25	Q				V
5+55	5.9939	5.21	Q				V
5+56	6.0010	5.17	Q				V
5+57	6.0081	5.13	Q				V
5+58	6.0151	5.09	Q				V
5+59	6.0221	5.05	Q				V
6+ 0	6.0290	5.02	Q				V
6+ 1	6.0358	4.98	Q				V

End of computations, total study area = 74.010 (Ac.)

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 06/21/21

108-001 Idaho (Escondido Estates)
Onsite Proposed Condition 301-401
Extrapolated storm from County Isopluvials
TH

***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:
6 hour, precipitation(inches) = 3.300
24 hour precipitation(inches) = 7.300
P6/P24 = 45.2%
San Diego hydrology manual 'C' values used

+++++
Process from Point/Station 301.000 to Point/Station 302.000
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[UNDISTURBED NATURAL TERRAIN]
(Permanent Open Space)
Impervious value, Ai = 0.000
Sub-Area C Value = 0.300
Initial subarea total flow distance = 92.000(Ft.)
Highest elevation = 765.000(Ft.)
Lowest elevation = 753.400(Ft.)
Elevation difference = 11.600(Ft.) Slope = 12.609 %
INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
The maximum overland flow distance is 100.00 (Ft)

for the top area slope value of 12.61 %, in a development type of
Permanent Open Space

In Accordance With Figure 3-3

Initial Area Time of Concentration = 6.19 minutes

$TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$

$TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (12.609^{(1/3)})] = 6.19$

Rainfall intensity (I) = 7.579(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.300

Subarea runoff = 0.318(CFS)

Total initial stream area = 0.140(Ac.)

+++++
Process from Point/Station 302.000 to Point/Station 303.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 1.128(CFS)

Depth of flow = 0.104(Ft.), Average velocity = 2.074(Ft/s)

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	50.00	0.00
3	100.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow = 1.128(CFS)
' ' flow top width = 10.429(Ft.)
' ' velocity = 2.074(Ft/s)
' ' area = 0.544(Sq.Ft)
' ' Froude number = 1.600

Upstream point elevation = 753.400(Ft.)

Downstream point elevation = 735.000(Ft.)

Flow length = 115.000(Ft.)

Travel time = 0.92 min.

Time of concentration = 7.11 min.

Depth of flow = 0.104(Ft.)

Average velocity = 2.074(Ft/s)

Total irregular channel flow = 1.128(CFS)

Irregular channel normal depth above invert elev. = 0.104(Ft.)

Average velocity of channel(s) = 2.074(Ft/s)

Adding area flow to channel

Rainfall intensity (I) = 6.928(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[UNDISTURBED NATURAL TERRAIN]

(Permanent Open Space)
 Impervious value, $A_i = 0.000$
 Sub-Area C Value = 0.300
 Rainfall intensity = 6.928(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is $C = 0.300$ $CA = 0.267$
 Subarea runoff = 1.531(CFS) for 0.750(Ac.)
 Total runoff = 1.850(CFS) Total area = 0.890(Ac.)
 Depth of flow = 0.126(Ft.), Average velocity = 2.347(Ft/s)

+++++
 Process from Point/Station 303.000 to Point/Station 304.000
 **** STREET FLOW TRAVEL TIME + SUBAREA FLOW ADDITION ****

Top of street segment elevation = 735.000(Ft.)
 End of street segment elevation = 705.400(Ft.)
 Length of street segment = 548.000(Ft.)
 Height of curb above gutter flowline = 6.0(In.)
 Width of half street (curb to crown) = 27.000(Ft.)
 Distance from crown to crossfall grade break = 25.500(Ft.)
 Slope from gutter to grade break (v/hz) = 0.020
 Slope from grade break to crown (v/hz) = 0.020
 Street flow is on [2] side(s) of the street
 Distance from curb to property line = 17.000(Ft.)
 Slope from curb to property line (v/hz) = 0.020
 Gutter width = 1.500(Ft.)
 Gutter hike from flowline = 2.000(In.)
 Manning's N in gutter = 0.0150
 Manning's N from gutter to grade break = 0.0150
 Manning's N from grade break to crown = 0.0150
 Estimated mean flow rate at midpoint of street = 4.446(CFS)
 Depth of flow = 0.267(Ft.), Average velocity = 4.194(Ft/s)
 Streetflow hydraulics at midpoint of street travel:
 Halfstreet flow width = 6.538(Ft.)
 Flow velocity = 4.19(Ft/s)
 Travel time = 2.18 min. TC = 9.29 min.
 Adding area flow to street
 Rainfall intensity (I) = 5.831(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 0.100
 Decimal fraction soil group D = 0.900
 [HIGH DENSITY RESIDENTIAL]
 (24.0 DU/A or Less)
 Impervious value, $A_i = 0.650$
 Sub-Area C Value = 0.708
 Rainfall intensity = 5.831(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is $C = 0.547$ $CA = 1.237$

Subarea runoff = 5.363(CFS) for 1.370(Ac.)
 Total runoff = 7.213(CFS) Total area = 2.260(Ac.)
 Street flow at end of street = 7.213(CFS)
 Half street flow at end of street = 3.607(CFS)
 Depth of flow = 0.301(Ft.), Average velocity = 4.658(Ft/s)
 Flow width (from curb towards crown) = 8.196(Ft.)

+++++
 Process from Point/Station 304.000 to Point/Station 308.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 701.400(Ft.)
 Downstream point/station elevation = 694.100(Ft.)
 Pipe length = 307.00(Ft.) Slope = 0.0238 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 7.213(CFS)
 Nearest computed pipe diameter = 15.00(In.)
 Calculated individual pipe flow = 7.213(CFS)
 Normal flow depth in pipe = 9.46(In.)
 Flow top width inside pipe = 14.48(In.)
 Critical Depth = 12.88(In.)
 Pipe flow velocity = 8.84(Ft/s)
 Travel time through pipe = 0.58 min.
 Time of concentration (TC) = 9.87 min.

+++++
 Process from Point/Station 304.000 to Point/Station 308.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 1 in normal stream number 1
 Stream flow area = 2.260(Ac.)
 Runoff from this stream = 7.213(CFS)
 Time of concentration = 9.87 min.
 Rainfall intensity = 5.608(In/Hr)

+++++
 Process from Point/Station 305.000 to Point/Station 306.000
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [LOW DENSITY RESIDENTIAL]
 (2.9 DU/A or Less)
 Impervious value, Ai = 0.250
 Sub-Area C Value = 0.450
 Initial subarea total flow distance = 98.000(Ft.)

Highest elevation = 752.800(Ft.)
 Lowest elevation = 741.000(Ft.)
 Elevation difference = 11.800(Ft.) Slope = 12.041 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 12.04 %, in a development type of
 2.9 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.10 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5}] / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4500) * (100.000^{.5})] / (12.041^{(1/3)}) = 5.10$
 Rainfall intensity (I) = 8.579(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.450
 Subarea runoff = 0.965(CFS)
 Total initial stream area = 0.250(Ac.)

++++++
 Process from Point/Station 306.000 to Point/Station 307.000
 **** STREET FLOW TRAVEL TIME + SUBAREA FLOW ADDITION ****

Top of street segment elevation = 741.000(Ft.)
 End of street segment elevation = 700.400(Ft.)
 Length of street segment = 399.000(Ft.)
 Height of curb above gutter flowline = 6.0(In.)
 Width of half street (curb to crown) = 12.000(Ft.)
 Distance from crown to crossfall grade break = 10.500(Ft.)
 Slope from gutter to grade break (v/hz) = 0.020
 Slope from grade break to crown (v/hz) = 0.020
 Street flow is on [2] side(s) of the street
 Distance from curb to property line = 8.000(Ft.)
 Slope from curb to property line (v/hz) = 0.020
 Gutter width = 1.500(Ft.)
 Gutter hike from flowline = 2.000(In.)
 Manning's N in gutter = 0.0150
 Manning's N from gutter to grade break = 0.0150
 Manning's N from grade break to crown = 0.0150
 Estimated mean flow rate at midpoint of street = 6.179(CFS)
 Depth of flow = 0.268(Ft.), Average velocity = 5.772(Ft/s)
 Streetflow hydraulics at midpoint of street travel:
 Halfstreet flow width = 6.579(Ft.)
 Flow velocity = 5.77(Ft/s)
 Travel time = 1.15 min. TC = 6.26 min.
 Adding area flow to street
 Rainfall intensity (I) = 7.524(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]

(4.3 DU/A or Less)
 Impervious value, $A_i = 0.300$
 Sub-Area C Value = 0.480
 Rainfall intensity = 7.524(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is $C = 0.478$ $CA = 1.500$
 Subarea runoff = 10.318(CFS) for 2.890(Ac.)
 Total runoff = 11.284(CFS) Total area = 3.140(Ac.)
 Street flow at end of street = 11.284(CFS)
 Half street flow at end of street = 5.642(CFS)
 Depth of flow = 0.310(Ft.), Average velocity = 6.585(Ft/s)
 Flow width (from curb towards crown)= 8.684(Ft.)

+++++
 Process from Point/Station 307.000 to Point/Station 308.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 696.400(Ft.)
 Downstream point/station elevation = 695.200(Ft.)
 Pipe length = 25.00(Ft.) Slope = 0.0480 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 11.284(CFS)
 Nearest computed pipe diameter = 15.00(In.)
 Calculated individual pipe flow = 11.284(CFS)
 Normal flow depth in pipe = 10.13(In.)
 Flow top width inside pipe = 14.05(In.)
 Critical depth could not be calculated.
 Pipe flow velocity = 12.81(Ft/s)
 Travel time through pipe = 0.03 min.
 Time of concentration (TC) = 6.29 min.

+++++
 Process from Point/Station 307.000 to Point/Station 308.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 1 in normal stream number 2
 Stream flow area = 3.140(Ac.)
 Runoff from this stream = 11.284(CFS)
 Time of concentration = 6.29 min.
 Rainfall intensity = 7.499(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	7.213	9.87	5.608
2	11.284	6.29	7.499

$Q_{\max}(1) =$

$$\begin{array}{rclcl}
 1.000 * & 1.000 * & 7.213) & + & \\
 0.748 * & 1.000 * & 11.284) & + = & 15.652 \\
 Q_{\max}(2) = & & & & \\
 1.000 * & 0.637 * & 7.213) & + & \\
 1.000 * & 1.000 * & 11.284) & + = & 15.881
 \end{array}$$

Total of 2 streams to confluence:

Flow rates before confluence point:

7.213 11.284

Maximum flow rates at confluence using above data:

15.652 15.881

Area of streams before confluence:

2.260 3.140

Results of confluence:

Total flow rate = 15.881(CFS)

Time of concentration = 6.289 min.

Effective stream area after confluence = 5.400(Ac.)

++++++
 Process from Point/Station 308.000 to Point/Station 311.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 695.200(Ft.)
 Downstream point/station elevation = 694.400(Ft.)
 Pipe length = 112.00(Ft.) Slope = 0.0071 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 15.881(CFS)
 Nearest computed pipe diameter = 24.00(In.)
 Calculated individual pipe flow = 15.881(CFS)
 Normal flow depth in pipe = 16.69(In.)
 Flow top width inside pipe = 22.09(In.)
 Critical Depth = 17.23(In.)
 Pipe flow velocity = 6.80(Ft/s)
 Travel time through pipe = 0.27 min.
 Time of concentration (TC) = 6.56 min.

++++++
 Process from Point/Station 308.000 to Point/Station 311.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 5.400(Ac.)

Runoff from this stream = 15.881(CFS)

Time of concentration = 6.56 min.

Rainfall intensity = 7.295(In/Hr)

Program is now starting with Main Stream No. 2

+++++
 Process from Point/Station 309.000 to Point/Station 310.000
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Initial subarea total flow distance = 60.000(Ft.)
 Highest elevation = 730.400(Ft.)
 Lowest elevation = 721.300(Ft.)
 Elevation difference = 9.100(Ft.) Slope = 15.167 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 15.17 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 4.51 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4800) * (100.000^{.5})] / (15.167^{(1/3)}) = 4.51$
 Calculated TC of 4.508 minutes is less than 5 minutes,
 resetting TC to 5.0 minutes for rainfall intensity calculations
 Rainfall intensity (I) = 8.695(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.480
 Subarea runoff = 0.167(CFS)
 Total initial stream area = 0.040(Ac.)

+++++
 Process from Point/Station 310.000 to Point/Station 311.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 0.649(CFS)
 Depth of flow = 0.240(Ft.), Average velocity = 2.255(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	5.00	0.00
3	10.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow = 0.649(CFS)
 ' ' flow top width = 2.399(Ft.)
 ' ' velocity = 2.255(Ft/s)

```

'      '      area =      0.288(Sq.Ft)
'      '      Froude number =      1.148

```

```

Upstream point elevation = 721.300(Ft.)
Downstream point elevation = 702.500(Ft.)
Flow length = 294.000(Ft.)
Travel time = 2.17 min.
Time of concentration = 6.68 min.
Depth of flow = 0.240(Ft.)
Average velocity = 2.255(Ft/s)
Total irregular channel flow = 0.649(CFS)
Irregular channel normal depth above invert elev. = 0.240(Ft.)
Average velocity of channel(s) = 2.255(Ft/s)
Adding area flow to channel
Rainfall intensity (I) = 7.212(In/Hr) for a 100.0 year storm
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[MEDIUM DENSITY RESIDENTIAL ]
(4.3 DU/A or Less )
Impervious value, Ai = 0.300
Sub-Area C Value = 0.480
Rainfall intensity = 7.212(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for total area
(Q=KCIA) is C = 0.480 CA = 0.149
Subarea runoff = 0.906(CFS) for 0.270(Ac.)
Total runoff = 1.073(CFS) Total area = 0.310(Ac.)
Depth of flow = 0.290(Ft.), Average velocity = 2.557(Ft/s)

```

```

+++++
Process from Point/Station 310.000 to Point/Station 311.000
**** CONFLUENCE OF MAIN STREAMS ****

```

The following data inside Main Stream is listed:

```

In Main Stream number: 2
Stream flow area = 0.310(Ac.)
Runoff from this stream = 1.073(CFS)
Time of concentration = 6.68 min.
Rainfall intensity = 7.212(In/Hr)
Summary of stream data:

```

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	15.881	6.56	7.295
2	1.073	6.68	7.212

Qmax(1) =

```

      1.000 *      1.000 *      15.881) +
      1.000 *      0.982 *      1.073) + =      16.935
Qmax(2) =
      0.989 *      1.000 *      15.881) +
      1.000 *      1.000 *      1.073) + =      16.773

```

Total of 2 main streams to confluence:

Flow rates before confluence point:

15.881 1.073

Maximum flow rates at confluence using above data:

16.935 16.773

Area of streams before confluence:

5.400 0.310

Results of confluence:

Total flow rate = 16.935(CFS)

Time of concentration = 6.564 min.

Effective stream area after confluence = 5.710(Ac.)

```

+++++
Process from Point/Station      311.000 to Point/Station      316.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

```

```

Upstream point/station elevation = 694.400(Ft.)
Downstream point/station elevation = 693.800(Ft.)
Pipe length = 87.00(Ft.) Slope = 0.0069 Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 16.935(CFS)
Nearest computed pipe diameter = 24.00(In.)
Calculated individual pipe flow = 16.935(CFS)
Normal flow depth in pipe = 17.81(In.)
Flow top width inside pipe = 21.00(In.)
Critical Depth = 17.79(In.)
Pipe flow velocity = 6.77(Ft/s)
Travel time through pipe = 0.21 min.
Time of concentration (TC) = 6.78 min.

```

```

+++++
Process from Point/Station      311.000 to Point/Station      316.000
**** CONFLUENCE OF MAIN STREAMS ****

```

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 5.710(Ac.)

Runoff from this stream = 16.935(CFS)

Time of concentration = 6.78 min.

Rainfall intensity = 7.146(In/Hr)

Program is now starting with Main Stream No. 2

+++++
 Process from Point/Station 312.000 to Point/Station 313.000
 ***** INITIAL AREA EVALUATION *****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Initial subarea total flow distance = 96.000(Ft.)
 Highest elevation = 754.200(Ft.)
 Lowest elevation = 741.000(Ft.)
 Elevation difference = 13.200(Ft.) Slope = 13.750 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 13.75 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 4.66 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4800) * (100.000^{.5}) / (13.750^{(1/3)})] = 4.66$
 Calculated TC of 4.658 minutes is less than 5 minutes,
 resetting TC to 5.0 minutes for rainfall intensity calculations
 Rainfall intensity (I) = 8.695(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.480
 Subarea runoff = 1.002(CFS)
 Total initial stream area = 0.240(Ac.)

+++++
 Process from Point/Station 313.000 to Point/Station 314.000
 ***** STREET FLOW TRAVEL TIME + SUBAREA FLOW ADDITION *****

Top of street segment elevation = 741.000(Ft.)
 End of street segment elevation = 727.200(Ft.)
 Length of street segment = 195.000(Ft.)
 Height of curb above gutter flowline = 6.0(In.)
 Width of half street (curb to crown) = 12.000(Ft.)
 Distance from crown to crossfall grade break = 10.500(Ft.)
 Slope from gutter to grade break (v/hz) = 0.020
 Slope from grade break to crown (v/hz) = 0.020
 Street flow is on [2] side(s) of the street
 Distance from curb to property line = 8.000(Ft.)
 Slope from curb to property line (v/hz) = 0.020
 Gutter width = 1.500(Ft.)

Gutter hike from flowline = 2.000(In.)
 Manning's N in gutter = 0.0150
 Manning's N from gutter to grade break = 0.0150
 Manning's N from grade break to crown = 0.0150
 Estimated mean flow rate at midpoint of street = 3.661(CFS)
 Depth of flow = 0.247(Ft.), Average velocity = 4.497(Ft/s)
 Streetflow hydraulics at midpoint of street travel:
 Halfstreet flow width = 5.519(Ft.)
 Flow velocity = 4.50(Ft/s)
 Travel time = 0.72 min. TC = 5.38 min.
 Adding area flow to street
 Rainfall intensity (I) = 8.292(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Rainfall intensity = 8.292(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.480 CA = 0.744
 Subarea runoff = 5.168(CFS) for 1.310(Ac.)
 Total runoff = 6.169(CFS) Total area = 1.550(Ac.)
 Street flow at end of street = 6.169(CFS)
 Half street flow at end of street = 3.085(CFS)
 Depth of flow = 0.280(Ft.), Average velocity = 5.001(Ft/s)
 Flow width (from curb towards crown)= 7.172(Ft.)

++++++
 Process from Point/Station 314.000 to Point/Station 315.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 723.200(Ft.)
 Downstream point/station elevation = 694.100(Ft.)
 Pipe length = 132.00(Ft.) Slope = 0.2205 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 6.169(CFS)
 Nearest computed pipe diameter = 9.00(In.)
 Calculated individual pipe flow = 6.169(CFS)
 Normal flow depth in pipe = 6.06(In.)
 Flow top width inside pipe = 8.44(In.)
 Critical depth could not be calculated.
 Pipe flow velocity = 19.51(Ft/s)
 Travel time through pipe = 0.11 min.
 Time of concentration (TC) = 5.49 min.

+++++

Process from Point/Station 315.000 to Point/Station 315.000
 **** SUBAREA FLOW ADDITION ****

Rainfall intensity (I) = 8.182(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Time of concentration = 5.49 min.
 Rainfall intensity = 8.182(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.480 CA = 0.989
 Subarea runoff = 1.921(CFS) for 0.510(Ac.)
 Total runoff = 8.090(CFS) Total area = 2.060(Ac.)

+++++
 Process from Point/Station 315.000 to Point/Station 316.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 694.100(Ft.)
 Downstream point/station elevation = 693.800(Ft.)
 Pipe length = 89.00(Ft.) Slope = 0.0034 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 8.090(CFS)
 Nearest computed pipe diameter = 21.00(In.)
 Calculated individual pipe flow = 8.090(CFS)
 Normal flow depth in pipe = 15.28(In.)
 Flow top width inside pipe = 18.70(In.)
 Critical Depth = 12.65(In.)
 Pipe flow velocity = 4.32(Ft/s)
 Travel time through pipe = 0.34 min.
 Time of concentration (TC) = 5.84 min.

+++++
 Process from Point/Station 315.000 to Point/Station 316.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 2
 Stream flow area = 2.060(Ac.)
 Runoff from this stream = 8.090(CFS)
 Time of concentration = 5.84 min.
 Rainfall intensity = 7.868(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	16.935	6.78	7.146
2	8.090	5.84	7.868

Qmax(1) =

1.000 *	1.000 *	16.935) +	
0.908 *	1.000 *	8.090) + =	24.283

Qmax(2) =

1.000 *	0.861 *	16.935) +	
1.000 *	1.000 *	8.090) + =	22.677

Total of 2 main streams to confluence:

Flow rates before confluence point:

16.935	8.090
--------	-------

Maximum flow rates at confluence using above data:

24.283	22.677
--------	--------

Area of streams before confluence:

5.710	2.060
-------	-------

Results of confluence:

Total flow rate = 24.283(CFS)

Time of concentration = 6.778 min.

Effective stream area after confluence = 7.770(Ac.)

+++++

Process from Point/Station 316.000 to Point/Station 401.000

**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 693.800(Ft.)

Downstream point/station elevation = 693.500(Ft.)

Pipe length = 47.00(Ft.) Slope = 0.0064 Manning's N = 0.013

No. of pipes = 1 Required pipe flow = 24.283(CFS)

Nearest computed pipe diameter = 27.00(In.)

Calculated individual pipe flow = 24.283(CFS)

Normal flow depth in pipe = 21.70(In.)

Flow top width inside pipe = 21.44(In.)

Critical Depth = 20.69(In.)

Pipe flow velocity = 7.09(Ft/s)

Travel time through pipe = 0.11 min.

Time of concentration (TC) = 6.89 min.

+++++

Process from Point/Station 401.000 to Point/Station 401.000

**** SUBAREA FLOW ADDITION ****

Rainfall intensity (I) = 7.071(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Time of concentration = 6.89 min.
 Rainfall intensity = 7.071(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.497 CA = 4.349
 Subarea runoff = 6.474(CFS) for 0.990(Ac.)
 Total runoff = 30.757(CFS) Total area = 8.760(Ac.)

++++++
 Process from Point/Station 316.000 to Point/Station 401.000
 **** 6 HOUR HYDROGRAPH ****

++++++
 Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 6.89
 Basin Area = 8.76 Acres
 6 Hour Rainfall = 3.300 Inches
 Runoff Coefficient = 0.497
 Peak Discharge = 30.76 CFS

Time (Min)	Discharge (CFS)
0	0.000
6	0.856
12	0.865
18	0.885
24	0.895
30	0.916
36	0.928
42	0.951
48	0.963
54	0.989
60	1.003
66	1.032
72	1.047
78	1.079
84	1.097
90	1.133
96	1.152
102	1.194
108	1.216

114	1.264
120	1.290
126	1.345
132	1.376
138	1.442
144	1.478
150	1.558
156	1.602
162	1.701
168	1.757
174	1.884
180	1.957
186	2.127
192	2.228
198	2.471
204	2.620
210	3.004
216	3.257
222	3.981
228	4.534
234	6.657
240	9.380
246	30.757
252	5.339
258	3.572
264	2.795
270	2.341
276	2.038
282	1.818
288	1.650
294	1.517
300	1.408
306	1.317
312	1.239
318	1.173
324	1.114
330	1.063
336	1.017
342	0.976
348	0.939
354	0.905
360	0.875
366	0.846

+++++

6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	7.7	15.4	23.1	30.8
0+ 0	0.0000		0.00	Q				
0+ 1	0.0002		0.14	Q				
0+ 2	0.0006		0.29	Q				
0+ 3	0.0012		0.43	Q				
0+ 4	0.0020		0.57	Q				
0+ 5	0.0029		0.71	Q				
0+ 6	0.0041		0.86	VQ				
0+ 7	0.0053		0.86	VQ				
0+ 8	0.0065		0.86	VQ				
0+ 9	0.0077		0.86	VQ				
0+10	0.0089		0.86	VQ				
0+11	0.0101		0.86	VQ				
0+12	0.0112		0.87	VQ				
0+13	0.0124		0.87	VQ				
0+14	0.0136		0.87	VQ				
0+15	0.0148		0.87	VQ				
0+16	0.0161		0.88	VQ				
0+17	0.0173		0.88	VQ				
0+18	0.0185		0.88	VQ				
0+19	0.0197		0.89	VQ				
0+20	0.0209		0.89	VQ				
0+21	0.0222		0.89	VQ				
0+22	0.0234		0.89	VQ				
0+23	0.0246		0.89	VQ				
0+24	0.0258		0.89	VQ				
0+25	0.0271		0.90	VQ				
0+26	0.0283		0.90	VQ				
0+27	0.0296		0.91	Q				
0+28	0.0308		0.91	Q				
0+29	0.0321		0.91	Q				
0+30	0.0333		0.92	Q				
0+31	0.0346		0.92	Q				
0+32	0.0359		0.92	Q				
0+33	0.0371		0.92	Q				
0+34	0.0384		0.92	Q				
0+35	0.0397		0.93	Q				
0+36	0.0410		0.93	Q				
0+37	0.0423		0.93	Q				
0+38	0.0435		0.94	Q				
0+39	0.0448		0.94	Q				
0+40	0.0461		0.94	Q				
0+41	0.0474		0.95	Q				
0+42	0.0488		0.95	Q				
0+43	0.0501		0.95	Q				
0+44	0.0514		0.96	Q				
0+45	0.0527		0.96	Q				
0+46	0.0540		0.96	Q				
0+47	0.0553		0.96	Q				

0+48	0.0567	0.96	Q				
0+49	0.0580	0.97	Q				
0+50	0.0593	0.97	QV				
0+51	0.0607	0.98	QV				
0+52	0.0620	0.98	QV				
0+53	0.0634	0.99	QV				
0+54	0.0648	0.99	QV				
0+55	0.0661	0.99	QV				
0+56	0.0675	0.99	QV				
0+57	0.0689	1.00	QV				
0+58	0.0702	1.00	QV				
0+59	0.0716	1.00	QV				
1+ 0	0.0730	1.00	QV				
1+ 1	0.0744	1.01	QV				
1+ 2	0.0758	1.01	QV				
1+ 3	0.0772	1.02	QV				
1+ 4	0.0786	1.02	QV				
1+ 5	0.0800	1.03	QV				
1+ 6	0.0814	1.03	QV				
1+ 7	0.0829	1.03	QV				
1+ 8	0.0843	1.04	QV				
1+ 9	0.0857	1.04	QV				
1+10	0.0872	1.04	QV				
1+11	0.0886	1.04	Q V				
1+12	0.0900	1.05	Q V				
1+13	0.0915	1.05	Q V				
1+14	0.0929	1.06	Q V				
1+15	0.0944	1.06	Q V				
1+16	0.0959	1.07	Q V				
1+17	0.0974	1.07	Q V				
1+18	0.0988	1.08	Q V				
1+19	0.1003	1.08	Q V				
1+20	0.1018	1.09	Q V				
1+21	0.1033	1.09	Q V				
1+22	0.1048	1.09	Q V				
1+23	0.1063	1.09	Q V				
1+24	0.1078	1.10	Q V				
1+25	0.1094	1.10	Q V				
1+26	0.1109	1.11	Q V				
1+27	0.1124	1.11	Q V				
1+28	0.1140	1.12	Q V				
1+29	0.1155	1.13	Q V				
1+30	0.1171	1.13	Q V				
1+31	0.1187	1.14	Q V				
1+32	0.1202	1.14	Q V				
1+33	0.1218	1.14	Q V				
1+34	0.1234	1.15	Q V				
1+35	0.1250	1.15	Q V				
1+36	0.1265	1.15	Q V				
1+37	0.1281	1.16	Q V				

1+38	0.1297	1.17	Q	V				
1+39	0.1314	1.17	Q	V				
1+40	0.1330	1.18	Q	V				
1+41	0.1346	1.19	Q	V				
1+42	0.1363	1.19	Q	V				
1+43	0.1379	1.20	Q	V				
1+44	0.1396	1.20	Q	V				
1+45	0.1412	1.20	Q	V				
1+46	0.1429	1.21	Q	V				
1+47	0.1446	1.21	Q	V				
1+48	0.1462	1.22	Q	V				
1+49	0.1479	1.22	Q	V				
1+50	0.1496	1.23	Q	V				
1+51	0.1513	1.24	Q	V				
1+52	0.1531	1.25	Q	V				
1+53	0.1548	1.26	Q	V				
1+54	0.1565	1.26	Q	V				
1+55	0.1583	1.27	Q	V				
1+56	0.1600	1.27	Q	V				
1+57	0.1618	1.28	Q	V				
1+58	0.1635	1.28	Q	V				
1+59	0.1653	1.29	Q	V				
2+ 0	0.1671	1.29	Q	V				
2+ 1	0.1689	1.30	Q	V				
2+ 2	0.1707	1.31	Q	V				
2+ 3	0.1725	1.32	Q	V				
2+ 4	0.1743	1.33	Q	V				
2+ 5	0.1762	1.34	Q	V				
2+ 6	0.1780	1.35	Q	V				
2+ 7	0.1799	1.35	Q	V				
2+ 8	0.1817	1.36	Q	V				
2+ 9	0.1836	1.36	Q	V				
2+10	0.1855	1.37	Q	V				
2+11	0.1874	1.37	Q	V				
2+12	0.1893	1.38	Q	V				
2+13	0.1912	1.39	Q	V				
2+14	0.1931	1.40	Q	V				
2+15	0.1951	1.41	Q	V				
2+16	0.1970	1.42	Q	V				
2+17	0.1990	1.43	Q	V				
2+18	0.2010	1.44	Q	V				
2+19	0.2030	1.45	Q	V				
2+20	0.2050	1.45	Q	V				
2+21	0.2070	1.46	Q	V				
2+22	0.2090	1.47	Q	V				
2+23	0.2110	1.47	Q	V				
2+24	0.2131	1.48	Q	V				
2+25	0.2151	1.49	Q	V				
2+26	0.2172	1.50	Q	V				
2+27	0.2193	1.52	Q	V				

2+28	0.2214	1.53	Q	V				
2+29	0.2235	1.54	Q	V				
2+30	0.2257	1.56	Q	V				
2+31	0.2278	1.57	Q	V				
2+32	0.2300	1.57	Q	V				
2+33	0.2322	1.58	Q	V				
2+34	0.2343	1.59	Q	V				
2+35	0.2365	1.59	Q	V				
2+36	0.2387	1.60	Q	V				
2+37	0.2410	1.62	Q	V				
2+38	0.2432	1.64	Q	V				
2+39	0.2455	1.65	Q	V				
2+40	0.2478	1.67	Q	V				
2+41	0.2501	1.68	Q	V				
2+42	0.2525	1.70	Q	V				
2+43	0.2548	1.71	Q	V				
2+44	0.2572	1.72	Q	V				
2+45	0.2596	1.73	Q	V				
2+46	0.2620	1.74	Q	V				
2+47	0.2644	1.75	Q	V				
2+48	0.2668	1.76	Q	V				
2+49	0.2692	1.78	Q	V				
2+50	0.2717	1.80	Q	V				
2+51	0.2742	1.82	Q	V				
2+52	0.2768	1.84	Q	V				
2+53	0.2793	1.86	Q	V				
2+54	0.2819	1.88	Q	V				
2+55	0.2845	1.90	Q	V				
2+56	0.2872	1.91	Q	V				
2+57	0.2898	1.92	Q	V				
2+58	0.2925	1.93	Q	V				
2+59	0.2952	1.95	Q	V				
3+ 0	0.2979	1.96	Q	V				
3+ 1	0.3006	1.99	Q	V				
3+ 2	0.3034	2.01	Q	V				
3+ 3	0.3062	2.04	Q	V				
3+ 4	0.3090	2.07	Q	V				
3+ 5	0.3119	2.10	Q	V				
3+ 6	0.3149	2.13	Q	V				
3+ 7	0.3178	2.14	Q	V				
3+ 8	0.3208	2.16	Q	V				
3+ 9	0.3238	2.18	Q	V				
3+10	0.3268	2.19	Q	V				
3+11	0.3298	2.21	Q	V				
3+12	0.3329	2.23	Q	V				
3+13	0.3360	2.27	Q	V				
3+14	0.3392	2.31	Q	V				
3+15	0.3425	2.35	Q	V				
3+16	0.3458	2.39	Q	V				
3+17	0.3491	2.43	Q	V				

3+18	0.3525	2.47	Q	V					
3+19	0.3559	2.50	Q	V					
3+20	0.3594	2.52	Q	V					
3+21	0.3629	2.55	Q	V					
3+22	0.3665	2.57	Q	V					
3+23	0.3700	2.60	Q	V					
3+24	0.3736	2.62	Q	V					
3+25	0.3773	2.68	Q	V					
3+26	0.3811	2.75	Q	V					
3+27	0.3850	2.81	Q	V					
3+28	0.3890	2.88	Q	V					
3+29	0.3930	2.94	Q	V					
3+30	0.3971	3.00	Q	V					
3+31	0.4013	3.05	Q	V					
3+32	0.4056	3.09	Q	V					
3+33	0.4099	3.13	Q	V					
3+34	0.4143	3.17	Q	V					
3+35	0.4187	3.21	Q	V					
3+36	0.4232	3.26	Q	V					
3+37	0.4278	3.38	Q	V					
3+38	0.4327	3.50	Q	V					
3+39	0.4376	3.62	Q	V					
3+40	0.4428	3.74	Q	V					
3+41	0.4481	3.86	Q	V					
3+42	0.4536	3.98	Q	V					
3+43	0.4592	4.07	Q	V					
3+44	0.4649	4.17	Q	V					
3+45	0.4708	4.26	Q	V					
3+46	0.4768	4.35	Q	V					
3+47	0.4829	4.44	Q	V					
3+48	0.4892	4.53	Q	V					
3+49	0.4959	4.89	Q	V					
3+50	0.5031	5.24	Q	V					
3+51	0.5108	5.60	Q	V					
3+52	0.5190	5.95	Q	V					
3+53	0.5277	6.30	Q	V					
3+54	0.5369	6.66	Q	V					
3+55	0.5467	7.11	Q	V					
3+56	0.5571	7.56	Q	V					
3+57	0.5681	8.02	Q	V					
3+58	0.5798	8.47	Q	V					
3+59	0.5921	8.93	Q	V					
4+ 0	0.6050	9.38	Q	V					
4+ 1	0.6228	12.94		Q	V				
4+ 2	0.6456	16.51			Q	V			
4+ 3	0.6732	20.07				Q	V		
4+ 4	0.7058	23.63					Q	V	
4+ 5	0.7432	27.19						Q	
4+ 6	0.7856	30.76							Q
4+ 7	0.8221	26.52							Q

4+ 8	0.8528	22.28				QV	
4+ 9	0.8777	18.05			Q	V	
4+10	0.8967	13.81		Q		V	
4+11	0.9099	9.58		Q		V	
4+12	0.9172	5.34	Q			V	
4+13	0.9242	5.04	Q			V	
4+14	0.9307	4.75	Q			V	
4+15	0.9369	4.46	Q			V	
4+16	0.9426	4.16	Q			V	
4+17	0.9479	3.87	Q			V	
4+18	0.9528	3.57	Q			V	
4+19	0.9576	3.44	Q			V	
4+20	0.9622	3.31	Q			V	
4+21	0.9665	3.18	Q			V	
4+22	0.9707	3.05	Q			V	
4+23	0.9748	2.92	Q			V	
4+24	0.9786	2.80	Q			V	
4+25	0.9824	2.72	Q			V	
4+26	0.9860	2.64	Q			V	
4+27	0.9895	2.57	Q			V	
4+28	0.9930	2.49	Q			V	
4+29	0.9963	2.42	Q			V	
4+30	0.9995	2.34	Q			V	
4+31	1.0027	2.29	Q			V	
4+32	1.0058	2.24	Q			V	
4+33	1.0088	2.19	Q			V	
4+34	1.0117	2.14	Q			V	
4+35	1.0146	2.09	Q			V	
4+36	1.0174	2.04	Q			V	
4+37	1.0202	2.00	Q			V	
4+38	1.0229	1.96	Q			V	
4+39	1.0255	1.93	Q			V	
4+40	1.0281	1.89	Q			V	
4+41	1.0307	1.85	Q			V	
4+42	1.0332	1.82	Q			V	
4+43	1.0357	1.79	Q			V	
4+44	1.0381	1.76	Q			V	
4+45	1.0405	1.73	Q			V	
4+46	1.0428	1.71	Q			V	
4+47	1.0451	1.68	Q			V	
4+48	1.0474	1.65	Q			V	
4+49	1.0497	1.63	Q			V	
4+50	1.0519	1.61	Q			V	
4+51	1.0541	1.58	Q			V	
4+52	1.0562	1.56	Q			V	
4+53	1.0583	1.54	Q			V	
4+54	1.0604	1.52	Q			V	
4+55	1.0625	1.50	Q			V	
4+56	1.0645	1.48	Q			V	
4+57	1.0665	1.46	Q			V	

4+58	1.0685	1.44	Q				V
4+59	1.0705	1.43	Q				V
5+ 0	1.0724	1.41	Q				V
5+ 1	1.0743	1.39	Q				V
5+ 2	1.0762	1.38	Q				V
5+ 3	1.0781	1.36	Q				V
5+ 4	1.0800	1.35	Q				V
5+ 5	1.0818	1.33	Q				V
5+ 6	1.0836	1.32	Q				V
5+ 7	1.0854	1.30	Q				V
5+ 8	1.0872	1.29	Q				V
5+ 9	1.0889	1.28	Q				V
5+10	1.0907	1.27	Q				V
5+11	1.0924	1.25	Q				V
5+12	1.0941	1.24	Q				V
5+13	1.0958	1.23	Q				V
5+14	1.0975	1.22	Q				V
5+15	1.0992	1.21	Q				V
5+16	1.1008	1.19	Q				V
5+17	1.1024	1.18	Q				V
5+18	1.1040	1.17	Q				V
5+19	1.1056	1.16	Q				V
5+20	1.1072	1.15	Q				V
5+21	1.1088	1.14	Q				V
5+22	1.1104	1.13	Q				V
5+23	1.1119	1.12	Q				V
5+24	1.1135	1.11	Q				V
5+25	1.1150	1.11	Q				V
5+26	1.1165	1.10	Q				V
5+27	1.1180	1.09	Q				V
5+28	1.1195	1.08	Q				V
5+29	1.1210	1.07	Q				V
5+30	1.1224	1.06	Q				V
5+31	1.1239	1.06	Q				V
5+32	1.1253	1.05	Q				V
5+33	1.1267	1.04	Q				V
5+34	1.1282	1.03	Q				V
5+35	1.1296	1.02	Q				V
5+36	1.1310	1.02	Q				V
5+37	1.1324	1.01	Q				V
5+38	1.1338	1.00	Q				V
5+39	1.1351	1.00	Q				V
5+40	1.1365	0.99	Q				V
5+41	1.1378	0.98	Q				V
5+42	1.1392	0.98	Q				V
5+43	1.1405	0.97	Q				V
5+44	1.1419	0.96	Q				V
5+45	1.1432	0.96	Q				V
5+46	1.1445	0.95	Q				V
5+47	1.1458	0.95	Q				V

5+48	1.1471	0.94	Q				V
5+49	1.1484	0.93	Q				V
5+50	1.1496	0.93	Q				V
5+51	1.1509	0.92	Q				V
5+52	1.1522	0.92	Q				V
5+53	1.1534	0.91	Q				V
5+54	1.1547	0.91	Q				V
5+55	1.1559	0.90	Q				V
5+56	1.1572	0.90	Q				V
5+57	1.1584	0.89	Q				V
5+58	1.1596	0.88	Q				V
5+59	1.1608	0.88	Q				V
6+ 0	1.1620	0.87	Q				V
6+ 1	1.1632	0.87	Q				V
6+ 2	1.1644	0.87	Q				V
6+ 3	1.1656	0.86	Q				V
6+ 4	1.1668	0.86	Q				V
6+ 5	1.1679	0.85	Q				V
6+ 6	1.1691	0.85	Q				V

End of computations, total study area = 8.760 (Ac.)

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 06/21/21

108-001 Idaho (Escondido Estates)
Onsite Proposed Condition 317-402
Extrapolated storm from County Isopluvials
TH

***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:
6 hour, precipitation(inches) = 3.300
24 hour precipitation(inches) = 7.300
P6/P24 = 45.2%
San Diego hydrology manual 'C' values used

+++++
Process from Point/Station 317.000 to Point/Station 318.000
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[MEDIUM DENSITY RESIDENTIAL]
(4.3 DU/A or Less)
Impervious value, Ai = 0.300
Sub-Area C Value = 0.480
Initial subarea total flow distance = 169.000(Ft.)
Highest elevation = 733.300(Ft.)
Lowest elevation = 724.000(Ft.)
Elevation difference = 9.300(Ft.) Slope = 5.503 %
INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
The maximum overland flow distance is 100.00 (Ft)

for the top area slope value of 5.50 %, in a development type of
4.3 DU/A or Less

In Accordance With Figure 3-3

Initial Area Time of Concentration = 6.32 minutes

$TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$

$TC = [1.8 * (1.1 - 0.4800) * (100.000^{.5}) / (5.503^{(1/3)})] = 6.32$

Rainfall intensity (I) = 7.474(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.480

Subarea runoff = 0.215(CFS)

Total initial stream area = 0.060(Ac.)

+++++
Process from Point/Station 318.000 to Point/Station 325.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 1.137(CFS)

Depth of flow = 0.541(Ft.), Average velocity = 7.772(Ft/s)

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	1.00	0.00
3	1.00	1.00

Manning's 'N' friction factor = 0.014

Sub-Channel flow = 1.137(CFS)
' ' flow top width = 0.541(Ft.)
' ' velocity = 7.772(Ft/s)
' ' area = 0.146(Sq.Ft)
' ' Froude number = 2.634

Upstream point elevation = 724.000(Ft.)

Downstream point elevation = 697.000(Ft.)

Flow length = 272.000(Ft.)

Travel time = 0.58 min.

Time of concentration = 6.90 min.

Depth of flow = 0.541(Ft.)

Average velocity = 7.772(Ft/s)

Total irregular channel flow = 1.137(CFS)

Irregular channel normal depth above invert elev. = 0.541(Ft.)

Average velocity of channel(s) = 7.772(Ft/s)

Adding area flow to channel

Rainfall intensity (I) = 7.061(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[MEDIUM DENSITY RESIDENTIAL]

(4.3 DU/A or Less)
 Impervious value, $A_i = 0.300$
 Sub-Area C Value = 0.480
 Rainfall intensity = 7.061(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is $C = 0.480$ $CA = 0.283$
 Subarea runoff = 1.784(CFS) for 0.530(Ac.)
 Total runoff = 2.000(CFS) Total area = 0.590(Ac.)
 Depth of flow = 0.668(Ft.), Average velocity = 8.950(Ft/s)

++++
 Process from Point/Station 318.000 to Point/Station 325.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 0.590(Ac.)
 Runoff from this stream = 2.000(CFS)
 Time of concentration = 6.90 min.
 Rainfall intensity = 7.061(In/Hr)
 Program is now starting with Main Stream No. 2

++++
 Process from Point/Station 319.000 to Point/Station 320.000
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [LOW DENSITY RESIDENTIAL]
 (1.0 DU/A or Less)
 Impervious value, $A_i = 0.100$
 Sub-Area C Value = 0.360
 Initial subarea total flow distance = 100.000(Ft.)
 Highest elevation = 767.000(Ft.)
 Lowest elevation = 759.100(Ft.)
 Elevation difference = 7.900(Ft.) Slope = 7.900 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 7.90 %, in a development type of
 1.0 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 6.69 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{0.5} / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.3600) * (100.000^{0.5}) / (7.900^{(1/3)})] = 6.69$
 Rainfall intensity (I) = 7.207(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is $C = 0.360$

Subarea runoff = 0.234(CFS)
 Total initial stream area = 0.090(Ac.)

+++++
 Process from Point/Station 320.000 to Point/Station 324.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 3.485(CFS)
 Depth of flow = 0.190(Ft.), Average velocity = 1.934(Ft/s)
 ***** Irregular Channel Data *****

 Information entered for subchannel number 1 :
 Point number 'X' coordinate 'Y' coordinate
 1 0.00 1.00
 2 50.00 0.00
 3 100.00 1.00
 Manning's 'N' friction factor = 0.040

 Sub-Channel flow = 3.485(CFS)
 ' ' flow top width = 18.982(Ft.)
 ' ' velocity= 1.934(Ft/s)
 ' ' area = 1.802(Sq.Ft)
 ' ' Froude number = 1.106

Upstream point elevation = 759.100(Ft.)
 Downstream point elevation = 730.600(Ft.)
 Flow length = 455.000(Ft.)
 Travel time = 3.92 min.
 Time of concentration = 10.61 min.
 Depth of flow = 0.190(Ft.)
 Average velocity = 1.934(Ft/s)
 Total irregular channel flow = 3.485(CFS)
 Irregular channel normal depth above invert elev. = 0.190(Ft.)
 Average velocity of channel(s) = 1.934(Ft/s)
 Adding area flow to channel
 Rainfall intensity (I) = 5.352(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [LOW DENSITY RESIDENTIAL]
 (1.0 DU/A or Less)
 Impervious value, Ai = 0.100
 Sub-Area C Value = 0.360
 Rainfall intensity = 5.352(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.360 CA = 1.242
 Subarea runoff = 6.414(CFS) for 3.360(Ac.)
 Total runoff = 6.648(CFS) Total area = 3.450(Ac.)

Depth of flow = 0.242(Ft.), Average velocity = 2.273(Ft/s)

```

+++++
Process from Point/Station      320.000 to Point/Station      324.000
**** CONFLUENCE OF MAIN STREAMS ****

```

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 3.450(Ac.)
 Runoff from this stream = 6.648(CFS)
 Time of concentration = 10.61 min.
 Rainfall intensity = 5.352(In/Hr)
 Program is now starting with Main Stream No. 2

```

+++++
Process from Point/Station      321.000 to Point/Station      322.000
**** INITIAL AREA EVALUATION ****

```

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Initial subarea total flow distance = 96.000(Ft.)
 Highest elevation = 767.000(Ft.)
 Lowest elevation = 752.700(Ft.)
 Elevation difference = 14.300(Ft.) Slope = 14.896 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 14.90 %, in a development type of
 Permanent Open Space
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.85 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5} / (% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.3000) * (100.000^{.5}) / (14.896^{(1/3)})] = 5.85$
 Rainfall intensity (I) = 7.855(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.300
 Subarea runoff = 0.165(CFS)
 Total initial stream area = 0.070(Ac.)

```

+++++
Process from Point/Station      322.000 to Point/Station      323.000
**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

```

Estimated mean flow rate at midpoint of channel = 0.365(CFS)
 Depth of flow = 0.423(Ft.), Average velocity = 2.044(Ft/s)

***** Irregular Channel Data *****

 Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	1.00	0.00
3	2.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow = 0.365(CFS)
 ' ' flow top width = 0.846(Ft.)
 ' ' velocity = 2.044(Ft/s)
 ' ' area = 0.179(Sq.Ft)
 ' ' Froude number = 0.783

Upstream point elevation = 759.300(Ft.)

Downstream point elevation = 752.700(Ft.)

Flow length = 173.000(Ft.)

Travel time = 1.41 min.

Time of concentration = 7.26 min.

Depth of flow = 0.423(Ft.)

Average velocity = 2.044(Ft/s)

Total irregular channel flow = 0.365(CFS)

Irregular channel normal depth above invert elev. = 0.423(Ft.)

Average velocity of channel(s) = 2.044(Ft/s)

Adding area flow to channel

Rainfall intensity (I) = 6.834(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[LOW DENSITY RESIDENTIAL]

(1.0 DU/A or Less)

Impervious value, Ai = 0.100

Sub-Area C Value = 0.360

Rainfall intensity = 6.834(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for total area

(Q=KCIA) is C = 0.343 CA = 0.082

Subarea runoff = 0.397(CFS) for 0.170(Ac.)

Total runoff = 0.562(CFS) Total area = 0.240(Ac.)

Depth of flow = 0.497(Ft.), Average velocity = 2.276(Ft/s)

+++++

Process from Point/Station 323.000 to Point/Station 324.000

**** IMPROVED CHANNEL TRAVEL TIME ****

 Upstream point elevation = 752.700(Ft.)

Downstream point elevation = 730.600(Ft.)
 Channel length thru subarea = 203.000(Ft.)
 Channel base width = 1.000(Ft.)
 Slope or 'Z' of left channel bank = 1.000
 Slope or 'Z' of right channel bank = 1.000
 Manning's 'N' = 0.015
 Maximum depth of channel = 1.000(Ft.)
 Flow(q) thru subarea = 0.562(CFS)
 Depth of flow = 0.088(Ft.), Average velocity = 5.888(Ft/s)
 Channel flow top width = 1.175(Ft.)
 Flow Velocity = 5.89(Ft/s)
 Travel time = 0.57 min.
 Time of concentration = 7.84 min.
 Critical depth = 0.199(Ft.)

++++++
 Process from Point/Station 323.000 to Point/Station 324.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 0.240(Ac.)
 Runoff from this stream = 0.562(CFS)
 Time of concentration = 7.84 min.
 Rainfall intensity = 6.506(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	6.648	10.61	5.352
2	0.562	7.84	6.506

Qmax(1) =
 1.000 * 1.000 * 6.648) +
 0.823 * 1.000 * 0.562) + = 7.110
 Qmax(2) =
 1.000 * 0.739 * 6.648) +
 1.000 * 1.000 * 0.562) + = 5.473

Total of 2 main streams to confluence:

Flow rates before confluence point:

6.648 0.562

Maximum flow rates at confluence using above data:

7.110 5.473

Area of streams before confluence:

3.450 0.240

Results of confluence:

Total flow rate = 7.110(CFS)
 Time of concentration = 10.608 min.
 Effective stream area after confluence = 3.690(Ac.)

+++++
 Process from Point/Station 324.000 to Point/Station 325.000
 **** IMPROVED CHANNEL TRAVEL TIME ****

Upstream point elevation = 730.600(Ft.)
 Downstream point elevation = 697.000(Ft.)
 Channel length thru subarea = 445.000(Ft.)
 Channel base width = 1.000(Ft.)
 Slope or 'Z' of left channel bank = 1.000
 Slope or 'Z' of right channel bank = 1.000
 Estimated mean flow rate at midpoint of channel = 7.192(CFS)
 Manning's 'N' = 0.015
 Maximum depth of channel = 1.000(Ft.)
 Flow(q) thru subarea = 7.192(CFS)
 Depth of flow = 0.432(Ft.), Average velocity = 11.612(Ft/s)
 Channel flow top width = 1.865(Ft.)
 Flow Velocity = 11.61(Ft/s)
 Travel time = 0.64 min.
 Time of concentration = 11.25 min.
 Critical depth = 0.875(Ft.)
 Adding area flow to channel
 Rainfall intensity (I) = 5.154(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 Rainfall intensity = 5.154(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.355 CA = 1.393
 Subarea runoff = 0.071(CFS) for 0.230(Ac.)
 Total runoff = 7.181(CFS) Total area = 3.920(Ac.)
 Depth of flow = 0.432(Ft.), Average velocity = 11.607(Ft/s)
 Critical depth = 0.875(Ft.)

+++++
 Process from Point/Station 324.000 to Point/Station 325.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 3.920(Ac.)
 Runoff from this stream = 7.181(CFS)
 Time of concentration = 11.25 min.
 Rainfall intensity = 5.154(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	6.648	10.61	5.352
---	-------	-------	-------

2	7.181	11.25	5.154
---	-------	-------	-------

Qmax(1) =

1.000 *	1.000 *	6.648)	+	
1.000 *	0.943 *	7.181)	+	13.421

Qmax(2) =

0.963 *	1.000 *	6.648)	+	
1.000 *	1.000 *	7.181)	+	13.582

Total of 2 main streams to confluence:

Flow rates before confluence point:

6.648	7.181
-------	-------

Maximum flow rates at confluence using above data:

13.421	13.582
--------	--------

Area of streams before confluence:

3.450	3.920
-------	-------

Results of confluence:

Total flow rate = 13.582(CFS)

Time of concentration = 11.247 min.

Effective stream area after confluence = 7.370(Ac.)

+++++
 Process from Point/Station 325.000 to Point/Station 402.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 697.000(Ft.)
 Downstream point/station elevation = 693.500(Ft.)
 Pipe length = 60.00(Ft.) Slope = 0.0583 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 13.582(CFS)
 Nearest computed pipe diameter = 15.00(In.)
 Calculated individual pipe flow = 13.582(CFS)
 Normal flow depth in pipe = 10.83(In.)
 Flow top width inside pipe = 13.44(In.)
 Critical depth could not be calculated.
 Pipe flow velocity = 14.32(Ft/s)
 Travel time through pipe = 0.07 min.

Time of concentration (TC) = 11.32 min.

+++++
 Process from Point/Station 325.000 to Point/Station 402.000
 **** 6 HOUR HYDROGRAPH ****

+++++
 Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 11.32
 Basin Area = 4.51 Acres
 6 Hour Rainfall = 3.300 Inches
 Runoff Coefficient = 0.584
 Peak Discharge = 13.58 CFS

Time (Min)	Discharge (CFS)
0	0.000
11	0.518
22	0.528
33	0.551
44	0.564
55	0.591
66	0.606
77	0.638
88	0.657
99	0.697
110	0.720
121	0.772
132	0.802
143	0.872
154	0.913
165	1.013
176	1.074
187	1.231
198	1.335
209	1.631
220	1.858
231	2.728
242	3.844
253	13.582
264	2.188
275	1.464
286	1.146
297	0.960
308	0.835
319	0.745
330	0.676
341	0.622
352	0.577

363

0.540

+++++

6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	3.4	6.8	10.2	13.6
0+ 0	0.0000		0.00	Q				
0+ 1	0.0001		0.05	Q				
0+ 2	0.0002		0.09	Q				
0+ 3	0.0004		0.14	Q				
0+ 4	0.0006		0.19	Q				
0+ 5	0.0010		0.24	Q				
0+ 6	0.0014		0.28	Q				
0+ 7	0.0018		0.33	Q				
0+ 8	0.0023		0.38	VQ				
0+ 9	0.0029		0.42	VQ				
0+10	0.0036		0.47	VQ				
0+11	0.0043		0.52	VQ				
0+12	0.0050		0.52	VQ				
0+13	0.0057		0.52	VQ				
0+14	0.0064		0.52	VQ				
0+15	0.0071		0.52	VQ				
0+16	0.0079		0.52	VQ				
0+17	0.0086		0.52	VQ				
0+18	0.0093		0.52	VQ				
0+19	0.0100		0.53	VQ				
0+20	0.0108		0.53	VQ				
0+21	0.0115		0.53	VQ				
0+22	0.0122		0.53	VQ				
0+23	0.0129		0.53	VQ				
0+24	0.0137		0.53	VQ				
0+25	0.0144		0.53	VQ				
0+26	0.0152		0.54	VQ				
0+27	0.0159		0.54	VQ				
0+28	0.0166		0.54	VQ				
0+29	0.0174		0.54	VQ				
0+30	0.0181		0.55	Q				
0+31	0.0189		0.55	Q				
0+32	0.0197		0.55	Q				
0+33	0.0204		0.55	Q				
0+34	0.0212		0.55	Q				
0+35	0.0219		0.55	Q				
0+36	0.0227		0.55	Q				
0+37	0.0235		0.56	Q				
0+38	0.0242		0.56	Q				
0+39	0.0250		0.56	Q				

0+40	0.0258	0.56	Q				
0+41	0.0265	0.56	Q				
0+42	0.0273	0.56	Q				
0+43	0.0281	0.56	Q				
0+44	0.0289	0.56	Q				
0+45	0.0296	0.57	Q				
0+46	0.0304	0.57	Q				
0+47	0.0312	0.57	Q				
0+48	0.0320	0.57	Q				
0+49	0.0328	0.58	Q				
0+50	0.0336	0.58	Q				
0+51	0.0344	0.58	Q				
0+52	0.0352	0.58	Q				
0+53	0.0360	0.59	QV				
0+54	0.0368	0.59	QV				
0+55	0.0376	0.59	QV				
0+56	0.0384	0.59	QV				
0+57	0.0393	0.59	QV				
0+58	0.0401	0.59	QV				
0+59	0.0409	0.60	QV				
1+ 0	0.0417	0.60	QV				
1+ 1	0.0426	0.60	QV				
1+ 2	0.0434	0.60	QV				
1+ 3	0.0442	0.60	QV				
1+ 4	0.0450	0.60	QV				
1+ 5	0.0459	0.60	QV				
1+ 6	0.0467	0.61	QV				
1+ 7	0.0475	0.61	QV				
1+ 8	0.0484	0.61	QV				
1+ 9	0.0492	0.61	QV				
1+10	0.0501	0.62	QV				
1+11	0.0509	0.62	QV				
1+12	0.0518	0.62	QV				
1+13	0.0527	0.63	QV				
1+14	0.0535	0.63	QV				
1+15	0.0544	0.63	Q V				
1+16	0.0553	0.64	Q V				
1+17	0.0562	0.64	Q V				
1+18	0.0570	0.64	Q V				
1+19	0.0579	0.64	Q V				
1+20	0.0588	0.64	Q V				
1+21	0.0597	0.65	Q V				
1+22	0.0606	0.65	Q V				
1+23	0.0615	0.65	Q V				
1+24	0.0624	0.65	Q V				
1+25	0.0633	0.65	Q V				
1+26	0.0642	0.65	Q V				
1+27	0.0651	0.65	Q V				
1+28	0.0660	0.66	Q V				
1+29	0.0669	0.66	Q V				

1+30	0.0678	0.66	Q V				
1+31	0.0687	0.67	Q V				
1+32	0.0696	0.67	Q V				
1+33	0.0706	0.68	Q V				
1+34	0.0715	0.68	Q V				
1+35	0.0725	0.68	Q V				
1+36	0.0734	0.69	Q V				
1+37	0.0743	0.69	Q V				
1+38	0.0753	0.69	Q V				
1+39	0.0763	0.70	Q V				
1+40	0.0772	0.70	Q V				
1+41	0.0782	0.70	Q V				
1+42	0.0792	0.70	Q V				
1+43	0.0801	0.71	Q V				
1+44	0.0811	0.71	Q V				
1+45	0.0821	0.71	Q V				
1+46	0.0831	0.71	Q V				
1+47	0.0841	0.71	Q V				
1+48	0.0850	0.72	Q V				
1+49	0.0860	0.72	Q V				
1+50	0.0870	0.72	Q V				
1+51	0.0880	0.72	Q V				
1+52	0.0890	0.73	Q V				
1+53	0.0900	0.73	Q V				
1+54	0.0911	0.74	Q V				
1+55	0.0921	0.74	Q V				
1+56	0.0931	0.75	Q V				
1+57	0.0941	0.75	Q V				
1+58	0.0952	0.76	Q V				
1+59	0.0962	0.76	Q V				
2+ 0	0.0973	0.77	Q V				
2+ 1	0.0984	0.77	Q V				
2+ 2	0.0994	0.77	Q V				
2+ 3	0.1005	0.78	Q V				
2+ 4	0.1016	0.78	Q V				
2+ 5	0.1027	0.78	Q V				
2+ 6	0.1037	0.79	Q V				
2+ 7	0.1048	0.79	Q V				
2+ 8	0.1059	0.79	Q V				
2+ 9	0.1070	0.79	Q V				
2+10	0.1081	0.80	Q V				
2+11	0.1092	0.80	Q V				
2+12	0.1103	0.80	Q V				
2+13	0.1114	0.81	Q V				
2+14	0.1125	0.81	Q V				
2+15	0.1137	0.82	Q V				
2+16	0.1148	0.83	Q V				
2+17	0.1160	0.83	Q V				
2+18	0.1171	0.84	Q V				
2+19	0.1183	0.85	Q V				

2+20	0.1195	0.85	Q	V				
2+21	0.1206	0.86	Q	V				
2+22	0.1218	0.87	Q	V				
2+23	0.1230	0.87	Q	V				
2+24	0.1242	0.88	Q	V				
2+25	0.1255	0.88	Q	V				
2+26	0.1267	0.88	Q	V				
2+27	0.1279	0.89	Q	V				
2+28	0.1291	0.89	Q	V				
2+29	0.1303	0.89	Q	V				
2+30	0.1316	0.90	Q	V				
2+31	0.1328	0.90	Q	V				
2+32	0.1341	0.91	Q	V				
2+33	0.1353	0.91	Q	V				
2+34	0.1366	0.91	Q	V				
2+35	0.1379	0.92	Q	V				
2+36	0.1391	0.93	Q	V				
2+37	0.1404	0.94	Q	V				
2+38	0.1417	0.95	Q	V				
2+39	0.1431	0.96	Q	V				
2+40	0.1444	0.97	Q	V				
2+41	0.1457	0.98	Q	V				
2+42	0.1471	0.99	Q	V				
2+43	0.1485	0.99	Q	V				
2+44	0.1498	1.00	Q	V				
2+45	0.1512	1.01	Q	V				
2+46	0.1526	1.02	Q	V				
2+47	0.1541	1.02	Q	V				
2+48	0.1555	1.03	Q	V				
2+49	0.1569	1.03	Q	V				
2+50	0.1583	1.04	Q	V				
2+51	0.1598	1.05	Q	V				
2+52	0.1612	1.05	Q	V				
2+53	0.1627	1.06	Q	V				
2+54	0.1641	1.06	Q	V				
2+55	0.1656	1.07	Q	V				
2+56	0.1671	1.07	Q	V				
2+57	0.1686	1.09	Q	V				
2+58	0.1701	1.10	Q	V				
2+59	0.1716	1.12	Q	V				
3+ 0	0.1732	1.13	Q	V				
3+ 1	0.1748	1.15	Q	V				
3+ 2	0.1764	1.16	Q	V				
3+ 3	0.1780	1.17	Q	V				
3+ 4	0.1796	1.19	Q	V				
3+ 5	0.1813	1.20	Q	V				
3+ 6	0.1830	1.22	Q	V				
3+ 7	0.1847	1.23	Q	V				
3+ 8	0.1864	1.24	Q	V				
3+ 9	0.1881	1.25	Q	V				

3+10	0.1898	1.26	Q	V		
3+11	0.1916	1.27	Q	V		
3+12	0.1933	1.28	Q	V		
3+13	0.1951	1.29	Q	V		
3+14	0.1969	1.30	Q	V		
3+15	0.1987	1.31	Q	V		
3+16	0.2005	1.32	Q	V		
3+17	0.2023	1.33	Q	V		
3+18	0.2042	1.33	Q	V		
3+19	0.2060	1.36	Q	V		
3+20	0.2080	1.39	Q	V		
3+21	0.2099	1.42	Q	V		
3+22	0.2119	1.44	Q	V		
3+23	0.2139	1.47	Q	V		
3+24	0.2160	1.50	Q	V		
3+25	0.2181	1.52	Q	V		
3+26	0.2202	1.55	Q	V		
3+27	0.2224	1.58	Q	V		
3+28	0.2246	1.60	Q	V		
3+29	0.2268	1.63	Q	V		
3+30	0.2291	1.65	Q	V		
3+31	0.2314	1.67	Q	V		
3+32	0.2337	1.69	Q	V		
3+33	0.2361	1.71	Q	V		
3+34	0.2385	1.73	Q	V		
3+35	0.2409	1.75	Q	V		
3+36	0.2434	1.78	Q	V		
3+37	0.2458	1.80	Q	V		
3+38	0.2483	1.82	Q	V		
3+39	0.2509	1.84	Q	V		
3+40	0.2534	1.86	Q	V		
3+41	0.2561	1.94	Q	V		
3+42	0.2589	2.02	Q	V		
3+43	0.2618	2.10	Q	V		
3+44	0.2648	2.17	Q	V		
3+45	0.2679	2.25	Q	V		
3+46	0.2711	2.33	Q	V		
3+47	0.2744	2.41	Q	V		
3+48	0.2778	2.49	Q	V		
3+49	0.2814	2.57	Q	V		
3+50	0.2850	2.65	Q	V		
3+51	0.2888	2.73	Q	V		
3+52	0.2927	2.83	Q	V		
3+53	0.2967	2.93	Q	V		
3+54	0.3009	3.03	Q	V		
3+55	0.3052	3.13	Q	V		
3+56	0.3097	3.24	Q	V		
3+57	0.3143	3.34	Q	V		
3+58	0.3190	3.44	Q	V		
3+59	0.3239	3.54	Q	V		

4+ 0	0.3289	3.64		Q	V			
4+ 1	0.3340	3.74		Q	V			
4+ 2	0.3393	3.84		Q	V			
4+ 3	0.3458	4.73			Q	V		
4+ 4	0.3536	5.61				Q	V	
4+ 5	0.3625	6.50					QV	
4+ 6	0.3727	7.39					VQ	
4+ 7	0.3841	8.27				V	Q	
4+ 8	0.3967	9.16				V		Q
4+ 9	0.4105	10.04				V		Q
4+10	0.4256	10.93					V	
4+11	0.4419	11.81					V	
4+12	0.4593	12.70					V	
4+13	0.4781	13.58					V	
4+14	0.4953	12.55					V	
4+15	0.5112	11.51					V	
4+16	0.5256	10.47					VQ	
4+17	0.5386	9.44					Q	V
4+18	0.5502	8.40					Q	V
4+19	0.5603	7.37				Q		V
4+20	0.5691	6.33				Q		V
4+21	0.5764	5.30				Q		V
4+22	0.5822	4.26			Q			V
4+23	0.5867	3.22		Q				V
4+24	0.5897	2.19		Q				V
4+25	0.5926	2.12		Q				V
4+26	0.5954	2.06		Q				V
4+27	0.5982	1.99		Q				V
4+28	0.6008	1.92		Q				V
4+29	0.6034	1.86		Q				V
4+30	0.6059	1.79		Q				V
4+31	0.6082	1.73		Q				V
4+32	0.6105	1.66		Q				V
4+33	0.6127	1.60		Q				V
4+34	0.6148	1.53		Q				V
4+35	0.6169	1.46		Q				V
4+36	0.6188	1.44		Q				V
4+37	0.6208	1.41		Q				V
4+38	0.6227	1.38		Q				V
4+39	0.6245	1.35		Q				V
4+40	0.6263	1.32		Q				V
4+41	0.6281	1.29		Q				V
4+42	0.6299	1.26		Q				V
4+43	0.6316	1.23		Q				V
4+44	0.6332	1.20		Q				V
4+45	0.6348	1.17		Q				V
4+46	0.6364	1.15		Q				V
4+47	0.6380	1.13		Q				V
4+48	0.6395	1.11		Q				V
4+49	0.6410	1.09		Q				V

4+50	0.6425	1.08	Q				V
4+51	0.6439	1.06	Q				V
4+52	0.6454	1.04	Q				V
4+53	0.6468	1.03	Q				V
4+54	0.6482	1.01	Q				V
4+55	0.6496	0.99	Q				V
4+56	0.6509	0.98	Q				V
4+57	0.6522	0.96	Q				V
4+58	0.6535	0.95	Q				V
4+59	0.6548	0.94	Q				V
5+ 0	0.6561	0.93	Q				V
5+ 1	0.6574	0.91	Q				V
5+ 2	0.6586	0.90	Q				V
5+ 3	0.6598	0.89	Q				V
5+ 4	0.6610	0.88	Q				V
5+ 5	0.6622	0.87	Q				V
5+ 6	0.6634	0.86	Q				V
5+ 7	0.6646	0.85	Q				V
5+ 8	0.6657	0.84	Q				V
5+ 9	0.6669	0.83	Q				V
5+10	0.6680	0.82	Q				V
5+11	0.6691	0.81	Q				V
5+12	0.6702	0.80	Q				V
5+13	0.6713	0.79	Q				V
5+14	0.6724	0.79	Q				V
5+15	0.6735	0.78	Q				V
5+16	0.6745	0.77	Q				V
5+17	0.6756	0.76	Q				V
5+18	0.6766	0.75	Q				V
5+19	0.6776	0.75	Q				V
5+20	0.6787	0.74	Q				V
5+21	0.6797	0.73	Q				V
5+22	0.6807	0.73	Q				V
5+23	0.6817	0.72	Q				V
5+24	0.6826	0.71	Q				V
5+25	0.6836	0.71	Q				V
5+26	0.6846	0.70	Q				V
5+27	0.6855	0.69	Q				V
5+28	0.6865	0.69	Q				V
5+29	0.6874	0.68	Q				V
5+30	0.6884	0.68	Q				V
5+31	0.6893	0.67	Q				V
5+32	0.6902	0.67	Q				V
5+33	0.6911	0.66	Q				V
5+34	0.6920	0.66	Q				V
5+35	0.6929	0.65	Q				V
5+36	0.6938	0.65	Q				V
5+37	0.6947	0.64	Q				V
5+38	0.6956	0.64	Q				V
5+39	0.6964	0.63	Q				V

5+40	0.6973	0.63	Q				V
5+41	0.6982	0.62	Q				V
5+42	0.6990	0.62	Q				V
5+43	0.6998	0.61	Q				V
5+44	0.7007	0.61	Q				V
5+45	0.7015	0.61	Q				V
5+46	0.7024	0.60	Q				V
5+47	0.7032	0.60	Q				V
5+48	0.7040	0.59	Q				V
5+49	0.7048	0.59	Q				V
5+50	0.7056	0.58	Q				V
5+51	0.7064	0.58	Q				V
5+52	0.7072	0.58	Q				V
5+53	0.7080	0.57	Q				V
5+54	0.7088	0.57	Q				V
5+55	0.7096	0.57	Q				V
5+56	0.7103	0.56	Q				V
5+57	0.7111	0.56	Q				V
5+58	0.7119	0.56	Q				V
5+59	0.7126	0.55	Q				V
6+ 0	0.7134	0.55	Q				V
6+ 1	0.7141	0.55	Q				V
6+ 2	0.7149	0.54	Q				V
6+ 3	0.7156	0.54	Q				V

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Process from Point/Station 325.000 to Point/Station 402.000

**** 6 HOUR HYDROGRAPH ****

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Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 11.32

Basin Area = 4.51 Acres

6 Hour Rainfall = 3.300 Inches

Runoff Coefficient = 0.584

Peak Discharge = 13.58 CFS

Time (Min)	Discharge (CFS)
0	0.000
11	0.518
22	0.528
33	0.551
44	0.564

55	0.591
66	0.606
77	0.638
88	0.657
99	0.697
110	0.720
121	0.772
132	0.802
143	0.872
154	0.913
165	1.013
176	1.074
187	1.231
198	1.335
209	1.631
220	1.858
231	2.728
242	3.844
253	13.582
264	2.188
275	1.464
286	1.146
297	0.960
308	0.835
319	0.745
330	0.676
341	0.622
352	0.577
363	0.540

+++++

6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	3.4	6.8	10.2	13.6
0+ 0	0.0000	0.00	Q					
0+ 1	0.0001	0.05	Q					
0+ 2	0.0002	0.09	Q					
0+ 3	0.0004	0.14	Q					
0+ 4	0.0006	0.19	Q					
0+ 5	0.0010	0.24	Q					
0+ 6	0.0014	0.28	Q					
0+ 7	0.0018	0.33	Q					
0+ 8	0.0023	0.38	VQ					
0+ 9	0.0029	0.42	VQ					
0+10	0.0036	0.47	VQ					
0+11	0.0043	0.52	VQ					

0+12	0.0050	0.52	VQ				
0+13	0.0057	0.52	VQ				
0+14	0.0064	0.52	VQ				
0+15	0.0071	0.52	VQ				
0+16	0.0079	0.52	VQ				
0+17	0.0086	0.52	VQ				
0+18	0.0093	0.52	VQ				
0+19	0.0100	0.53	VQ				
0+20	0.0108	0.53	VQ				
0+21	0.0115	0.53	VQ				
0+22	0.0122	0.53	VQ				
0+23	0.0129	0.53	VQ				
0+24	0.0137	0.53	VQ				
0+25	0.0144	0.53	VQ				
0+26	0.0152	0.54	VQ				
0+27	0.0159	0.54	VQ				
0+28	0.0166	0.54	VQ				
0+29	0.0174	0.54	VQ				
0+30	0.0181	0.55	Q				
0+31	0.0189	0.55	Q				
0+32	0.0197	0.55	Q				
0+33	0.0204	0.55	Q				
0+34	0.0212	0.55	Q				
0+35	0.0219	0.55	Q				
0+36	0.0227	0.55	Q				
0+37	0.0235	0.56	Q				
0+38	0.0242	0.56	Q				
0+39	0.0250	0.56	Q				
0+40	0.0258	0.56	Q				
0+41	0.0265	0.56	Q				
0+42	0.0273	0.56	Q				
0+43	0.0281	0.56	Q				
0+44	0.0289	0.56	Q				
0+45	0.0296	0.57	Q				
0+46	0.0304	0.57	Q				
0+47	0.0312	0.57	Q				
0+48	0.0320	0.57	Q				
0+49	0.0328	0.58	Q				
0+50	0.0336	0.58	Q				
0+51	0.0344	0.58	Q				
0+52	0.0352	0.58	Q				
0+53	0.0360	0.59	QV				
0+54	0.0368	0.59	QV				
0+55	0.0376	0.59	QV				
0+56	0.0384	0.59	QV				
0+57	0.0393	0.59	QV				
0+58	0.0401	0.59	QV				
0+59	0.0409	0.60	QV				
1+ 0	0.0417	0.60	QV				
1+ 1	0.0426	0.60	QV				

1+ 2	0.0434	0.60	QV				
1+ 3	0.0442	0.60	QV				
1+ 4	0.0450	0.60	QV				
1+ 5	0.0459	0.60	QV				
1+ 6	0.0467	0.61	QV				
1+ 7	0.0475	0.61	QV				
1+ 8	0.0484	0.61	QV				
1+ 9	0.0492	0.61	QV				
1+10	0.0501	0.62	QV				
1+11	0.0509	0.62	QV				
1+12	0.0518	0.62	QV				
1+13	0.0527	0.63	QV				
1+14	0.0535	0.63	QV				
1+15	0.0544	0.63	Q V				
1+16	0.0553	0.64	Q V				
1+17	0.0562	0.64	Q V				
1+18	0.0570	0.64	Q V				
1+19	0.0579	0.64	Q V				
1+20	0.0588	0.64	Q V				
1+21	0.0597	0.65	Q V				
1+22	0.0606	0.65	Q V				
1+23	0.0615	0.65	Q V				
1+24	0.0624	0.65	Q V				
1+25	0.0633	0.65	Q V				
1+26	0.0642	0.65	Q V				
1+27	0.0651	0.65	Q V				
1+28	0.0660	0.66	Q V				
1+29	0.0669	0.66	Q V				
1+30	0.0678	0.66	Q V				
1+31	0.0687	0.67	Q V				
1+32	0.0696	0.67	Q V				
1+33	0.0706	0.68	Q V				
1+34	0.0715	0.68	Q V				
1+35	0.0725	0.68	Q V				
1+36	0.0734	0.69	Q V				
1+37	0.0743	0.69	Q V				
1+38	0.0753	0.69	Q V				
1+39	0.0763	0.70	Q V				
1+40	0.0772	0.70	Q V				
1+41	0.0782	0.70	Q V				
1+42	0.0792	0.70	Q V				
1+43	0.0801	0.71	Q V				
1+44	0.0811	0.71	Q V				
1+45	0.0821	0.71	Q V				
1+46	0.0831	0.71	Q V				
1+47	0.0841	0.71	Q V				
1+48	0.0850	0.72	Q V				
1+49	0.0860	0.72	Q V				
1+50	0.0870	0.72	Q V				
1+51	0.0880	0.72	Q V				

1+52	0.0890	0.73	Q V				
1+53	0.0900	0.73	Q V				
1+54	0.0911	0.74	Q V				
1+55	0.0921	0.74	Q V				
1+56	0.0931	0.75	Q V				
1+57	0.0941	0.75	Q V				
1+58	0.0952	0.76	Q V				
1+59	0.0962	0.76	Q V				
2+ 0	0.0973	0.77	Q V				
2+ 1	0.0984	0.77	Q V				
2+ 2	0.0994	0.77	Q V				
2+ 3	0.1005	0.78	Q V				
2+ 4	0.1016	0.78	Q V				
2+ 5	0.1027	0.78	Q V				
2+ 6	0.1037	0.79	Q V				
2+ 7	0.1048	0.79	Q V				
2+ 8	0.1059	0.79	Q V				
2+ 9	0.1070	0.79	Q V				
2+10	0.1081	0.80	Q V				
2+11	0.1092	0.80	Q V				
2+12	0.1103	0.80	Q V				
2+13	0.1114	0.81	Q V				
2+14	0.1125	0.81	Q V				
2+15	0.1137	0.82	Q V				
2+16	0.1148	0.83	Q V				
2+17	0.1160	0.83	Q V				
2+18	0.1171	0.84	Q V				
2+19	0.1183	0.85	Q V				
2+20	0.1195	0.85	Q V				
2+21	0.1206	0.86	Q V				
2+22	0.1218	0.87	Q V				
2+23	0.1230	0.87	Q V				
2+24	0.1242	0.88	Q V				
2+25	0.1255	0.88	Q V				
2+26	0.1267	0.88	Q V				
2+27	0.1279	0.89	Q V				
2+28	0.1291	0.89	Q V				
2+29	0.1303	0.89	Q V				
2+30	0.1316	0.90	Q V				
2+31	0.1328	0.90	Q V				
2+32	0.1341	0.91	Q V				
2+33	0.1353	0.91	Q V				
2+34	0.1366	0.91	Q V				
2+35	0.1379	0.92	Q V				
2+36	0.1391	0.93	Q V				
2+37	0.1404	0.94	Q V				
2+38	0.1417	0.95	Q V				
2+39	0.1431	0.96	Q V				
2+40	0.1444	0.97	Q V				
2+41	0.1457	0.98	Q V				

2+42	0.1471	0.99	Q	V				
2+43	0.1485	0.99	Q	V				
2+44	0.1498	1.00	Q	V				
2+45	0.1512	1.01	Q	V				
2+46	0.1526	1.02	Q	V				
2+47	0.1541	1.02	Q	V				
2+48	0.1555	1.03	Q	V				
2+49	0.1569	1.03	Q	V				
2+50	0.1583	1.04	Q	V				
2+51	0.1598	1.05	Q	V				
2+52	0.1612	1.05	Q	V				
2+53	0.1627	1.06	Q	V				
2+54	0.1641	1.06	Q	V				
2+55	0.1656	1.07	Q	V				
2+56	0.1671	1.07	Q	V				
2+57	0.1686	1.09	Q	V				
2+58	0.1701	1.10	Q	V				
2+59	0.1716	1.12	Q	V				
3+ 0	0.1732	1.13	Q	V				
3+ 1	0.1748	1.15	Q	V				
3+ 2	0.1764	1.16	Q	V				
3+ 3	0.1780	1.17	Q	V				
3+ 4	0.1796	1.19	Q	V				
3+ 5	0.1813	1.20	Q	V				
3+ 6	0.1830	1.22	Q	V				
3+ 7	0.1847	1.23	Q	V				
3+ 8	0.1864	1.24	Q	V				
3+ 9	0.1881	1.25	Q	V				
3+10	0.1898	1.26	Q	V				
3+11	0.1916	1.27	Q	V				
3+12	0.1933	1.28	Q	V				
3+13	0.1951	1.29	Q	V				
3+14	0.1969	1.30	Q	V				
3+15	0.1987	1.31	Q	V				
3+16	0.2005	1.32	Q	V				
3+17	0.2023	1.33	Q	V				
3+18	0.2042	1.33	Q	V				
3+19	0.2060	1.36	Q	V				
3+20	0.2080	1.39	Q	V				
3+21	0.2099	1.42	Q	V				
3+22	0.2119	1.44	Q	V				
3+23	0.2139	1.47	Q	V				
3+24	0.2160	1.50	Q	V				
3+25	0.2181	1.52	Q	V				
3+26	0.2202	1.55	Q	V				
3+27	0.2224	1.58	Q	V				
3+28	0.2246	1.60	Q	V				
3+29	0.2268	1.63	Q	V				
3+30	0.2291	1.65	Q	V				
3+31	0.2314	1.67	Q	V				

3+32	0.2337	1.69	Q	V					
3+33	0.2361	1.71	Q	V					
3+34	0.2385	1.73	Q	V					
3+35	0.2409	1.75	Q	V					
3+36	0.2434	1.78	Q	V					
3+37	0.2458	1.80	Q	V					
3+38	0.2483	1.82	Q	V					
3+39	0.2509	1.84	Q	V					
3+40	0.2534	1.86	Q	V					
3+41	0.2561	1.94	Q	V					
3+42	0.2589	2.02	Q	V					
3+43	0.2618	2.10	Q	V					
3+44	0.2648	2.17	Q	V					
3+45	0.2679	2.25	Q	V					
3+46	0.2711	2.33	Q	V					
3+47	0.2744	2.41	Q	V					
3+48	0.2778	2.49	Q	V					
3+49	0.2814	2.57	Q	V					
3+50	0.2850	2.65	Q	V					
3+51	0.2888	2.73	Q	V					
3+52	0.2927	2.83	Q	V					
3+53	0.2967	2.93	Q	V					
3+54	0.3009	3.03	Q	V					
3+55	0.3052	3.13	Q	V					
3+56	0.3097	3.24	Q	V					
3+57	0.3143	3.34	Q	V					
3+58	0.3190	3.44	Q	V					
3+59	0.3239	3.54	Q	V					
4+ 0	0.3289	3.64	Q	V					
4+ 1	0.3340	3.74	Q	V					
4+ 2	0.3393	3.84	Q	V					
4+ 3	0.3458	4.73	Q	V					
4+ 4	0.3536	5.61	Q	V					
4+ 5	0.3625	6.50	Q	V					
4+ 6	0.3727	7.39	Q	V					
4+ 7	0.3841	8.27	Q	V					
4+ 8	0.3967	9.16	Q	V					
4+ 9	0.4105	10.04	Q	V					
4+10	0.4256	10.93	Q	V					
4+11	0.4419	11.81	Q	V					
4+12	0.4593	12.70	Q	V					
4+13	0.4781	13.58	Q	V					
4+14	0.4953	12.55	Q	V					
4+15	0.5112	11.51	Q	V					
4+16	0.5256	10.47	Q	V					
4+17	0.5386	9.44	Q	V					
4+18	0.5502	8.40	Q	V					
4+19	0.5603	7.37	Q	V					
4+20	0.5691	6.33	Q	V					
4+21	0.5764	5.30	Q	V					

4+22	0.5822	4.26		Q	V
4+23	0.5867	3.22		Q	V
4+24	0.5897	2.19	Q		V
4+25	0.5926	2.12	Q		V
4+26	0.5954	2.06	Q		V
4+27	0.5982	1.99	Q		V
4+28	0.6008	1.92	Q		V
4+29	0.6034	1.86	Q		V
4+30	0.6059	1.79	Q		V
4+31	0.6082	1.73	Q		V
4+32	0.6105	1.66	Q		V
4+33	0.6127	1.60	Q		V
4+34	0.6148	1.53	Q		V
4+35	0.6169	1.46	Q		V
4+36	0.6188	1.44	Q		V
4+37	0.6208	1.41	Q		V
4+38	0.6227	1.38	Q		V
4+39	0.6245	1.35	Q		V
4+40	0.6263	1.32	Q		V
4+41	0.6281	1.29	Q		V
4+42	0.6299	1.26	Q		V
4+43	0.6316	1.23	Q		V
4+44	0.6332	1.20	Q		V
4+45	0.6348	1.17	Q		V
4+46	0.6364	1.15	Q		V
4+47	0.6380	1.13	Q		V
4+48	0.6395	1.11	Q		V
4+49	0.6410	1.09	Q		V
4+50	0.6425	1.08	Q		V
4+51	0.6439	1.06	Q		V
4+52	0.6454	1.04	Q		V
4+53	0.6468	1.03	Q		V
4+54	0.6482	1.01	Q		V
4+55	0.6496	0.99	Q		V
4+56	0.6509	0.98	Q		V
4+57	0.6522	0.96	Q		V
4+58	0.6535	0.95	Q		V
4+59	0.6548	0.94	Q		V
5+ 0	0.6561	0.93	Q		V
5+ 1	0.6574	0.91	Q		V
5+ 2	0.6586	0.90	Q		V
5+ 3	0.6598	0.89	Q		V
5+ 4	0.6610	0.88	Q		V
5+ 5	0.6622	0.87	Q		V
5+ 6	0.6634	0.86	Q		V
5+ 7	0.6646	0.85	Q		V
5+ 8	0.6657	0.84	Q		V
5+ 9	0.6669	0.83	Q		V
5+10	0.6680	0.82	Q		V
5+11	0.6691	0.81	Q		V

5+12	0.6702	0.80	Q				V
5+13	0.6713	0.79	Q				V
5+14	0.6724	0.79	Q				V
5+15	0.6735	0.78	Q				V
5+16	0.6745	0.77	Q				V
5+17	0.6756	0.76	Q				V
5+18	0.6766	0.75	Q				V
5+19	0.6776	0.75	Q				V
5+20	0.6787	0.74	Q				V
5+21	0.6797	0.73	Q				V
5+22	0.6807	0.73	Q				V
5+23	0.6817	0.72	Q				V
5+24	0.6826	0.71	Q				V
5+25	0.6836	0.71	Q				V
5+26	0.6846	0.70	Q				V
5+27	0.6855	0.69	Q				V
5+28	0.6865	0.69	Q				V
5+29	0.6874	0.68	Q				V
5+30	0.6884	0.68	Q				V
5+31	0.6893	0.67	Q				V
5+32	0.6902	0.67	Q				V
5+33	0.6911	0.66	Q				V
5+34	0.6920	0.66	Q				V
5+35	0.6929	0.65	Q				V
5+36	0.6938	0.65	Q				V
5+37	0.6947	0.64	Q				V
5+38	0.6956	0.64	Q				V
5+39	0.6964	0.63	Q				V
5+40	0.6973	0.63	Q				V
5+41	0.6982	0.62	Q				V
5+42	0.6990	0.62	Q				V
5+43	0.6998	0.61	Q				V
5+44	0.7007	0.61	Q				V
5+45	0.7015	0.61	Q				V
5+46	0.7024	0.60	Q				V
5+47	0.7032	0.60	Q				V
5+48	0.7040	0.59	Q				V
5+49	0.7048	0.59	Q				V
5+50	0.7056	0.58	Q				V
5+51	0.7064	0.58	Q				V
5+52	0.7072	0.58	Q				V
5+53	0.7080	0.57	Q				V
5+54	0.7088	0.57	Q				V
5+55	0.7096	0.57	Q				V
5+56	0.7103	0.56	Q				V
5+57	0.7111	0.56	Q				V
5+58	0.7119	0.56	Q				V
5+59	0.7126	0.55	Q				V
6+ 0	0.7134	0.55	Q				V
6+ 1	0.7141	0.55	Q				V

6+ 2	0.7149	0.54	Q				V
6+ 3	0.7156	0.54	Q				V

End of computations, total study area = 4.510 (Ac.)

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 06/21/21

108-001 Idaho (Escondido Estates)
Onsite Proposed Condition BMP1 Confluence
Extrapolated storm from County Isopluvials - Unmitigated
TH

***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:
6 hour, precipitation(inches) = 3.300
24 hour precipitation(inches) = 7.300
P6/P24 = 45.2%
San Diego hydrology manual 'C' values used

++++
Process from Point/Station 401.000 to Point/Station 403.000
**** USER DEFINED FLOW INFORMATION AT A POINT ****

User specified 'C' value of 0.497 given for subarea
Rainfall intensity (I) = 7.070(In/Hr) for a 100.0 year storm
User specified values are as follows:
TC = 6.89 min. Rain intensity = 7.07(In/Hr)
Total area = 8.760(Ac.) Total runoff = 30.757(CFS)

++++
Process from Point/Station 401.000 to Point/Station 403.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
In Main Stream number: 1
Stream flow area = 8.760(Ac.)

Runoff from this stream = 30.757(CFS)
 Time of concentration = 6.89 min.
 Rainfall intensity = 7.070(In/Hr)
 Program is now starting with Main Stream No. 2

+++++
 Process from Point/Station 402.000 to Point/Station 403.000
 **** USER DEFINED FLOW INFORMATION AT A POINT ****

User specified 'C' value of 0.584 given for subarea
 Rainfall intensity (I) = 4.564(In/Hr) for a 100.0 year storm
 User specified values are as follows:
 TC = 13.58 min. Rain intensity = 4.56(In/Hr)
 Total area = 4.510(Ac.) Total runoff = 11.320(CFS)

+++++
 Process from Point/Station 402.000 to Point/Station 403.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 4.510(Ac.)
 Runoff from this stream = 11.320(CFS)
 Time of concentration = 13.58 min.
 Rainfall intensity = 4.564(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	30.757	6.89	7.070
2	11.320	13.58	4.564

Qmax(1) =

1.000 *	1.000 *	30.757)	+	
1.000 *	0.507 *	11.320)	+	36.500

Qmax(2) =

0.646 *	1.000 *	30.757)	+	
1.000 *	1.000 *	11.320)	+	31.175

Total of 2 main streams to confluence:

Flow rates before confluence point:

30.757 11.320

Maximum flow rates at confluence using above data:

36.500 31.175

Area of streams before confluence:

8.760 4.510

Results of confluence:

Total flow rate = 36.500(CFS)

Time of concentration = 6.890 min.

Effective stream area after confluence = 13.270(Ac.)

+++++

Process from Point/Station 402.000 to Point/Station 403.000

**** 6 HOUR HYDROGRAPH ****

+++++

Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 6.89

Basin Area = 13.27 Acres

6 Hour Rainfall = 3.300 Inches

Runoff Coefficient = 0.527

Peak Discharge = 36.50 CFS

Time (Min)	Discharge (CFS)
0	0.000
6	1.375
12	1.390
18	1.421
24	1.438
30	1.472
36	1.490
42	1.528
48	1.548
54	1.590
60	1.612
66	1.658
72	1.682
78	1.734
84	1.762
90	1.820
96	1.851
102	1.918
108	1.954
114	2.030
120	2.072
126	2.161
132	2.210
138	2.316
144	2.374
150	2.503
156	2.574
162	2.733
168	2.823

174	3.027
180	3.144
186	3.418
192	3.579
198	3.969
204	4.210
210	4.825
216	5.232
222	6.395
228	7.284
234	10.695
240	15.069
246	36.500
252	8.578
258	5.739
264	4.491
270	3.761
276	3.274
282	2.921
288	2.651
294	2.436
300	2.261
306	2.115
312	1.991
318	1.884
324	1.790
330	1.708
336	1.634
342	1.568
348	1.509
354	1.455
360	1.405
366	1.360

+++++

6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	9.1	18.3	27.4	36.5
0+ 0	0.0000	0.00	Q					
0+ 1	0.0003	0.23	Q					
0+ 2	0.0009	0.46	Q					
0+ 3	0.0019	0.69	Q					
0+ 4	0.0032	0.92	VQ					
0+ 5	0.0047	1.15	VQ					
0+ 6	0.0066	1.37	VQ					
0+ 7	0.0085	1.38	VQ					

0+ 8	0.0104	1.38	VQ				
0+ 9	0.0123	1.38	VQ				
0+10	0.0142	1.38	VQ				
0+11	0.0161	1.39	VQ				
0+12	0.0181	1.39	VQ				
0+13	0.0200	1.39	VQ				
0+14	0.0219	1.40	VQ				
0+15	0.0238	1.41	VQ				
0+16	0.0258	1.41	VQ				
0+17	0.0277	1.42	VQ				
0+18	0.0297	1.42	VQ				
0+19	0.0317	1.42	VQ				
0+20	0.0336	1.43	VQ				
0+21	0.0356	1.43	VQ				
0+22	0.0376	1.43	VQ				
0+23	0.0395	1.43	VQ				
0+24	0.0415	1.44	VQ				
0+25	0.0435	1.44	VQ				
0+26	0.0455	1.45	Q				
0+27	0.0475	1.45	Q				
0+28	0.0495	1.46	Q				
0+29	0.0515	1.47	Q				
0+30	0.0536	1.47	Q				
0+31	0.0556	1.48	Q				
0+32	0.0576	1.48	Q				
0+33	0.0597	1.48	Q				
0+34	0.0617	1.48	Q				
0+35	0.0638	1.49	Q				
0+36	0.0658	1.49	Q				
0+37	0.0679	1.50	Q				
0+38	0.0700	1.50	Q				
0+39	0.0720	1.51	Q				
0+40	0.0741	1.52	Q				
0+41	0.0762	1.52	Q				
0+42	0.0783	1.53	Q				
0+43	0.0804	1.53	Q				
0+44	0.0825	1.53	Q				
0+45	0.0847	1.54	Q				
0+46	0.0868	1.54	Q				
0+47	0.0889	1.54	QV				
0+48	0.0910	1.55	QV				
0+49	0.0932	1.55	QV				
0+50	0.0953	1.56	QV				
0+51	0.0975	1.57	QV				
0+52	0.0997	1.58	QV				
0+53	0.1018	1.58	QV				
0+54	0.1040	1.59	QV				
0+55	0.1062	1.59	QV				
0+56	0.1084	1.60	QV				
0+57	0.1106	1.60	QV				

0+58	0.1128	1.60	QV				
0+59	0.1151	1.61	QV				
1+ 0	0.1173	1.61	QV				
1+ 1	0.1195	1.62	QV				
1+ 2	0.1218	1.63	QV				
1+ 3	0.1240	1.63	QV				
1+ 4	0.1263	1.64	QV				
1+ 5	0.1285	1.65	QV				
1+ 6	0.1308	1.66	QV				
1+ 7	0.1331	1.66	Q V				
1+ 8	0.1354	1.67	Q V				
1+ 9	0.1377	1.67	Q V				
1+10	0.1400	1.67	Q V				
1+11	0.1423	1.68	Q V				
1+12	0.1446	1.68	Q V				
1+13	0.1470	1.69	Q V				
1+14	0.1493	1.70	Q V				
1+15	0.1517	1.71	Q V				
1+16	0.1540	1.72	Q V				
1+17	0.1564	1.73	Q V				
1+18	0.1588	1.73	Q V				
1+19	0.1612	1.74	Q V				
1+20	0.1636	1.74	Q V				
1+21	0.1660	1.75	Q V				
1+22	0.1684	1.75	Q V				
1+23	0.1708	1.76	Q V				
1+24	0.1733	1.76	Q V				
1+25	0.1757	1.77	Q V				
1+26	0.1782	1.78	Q V				
1+27	0.1806	1.79	Q V				
1+28	0.1831	1.80	Q V				
1+29	0.1856	1.81	Q V				
1+30	0.1881	1.82	Q V				
1+31	0.1906	1.83	Q V				
1+32	0.1931	1.83	Q V				
1+33	0.1957	1.84	Q V				
1+34	0.1982	1.84	Q V				
1+35	0.2007	1.85	Q V				
1+36	0.2033	1.85	Q V				
1+37	0.2059	1.86	Q V				
1+38	0.2084	1.87	Q V				
1+39	0.2110	1.88	Q V				
1+40	0.2136	1.90	Q V				
1+41	0.2163	1.91	Q V				
1+42	0.2189	1.92	Q V				
1+43	0.2216	1.92	Q V				
1+44	0.2242	1.93	Q V				
1+45	0.2269	1.94	Q V				
1+46	0.2296	1.94	Q V				
1+47	0.2323	1.95	Q V				

1+48	0.2349	1.95	Q	V				
1+49	0.2376	1.97	Q	V				
1+50	0.2404	1.98	Q	V				
1+51	0.2431	1.99	Q	V				
1+52	0.2459	2.00	Q	V				
1+53	0.2487	2.02	Q	V				
1+54	0.2515	2.03	Q	V				
1+55	0.2543	2.04	Q	V				
1+56	0.2571	2.04	Q	V				
1+57	0.2599	2.05	Q	V				
1+58	0.2627	2.06	Q	V				
1+59	0.2656	2.06	Q	V				
2+ 0	0.2684	2.07	Q	V				
2+ 1	0.2713	2.09	Q	V				
2+ 2	0.2742	2.10	Q	V				
2+ 3	0.2771	2.12	Q	V				
2+ 4	0.2801	2.13	Q	V				
2+ 5	0.2830	2.15	Q	V				
2+ 6	0.2860	2.16	Q	V				
2+ 7	0.2890	2.17	Q	V				
2+ 8	0.2920	2.18	Q	V				
2+ 9	0.2950	2.19	Q	V				
2+10	0.2980	2.19	Q	V				
2+11	0.3010	2.20	Q	V				
2+12	0.3041	2.21	Q	V				
2+13	0.3072	2.23	Q	V				
2+14	0.3102	2.25	Q	V				
2+15	0.3134	2.26	Q	V				
2+16	0.3165	2.28	Q	V				
2+17	0.3197	2.30	Q	V				
2+18	0.3229	2.32	Q	V				
2+19	0.3261	2.33	Q	V				
2+20	0.3293	2.34	Q	V				
2+21	0.3325	2.35	Q	V				
2+22	0.3358	2.35	Q	V				
2+23	0.3390	2.36	Q	V				
2+24	0.3423	2.37	Q	V				
2+25	0.3456	2.40	Q	V				
2+26	0.3489	2.42	Q	V				
2+27	0.3523	2.44	Q	V				
2+28	0.3557	2.46	Q	V				
2+29	0.3591	2.48	Q	V				
2+30	0.3625	2.50	Q	V				
2+31	0.3660	2.51	Q	V				
2+32	0.3695	2.53	Q	V				
2+33	0.3730	2.54	Q	V				
2+34	0.3765	2.55	Q	V				
2+35	0.3800	2.56	Q	V				
2+36	0.3836	2.57	Q	V				
2+37	0.3871	2.60	Q	V				

2+38	0.3908	2.63	Q	V				
2+39	0.3944	2.65	Q	V				
2+40	0.3981	2.68	Q	V				
2+41	0.4018	2.71	Q	V				
2+42	0.4056	2.73	Q	V				
2+43	0.4094	2.75	Q	V				
2+44	0.4132	2.76	Q	V				
2+45	0.4170	2.78	Q	V				
2+46	0.4209	2.79	Q	V				
2+47	0.4247	2.81	Q	V				
2+48	0.4286	2.82	Q	V				
2+49	0.4326	2.86	Q	V				
2+50	0.4365	2.89	Q	V				
2+51	0.4406	2.93	Q	V				
2+52	0.4446	2.96	Q	V				
2+53	0.4488	2.99	Q	V				
2+54	0.4529	3.03	Q	V				
2+55	0.4571	3.05	Q	V				
2+56	0.4614	3.07	Q	V				
2+57	0.4656	3.09	Q	V				
2+58	0.4699	3.11	Q	V				
2+59	0.4742	3.12	Q	V				
3+ 0	0.4785	3.14	Q	V				
3+ 1	0.4829	3.19	Q	V				
3+ 2	0.4874	3.24	Q	V				
3+ 3	0.4919	3.28	Q	V				
3+ 4	0.4965	3.33	Q	V				
3+ 5	0.5011	3.37	Q	V				
3+ 6	0.5058	3.42	Q	V				
3+ 7	0.5106	3.44	Q	V				
3+ 8	0.5153	3.47	Q	V				
3+ 9	0.5202	3.50	Q	V				
3+10	0.5250	3.53	Q	V				
3+11	0.5299	3.55	Q	V				
3+12	0.5348	3.58	Q	V				
3+13	0.5399	3.64	Q	V				
3+14	0.5450	3.71	Q	V				
3+15	0.5502	3.77	Q	V				
3+16	0.5555	3.84	Q	V				
3+17	0.5608	3.90	Q	V				
3+18	0.5663	3.97	Q	V				
3+19	0.5718	4.01	Q	V				
3+20	0.5774	4.05	Q	V				
3+21	0.5830	4.09	Q	V				
3+22	0.5887	4.13	Q	V				
3+23	0.5945	4.17	Q	V				
3+24	0.6003	4.21	Q	V				
3+25	0.6062	4.31	Q	V				
3+26	0.6123	4.41	Q	V				
3+27	0.6185	4.52	Q	V				

3+28	0.6249	4.62	Q	V					
3+29	0.6314	4.72	Q	V					
3+30	0.6380	4.83	Q	V					
3+31	0.6448	4.89	Q	V					
3+32	0.6516	4.96	Q	V					
3+33	0.6585	5.03	Q	V					
3+34	0.6655	5.10	Q	V					
3+35	0.6727	5.16	Q	V					
3+36	0.6799	5.23	Q	V					
3+37	0.6873	5.43	Q	V					
3+38	0.6951	5.62	Q	V					
3+39	0.7031	5.81	Q	V					
3+40	0.7114	6.01	Q	V					
3+41	0.7199	6.20	Q	V					
3+42	0.7287	6.39	Q	V					
3+43	0.7377	6.54	Q	V					
3+44	0.7469	6.69	Q	V					
3+45	0.7564	6.84	Q	V					
3+46	0.7660	6.99	Q	V					
3+47	0.7758	7.14	Q	V					
3+48	0.7858	7.28	Q	V					
3+49	0.7967	7.85	Q	V					
3+50	0.8083	8.42	Q	V					
3+51	0.8206	8.99	Q	V					
3+52	0.8338	9.56	Q	V					
3+53	0.8478	10.13	Q	V					
3+54	0.8625	10.70	Q	V					
3+55	0.8782	11.42	Q	V					
3+56	0.8950	12.15	Q	V					
3+57	0.9127	12.88	Q	V					
3+58	0.9315	13.61	Q	V					
3+59	0.9512	14.34	Q	V					
4+ 0	0.9720	15.07	Q	V					
4+ 1	0.9976	18.64	Q	V					
4+ 2	1.0282	22.21	Q	V					
4+ 3	1.0638	25.78	Q	V					
4+ 4	1.1042	29.36	Q	V					
4+ 5	1.1495	32.93	Q	V					
4+ 6	1.1998	36.50	Q	V					
4+ 7	1.2437	31.85	Q	V					
4+ 8	1.2811	27.19	Q	V					
4+ 9	1.3122	22.54	Q	V					
4+10	1.3368	17.89	Q	V					
4+11	1.3551	13.23	Q	V					
4+12	1.3669	8.58	Q	V					
4+13	1.3780	8.10	Q	V					
4+14	1.3885	7.63	Q	V					
4+15	1.3984	7.16	Q	V					
4+16	1.4076	6.69	Q	V					
4+17	1.4162	6.21	Q	V					

4+18	1.4241	5.74	Q			V
4+19	1.4317	5.53	Q			V
4+20	1.4390	5.32	Q			V
4+21	1.4461	5.12	Q			V
4+22	1.4528	4.91	Q			V
4+23	1.4593	4.70	Q			V
4+24	1.4655	4.49	Q			V
4+25	1.4715	4.37	Q			V
4+26	1.4774	4.25	Q			V
4+27	1.4830	4.13	Q			V
4+28	1.4886	4.00	Q			V
4+29	1.4939	3.88	Q			V
4+30	1.4991	3.76	Q			V
4+31	1.5042	3.68	Q			V
4+32	1.5091	3.60	Q			V
4+33	1.5140	3.52	Q			V
4+34	1.5187	3.44	Q			V
4+35	1.5233	3.36	Q			V
4+36	1.5278	3.27	Q			V
4+37	1.5322	3.21	Q			V
4+38	1.5366	3.16	Q			V
4+39	1.5409	3.10	Q			V
4+40	1.5450	3.04	Q			V
4+41	1.5492	2.98	Q			V
4+42	1.5532	2.92	Q			V
4+43	1.5571	2.88	Q			V
4+44	1.5610	2.83	Q			V
4+45	1.5649	2.79	Q			V
4+46	1.5686	2.74	Q			V
4+47	1.5724	2.70	Q			V
4+48	1.5760	2.65	Q			V
4+49	1.5796	2.61	Q			V
4+50	1.5832	2.58	Q			V
4+51	1.5867	2.54	Q			V
4+52	1.5901	2.51	Q			V
4+53	1.5935	2.47	Q			V
4+54	1.5969	2.44	Q			V
4+55	1.6002	2.41	Q			V
4+56	1.6035	2.38	Q			V
4+57	1.6067	2.35	Q			V
4+58	1.6099	2.32	Q			V
4+59	1.6131	2.29	Q			V
5+ 0	1.6162	2.26	Q			V
5+ 1	1.6193	2.24	Q			V
5+ 2	1.6223	2.21	Q			V
5+ 3	1.6253	2.19	Q			V
5+ 4	1.6283	2.16	Q			V
5+ 5	1.6312	2.14	Q			V
5+ 6	1.6342	2.12	Q			V
5+ 7	1.6370	2.09	Q			V

5+ 8	1.6399	2.07	Q				V
5+ 9	1.6427	2.05	Q				V
5+10	1.6455	2.03	Q				V
5+11	1.6483	2.01	Q				V
5+12	1.6510	1.99	Q				V
5+13	1.6538	1.97	Q				V
5+14	1.6565	1.96	Q				V
5+15	1.6591	1.94	Q				V
5+16	1.6618	1.92	Q				V
5+17	1.6644	1.90	Q				V
5+18	1.6670	1.88	Q				V
5+19	1.6696	1.87	Q				V
5+20	1.6721	1.85	Q				V
5+21	1.6746	1.84	Q				V
5+22	1.6771	1.82	Q				V
5+23	1.6796	1.81	Q				V
5+24	1.6821	1.79	Q				V
5+25	1.6845	1.78	Q				V
5+26	1.6870	1.76	Q				V
5+27	1.6894	1.75	Q				V
5+28	1.6918	1.74	Q				V
5+29	1.6941	1.72	Q				V
5+30	1.6965	1.71	Q				V
5+31	1.6988	1.70	Q				V
5+32	1.7012	1.68	Q				V
5+33	1.7035	1.67	Q				V
5+34	1.7057	1.66	Q				V
5+35	1.7080	1.65	Q				V
5+36	1.7103	1.63	Q				V
5+37	1.7125	1.62	Q				V
5+38	1.7147	1.61	Q				V
5+39	1.7169	1.60	Q				V
5+40	1.7191	1.59	Q				V
5+41	1.7213	1.58	Q				V
5+42	1.7234	1.57	Q				V
5+43	1.7256	1.56	Q				V
5+44	1.7277	1.55	Q				V
5+45	1.7298	1.54	Q				V
5+46	1.7320	1.53	Q				V
5+47	1.7340	1.52	Q				V
5+48	1.7361	1.51	Q				V
5+49	1.7382	1.50	Q				V
5+50	1.7402	1.49	Q				V
5+51	1.7423	1.48	Q				V
5+52	1.7443	1.47	Q				V
5+53	1.7463	1.46	Q				V
5+54	1.7483	1.45	Q				V
5+55	1.7503	1.45	Q				V
5+56	1.7523	1.44	Q				V
5+57	1.7543	1.43	Q				V

5+58	1.7562	1.42	Q				V
5+59	1.7582	1.41	Q				V
6+ 0	1.7601	1.41	Q				V
6+ 1	1.7620	1.40	Q				V
6+ 2	1.7640	1.39	Q				V
6+ 3	1.7659	1.38	Q				V
6+ 4	1.7678	1.38	Q				V
6+ 5	1.7696	1.37	Q				V
6+ 6	1.7715	1.36	Q				V

End of computations, total study area = 13.270 (Ac.)

FLOOD HYDROGRAPH ROUTING PROGRAM
 Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2018
 Study date: 06/21/21

 108-001 Idaho (Escondido Estates)
 Proposed Condition 100-Year Storm
 SSD Starting Peak @ Top of WQ Ponding
 TH

Program License Serial Number 6440

 ***** HYDROGRAPH INFORMATION *****

From study/file name: 108001OnsitePropBMPNR.rte
 *****HYDROGRAPH DATA*****
 Number of intervals = 366
 Time interval = 1.0 (Min.)
 Maximum/Peak flow rate = 36.500 (CFS)
 Total volume = 1.772 (Ac.Ft)
 Status of hydrographs being held in storage

	Stream 1	Stream 2	Stream 3	Stream 4	Stream 5
Peak (CFS)	0.000	0.000	0.000	0.000	0.000
Vol (Ac.Ft)	0.000	0.000	0.000	0.000	0.000

+++++
 Process from Point/Station 403.000 to Point/Station 403.000
 **** RETARDING BASIN ROUTING ****

 User entry of depth-outflow-storage data

Total number of inflow hydrograph intervals = 366
 Hydrograph time unit = 1.000 (Min.)
 Initial depth in storage basin = 3.25(Ft.)

 Initial basin depth = 3.25 (Ft.)
 Initial basin storage = 0.37 (Ac.Ft)
 Initial basin outflow = 0.43 (CFS)

 Depth vs. Storage and Depth vs. Discharge data:

Basin Depth (Ft.)	Storage (Ac.Ft)	Outflow (CFS)	(S-O*dt/2) (Ac.Ft)	(S+O*dt/2) (Ac.Ft)
0.000	0.000	0.000	0.000	0.000
0.125	0.010	0.040	0.010	0.010
0.250	0.020	0.090	0.020	0.020
0.750	0.060	0.190	0.060	0.060
2.250	0.150	0.360	0.150	0.150
2.750	0.260	0.400	0.260	0.260
3.250	0.370	0.430	0.370	0.370
3.750	0.490	2.820	0.488	0.492
4.750	0.750	12.760	0.741	0.759
5.250	0.900	19.390	0.887	0.913

 Hydrograph Detention Basin Routing

Graph values: 'I'= unit inflow; 'O'=outflow at time shown

Time (Hours)	Inflow (CFS)	Outflow (CFS)	Storage (Ac.Ft)	0	9.1	18.25	27.38	36.50	Depth (Ft.)
0.017	0.23	0.43	0.369	0					3.25
0.033	0.46	0.43	0.369	0					3.25
0.050	0.69	0.43	0.369	0					3.25
0.067	0.92	0.43	0.370	0					3.25
0.083	1.15	0.44	0.371	OI					3.25
0.100	1.37	0.47	0.372	OI					3.26
0.117	1.38	0.49	0.373	OI					3.26
0.133	1.38	0.51	0.374	OI					3.27
0.150	1.38	0.54	0.375	OI					3.27
0.167	1.38	0.56	0.377	OI					3.28
0.183	1.39	0.58	0.378	OI					3.28
0.200	1.39	0.60	0.379	OI					3.29
0.217	1.39	0.63	0.380	OI					3.29
0.233	1.40	0.65	0.381	OI					3.30
0.250	1.41	0.67	0.382	OI					3.30
0.267	1.41	0.69	0.383	OI					3.30
0.283	1.42	0.71	0.384	OI					3.31
0.300	1.42	0.73	0.385	OI					3.31
0.317	1.42	0.75	0.386	OI					3.32
0.333	1.43	0.76	0.387	OI					3.32
0.350	1.43	0.78	0.388	OI					3.32
0.367	1.43	0.80	0.389	OI					3.33
0.383	1.43	0.82	0.389	OI					3.33
0.400	1.44	0.83	0.390	OI					3.33
0.417	1.44	0.85	0.391	OI					3.34
0.433	1.45	0.87	0.392	OI					3.34

0.450	1.45	0.88	0.393	OI					3.34
0.467	1.46	0.90	0.393	OI					3.35
0.483	1.47	0.91	0.394	OI					3.35
0.500	1.47	0.93	0.395	OI					3.35
0.517	1.48	0.94	0.396	OI					3.36
0.533	1.48	0.96	0.396	OI					3.36
0.550	1.48	0.97	0.397	OI					3.36
0.567	1.48	0.98	0.398	OI					3.37
0.583	1.49	1.00	0.399	OI					3.37
0.600	1.49	1.01	0.399	OI					3.37
0.617	1.50	1.02	0.400	OI					3.37
0.633	1.50	1.04	0.401	OI					3.38
0.650	1.51	1.05	0.401	OI					3.38
0.667	1.52	1.06	0.402	OI					3.38
0.683	1.52	1.08	0.402	OI					3.38
0.700	1.53	1.09	0.403	OI					3.39
0.717	1.53	1.10	0.404	OI					3.39
0.733	1.53	1.11	0.404	OI					3.39
0.750	1.54	1.12	0.405	OI					3.39
0.767	1.54	1.13	0.405	OI					3.40
0.783	1.54	1.14	0.406	0					3.40
0.800	1.55	1.16	0.406	0					3.40
0.817	1.55	1.17	0.407	0					3.40
0.833	1.56	1.18	0.408	0					3.41
0.850	1.57	1.19	0.408	0					3.41
0.867	1.58	1.20	0.409	0					3.41
0.883	1.58	1.21	0.409	0					3.41
0.900	1.59	1.22	0.410	0					3.41
0.917	1.59	1.23	0.410	0					3.42
0.933	1.60	1.24	0.411	0					3.42
0.950	1.60	1.25	0.411	0					3.42
0.967	1.60	1.26	0.412	0					3.42
0.983	1.61	1.27	0.412	0					3.43
1.000	1.61	1.28	0.412	0					3.43
1.017	1.62	1.29	0.413	0					3.43
1.033	1.63	1.29	0.413	0					3.43
1.050	1.63	1.30	0.414	0					3.43
1.067	1.64	1.31	0.414	0					3.43
1.083	1.65	1.32	0.415	0					3.44
1.100	1.66	1.33	0.415	0					3.44
1.117	1.66	1.34	0.416	0					3.44
1.133	1.67	1.35	0.416	0					3.44
1.150	1.67	1.36	0.417	0					3.44
1.167	1.67	1.37	0.417	0					3.45
1.183	1.68	1.37	0.417	0					3.45
1.200	1.68	1.38	0.418	0					3.45
1.217	1.69	1.39	0.418	0					3.45
1.233	1.70	1.40	0.419	0					3.45
1.250	1.71	1.41	0.419	0					3.45
1.267	1.72	1.42	0.419	0					3.46

1.283	1.73	1.42	0.420	0					3.46
1.300	1.73	1.43	0.420	0					3.46
1.317	1.74	1.44	0.421	0					3.46
1.333	1.74	1.45	0.421	0					3.46
1.350	1.75	1.46	0.422	0					3.46
1.367	1.75	1.46	0.422	0					3.47
1.383	1.76	1.47	0.422	0					3.47
1.400	1.76	1.48	0.423	0					3.47
1.417	1.77	1.49	0.423	0					3.47
1.433	1.78	1.50	0.424	0					3.47
1.450	1.79	1.50	0.424	0					3.47
1.467	1.80	1.51	0.424	0					3.48
1.483	1.81	1.52	0.425	0					3.48
1.500	1.82	1.53	0.425	0					3.48
1.517	1.83	1.54	0.425	0					3.48
1.533	1.83	1.54	0.426	0					3.48
1.550	1.84	1.55	0.426	0					3.48
1.567	1.84	1.56	0.427	0					3.49
1.583	1.85	1.57	0.427	0					3.49
1.600	1.85	1.57	0.427	0					3.49
1.617	1.86	1.58	0.428	0					3.49
1.633	1.87	1.59	0.428	0					3.49
1.650	1.88	1.60	0.429	0					3.49
1.667	1.90	1.61	0.429	0					3.50
1.683	1.91	1.61	0.429	0					3.50
1.700	1.92	1.62	0.430	0					3.50
1.717	1.92	1.63	0.430	0					3.50
1.733	1.93	1.64	0.431	0					3.50
1.750	1.94	1.65	0.431	0					3.50
1.767	1.94	1.65	0.431	0					3.51
1.783	1.95	1.66	0.432	0					3.51
1.800	1.95	1.67	0.432	0					3.51
1.817	1.97	1.68	0.433	0					3.51
1.833	1.98	1.69	0.433	0					3.51
1.850	1.99	1.69	0.433	0					3.51
1.867	2.00	1.70	0.434	0					3.52
1.883	2.02	1.71	0.434	0					3.52
1.900	2.03	1.72	0.435	0					3.52
1.917	2.04	1.73	0.435	0					3.52
1.933	2.04	1.74	0.436	0					3.52
1.950	2.05	1.74	0.436	0					3.52
1.967	2.06	1.75	0.436	0					3.53
1.983	2.06	1.76	0.437	0					3.53
2.000	2.07	1.77	0.437	0					3.53
2.017	2.09	1.78	0.438	0					3.53
2.033	2.10	1.79	0.438	0					3.53
2.050	2.12	1.79	0.439	0					3.54
2.067	2.13	1.80	0.439	0					3.54
2.083	2.15	1.81	0.439	0					3.54
2.100	2.16	1.82	0.440	0					3.54

2.117	2.17	1.83	0.440	0					3.54
2.133	2.18	1.84	0.441	0					3.55
2.150	2.19	1.85	0.441	0					3.55
2.167	2.19	1.86	0.442	0					3.55
2.183	2.20	1.87	0.442	0					3.55
2.200	2.21	1.88	0.443	0					3.55
2.217	2.23	1.89	0.443	0					3.55
2.233	2.25	1.90	0.444	0					3.56
2.250	2.26	1.91	0.444	0					3.56
2.267	2.28	1.92	0.445	0					3.56
2.283	2.30	1.93	0.445	OI					3.56
2.300	2.32	1.94	0.446	OI					3.57
2.317	2.33	1.95	0.446	OI					3.57
2.333	2.34	1.96	0.447	OI					3.57
2.350	2.35	1.97	0.447	OI					3.57
2.367	2.35	1.98	0.448	OI					3.57
2.383	2.36	1.99	0.448	OI					3.58
2.400	2.37	2.00	0.449	OI					3.58
2.417	2.40	2.01	0.449	OI					3.58
2.433	2.42	2.02	0.450	OI					3.58
2.450	2.44	2.03	0.450	OI					3.58
2.467	2.46	2.04	0.451	OI					3.59
2.483	2.48	2.05	0.452	OI					3.59
2.500	2.50	2.07	0.452	OI					3.59
2.517	2.51	2.08	0.453	OI					3.59
2.533	2.53	2.09	0.453	OI					3.60
2.550	2.54	2.10	0.454	OI					3.60
2.567	2.55	2.11	0.455	OI					3.60
2.583	2.56	2.13	0.455	OI					3.60
2.600	2.57	2.14	0.456	OI					3.61
2.617	2.60	2.15	0.456	OI					3.61
2.633	2.63	2.16	0.457	OI					3.61
2.650	2.65	2.17	0.458	OI					3.62
2.667	2.68	2.19	0.458	OI					3.62
2.683	2.71	2.20	0.459	OI					3.62
2.700	2.73	2.22	0.460	OI					3.62
2.717	2.75	2.23	0.460	OI					3.63
2.733	2.76	2.24	0.461	OI					3.63
2.750	2.78	2.26	0.462	OI					3.63
2.767	2.79	2.27	0.463	OI					3.64
2.783	2.81	2.29	0.463	0					3.64
2.800	2.82	2.30	0.464	0					3.64
2.817	2.86	2.32	0.465	0					3.64
2.833	2.89	2.33	0.465	0					3.65
2.850	2.93	2.35	0.466	0					3.65
2.867	2.96	2.36	0.467	0					3.65
2.883	2.99	2.38	0.468	0					3.66
2.900	3.03	2.40	0.469	0					3.66
2.917	3.05	2.41	0.470	0					3.67
2.933	3.07	2.43	0.470	0					3.67

2.950	3.09	2.45	0.471	0					3.67
2.967	3.11	2.47	0.472	0					3.68
2.983	3.12	2.48	0.473	0					3.68
3.000	3.14	2.50	0.474	0					3.68
3.017	3.19	2.52	0.475	0					3.69
3.033	3.24	2.54	0.476	0					3.69
3.050	3.28	2.56	0.477	0					3.70
3.067	3.33	2.58	0.478	0					3.70
3.083	3.37	2.60	0.479	0					3.70
3.100	3.42	2.62	0.480	0					3.71
3.117	3.44	2.64	0.481	OI					3.71
3.133	3.47	2.66	0.482	OI					3.72
3.150	3.50	2.69	0.483	OI					3.72
3.167	3.53	2.71	0.484	OI					3.73
3.183	3.55	2.73	0.486	OI					3.73
3.200	3.58	2.75	0.487	OI					3.74
3.217	3.64	2.78	0.488	OI					3.74
3.233	3.71	2.80	0.489	OI					3.75
3.250	3.77	2.83	0.490	OI					3.75
3.267	3.84	2.88	0.492	OI					3.76
3.283	3.90	2.93	0.493	OI					3.76
3.300	3.97	2.99	0.494	OI					3.77
3.317	4.01	3.04	0.496	OI					3.77
3.333	4.05	3.09	0.497	OI					3.78
3.350	4.09	3.14	0.498	OI					3.78
3.367	4.13	3.19	0.500	OI					3.79
3.383	4.17	3.24	0.501	OI					3.79
3.400	4.21	3.29	0.502	OI					3.80
3.417	4.31	3.34	0.504	OI					3.80
3.433	4.41	3.39	0.505	OI					3.81
3.450	4.52	3.44	0.506	0					3.81
3.467	4.62	3.50	0.508	OI					3.82
3.483	4.72	3.56	0.509	OI					3.82
3.500	4.83	3.62	0.511	OI					3.83
3.517	4.89	3.69	0.513	OI					3.84
3.533	4.96	3.75	0.514	OI					3.84
3.550	5.03	3.81	0.516	OI					3.85
3.567	5.10	3.88	0.518	OI					3.86
3.583	5.16	3.94	0.519	OI					3.86
3.600	5.23	4.01	0.521	OI					3.87
3.617	5.43	4.08	0.523	OI					3.88
3.633	5.62	4.15	0.525	OI					3.88
3.650	5.81	4.23	0.527	0 I					3.89
3.667	6.01	4.32	0.529	0 I					3.90
3.683	6.20	4.41	0.532	0 I					3.91
3.700	6.39	4.50	0.534	0 I					3.92
3.717	6.54	4.61	0.537	OI					3.93
3.733	6.69	4.71	0.539	OI					3.94
3.750	6.84	4.81	0.542	OI					3.95
3.767	6.99	4.92	0.545	0 I					3.96

3.783	7.14	5.03	0.548	0	I							3.97
3.800	7.28	5.14	0.551	0	I							3.98
3.817	7.85	5.27	0.554	0	I							4.00
3.833	8.42	5.42	0.558	0	I							4.01
3.850	8.99	5.58	0.562	0	I							4.03
3.867	9.56	5.77	0.567	0	I							4.05
3.883	10.13	5.98	0.573	0	I							4.07
3.900	10.70	6.21	0.579	0	I							4.09
3.917	11.42	6.46	0.585	0	I							4.12
3.933	12.15	6.73	0.592	0	I							4.14
3.950	12.88	7.03	0.600	0	I							4.17
3.967	13.61	7.35	0.608	0	I							4.21
3.983	14.34	7.69	0.617	0	I							4.24
4.000	15.07	8.05	0.627	0	I							4.28
4.017	18.64	8.50	0.639	0		I						4.32
4.033	22.21	9.11	0.655	0			I					4.38
4.050	25.78	9.88	0.675	0				I				4.46
4.067	29.36	10.78	0.698	0					I			4.55
4.083	32.93	11.83	0.726	0						I		4.66
4.100	36.50	13.04	0.756	0							I	4.77
4.117	31.85	14.29	0.785	0						I		4.87
4.133	27.19	15.19	0.805	0				I				4.93
4.150	22.54	15.76	0.818	0			I					4.98
4.167	17.89	16.02	0.824	0	I							5.00
4.183	13.23	16.00	0.823	I	0							4.99
4.200	8.58	15.69	0.816	I	0							4.97
4.217	8.10	15.26	0.807	I	0							4.94
4.233	7.63	14.82	0.797	I	0							4.91
4.250	7.16	14.38	0.787	I	0							4.87
4.267	6.69	13.94	0.777	I	0							4.84
4.283	6.21	13.50	0.767	I	0							4.81
4.300	5.74	13.06	0.757	I	0							4.77
4.317	5.53	12.64	0.747	I	0							4.74
4.333	5.32	12.27	0.737	I	0							4.70
4.350	5.12	11.90	0.728	I	0							4.66
4.367	4.91	11.55	0.718	I	0							4.63
4.383	4.70	11.20	0.709	I	0							4.59
4.400	4.49	10.87	0.700	I	0							4.56
4.417	4.37	10.54	0.692	I	0							4.53
4.433	4.25	10.22	0.683	I	0							4.49
4.450	4.13	9.91	0.675	I	0							4.46
4.467	4.00	9.61	0.668	I	0							4.43
4.483	3.88	9.32	0.660	I	0							4.40
4.500	3.76	9.03	0.653	I	0							4.38
4.517	3.68	8.76	0.645	I	0							4.35
4.533	3.60	8.50	0.639	I	0							4.32
4.550	3.52	8.25	0.632	I	0							4.30
4.567	3.44	8.00	0.626	I	0							4.27
4.583	3.36	7.76	0.619	I	0							4.25
4.600	3.27	7.54	0.613	I	0							4.22

4.617	3.21	7.32	0.608	I	0					4.20
4.633	3.16	7.10	0.602	I	0					4.18
4.650	3.10	6.90	0.597	I	0					4.16
4.667	3.04	6.70	0.592	I	0					4.14
4.683	2.98	6.51	0.587	I	0					4.12
4.700	2.92	6.33	0.582	I	0					4.10
4.717	2.88	6.15	0.577	I	0					4.09
4.733	2.83	5.99	0.573	I	0					4.07
4.750	2.79	5.82	0.569	I	0					4.05
4.767	2.74	5.67	0.564	I	0					4.04
4.783	2.70	5.51	0.560	I	0					4.02
4.800	2.65	5.37	0.557	I	0					4.01
4.817	2.61	5.23	0.553	I	0					3.99
4.833	2.58	5.09	0.549	I	0					3.98
4.850	2.54	4.96	0.546	I	0					3.97
4.867	2.51	4.84	0.543	I	0					3.95
4.883	2.47	4.72	0.540	I	0					3.94
4.900	2.44	4.60	0.537	I	0					3.93
4.917	2.41	4.49	0.534	I	0					3.92
4.933	2.38	4.38	0.531	I	0					3.91
4.950	2.35	4.28	0.528	I	0					3.90
4.967	2.32	4.18	0.526	I	0					3.89
4.983	2.29	4.08	0.523	I	0					3.88
5.000	2.26	3.99	0.521	I	0					3.87
5.017	2.24	3.90	0.518	I	0					3.86
5.033	2.21	3.81	0.516	I	0					3.85
5.050	2.19	3.73	0.514	I	0					3.84
5.067	2.16	3.65	0.512	I	0					3.83
5.083	2.14	3.58	0.510	I	0					3.83
5.100	2.12	3.50	0.508	I	0					3.82
5.117	2.09	3.43	0.506	I	0					3.81
5.133	2.07	3.36	0.504	I	0					3.80
5.150	2.05	3.29	0.502	I	0					3.80
5.167	2.03	3.23	0.501	I	0					3.79
5.183	2.01	3.17	0.499	I	0					3.78
5.200	1.99	3.11	0.498	I	0					3.78
5.217	1.97	3.05	0.496	I	0					3.77
5.233	1.96	2.99	0.495	I	0					3.77
5.250	1.94	2.94	0.493	I	0					3.76
5.267	1.92	2.89	0.492	I	0					3.76
5.283	1.90	2.84	0.490	I	0					3.75
5.300	1.88	2.80	0.489	I	0					3.75
5.317	1.87	2.78	0.488	I	0					3.74
5.333	1.85	2.75	0.487	I	0					3.74
5.350	1.84	2.73	0.485	I	0					3.73
5.367	1.82	2.71	0.484	I	0					3.73
5.383	1.81	2.68	0.483	I	0					3.72
5.400	1.79	2.66	0.482	I	0					3.72
5.417	1.78	2.63	0.481	I	0					3.71
5.433	1.76	2.61	0.479	I	0					3.71

5.450	1.75	2.59	0.478	IO					3.70
5.467	1.74	2.56	0.477	IO					3.70
5.483	1.72	2.54	0.476	IO					3.69
5.500	1.71	2.52	0.475	IO					3.69
5.517	1.70	2.50	0.474	IO					3.68
5.533	1.68	2.48	0.473	IO					3.68
5.550	1.67	2.45	0.472	IO					3.67
5.567	1.66	2.43	0.471	IO					3.67
5.583	1.65	2.41	0.469	IO					3.66
5.600	1.63	2.39	0.468	IO					3.66
5.617	1.62	2.37	0.467	IO					3.66
5.633	1.61	2.35	0.466	IO					3.65
5.650	1.60	2.33	0.465	IO					3.65
5.667	1.59	2.31	0.464	IO					3.64
5.683	1.58	2.29	0.463	IO					3.64
5.700	1.57	2.27	0.462	O					3.64
5.717	1.56	2.25	0.461	O					3.63
5.733	1.55	2.23	0.460	O					3.63
5.750	1.54	2.21	0.460	O					3.62
5.767	1.53	2.20	0.459	O					3.62
5.783	1.52	2.18	0.458	O					3.62
5.800	1.51	2.16	0.457	O					3.61
5.817	1.50	2.14	0.456	O					3.61
5.833	1.49	2.12	0.455	O					3.60
5.850	1.48	2.11	0.454	O					3.60
5.867	1.47	2.09	0.453	O					3.60
5.883	1.46	2.07	0.452	O					3.59
5.900	1.45	2.06	0.452	O					3.59
5.917	1.45	2.04	0.451	O					3.59
5.933	1.44	2.02	0.450	O					3.58
5.950	1.43	2.01	0.449	O					3.58
5.967	1.42	1.99	0.448	O					3.58
5.983	1.41	1.98	0.448	O					3.57
6.000	1.41	1.96	0.447	O					3.57
6.017	1.40	1.95	0.446	O					3.57
6.033	1.39	1.93	0.445	O					3.56
6.050	1.38	1.92	0.445	O					3.56
6.067	1.38	1.90	0.444	O					3.56
6.083	1.37	1.89	0.443	O					3.55
6.100	1.36	1.87	0.442	O					3.55
6.117	0.00	1.84	0.441	IO					3.55
6.133	0.00	1.79	0.438	IO					3.53
6.150	0.00	1.74	0.436	IO					3.52
6.167	0.00	1.70	0.434	IO					3.51
6.183	0.00	1.65	0.431	IO					3.51
6.200	0.00	1.60	0.429	IO					3.50
6.217	0.00	1.56	0.427	IO					3.49
6.233	0.00	1.52	0.425	IO					3.48
6.250	0.00	1.48	0.423	IO					3.47
6.267	0.00	1.44	0.421	IO					3.46

6.283	0.00	1.40	0.419	IO					3.45
6.300	0.00	1.36	0.417	IO					3.44
6.317	0.00	1.32	0.415	IO					3.44
6.333	0.00	1.29	0.413	IO					3.43
6.350	0.00	1.25	0.411	IO					3.42
6.367	0.00	1.22	0.410	IO					3.42
6.383	0.00	1.19	0.408	IO					3.41
6.400	0.00	1.15	0.406	IO					3.40
6.417	0.00	1.12	0.405	O					3.40
6.433	0.00	1.09	0.403	O					3.39
6.450	0.00	1.06	0.402	O					3.38
6.467	0.00	1.03	0.400	O					3.38
6.483	0.00	1.01	0.399	O					3.37
6.500	0.00	0.98	0.398	O					3.36
6.517	0.00	0.95	0.396	O					3.36
6.533	0.00	0.93	0.395	O					3.35
6.550	0.00	0.90	0.394	O					3.35
6.567	0.00	0.88	0.392	O					3.34
6.583	0.00	0.85	0.391	O					3.34
6.600	0.00	0.83	0.390	O					3.33
6.617	0.00	0.81	0.389	O					3.33
6.633	0.00	0.79	0.388	O					3.32
6.650	0.00	0.77	0.387	O					3.32
6.667	0.00	0.74	0.386	O					3.32
6.683	0.00	0.72	0.385	O					3.31
6.700	0.00	0.70	0.384	O					3.31
6.717	0.00	0.69	0.383	O					3.30
6.733	0.00	0.67	0.382	O					3.30
6.750	0.00	0.65	0.381	O					3.30
6.767	0.00	0.63	0.380	O					3.29
6.783	0.00	0.61	0.379	O					3.29
6.800	0.00	0.60	0.378	O					3.29
6.817	0.00	0.58	0.378	O					3.28
6.833	0.00	0.57	0.377	O					3.28
6.850	0.00	0.55	0.376	O					3.28
6.867	0.00	0.54	0.375	O					3.27
6.883	0.00	0.52	0.375	O					3.27
6.900	0.00	0.51	0.374	O					3.27
6.917	0.00	0.49	0.373	O					3.26
6.933	0.00	0.48	0.373	O					3.26
6.950	0.00	0.47	0.372	O					3.26
6.967	0.00	0.45	0.371	O					3.26
6.983	0.00	0.44	0.371	O					3.25
7.000	0.00	0.43	0.370	O					3.25
7.017	0.00	0.43	0.369	O					3.25
7.033	0.00	0.43	0.369	O					3.24
7.050	0.00	0.43	0.368	O					3.24
7.067	0.00	0.43	0.368	O					3.24
7.083	0.00	0.43	0.367	O					3.24
7.100	0.00	0.43	0.366	O					3.23

7.117	0.00	0.43	0.366	0					3.23
7.133	0.00	0.43	0.365	0					3.23
7.150	0.00	0.43	0.365	0					3.23
7.167	0.00	0.43	0.364	0					3.22
7.183	0.00	0.43	0.364	0					3.22
7.200	0.00	0.43	0.363	0					3.22
7.217	0.00	0.43	0.362	0					3.22
7.233	0.00	0.43	0.362	0					3.21
7.250	0.00	0.43	0.361	0					3.21
7.267	0.00	0.43	0.361	0					3.21
7.283	0.00	0.43	0.360	0					3.20
7.300	0.00	0.43	0.359	0					3.20
7.317	0.00	0.43	0.359	0					3.20
7.333	0.00	0.43	0.358	0					3.20
7.350	0.00	0.43	0.358	0					3.19
7.367	0.00	0.43	0.357	0					3.19
7.383	0.00	0.43	0.356	0					3.19
7.400	0.00	0.43	0.356	0					3.19
7.417	0.00	0.43	0.355	0					3.18
7.433	0.00	0.43	0.355	0					3.18
7.450	0.00	0.43	0.354	0					3.18
7.467	0.00	0.43	0.354	0					3.18
7.483	0.00	0.43	0.353	0					3.17
7.500	0.00	0.43	0.352	0					3.17
7.517	0.00	0.43	0.352	0					3.17
7.533	0.00	0.42	0.351	0					3.16
7.550	0.00	0.42	0.351	0					3.16
7.567	0.00	0.42	0.350	0					3.16
7.583	0.00	0.42	0.349	0					3.16
7.600	0.00	0.42	0.349	0					3.15
7.617	0.00	0.42	0.348	0					3.15
7.633	0.00	0.42	0.348	0					3.15
7.650	0.00	0.42	0.347	0					3.15
7.667	0.00	0.42	0.346	0					3.14
7.683	0.00	0.42	0.346	0					3.14
7.700	0.00	0.42	0.345	0					3.14
7.717	0.00	0.42	0.345	0					3.14
7.733	0.00	0.42	0.344	0					3.13
7.750	0.00	0.42	0.344	0					3.13
7.767	0.00	0.42	0.343	0					3.13
7.783	0.00	0.42	0.342	0					3.12
7.800	0.00	0.42	0.342	0					3.12
7.817	0.00	0.42	0.341	0					3.12
7.833	0.00	0.42	0.341	0					3.12
7.850	0.00	0.42	0.340	0					3.11
7.867	0.00	0.42	0.340	0					3.11
7.883	0.00	0.42	0.339	0					3.11
7.900	0.00	0.42	0.338	0					3.11
7.917	0.00	0.42	0.338	0					3.10
7.933	0.00	0.42	0.337	0					3.10

7.950	0.00	0.42	0.337	0					3.10
7.967	0.00	0.42	0.336	0					3.10
7.983	0.00	0.42	0.335	0					3.09
8.000	0.00	0.42	0.335	0					3.09
8.017	0.00	0.42	0.334	0					3.09
8.033	0.00	0.42	0.334	0					3.09
8.050	0.00	0.42	0.333	0					3.08
8.067	0.00	0.42	0.333	0					3.08
8.083	0.00	0.42	0.332	0					3.08
8.100	0.00	0.42	0.331	0					3.07
8.117	0.00	0.42	0.331	0					3.07
8.133	0.00	0.42	0.330	0					3.07
8.150	0.00	0.42	0.330	0					3.07
8.167	0.00	0.42	0.329	0					3.06
8.183	0.00	0.42	0.329	0					3.06
8.200	0.00	0.42	0.328	0					3.06
8.217	0.00	0.42	0.327	0					3.06
8.233	0.00	0.42	0.327	0					3.05
8.250	0.00	0.42	0.326	0					3.05
8.267	0.00	0.42	0.326	0					3.05
8.283	0.00	0.42	0.325	0					3.05
8.300	0.00	0.42	0.324	0					3.04
8.317	0.00	0.42	0.324	0					3.04
8.333	0.00	0.42	0.323	0					3.04
8.350	0.00	0.42	0.323	0					3.04
8.367	0.00	0.42	0.322	0					3.03
8.383	0.00	0.42	0.322	0					3.03
8.400	0.00	0.42	0.321	0					3.03
8.417	0.00	0.42	0.320	0					3.02
8.433	0.00	0.42	0.320	0					3.02
8.450	0.00	0.42	0.319	0					3.02
8.467	0.00	0.42	0.319	0					3.02
8.483	0.00	0.42	0.318	0					3.01
8.500	0.00	0.42	0.318	0					3.01
8.517	0.00	0.42	0.317	0					3.01
8.533	0.00	0.42	0.316	0					3.01
8.550	0.00	0.42	0.316	0					3.00
8.567	0.00	0.42	0.315	0					3.00
8.583	0.00	0.41	0.315	0					3.00
8.600	0.00	0.41	0.314	0					3.00
8.617	0.00	0.41	0.314	0					2.99
8.633	0.00	0.41	0.313	0					2.99
8.650	0.00	0.41	0.312	0					2.99
8.667	0.00	0.41	0.312	0					2.99
8.683	0.00	0.41	0.311	0					2.98
8.700	0.00	0.41	0.311	0					2.98
8.717	0.00	0.41	0.310	0					2.98
8.733	0.00	0.41	0.310	0					2.98
8.750	0.00	0.41	0.309	0					2.97
8.767	0.00	0.41	0.308	0					2.97

8.783	0.00	0.41	0.308	0					2.97
8.800	0.00	0.41	0.307	0					2.97
8.817	0.00	0.41	0.307	0					2.96
8.833	0.00	0.41	0.306	0					2.96
8.850	0.00	0.41	0.306	0					2.96
8.867	0.00	0.41	0.305	0					2.95
8.883	0.00	0.41	0.304	0					2.95
8.900	0.00	0.41	0.304	0					2.95
8.917	0.00	0.41	0.303	0					2.95
8.933	0.00	0.41	0.303	0					2.94
8.950	0.00	0.41	0.302	0					2.94
8.967	0.00	0.41	0.302	0					2.94
8.983	0.00	0.41	0.301	0					2.94
9.000	0.00	0.41	0.301	0					2.93
9.017	0.00	0.41	0.300	0					2.93
9.033	0.00	0.41	0.299	0					2.93
9.050	0.00	0.41	0.299	0					2.93
9.067	0.00	0.41	0.298	0					2.92
9.083	0.00	0.41	0.298	0					2.92
9.100	0.00	0.41	0.297	0					2.92
9.117	0.00	0.41	0.297	0					2.92
9.133	0.00	0.41	0.296	0					2.91
9.150	0.00	0.41	0.295	0					2.91
9.167	0.00	0.41	0.295	0					2.91
9.183	0.00	0.41	0.294	0					2.91
9.200	0.00	0.41	0.294	0					2.90
9.217	0.00	0.41	0.293	0					2.90
9.233	0.00	0.41	0.293	0					2.90
9.250	0.00	0.41	0.292	0					2.90
9.267	0.00	0.41	0.291	0					2.89
9.283	0.00	0.41	0.291	0					2.89
9.300	0.00	0.41	0.290	0					2.89
9.317	0.00	0.41	0.290	0					2.89
9.333	0.00	0.41	0.289	0					2.88
9.350	0.00	0.41	0.289	0					2.88
9.367	0.00	0.41	0.288	0					2.88
9.383	0.00	0.41	0.288	0					2.88
9.400	0.00	0.41	0.287	0					2.87
9.417	0.00	0.41	0.286	0					2.87
9.433	0.00	0.41	0.286	0					2.87
9.450	0.00	0.41	0.285	0					2.86
9.467	0.00	0.41	0.285	0					2.86
9.483	0.00	0.41	0.284	0					2.86
9.500	0.00	0.41	0.284	0					2.86
9.517	0.00	0.41	0.283	0					2.85
9.533	0.00	0.41	0.282	0					2.85
9.550	0.00	0.41	0.282	0					2.85
9.567	0.00	0.41	0.281	0					2.85
9.583	0.00	0.41	0.281	0					2.84
9.600	0.00	0.41	0.280	0					2.84

9.617	0.00	0.41	0.280	0					2.84
9.633	0.00	0.41	0.279	0					2.84
9.650	0.00	0.41	0.279	0					2.83
9.667	0.00	0.40	0.278	0					2.83
9.683	0.00	0.40	0.277	0					2.83
9.700	0.00	0.40	0.277	0					2.83
9.717	0.00	0.40	0.276	0					2.82
9.733	0.00	0.40	0.276	0					2.82
9.750	0.00	0.40	0.275	0					2.82
9.767	0.00	0.40	0.275	0					2.82
9.783	0.00	0.40	0.274	0					2.81
9.800	0.00	0.40	0.274	0					2.81
9.817	0.00	0.40	0.273	0					2.81
9.833	0.00	0.40	0.272	0					2.81
9.850	0.00	0.40	0.272	0					2.80
9.867	0.00	0.40	0.271	0					2.80
9.883	0.00	0.40	0.271	0					2.80
9.900	0.00	0.40	0.270	0					2.80
9.917	0.00	0.40	0.270	0					2.79
9.933	0.00	0.40	0.269	0					2.79
9.950	0.00	0.40	0.269	0					2.79
9.967	0.00	0.40	0.268	0					2.79
9.983	0.00	0.40	0.267	0					2.78
10.000	0.00	0.40	0.267	0					2.78
10.017	0.00	0.40	0.266	0					2.78
10.033	0.00	0.40	0.266	0					2.78
10.050	0.00	0.40	0.265	0					2.77
10.067	0.00	0.40	0.265	0					2.77
10.083	0.00	0.40	0.264	0					2.77
10.100	0.00	0.40	0.264	0					2.77
10.117	0.00	0.40	0.263	0					2.76
10.133	0.00	0.40	0.262	0					2.76
10.150	0.00	0.40	0.262	0					2.76
10.167	0.00	0.40	0.261	0					2.76
10.183	0.00	0.40	0.261	0					2.75
10.200	0.00	0.40	0.260	0					2.75
10.217	0.00	0.40	0.260	0					2.75
10.233	0.00	0.40	0.259	0					2.75
10.250	0.00	0.40	0.259	0					2.74
10.267	0.00	0.40	0.258	0					2.74
10.283	0.00	0.40	0.258	0					2.74
10.300	0.00	0.40	0.257	0					2.74
10.317	0.00	0.40	0.256	0					2.73
10.333	0.00	0.40	0.256	0					2.73
10.350	0.00	0.40	0.255	0					2.73
10.367	0.00	0.40	0.255	0					2.73
10.383	0.00	0.40	0.254	0					2.72
10.400	0.00	0.40	0.254	0					2.72
10.417	0.00	0.40	0.253	0					2.72
10.433	0.00	0.40	0.253	0					2.72

10.450	0.00	0.40	0.252	0					2.71
10.467	0.00	0.40	0.252	0					2.71
10.483	0.00	0.40	0.251	0					2.71
10.500	0.00	0.40	0.250	0					2.71
10.517	0.00	0.40	0.250	0					2.70
10.533	0.00	0.40	0.249	0					2.70
10.550	0.00	0.40	0.249	0					2.70
10.567	0.00	0.40	0.248	0					2.70
10.583	0.00	0.40	0.248	0					2.69
10.600	0.00	0.40	0.247	0					2.69
10.617	0.00	0.40	0.247	0					2.69
10.633	0.00	0.39	0.246	0					2.69
10.650	0.00	0.39	0.246	0					2.68
10.667	0.00	0.39	0.245	0					2.68
10.683	0.00	0.39	0.244	0					2.68
10.700	0.00	0.39	0.244	0					2.68
10.717	0.00	0.39	0.243	0					2.67
10.733	0.00	0.39	0.243	0					2.67
10.750	0.00	0.39	0.242	0					2.67
10.767	0.00	0.39	0.242	0					2.67
10.783	0.00	0.39	0.241	0					2.66
10.800	0.00	0.39	0.241	0					2.66
10.817	0.00	0.39	0.240	0					2.66
10.833	0.00	0.39	0.240	0					2.66
10.850	0.00	0.39	0.239	0					2.65
10.867	0.00	0.39	0.238	0					2.65
10.883	0.00	0.39	0.238	0					2.65
10.900	0.00	0.39	0.237	0					2.65
10.917	0.00	0.39	0.237	0					2.64
10.933	0.00	0.39	0.236	0					2.64
10.950	0.00	0.39	0.236	0					2.64
10.967	0.00	0.39	0.235	0					2.64
10.983	0.00	0.39	0.235	0					2.63
11.000	0.00	0.39	0.234	0					2.63
11.017	0.00	0.39	0.234	0					2.63
11.033	0.00	0.39	0.233	0					2.63
11.050	0.00	0.39	0.233	0					2.63
11.067	0.00	0.39	0.232	0					2.62
11.083	0.00	0.39	0.231	0					2.62
11.100	0.00	0.39	0.231	0					2.62
11.117	0.00	0.39	0.230	0					2.62
11.133	0.00	0.39	0.230	0					2.61
11.150	0.00	0.39	0.229	0					2.61
11.167	0.00	0.39	0.229	0					2.61
11.183	0.00	0.39	0.228	0					2.61
11.200	0.00	0.39	0.228	0					2.60
11.217	0.00	0.39	0.227	0					2.60
11.233	0.00	0.39	0.227	0					2.60
11.250	0.00	0.39	0.226	0					2.60
11.267	0.00	0.39	0.226	0					2.59

11.283	0.00	0.39	0.225	0					2.59
11.300	0.00	0.39	0.225	0					2.59
11.317	0.00	0.39	0.224	0					2.59
11.333	0.00	0.39	0.223	0					2.58
11.350	0.00	0.39	0.223	0					2.58
11.367	0.00	0.39	0.222	0					2.58
11.383	0.00	0.39	0.222	0					2.58
11.400	0.00	0.39	0.221	0					2.57
11.417	0.00	0.39	0.221	0					2.57
11.433	0.00	0.39	0.220	0					2.57
11.450	0.00	0.39	0.220	0					2.57
11.467	0.00	0.39	0.219	0					2.56
11.483	0.00	0.38	0.219	0					2.56
11.500	0.00	0.38	0.218	0					2.56
11.517	0.00	0.38	0.218	0					2.56
11.533	0.00	0.38	0.217	0					2.55
11.550	0.00	0.38	0.217	0					2.55
11.567	0.00	0.38	0.216	0					2.55
11.583	0.00	0.38	0.215	0					2.55
11.600	0.00	0.38	0.215	0					2.55
11.617	0.00	0.38	0.214	0					2.54
11.633	0.00	0.38	0.214	0					2.54
11.650	0.00	0.38	0.213	0					2.54
11.667	0.00	0.38	0.213	0					2.54
11.683	0.00	0.38	0.212	0					2.53
11.700	0.00	0.38	0.212	0					2.53
11.717	0.00	0.38	0.211	0					2.53
11.733	0.00	0.38	0.211	0					2.53
11.750	0.00	0.38	0.210	0					2.52
11.767	0.00	0.38	0.210	0					2.52
11.783	0.00	0.38	0.209	0					2.52
11.800	0.00	0.38	0.209	0					2.52
11.817	0.00	0.38	0.208	0					2.51
11.833	0.00	0.38	0.208	0					2.51
11.850	0.00	0.38	0.207	0					2.51
11.867	0.00	0.38	0.207	0					2.51
11.883	0.00	0.38	0.206	0					2.50
11.900	0.00	0.38	0.205	0					2.50
11.917	0.00	0.38	0.205	0					2.50
11.933	0.00	0.38	0.204	0					2.50
11.950	0.00	0.38	0.204	0					2.50
11.967	0.00	0.38	0.203	0					2.49
11.983	0.00	0.38	0.203	0					2.49
12.000	0.00	0.38	0.202	0					2.49
12.017	0.00	0.38	0.202	0					2.49
12.033	0.00	0.38	0.201	0					2.48
12.050	0.00	0.38	0.201	0					2.48
12.067	0.00	0.38	0.200	0					2.48
12.083	0.00	0.38	0.200	0					2.48
12.100	0.00	0.38	0.199	0					2.47

12.117	0.00	0.38	0.199	0					2.47
12.133	0.00	0.38	0.198	0					2.47
12.150	0.00	0.38	0.198	0					2.47
12.167	0.00	0.38	0.197	0					2.46
12.183	0.00	0.38	0.197	0					2.46
12.200	0.00	0.38	0.196	0					2.46
12.217	0.00	0.38	0.196	0					2.46
12.233	0.00	0.38	0.195	0					2.45
12.250	0.00	0.38	0.195	0					2.45
12.267	0.00	0.38	0.194	0					2.45
12.283	0.00	0.38	0.194	0					2.45
12.300	0.00	0.38	0.193	0					2.45
12.317	0.00	0.38	0.192	0					2.44
12.333	0.00	0.38	0.192	0					2.44
12.350	0.00	0.38	0.191	0					2.44
12.367	0.00	0.37	0.191	0					2.44
12.383	0.00	0.37	0.190	0					2.43
12.400	0.00	0.37	0.190	0					2.43
12.417	0.00	0.37	0.189	0					2.43
12.433	0.00	0.37	0.189	0					2.43
12.450	0.00	0.37	0.188	0					2.42
12.467	0.00	0.37	0.188	0					2.42
12.483	0.00	0.37	0.187	0					2.42
12.500	0.00	0.37	0.187	0					2.42
12.517	0.00	0.37	0.186	0					2.41
12.533	0.00	0.37	0.186	0					2.41
12.550	0.00	0.37	0.185	0					2.41
12.567	0.00	0.37	0.185	0					2.41
12.583	0.00	0.37	0.184	0					2.41
12.600	0.00	0.37	0.184	0					2.40
12.617	0.00	0.37	0.183	0					2.40
12.633	0.00	0.37	0.183	0					2.40
12.650	0.00	0.37	0.182	0					2.40
12.667	0.00	0.37	0.182	0					2.39
12.683	0.00	0.37	0.181	0					2.39
12.700	0.00	0.37	0.181	0					2.39
12.717	0.00	0.37	0.180	0					2.39
12.733	0.00	0.37	0.180	0					2.38
12.750	0.00	0.37	0.179	0					2.38
12.767	0.00	0.37	0.179	0					2.38
12.783	0.00	0.37	0.178	0					2.38
12.800	0.00	0.37	0.178	0					2.38
12.817	0.00	0.37	0.177	0					2.37
12.833	0.00	0.37	0.177	0					2.37
12.850	0.00	0.37	0.176	0					2.37
12.867	0.00	0.37	0.176	0					2.37
12.883	0.00	0.37	0.175	0					2.36
12.900	0.00	0.37	0.175	0					2.36
12.917	0.00	0.37	0.174	0					2.36
12.933	0.00	0.37	0.174	0					2.36

12.950	0.00	0.37	0.173	0					2.35
12.967	0.00	0.37	0.173	0					2.35
12.983	0.00	0.37	0.172	0					2.35
13.000	0.00	0.37	0.171	0					2.35
13.017	0.00	0.37	0.171	0					2.35
13.033	0.00	0.37	0.170	0					2.34
13.050	0.00	0.37	0.170	0					2.34
13.067	0.00	0.37	0.169	0					2.34
13.083	0.00	0.37	0.169	0					2.34
13.100	0.00	0.37	0.168	0					2.33
13.117	0.00	0.37	0.168	0					2.33
13.133	0.00	0.37	0.167	0					2.33
13.150	0.00	0.37	0.167	0					2.33
13.167	0.00	0.37	0.166	0					2.32
13.183	0.00	0.37	0.166	0					2.32
13.200	0.00	0.37	0.165	0					2.32
13.217	0.00	0.37	0.165	0					2.32
13.233	0.00	0.37	0.164	0					2.32
13.250	0.00	0.37	0.164	0					2.31
13.267	0.00	0.36	0.163	0					2.31
13.283	0.00	0.36	0.163	0					2.31
13.300	0.00	0.36	0.162	0					2.31
13.317	0.00	0.36	0.162	0					2.30
13.333	0.00	0.36	0.161	0					2.30
13.350	0.00	0.36	0.161	0					2.30
13.367	0.00	0.36	0.160	0					2.30
13.383	0.00	0.36	0.160	0					2.30
13.400	0.00	0.36	0.159	0					2.29
13.417	0.00	0.36	0.159	0					2.29
13.433	0.00	0.36	0.158	0					2.29
13.450	0.00	0.36	0.158	0					2.29
13.467	0.00	0.36	0.157	0					2.28
13.483	0.00	0.36	0.157	0					2.28
13.500	0.00	0.36	0.156	0					2.28
13.517	0.00	0.36	0.156	0					2.28
13.533	0.00	0.36	0.155	0					2.27
13.550	0.00	0.36	0.155	0					2.27
13.567	0.00	0.36	0.154	0					2.27
13.583	0.00	0.36	0.154	0					2.27
13.600	0.00	0.36	0.153	0					2.27
13.617	0.00	0.36	0.153	0					2.26
13.633	0.00	0.36	0.152	0					2.26
13.650	0.00	0.36	0.152	0					2.26
13.667	0.00	0.36	0.151	0					2.26
13.683	0.00	0.36	0.151	0					2.25
13.700	0.00	0.36	0.150	0					2.25
13.717	0.00	0.36	0.150	0					2.25
13.733	0.00	0.36	0.149	0					2.24
13.750	0.00	0.36	0.149	0					2.23
13.767	0.00	0.36	0.148	0					2.22

13.783	0.00	0.36	0.148	0					2.22
13.800	0.00	0.36	0.147	0					2.21
13.817	0.00	0.35	0.147	0					2.20
13.833	0.00	0.35	0.147	0					2.19
13.850	0.00	0.35	0.146	0					2.18
13.867	0.00	0.35	0.146	0					2.18
13.883	0.00	0.35	0.145	0					2.17
13.900	0.00	0.35	0.145	0					2.16
13.917	0.00	0.35	0.144	0					2.15
13.933	0.00	0.35	0.144	0					2.14
13.950	0.00	0.35	0.143	0					2.14
13.967	0.00	0.35	0.143	0					2.13
13.983	0.00	0.35	0.142	0					2.12
14.000	0.00	0.34	0.142	0					2.11
14.017	0.00	0.34	0.141	0					2.10
14.033	0.00	0.34	0.141	0					2.10
14.050	0.00	0.34	0.140	0					2.09
14.067	0.00	0.34	0.140	0					2.08
14.083	0.00	0.34	0.139	0					2.07
14.100	0.00	0.34	0.139	0					2.06
14.117	0.00	0.34	0.138	0					2.06
14.133	0.00	0.34	0.138	0					2.05
14.150	0.00	0.34	0.137	0					2.04
14.167	0.00	0.34	0.137	0					2.03
14.183	0.00	0.33	0.137	0					2.03
14.200	0.00	0.33	0.136	0					2.02
14.217	0.00	0.33	0.136	0					2.01
14.233	0.00	0.33	0.135	0					2.00
14.250	0.00	0.33	0.135	0					2.00
14.267	0.00	0.33	0.134	0					1.99
14.283	0.00	0.33	0.134	0					1.98
14.300	0.00	0.33	0.133	0					1.97
14.317	0.00	0.33	0.133	0					1.97
14.333	0.00	0.33	0.132	0					1.96
14.350	0.00	0.33	0.132	0					1.95
14.367	0.00	0.33	0.132	0					1.94
14.383	0.00	0.32	0.131	0					1.94
14.400	0.00	0.32	0.131	0					1.93
14.417	0.00	0.32	0.130	0					1.92
14.433	0.00	0.32	0.130	0					1.91
14.450	0.00	0.32	0.129	0					1.91
14.467	0.00	0.32	0.129	0					1.90
14.483	0.00	0.32	0.128	0					1.89
14.500	0.00	0.32	0.128	0					1.88
14.517	0.00	0.32	0.128	0					1.88
14.533	0.00	0.32	0.127	0					1.87
14.550	0.00	0.32	0.127	0					1.86
14.567	0.00	0.32	0.126	0					1.85
14.583	0.00	0.31	0.126	0					1.85
14.600	0.00	0.31	0.125	0					1.84

14.617	0.00	0.31	0.125	0					1.83
14.633	0.00	0.31	0.125	0					1.83
14.650	0.00	0.31	0.124	0					1.82
14.667	0.00	0.31	0.124	0					1.81
14.683	0.00	0.31	0.123	0					1.80
14.700	0.00	0.31	0.123	0					1.80
14.717	0.00	0.31	0.122	0					1.79
14.733	0.00	0.31	0.122	0					1.78
14.750	0.00	0.31	0.122	0					1.78
14.767	0.00	0.31	0.121	0					1.77
14.783	0.00	0.30	0.121	0					1.76
14.800	0.00	0.30	0.120	0					1.75
14.817	0.00	0.30	0.120	0					1.75
14.833	0.00	0.30	0.119	0					1.74
14.850	0.00	0.30	0.119	0					1.73
14.867	0.00	0.30	0.119	0					1.73
14.883	0.00	0.30	0.118	0					1.72
14.900	0.00	0.30	0.118	0					1.71
14.917	0.00	0.30	0.117	0					1.71
14.933	0.00	0.30	0.117	0					1.70
14.950	0.00	0.30	0.117	0					1.69
14.967	0.00	0.30	0.116	0					1.69
14.983	0.00	0.30	0.116	0					1.68
15.000	0.00	0.29	0.115	0					1.67
15.017	0.00	0.29	0.115	0					1.67
15.033	0.00	0.29	0.115	0					1.66
15.050	0.00	0.29	0.114	0					1.65
15.067	0.00	0.29	0.114	0					1.65
15.083	0.00	0.29	0.113	0					1.64
15.100	0.00	0.29	0.113	0					1.63
15.117	0.00	0.29	0.113	0					1.63
15.133	0.00	0.29	0.112	0					1.62
15.150	0.00	0.29	0.112	0					1.61
15.167	0.00	0.29	0.111	0					1.61
15.183	0.00	0.29	0.111	0					1.60
15.200	0.00	0.29	0.111	0					1.59
15.217	0.00	0.28	0.110	0					1.59
15.233	0.00	0.28	0.110	0					1.58
15.250	0.00	0.28	0.109	0					1.57
15.267	0.00	0.28	0.109	0					1.57
15.283	0.00	0.28	0.109	0					1.56
15.300	0.00	0.28	0.108	0					1.55
15.317	0.00	0.28	0.108	0					1.55
15.333	0.00	0.28	0.107	0					1.54
15.350	0.00	0.28	0.107	0					1.53
15.367	0.00	0.28	0.107	0					1.53
15.383	0.00	0.28	0.106	0					1.52
15.400	0.00	0.28	0.106	0					1.52
15.417	0.00	0.28	0.106	0					1.51
15.433	0.00	0.28	0.105	0					1.50

15.450	0.00	0.27	0.105	0					1.50
15.467	0.00	0.27	0.104	0					1.49
15.483	0.00	0.27	0.104	0					1.48
15.500	0.00	0.27	0.104	0					1.48
15.517	0.00	0.27	0.103	0					1.47
15.533	0.00	0.27	0.103	0					1.46
15.550	0.00	0.27	0.103	0					1.46
15.567	0.00	0.27	0.102	0					1.45
15.583	0.00	0.27	0.102	0					1.45
15.600	0.00	0.27	0.101	0					1.44
15.617	0.00	0.27	0.101	0					1.43
15.633	0.00	0.27	0.101	0					1.43
15.650	0.00	0.27	0.100	0					1.42
15.667	0.00	0.27	0.100	0					1.42
15.683	0.00	0.26	0.100	0					1.41
15.700	0.00	0.26	0.099	0					1.40
15.717	0.00	0.26	0.099	0					1.40
15.733	0.00	0.26	0.098	0					1.39
15.750	0.00	0.26	0.098	0					1.39
15.767	0.00	0.26	0.098	0					1.38
15.783	0.00	0.26	0.097	0					1.37
15.800	0.00	0.26	0.097	0					1.37
15.817	0.00	0.26	0.097	0					1.36
15.833	0.00	0.26	0.096	0					1.36
15.850	0.00	0.26	0.096	0					1.35
15.867	0.00	0.26	0.096	0					1.34
15.883	0.00	0.26	0.095	0					1.34
15.900	0.00	0.26	0.095	0					1.33
15.917	0.00	0.26	0.095	0					1.33
15.933	0.00	0.25	0.094	0					1.32
15.950	0.00	0.25	0.094	0					1.31
15.967	0.00	0.25	0.094	0					1.31
15.983	0.00	0.25	0.093	0					1.30
16.000	0.00	0.25	0.093	0					1.30
16.017	0.00	0.25	0.092	0					1.29
16.033	0.00	0.25	0.092	0					1.29
16.050	0.00	0.25	0.092	0					1.28
16.067	0.00	0.25	0.091	0					1.27
16.083	0.00	0.25	0.091	0					1.27
16.100	0.00	0.25	0.091	0					1.26
16.117	0.00	0.25	0.090	0					1.26
16.133	0.00	0.25	0.090	0					1.25
16.150	0.00	0.25	0.090	0					1.25
16.167	0.00	0.25	0.089	0					1.24
16.183	0.00	0.24	0.089	0					1.23
16.200	0.00	0.24	0.089	0					1.23
16.217	0.00	0.24	0.088	0					1.22
16.233	0.00	0.24	0.088	0					1.22
16.250	0.00	0.24	0.088	0					1.21
16.267	0.00	0.24	0.087	0					1.21

16.283	0.00	0.24	0.087	0					1.20
16.300	0.00	0.24	0.087	0					1.20
16.317	0.00	0.24	0.086	0					1.19
16.333	0.00	0.24	0.086	0					1.18
16.350	0.00	0.24	0.086	0					1.18
16.367	0.00	0.24	0.085	0					1.17
16.383	0.00	0.24	0.085	0					1.17
16.400	0.00	0.24	0.085	0					1.16
16.417	0.00	0.24	0.084	0					1.16
16.433	0.00	0.24	0.084	0					1.15
16.450	0.00	0.23	0.084	0					1.15
16.467	0.00	0.23	0.083	0					1.14
16.483	0.00	0.23	0.083	0					1.14
16.500	0.00	0.23	0.083	0					1.13
16.517	0.00	0.23	0.082	0					1.12
16.533	0.00	0.23	0.082	0					1.12
16.550	0.00	0.23	0.082	0					1.11
16.567	0.00	0.23	0.082	0					1.11
16.583	0.00	0.23	0.081	0					1.10
16.600	0.00	0.23	0.081	0					1.10
16.617	0.00	0.23	0.081	0					1.09
16.633	0.00	0.23	0.080	0					1.09
16.650	0.00	0.23	0.080	0					1.08
16.667	0.00	0.23	0.080	0					1.08
16.683	0.00	0.23	0.079	0					1.07
16.700	0.00	0.23	0.079	0					1.07
16.717	0.00	0.23	0.079	0					1.06
16.733	0.00	0.22	0.078	0					1.06
16.750	0.00	0.22	0.078	0					1.05
16.767	0.00	0.22	0.078	0					1.05
16.783	0.00	0.22	0.077	0					1.04
16.800	0.00	0.22	0.077	0					1.04
16.817	0.00	0.22	0.077	0					1.03
16.833	0.00	0.22	0.077	0					1.03
16.850	0.00	0.22	0.076	0					1.02
16.867	0.00	0.22	0.076	0					1.02
16.883	0.00	0.22	0.076	0					1.01
16.900	0.00	0.22	0.075	0					1.01
16.917	0.00	0.22	0.075	0					1.00
16.933	0.00	0.22	0.075	0					1.00
16.950	0.00	0.22	0.074	0					0.99
16.967	0.00	0.22	0.074	0					0.99
16.983	0.00	0.22	0.074	0					0.98
17.000	0.00	0.22	0.074	0					0.98
17.017	0.00	0.21	0.073	0					0.97
17.033	0.00	0.21	0.073	0					0.97
17.050	0.00	0.21	0.073	0					0.96
17.067	0.00	0.21	0.072	0					0.96
17.083	0.00	0.21	0.072	0					0.95
17.100	0.00	0.21	0.072	0					0.95

17.117	0.00	0.21	0.071	0				0.94
17.133	0.00	0.21	0.071	0				0.94
17.150	0.00	0.21	0.071	0				0.93
17.167	0.00	0.21	0.071	0				0.93
17.183	0.00	0.21	0.070	0				0.92
17.200	0.00	0.21	0.070	0				0.92
17.217	0.00	0.21	0.070	0				0.91
17.233	0.00	0.21	0.069	0				0.91
17.250	0.00	0.21	0.069	0				0.90
17.267	0.00	0.21	0.069	0				0.90
17.283	0.00	0.21	0.069	0				0.89
17.300	0.00	0.21	0.068	0				0.89
17.317	0.00	0.21	0.068	0				0.88
17.333	0.00	0.20	0.068	0				0.88
17.350	0.00	0.20	0.067	0				0.87
17.367	0.00	0.20	0.067	0				0.87
17.383	0.00	0.20	0.067	0				0.87
17.400	0.00	0.20	0.067	0				0.86
17.417	0.00	0.20	0.066	0				0.86
17.433	0.00	0.20	0.066	0				0.85
17.450	0.00	0.20	0.066	0				0.85
17.467	0.00	0.20	0.066	0				0.84
17.483	0.00	0.20	0.065	0				0.84
17.500	0.00	0.20	0.065	0				0.83
17.517	0.00	0.20	0.065	0				0.83
17.533	0.00	0.20	0.064	0				0.82
17.550	0.00	0.20	0.064	0				0.82
17.567	0.00	0.20	0.064	0				0.81
17.583	0.00	0.20	0.064	0				0.81
17.600	0.00	0.20	0.063	0				0.81
17.617	0.00	0.20	0.063	0				0.80
17.633	0.00	0.20	0.063	0				0.80
17.650	0.00	0.19	0.063	0				0.79
17.667	0.00	0.19	0.062	0				0.79
17.683	0.00	0.19	0.062	0				0.78
17.700	0.00	0.19	0.062	0				0.78
17.717	0.00	0.19	0.061	0				0.77
17.733	0.00	0.19	0.061	0				0.77
17.750	0.00	0.19	0.061	0				0.77
17.767	0.00	0.19	0.061	0				0.76
17.783	0.00	0.19	0.060	0				0.76
17.800	0.00	0.19	0.060	0				0.75
17.817	0.00	0.19	0.060	0				0.75
17.833	0.00	0.19	0.060	0				0.75
17.850	0.00	0.19	0.059	0				0.74
17.867	0.00	0.19	0.059	0				0.74
17.883	0.00	0.19	0.059	0				0.74
17.900	0.00	0.19	0.059	0				0.73
17.917	0.00	0.19	0.058	0				0.73
17.933	0.00	0.19	0.058	0				0.73

17.950	0.00	0.18	0.058	0					0.72
17.967	0.00	0.18	0.058	0					0.72
17.983	0.00	0.18	0.057	0					0.72
18.000	0.00	0.18	0.057	0					0.71
18.017	0.00	0.18	0.057	0					0.71
18.033	0.00	0.18	0.057	0					0.71
18.050	0.00	0.18	0.056	0					0.70
18.067	0.00	0.18	0.056	0					0.70
18.083	0.00	0.18	0.056	0					0.70
18.100	0.00	0.18	0.056	0					0.69
18.117	0.00	0.18	0.055	0					0.69
18.133	0.00	0.18	0.055	0					0.69
18.150	0.00	0.18	0.055	0					0.69
18.167	0.00	0.18	0.055	0					0.68
18.183	0.00	0.18	0.054	0					0.68
18.200	0.00	0.18	0.054	0					0.68
18.217	0.00	0.17	0.054	0					0.67
18.233	0.00	0.17	0.054	0					0.67
18.250	0.00	0.17	0.053	0					0.67
18.267	0.00	0.17	0.053	0					0.66
18.283	0.00	0.17	0.053	0					0.66
18.300	0.00	0.17	0.053	0					0.66
18.317	0.00	0.17	0.052	0					0.66
18.333	0.00	0.17	0.052	0					0.65
18.350	0.00	0.17	0.052	0					0.65
18.367	0.00	0.17	0.052	0					0.65
18.383	0.00	0.17	0.051	0					0.64
18.400	0.00	0.17	0.051	0					0.64
18.417	0.00	0.17	0.051	0					0.64
18.433	0.00	0.17	0.051	0					0.63
18.450	0.00	0.17	0.051	0					0.63
18.467	0.00	0.17	0.050	0					0.63
18.483	0.00	0.17	0.050	0					0.63
18.500	0.00	0.16	0.050	0					0.62
18.517	0.00	0.16	0.050	0					0.62
18.533	0.00	0.16	0.049	0					0.62
18.550	0.00	0.16	0.049	0					0.62
18.567	0.00	0.16	0.049	0					0.61
18.583	0.00	0.16	0.049	0					0.61
18.600	0.00	0.16	0.049	0					0.61
18.617	0.00	0.16	0.048	0					0.60
18.633	0.00	0.16	0.048	0					0.60
18.650	0.00	0.16	0.048	0					0.60
18.667	0.00	0.16	0.048	0					0.60
18.683	0.00	0.16	0.047	0					0.59
18.700	0.00	0.16	0.047	0					0.59
18.717	0.00	0.16	0.047	0					0.59
18.733	0.00	0.16	0.047	0					0.58
18.750	0.00	0.16	0.047	0					0.58
18.767	0.00	0.16	0.046	0					0.58

18.783	0.00	0.16	0.046	0					0.58
18.800	0.00	0.15	0.046	0					0.57
18.817	0.00	0.15	0.046	0					0.57
18.833	0.00	0.15	0.045	0					0.57
18.850	0.00	0.15	0.045	0					0.57
18.867	0.00	0.15	0.045	0					0.56
18.883	0.00	0.15	0.045	0					0.56
18.900	0.00	0.15	0.045	0					0.56
18.917	0.00	0.15	0.044	0					0.56
18.933	0.00	0.15	0.044	0					0.55
18.950	0.00	0.15	0.044	0					0.55
18.967	0.00	0.15	0.044	0					0.55
18.983	0.00	0.15	0.044	0					0.55
19.000	0.00	0.15	0.043	0					0.54
19.017	0.00	0.15	0.043	0					0.54
19.033	0.00	0.15	0.043	0					0.54
19.050	0.00	0.15	0.043	0					0.54
19.067	0.00	0.15	0.043	0					0.53
19.083	0.00	0.15	0.042	0					0.53
19.100	0.00	0.15	0.042	0					0.53
19.117	0.00	0.15	0.042	0					0.53
19.133	0.00	0.14	0.042	0					0.52
19.150	0.00	0.14	0.042	0					0.52
19.167	0.00	0.14	0.041	0					0.52
19.183	0.00	0.14	0.041	0					0.52
19.200	0.00	0.14	0.041	0					0.51
19.217	0.00	0.14	0.041	0					0.51
19.233	0.00	0.14	0.041	0					0.51
19.250	0.00	0.14	0.040	0					0.51
19.267	0.00	0.14	0.040	0					0.50
19.283	0.00	0.14	0.040	0					0.50
19.300	0.00	0.14	0.040	0					0.50
19.317	0.00	0.14	0.040	0					0.50
19.333	0.00	0.14	0.039	0					0.49
19.350	0.00	0.14	0.039	0					0.49
19.367	0.00	0.14	0.039	0					0.49
19.383	0.00	0.14	0.039	0					0.49
19.400	0.00	0.14	0.039	0					0.48
19.417	0.00	0.14	0.039	0					0.48
19.433	0.00	0.14	0.038	0					0.48
19.450	0.00	0.14	0.038	0					0.48
19.467	0.00	0.13	0.038	0					0.47
19.483	0.00	0.13	0.038	0					0.47
19.500	0.00	0.13	0.038	0					0.47
19.517	0.00	0.13	0.037	0					0.47
19.533	0.00	0.13	0.037	0					0.47
19.550	0.00	0.13	0.037	0					0.46
19.567	0.00	0.13	0.037	0					0.46
19.583	0.00	0.13	0.037	0					0.46
19.600	0.00	0.13	0.036	0					0.46

19.617	0.00	0.13	0.036	0					0.45
19.633	0.00	0.13	0.036	0					0.45
19.650	0.00	0.13	0.036	0					0.45
19.667	0.00	0.13	0.036	0					0.45
19.683	0.00	0.13	0.036	0					0.44
19.700	0.00	0.13	0.035	0					0.44
19.717	0.00	0.13	0.035	0					0.44
19.733	0.00	0.13	0.035	0					0.44
19.750	0.00	0.13	0.035	0					0.44
19.767	0.00	0.13	0.035	0					0.43
19.783	0.00	0.13	0.035	0					0.43
19.800	0.00	0.13	0.034	0					0.43
19.817	0.00	0.13	0.034	0					0.43
19.833	0.00	0.13	0.034	0					0.43
19.850	0.00	0.12	0.034	0					0.42
19.867	0.00	0.12	0.034	0					0.42
19.883	0.00	0.12	0.034	0					0.42
19.900	0.00	0.12	0.033	0					0.42
19.917	0.00	0.12	0.033	0					0.41
19.933	0.00	0.12	0.033	0					0.41
19.950	0.00	0.12	0.033	0					0.41
19.967	0.00	0.12	0.033	0					0.41
19.983	0.00	0.12	0.032	0					0.41
20.000	0.00	0.12	0.032	0					0.40
20.017	0.00	0.12	0.032	0					0.40
20.033	0.00	0.12	0.032	0					0.40
20.050	0.00	0.12	0.032	0					0.40
20.067	0.00	0.12	0.032	0					0.40
20.083	0.00	0.12	0.031	0					0.39
20.100	0.00	0.12	0.031	0					0.39
20.117	0.00	0.12	0.031	0					0.39
20.133	0.00	0.12	0.031	0					0.39
20.150	0.00	0.12	0.031	0					0.39
20.167	0.00	0.12	0.031	0					0.38
20.183	0.00	0.12	0.031	0					0.38
20.200	0.00	0.12	0.030	0					0.38
20.217	0.00	0.12	0.030	0					0.38
20.233	0.00	0.12	0.030	0					0.38
20.250	0.00	0.11	0.030	0					0.37
20.267	0.00	0.11	0.030	0					0.37
20.283	0.00	0.11	0.030	0					0.37
20.300	0.00	0.11	0.029	0					0.37
20.317	0.00	0.11	0.029	0					0.37
20.333	0.00	0.11	0.029	0					0.36
20.350	0.00	0.11	0.029	0					0.36
20.367	0.00	0.11	0.029	0					0.36
20.383	0.00	0.11	0.029	0					0.36
20.400	0.00	0.11	0.028	0					0.36
20.417	0.00	0.11	0.028	0					0.35
20.433	0.00	0.11	0.028	0					0.35

20.450	0.00	0.11	0.028	0					0.35
20.467	0.00	0.11	0.028	0					0.35
20.483	0.00	0.11	0.028	0					0.35
20.500	0.00	0.11	0.028	0					0.34
20.517	0.00	0.11	0.027	0					0.34
20.533	0.00	0.11	0.027	0					0.34
20.550	0.00	0.11	0.027	0					0.34
20.567	0.00	0.11	0.027	0					0.34
20.583	0.00	0.11	0.027	0					0.34
20.600	0.00	0.11	0.027	0					0.33
20.617	0.00	0.11	0.027	0					0.33
20.633	0.00	0.11	0.026	0					0.33
20.650	0.00	0.11	0.026	0					0.33
20.667	0.00	0.11	0.026	0					0.33
20.683	0.00	0.10	0.026	0					0.32
20.700	0.00	0.10	0.026	0					0.32
20.717	0.00	0.10	0.026	0					0.32
20.733	0.00	0.10	0.026	0					0.32
20.750	0.00	0.10	0.025	0					0.32
20.767	0.00	0.10	0.025	0					0.32
20.783	0.00	0.10	0.025	0					0.31
20.800	0.00	0.10	0.025	0					0.31
20.817	0.00	0.10	0.025	0					0.31
20.833	0.00	0.10	0.025	0					0.31
20.850	0.00	0.10	0.025	0					0.31
20.867	0.00	0.10	0.024	0					0.31
20.883	0.00	0.10	0.024	0					0.30
20.900	0.00	0.10	0.024	0					0.30
20.917	0.00	0.10	0.024	0					0.30
20.933	0.00	0.10	0.024	0					0.30
20.950	0.00	0.10	0.024	0					0.30
20.967	0.00	0.10	0.024	0					0.29
20.983	0.00	0.10	0.023	0					0.29
21.000	0.00	0.10	0.023	0					0.29
21.017	0.00	0.10	0.023	0					0.29
21.033	0.00	0.10	0.023	0					0.29
21.050	0.00	0.10	0.023	0					0.29
21.067	0.00	0.10	0.023	0					0.28
21.083	0.00	0.10	0.023	0					0.28
21.100	0.00	0.10	0.023	0					0.28
21.117	0.00	0.10	0.022	0					0.28
21.133	0.00	0.10	0.022	0					0.28
21.150	0.00	0.10	0.022	0					0.28
21.167	0.00	0.09	0.022	0					0.27
21.183	0.00	0.09	0.022	0					0.27
21.200	0.00	0.09	0.022	0					0.27
21.217	0.00	0.09	0.022	0					0.27
21.233	0.00	0.09	0.021	0					0.27
21.250	0.00	0.09	0.021	0					0.27
21.267	0.00	0.09	0.021	0					0.26

21.283	0.00	0.09	0.021	0					0.26
21.300	0.00	0.09	0.021	0					0.26
21.317	0.00	0.09	0.021	0					0.26
21.333	0.00	0.09	0.021	0					0.26
21.350	0.00	0.09	0.021	0					0.26
21.367	0.00	0.09	0.020	0					0.26
21.383	0.00	0.09	0.020	0					0.25
21.400	0.00	0.09	0.020	0					0.25
21.417	0.00	0.09	0.020	0					0.25
21.433	0.00	0.09	0.020	0					0.25
21.450	0.00	0.09	0.020	0					0.25
21.467	0.00	0.09	0.020	0					0.25
21.483	0.00	0.09	0.020	0					0.24
21.500	0.00	0.09	0.019	0					0.24
21.517	0.00	0.09	0.019	0					0.24
21.533	0.00	0.09	0.019	0					0.24
21.550	0.00	0.09	0.019	0					0.24
21.567	0.00	0.08	0.019	0					0.24
21.583	0.00	0.08	0.019	0					0.24
21.600	0.00	0.08	0.019	0					0.23
21.617	0.00	0.08	0.019	0					0.23
21.633	0.00	0.08	0.019	0					0.23
21.650	0.00	0.08	0.018	0					0.23
21.667	0.00	0.08	0.018	0					0.23
21.683	0.00	0.08	0.018	0					0.23
21.700	0.00	0.08	0.018	0					0.23
21.717	0.00	0.08	0.018	0					0.22
21.733	0.00	0.08	0.018	0					0.22
21.750	0.00	0.08	0.018	0					0.22
21.767	0.00	0.08	0.018	0					0.22
21.783	0.00	0.08	0.018	0					0.22
21.800	0.00	0.08	0.017	0					0.22
21.817	0.00	0.08	0.017	0					0.22
21.833	0.00	0.08	0.017	0					0.22
21.850	0.00	0.08	0.017	0					0.21
21.867	0.00	0.07	0.017	0					0.21
21.883	0.00	0.07	0.017	0					0.21
21.900	0.00	0.07	0.017	0					0.21
21.917	0.00	0.07	0.017	0					0.21
21.933	0.00	0.07	0.017	0					0.21
21.950	0.00	0.07	0.016	0					0.21
21.967	0.00	0.07	0.016	0					0.20
21.983	0.00	0.07	0.016	0					0.20
22.000	0.00	0.07	0.016	0					0.20
22.017	0.00	0.07	0.016	0					0.20
22.033	0.00	0.07	0.016	0					0.20
22.050	0.00	0.07	0.016	0					0.20
22.067	0.00	0.07	0.016	0					0.20
22.083	0.00	0.07	0.016	0					0.20
22.100	0.00	0.07	0.016	0					0.20

22.117	0.00	0.07	0.016	0					0.19
22.133	0.00	0.07	0.015	0					0.19
22.150	0.00	0.07	0.015	0					0.19
22.167	0.00	0.07	0.015	0					0.19
22.183	0.00	0.07	0.015	0					0.19
22.200	0.00	0.07	0.015	0					0.19
22.217	0.00	0.06	0.015	0					0.19
22.233	0.00	0.06	0.015	0					0.19
22.250	0.00	0.06	0.015	0					0.19
22.267	0.00	0.06	0.015	0					0.18
22.283	0.00	0.06	0.015	0					0.18
22.300	0.00	0.06	0.015	0					0.18
22.317	0.00	0.06	0.014	0					0.18
22.333	0.00	0.06	0.014	0					0.18
22.350	0.00	0.06	0.014	0					0.18
22.367	0.00	0.06	0.014	0					0.18
22.383	0.00	0.06	0.014	0					0.18
22.400	0.00	0.06	0.014	0					0.18
22.417	0.00	0.06	0.014	0					0.17
22.433	0.00	0.06	0.014	0					0.17
22.450	0.00	0.06	0.014	0					0.17
22.467	0.00	0.06	0.014	0					0.17
22.483	0.00	0.06	0.014	0					0.17
22.500	0.00	0.06	0.014	0					0.17
22.517	0.00	0.06	0.013	0					0.17
22.533	0.00	0.06	0.013	0					0.17
22.550	0.00	0.06	0.013	0					0.17
22.567	0.00	0.06	0.013	0					0.17
22.583	0.00	0.06	0.013	0					0.16
22.600	0.00	0.06	0.013	0					0.16
22.617	0.00	0.06	0.013	0					0.16
22.633	0.00	0.05	0.013	0					0.16
22.650	0.00	0.05	0.013	0					0.16
22.667	0.00	0.05	0.013	0					0.16
22.683	0.00	0.05	0.013	0					0.16
22.700	0.00	0.05	0.013	0					0.16
22.717	0.00	0.05	0.013	0					0.16
22.733	0.00	0.05	0.012	0					0.16
22.750	0.00	0.05	0.012	0					0.16
22.767	0.00	0.05	0.012	0					0.15
22.783	0.00	0.05	0.012	0					0.15
22.800	0.00	0.05	0.012	0					0.15
22.817	0.00	0.05	0.012	0					0.15
22.833	0.00	0.05	0.012	0					0.15
22.850	0.00	0.05	0.012	0					0.15
22.867	0.00	0.05	0.012	0					0.15
22.883	0.00	0.05	0.012	0					0.15
22.900	0.00	0.05	0.012	0					0.15
22.917	0.00	0.05	0.012	0					0.15
22.933	0.00	0.05	0.012	0					0.15

22.950	0.00	0.05	0.012	0					0.14
22.967	0.00	0.05	0.012	0					0.14
22.983	0.00	0.05	0.011	0					0.14
23.000	0.00	0.05	0.011	0					0.14
23.017	0.00	0.05	0.011	0					0.14
23.033	0.00	0.05	0.011	0					0.14
23.050	0.00	0.05	0.011	0					0.14
23.067	0.00	0.05	0.011	0					0.14
23.083	0.00	0.05	0.011	0					0.14
23.100	0.00	0.05	0.011	0					0.14
23.117	0.00	0.04	0.011	0					0.14
23.133	0.00	0.04	0.011	0					0.14
23.150	0.00	0.04	0.011	0					0.14
23.167	0.00	0.04	0.011	0					0.13
23.183	0.00	0.04	0.011	0					0.13
23.200	0.00	0.04	0.011	0					0.13
23.217	0.00	0.04	0.011	0					0.13
23.233	0.00	0.04	0.011	0					0.13
23.250	0.00	0.04	0.010	0					0.13
23.267	0.00	0.04	0.010	0					0.13
23.283	0.00	0.04	0.010	0					0.13
23.300	0.00	0.04	0.010	0					0.13
23.317	0.00	0.04	0.010	0					0.13
23.333	0.00	0.04	0.010	0					0.13
23.350	0.00	0.04	0.010	0					0.13
23.367	0.00	0.04	0.010	0					0.13
23.383	0.00	0.04	0.010	0					0.13
23.400	0.00	0.04	0.010	0					0.12
23.417	0.00	0.04	0.010	0					0.12
23.433	0.00	0.04	0.010	0					0.12
23.450	0.00	0.04	0.010	0					0.12
23.467	0.00	0.04	0.010	0					0.12
23.483	0.00	0.04	0.010	0					0.12
23.500	0.00	0.04	0.010	0					0.12
23.517	0.00	0.04	0.010	0					0.12
23.533	0.00	0.04	0.010	0					0.12
23.550	0.00	0.04	0.009	0					0.12
23.567	0.00	0.04	0.009	0					0.12
23.583	0.00	0.04	0.009	0					0.12
23.600	0.00	0.04	0.009	0					0.12
23.617	0.00	0.04	0.009	0					0.12
23.633	0.00	0.04	0.009	0					0.12
23.650	0.00	0.04	0.009	0					0.11
23.667	0.00	0.04	0.009	0					0.11
23.683	0.00	0.04	0.009	0					0.11
23.700	0.00	0.04	0.009	0					0.11
23.717	0.00	0.04	0.009	0					0.11
23.733	0.00	0.04	0.009	0					0.11
23.750	0.00	0.04	0.009	0					0.11
23.767	0.00	0.04	0.009	0					0.11

23.783	0.00	0.04	0.009	0					0.11
23.800	0.00	0.03	0.009	0					0.11
23.817	0.00	0.03	0.009	0					0.11
23.833	0.00	0.03	0.009	0					0.11
23.850	0.00	0.03	0.009	0					0.11
23.867	0.00	0.03	0.009	0					0.11
23.883	0.00	0.03	0.008	0					0.11
23.900	0.00	0.03	0.008	0					0.11
23.917	0.00	0.03	0.008	0					0.10
23.933	0.00	0.03	0.008	0					0.10
23.950	0.00	0.03	0.008	0					0.10
23.967	0.00	0.03	0.008	0					0.10
23.983	0.00	0.03	0.008	0					0.10
24.000	0.00	0.03	0.008	0					0.10
24.017	0.00	0.03	0.008	0					0.10
24.033	0.00	0.03	0.008	0					0.10
24.050	0.00	0.03	0.008	0					0.10
24.067	0.00	0.03	0.008	0					0.10
24.083	0.00	0.03	0.008	0					0.10
24.100	0.00	0.03	0.008	0					0.10
24.117	0.00	0.03	0.008	0					0.10
24.133	0.00	0.03	0.008	0					0.10
24.150	0.00	0.03	0.008	0					0.10
24.167	0.00	0.03	0.008	0					0.10
24.183	0.00	0.03	0.008	0					0.10
24.200	0.00	0.03	0.008	0					0.10
24.217	0.00	0.03	0.008	0					0.10
24.233	0.00	0.03	0.008	0					0.09
24.250	0.00	0.03	0.008	0					0.09
24.267	0.00	0.03	0.007	0					0.09
24.283	0.00	0.03	0.007	0					0.09
24.300	0.00	0.03	0.007	0					0.09
24.317	0.00	0.03	0.007	0					0.09
24.333	0.00	0.03	0.007	0					0.09
24.350	0.00	0.03	0.007	0					0.09
24.367	0.00	0.03	0.007	0					0.09
24.383	0.00	0.03	0.007	0					0.09
24.400	0.00	0.03	0.007	0					0.09
24.417	0.00	0.03	0.007	0					0.09
24.433	0.00	0.03	0.007	0					0.09
24.450	0.00	0.03	0.007	0					0.09
24.467	0.00	0.03	0.007	0					0.09
24.483	0.00	0.03	0.007	0					0.09
24.500	0.00	0.03	0.007	0					0.09
24.517	0.00	0.03	0.007	0					0.09
24.533	0.00	0.03	0.007	0					0.09
24.550	0.00	0.03	0.007	0					0.09
24.567	0.00	0.03	0.007	0					0.08
24.583	0.00	0.03	0.007	0					0.08
24.600	0.00	0.03	0.007	0					0.08

24.617	0.00	0.03	0.007	0					0.08
24.633	0.00	0.03	0.007	0					0.08
24.650	0.00	0.03	0.007	0					0.08
24.667	0.00	0.03	0.007	0					0.08
24.683	0.00	0.03	0.007	0					0.08
24.700	0.00	0.03	0.006	0					0.08
24.717	0.00	0.03	0.006	0					0.08
24.733	0.00	0.03	0.006	0					0.08
24.750	0.00	0.03	0.006	0					0.08
24.767	0.00	0.03	0.006	0					0.08
24.783	0.00	0.03	0.006	0					0.08
24.800	0.00	0.03	0.006	0					0.08
24.817	0.00	0.02	0.006	0					0.08
24.833	0.00	0.02	0.006	0					0.08
24.850	0.00	0.02	0.006	0					0.08
24.867	0.00	0.02	0.006	0					0.08
24.883	0.00	0.02	0.006	0					0.08
24.900	0.00	0.02	0.006	0					0.08
24.917	0.00	0.02	0.006	0					0.08
24.933	0.00	0.02	0.006	0					0.07
24.950	0.00	0.02	0.006	0					0.07
24.967	0.00	0.02	0.006	0					0.07
24.983	0.00	0.02	0.006	0					0.07
25.000	0.00	0.02	0.006	0					0.07
25.017	0.00	0.02	0.006	0					0.07
25.033	0.00	0.02	0.006	0					0.07
25.050	0.00	0.02	0.006	0					0.07
25.067	0.00	0.02	0.006	0					0.07
25.083	0.00	0.02	0.006	0					0.07
25.100	0.00	0.02	0.006	0					0.07
25.117	0.00	0.02	0.006	0					0.07
25.133	0.00	0.02	0.006	0					0.07
25.150	0.00	0.02	0.006	0					0.07
25.167	0.00	0.02	0.006	0					0.07
25.183	0.00	0.02	0.006	0					0.07
25.200	0.00	0.02	0.005	0					0.07
25.217	0.00	0.02	0.005	0					0.07
25.233	0.00	0.02	0.005	0					0.07
25.250	0.00	0.02	0.005	0					0.07
25.267	0.00	0.02	0.005	0					0.07
25.283	0.00	0.02	0.005	0					0.07
25.300	0.00	0.02	0.005	0					0.07
25.317	0.00	0.02	0.005	0					0.07
25.333	0.00	0.02	0.005	0					0.07
25.350	0.00	0.02	0.005	0					0.07
25.367	0.00	0.02	0.005	0					0.06
25.383	0.00	0.02	0.005	0					0.06
25.400	0.00	0.02	0.005	0					0.06
25.417	0.00	0.02	0.005	0					0.06
25.433	0.00	0.02	0.005	0					0.06

25.450	0.00	0.02	0.005	0					0.06
25.467	0.00	0.02	0.005	0					0.06
25.483	0.00	0.02	0.005	0					0.06
25.500	0.00	0.02	0.005	0					0.06
25.517	0.00	0.02	0.005	0					0.06
25.533	0.00	0.02	0.005	0					0.06
25.550	0.00	0.02	0.005	0					0.06
25.567	0.00	0.02	0.005	0					0.06
25.583	0.00	0.02	0.005	0					0.06
25.600	0.00	0.02	0.005	0					0.06
25.617	0.00	0.02	0.005	0					0.06
25.633	0.00	0.02	0.005	0					0.06
25.650	0.00	0.02	0.005	0					0.06
25.667	0.00	0.02	0.005	0					0.06
25.683	0.00	0.02	0.005	0					0.06
25.700	0.00	0.02	0.005	0					0.06
25.717	0.00	0.02	0.005	0					0.06
25.733	0.00	0.02	0.005	0					0.06
25.750	0.00	0.02	0.005	0					0.06
25.767	0.00	0.02	0.005	0					0.06
25.783	0.00	0.02	0.005	0					0.06
25.800	0.00	0.02	0.005	0					0.06
25.817	0.00	0.02	0.004	0					0.06
25.833	0.00	0.02	0.004	0					0.06
25.850	0.00	0.02	0.004	0					0.06
25.867	0.00	0.02	0.004	0					0.06
25.883	0.00	0.02	0.004	0					0.05
25.900	0.00	0.02	0.004	0					0.05
25.917	0.00	0.02	0.004	0					0.05
25.933	0.00	0.02	0.004	0					0.05
25.950	0.00	0.02	0.004	0					0.05
25.967	0.00	0.02	0.004	0					0.05
25.983	0.00	0.02	0.004	0					0.05
26.000	0.00	0.02	0.004	0					0.05
26.017	0.00	0.02	0.004	0					0.05
26.033	0.00	0.02	0.004	0					0.05
26.050	0.00	0.02	0.004	0					0.05
26.067	0.00	0.02	0.004	0					0.05
26.083	0.00	0.02	0.004	0					0.05
26.100	0.00	0.02	0.004	0					0.05
26.117	0.00	0.02	0.004	0					0.05
26.133	0.00	0.02	0.004	0					0.05
26.150	0.00	0.02	0.004	0					0.05
26.167	0.00	0.02	0.004	0					0.05
26.183	0.00	0.02	0.004	0					0.05
26.200	0.00	0.02	0.004	0					0.05
26.217	0.00	0.02	0.004	0					0.05
26.233	0.00	0.02	0.004	0					0.05
26.250	0.00	0.02	0.004	0					0.05
26.267	0.00	0.02	0.004	0					0.05

26.283	0.00	0.02	0.004	0					0.05
26.300	0.00	0.02	0.004	0					0.05
26.317	0.00	0.02	0.004	0					0.05
26.333	0.00	0.02	0.004	0					0.05
26.350	0.00	0.02	0.004	0					0.05
26.367	0.00	0.01	0.004	0					0.05
26.383	0.00	0.01	0.004	0					0.05
26.400	0.00	0.01	0.004	0					0.05
26.417	0.00	0.01	0.004	0					0.05
26.433	0.00	0.01	0.004	0					0.05
26.450	0.00	0.01	0.004	0					0.05
26.467	0.00	0.01	0.004	0					0.05
26.483	0.00	0.01	0.004	0					0.04
26.500	0.00	0.01	0.004	0					0.04
26.517	0.00	0.01	0.004	0					0.04
26.533	0.00	0.01	0.004	0					0.04
26.550	0.00	0.01	0.004	0					0.04
26.567	0.00	0.01	0.003	0					0.04
26.583	0.00	0.01	0.003	0					0.04
26.600	0.00	0.01	0.003	0					0.04
26.617	0.00	0.01	0.003	0					0.04
26.633	0.00	0.01	0.003	0					0.04
26.650	0.00	0.01	0.003	0					0.04
26.667	0.00	0.01	0.003	0					0.04
26.683	0.00	0.01	0.003	0					0.04
26.700	0.00	0.01	0.003	0					0.04
26.717	0.00	0.01	0.003	0					0.04
26.733	0.00	0.01	0.003	0					0.04
26.750	0.00	0.01	0.003	0					0.04
26.767	0.00	0.01	0.003	0					0.04
26.783	0.00	0.01	0.003	0					0.04
26.800	0.00	0.01	0.003	0					0.04
26.817	0.00	0.01	0.003	0					0.04
26.833	0.00	0.01	0.003	0					0.04
26.850	0.00	0.01	0.003	0					0.04
26.867	0.00	0.01	0.003	0					0.04
26.883	0.00	0.01	0.003	0					0.04
26.900	0.00	0.01	0.003	0					0.04
26.917	0.00	0.01	0.003	0					0.04
26.933	0.00	0.01	0.003	0					0.04
26.950	0.00	0.01	0.003	0					0.04
26.967	0.00	0.01	0.003	0					0.04
26.983	0.00	0.01	0.003	0					0.04
27.000	0.00	0.01	0.003	0					0.04
27.017	0.00	0.01	0.003	0					0.04
27.033	0.00	0.01	0.003	0					0.04
27.050	0.00	0.01	0.003	0					0.04
27.067	0.00	0.01	0.003	0					0.04
27.083	0.00	0.01	0.003	0					0.04
27.100	0.00	0.01	0.003	0					0.04

27.117	0.00	0.01	0.003	0				0.04
27.133	0.00	0.01	0.003	0				0.04
27.150	0.00	0.01	0.003	0				0.04
27.167	0.00	0.01	0.003	0				0.04
27.183	0.00	0.01	0.003	0				0.04
27.200	0.00	0.01	0.003	0				0.04
27.217	0.00	0.01	0.003	0				0.04
27.233	0.00	0.01	0.003	0				0.04
27.250	0.00	0.01	0.003	0				0.03
27.267	0.00	0.01	0.003	0				0.03
27.283	0.00	0.01	0.003	0				0.03
27.300	0.00	0.01	0.003	0				0.03
27.317	0.00	0.01	0.003	0				0.03
27.333	0.00	0.01	0.003	0				0.03
27.350	0.00	0.01	0.003	0				0.03
27.367	0.00	0.01	0.003	0				0.03
27.383	0.00	0.01	0.003	0				0.03
27.400	0.00	0.01	0.003	0				0.03
27.417	0.00	0.01	0.003	0				0.03
27.433	0.00	0.01	0.003	0				0.03
27.450	0.00	0.01	0.003	0				0.03
27.467	0.00	0.01	0.003	0				0.03
27.483	0.00	0.01	0.003	0				0.03
27.500	0.00	0.01	0.003	0				0.03
27.517	0.00	0.01	0.003	0				0.03
27.533	0.00	0.01	0.003	0				0.03
27.550	0.00	0.01	0.003	0				0.03
27.567	0.00	0.01	0.003	0				0.03
27.583	0.00	0.01	0.002	0				0.03
27.600	0.00	0.01	0.002	0				0.03
27.617	0.00	0.01	0.002	0				0.03
27.633	0.00	0.01	0.002	0				0.03
27.650	0.00	0.01	0.002	0				0.03
27.667	0.00	0.01	0.002	0				0.03
27.683	0.00	0.01	0.002	0				0.03
27.700	0.00	0.01	0.002	0				0.03
27.717	0.00	0.01	0.002	0				0.03
27.733	0.00	0.01	0.002	0				0.03
27.750	0.00	0.01	0.002	0				0.03
27.767	0.00	0.01	0.002	0				0.03
27.783	0.00	0.01	0.002	0				0.03
27.800	0.00	0.01	0.002	0				0.03
27.817	0.00	0.01	0.002	0				0.03
27.833	0.00	0.01	0.002	0				0.03
27.850	0.00	0.01	0.002	0				0.03
27.867	0.00	0.01	0.002	0				0.03
27.883	0.00	0.01	0.002	0				0.03
27.900	0.00	0.01	0.002	0				0.03
27.917	0.00	0.01	0.002	0				0.03
27.933	0.00	0.01	0.002	0				0.03

27.950	0.00	0.01	0.002	0					0.03
27.967	0.00	0.01	0.002	0					0.03
27.983	0.00	0.01	0.002	0					0.03
28.000	0.00	0.01	0.002	0					0.03
28.017	0.00	0.01	0.002	0					0.03
28.033	0.00	0.01	0.002	0					0.03
28.050	0.00	0.01	0.002	0					0.03
28.067	0.00	0.01	0.002	0					0.03
28.083	0.00	0.01	0.002	0					0.03
28.100	0.00	0.01	0.002	0					0.03
28.117	0.00	0.01	0.002	0					0.03
28.133	0.00	0.01	0.002	0					0.03
28.150	0.00	0.01	0.002	0					0.03
28.167	0.00	0.01	0.002	0					0.03
28.183	0.00	0.01	0.002	0					0.03
28.200	0.00	0.01	0.002	0					0.03
28.217	0.00	0.01	0.002	0					0.03
28.233	0.00	0.01	0.002	0					0.03
28.250	0.00	0.01	0.002	0					0.03
28.267	0.00	0.01	0.002	0					0.02
28.283	0.00	0.01	0.002	0					0.02
28.300	0.00	0.01	0.002	0					0.02
28.317	0.00	0.01	0.002	0					0.02
28.333	0.00	0.01	0.002	0					0.02
28.350	0.00	0.01	0.002	0					0.02
28.367	0.00	0.01	0.002	0					0.02
28.383	0.00	0.01	0.002	0					0.02
28.400	0.00	0.01	0.002	0					0.02
28.417	0.00	0.01	0.002	0					0.02
28.433	0.00	0.01	0.002	0					0.02
28.450	0.00	0.01	0.002	0					0.02
28.467	0.00	0.01	0.002	0					0.02
28.483	0.00	0.01	0.002	0					0.02
28.500	0.00	0.01	0.002	0					0.02
28.517	0.00	0.01	0.002	0					0.02
28.533	0.00	0.01	0.002	0					0.02
28.550	0.00	0.01	0.002	0					0.02
28.567	0.00	0.01	0.002	0					0.02
28.583	0.00	0.01	0.002	0					0.02
28.600	0.00	0.01	0.002	0					0.02
28.617	0.00	0.01	0.002	0					0.02
28.633	0.00	0.01	0.002	0					0.02
28.650	0.00	0.01	0.002	0					0.02
28.667	0.00	0.01	0.002	0					0.02
28.683	0.00	0.01	0.002	0					0.02
28.700	0.00	0.01	0.002	0					0.02
28.717	0.00	0.01	0.002	0					0.02
28.733	0.00	0.01	0.002	0					0.02
28.750	0.00	0.01	0.002	0					0.02
28.767	0.00	0.01	0.002	0					0.02

28.783	0.00	0.01	0.002	0					0.02
28.800	0.00	0.01	0.002	0					0.02
28.817	0.00	0.01	0.002	0					0.02
28.833	0.00	0.01	0.002	0					0.02
28.850	0.00	0.01	0.002	0					0.02
28.867	0.00	0.01	0.002	0					0.02
28.883	0.00	0.01	0.002	0					0.02
28.900	0.00	0.01	0.002	0					0.02
28.917	0.00	0.01	0.002	0					0.02
28.933	0.00	0.01	0.002	0					0.02
28.950	0.00	0.01	0.002	0					0.02
28.967	0.00	0.01	0.002	0					0.02
28.983	0.00	0.01	0.002	0					0.02
29.000	0.00	0.01	0.002	0					0.02
29.017	0.00	0.01	0.002	0					0.02
29.033	0.00	0.01	0.002	0					0.02
29.050	0.00	0.01	0.002	0					0.02
29.067	0.00	0.01	0.002	0					0.02
29.083	0.00	0.01	0.002	0					0.02
29.100	0.00	0.01	0.002	0					0.02
29.117	0.00	0.01	0.002	0					0.02
29.133	0.00	0.01	0.001	0					0.02
29.150	0.00	0.01	0.001	0					0.02
29.167	0.00	0.01	0.001	0					0.02
29.183	0.00	0.01	0.001	0					0.02
29.200	0.00	0.01	0.001	0					0.02
29.217	0.00	0.01	0.001	0					0.02
29.233	0.00	0.01	0.001	0					0.02
29.250	0.00	0.01	0.001	0					0.02
29.267	0.00	0.01	0.001	0					0.02
29.283	0.00	0.01	0.001	0					0.02
29.300	0.00	0.01	0.001	0					0.02
29.317	0.00	0.01	0.001	0					0.02
29.333	0.00	0.01	0.001	0					0.02
29.350	0.00	0.01	0.001	0					0.02
29.367	0.00	0.01	0.001	0					0.02
29.383	0.00	0.01	0.001	0					0.02
29.400	0.00	0.01	0.001	0					0.02
29.417	0.00	0.01	0.001	0					0.02
29.433	0.00	0.01	0.001	0					0.02
29.450	0.00	0.01	0.001	0					0.02
29.467	0.00	0.01	0.001	0					0.02
29.483	0.00	0.01	0.001	0					0.02
29.500	0.00	0.01	0.001	0					0.02
29.517	0.00	0.01	0.001	0					0.02
29.533	0.00	0.01	0.001	0					0.02
29.550	0.00	0.01	0.001	0					0.02
29.567	0.00	0.01	0.001	0					0.02
29.583	0.00	0.01	0.001	0					0.02
29.600	0.00	0.01	0.001	0					0.02

29.617	0.00	0.01	0.001	0					0.02
29.633	0.00	0.01	0.001	0					0.02
29.650	0.00	0.01	0.001	0					0.02
29.667	0.00	0.01	0.001	0					0.02
29.683	0.00	0.00	0.001	0					0.02
29.700	0.00	0.00	0.001	0					0.02
29.717	0.00	0.00	0.001	0					0.02
29.733	0.00	0.00	0.001	0					0.02
29.750	0.00	0.00	0.001	0					0.02
29.767	0.00	0.00	0.001	0					0.02
29.783	0.00	0.00	0.001	0					0.02
29.800	0.00	0.00	0.001	0					0.02
29.817	0.00	0.00	0.001	0					0.01
29.833	0.00	0.00	0.001	0					0.01
29.850	0.00	0.00	0.001	0					0.01
29.867	0.00	0.00	0.001	0					0.01
29.883	0.00	0.00	0.001	0					0.01
29.900	0.00	0.00	0.001	0					0.01
29.917	0.00	0.00	0.001	0					0.01
29.933	0.00	0.00	0.001	0					0.01
29.950	0.00	0.00	0.001	0					0.01
29.967	0.00	0.00	0.001	0					0.01
29.983	0.00	0.00	0.001	0					0.01
30.000	0.00	0.00	0.001	0					0.01
30.017	0.00	0.00	0.001	0					0.01
30.033	0.00	0.00	0.001	0					0.01
30.050	0.00	0.00	0.001	0					0.01
30.067	0.00	0.00	0.001	0					0.01
30.083	0.00	0.00	0.001	0					0.01
30.100	0.00	0.00	0.001	0					0.01
30.117	0.00	0.00	0.001	0					0.01
30.133	0.00	0.00	0.001	0					0.01
30.150	0.00	0.00	0.001	0					0.01
30.167	0.00	0.00	0.001	0					0.01
30.183	0.00	0.00	0.001	0					0.01
30.200	0.00	0.00	0.001	0					0.01
30.217	0.00	0.00	0.001	0					0.01
30.233	0.00	0.00	0.001	0					0.01
30.250	0.00	0.00	0.001	0					0.01
30.267	0.00	0.00	0.001	0					0.01
30.283	0.00	0.00	0.001	0					0.01
30.300	0.00	0.00	0.001	0					0.01
30.317	0.00	0.00	0.001	0					0.01
30.333	0.00	0.00	0.001	0					0.01
30.350	0.00	0.00	0.001	0					0.01
30.367	0.00	0.00	0.001	0					0.01
30.383	0.00	0.00	0.001	0					0.01
30.400	0.00	0.00	0.001	0					0.01
30.417	0.00	0.00	0.001	0					0.01
30.433	0.00	0.00	0.001	0					0.01

30.450	0.00	0.00	0.001	0				0.01
30.467	0.00	0.00	0.001	0				0.01
30.483	0.00	0.00	0.001	0				0.01
30.500	0.00	0.00	0.001	0				0.01
30.517	0.00	0.00	0.001	0				0.01
30.533	0.00	0.00	0.001	0				0.01
30.550	0.00	0.00	0.001	0				0.01
30.567	0.00	0.00	0.001	0				0.01
30.583	0.00	0.00	0.001	0				0.01
30.600	0.00	0.00	0.001	0				0.01
30.617	0.00	0.00	0.001	0				0.01
30.633	0.00	0.00	0.001	0				0.01
30.650	0.00	0.00	0.001	0				0.01
30.667	0.00	0.00	0.001	0				0.01
30.683	0.00	0.00	0.001	0				0.01
30.700	0.00	0.00	0.001	0				0.01
30.717	0.00	0.00	0.001	0				0.01
30.733	0.00	0.00	0.001	0				0.01
30.750	0.00	0.00	0.001	0				0.01
30.767	0.00	0.00	0.001	0				0.01
30.783	0.00	0.00	0.001	0				0.01
30.800	0.00	0.00	0.001	0				0.01
30.817	0.00	0.00	0.001	0				0.01
30.833	0.00	0.00	0.001	0				0.01
30.850	0.00	0.00	0.001	0				0.01
30.867	0.00	0.00	0.001	0				0.01
30.883	0.00	0.00	0.001	0				0.01
30.900	0.00	0.00	0.001	0				0.01
30.917	0.00	0.00	0.001	0				0.01
30.933	0.00	0.00	0.001	0				0.01
30.950	0.00	0.00	0.001	0				0.01
30.967	0.00	0.00	0.001	0				0.01
30.983	0.00	0.00	0.001	0				0.01
31.000	0.00	0.00	0.001	0				0.01
31.017	0.00	0.00	0.001	0				0.01
31.033	0.00	0.00	0.001	0				0.01
31.050	0.00	0.00	0.001	0				0.01
31.067	0.00	0.00	0.001	0				0.01
31.083	0.00	0.00	0.001	0				0.01
31.100	0.00	0.00	0.001	0				0.01
31.117	0.00	0.00	0.001	0				0.01
31.133	0.00	0.00	0.001	0				0.01
31.150	0.00	0.00	0.001	0				0.01
31.167	0.00	0.00	0.001	0				0.01
31.183	0.00	0.00	0.001	0				0.01
31.200	0.00	0.00	0.001	0				0.01
31.217	0.00	0.00	0.001	0				0.01
31.233	0.00	0.00	0.001	0				0.01
31.250	0.00	0.00	0.001	0				0.01
31.267	0.00	0.00	0.001	0				0.01

31.283	0.00	0.00	0.001	0				0.01
31.300	0.00	0.00	0.001	0				0.01
31.317	0.00	0.00	0.001	0				0.01
31.333	0.00	0.00	0.001	0				0.01
31.350	0.00	0.00	0.001	0				0.01
31.367	0.00	0.00	0.001	0				0.01
31.383	0.00	0.00	0.001	0				0.01
31.400	0.00	0.00	0.001	0				0.01
31.417	0.00	0.00	0.001	0				0.01
31.433	0.00	0.00	0.001	0				0.01
31.450	0.00	0.00	0.001	0				0.01
31.467	0.00	0.00	0.001	0				0.01
31.483	0.00	0.00	0.001	0				0.01
31.500	0.00	0.00	0.001	0				0.01
31.517	0.00	0.00	0.001	0				0.01
31.533	0.00	0.00	0.001	0				0.01
31.550	0.00	0.00	0.001	0				0.01
31.567	0.00	0.00	0.001	0				0.01
31.583	0.00	0.00	0.001	0				0.01
31.600	0.00	0.00	0.001	0				0.01
31.617	0.00	0.00	0.001	0				0.01
31.633	0.00	0.00	0.001	0				0.01
31.650	0.00	0.00	0.001	0				0.01
31.667	0.00	0.00	0.001	0				0.01
31.683	0.00	0.00	0.001	0				0.01
31.700	0.00	0.00	0.001	0				0.01
31.717	0.00	0.00	0.001	0				0.01
31.733	0.00	0.00	0.001	0				0.01
31.750	0.00	0.00	0.001	0				0.01
31.767	0.00	0.00	0.001	0				0.01
31.783	0.00	0.00	0.001	0				0.01
31.800	0.00	0.00	0.001	0				0.01
31.817	0.00	0.00	0.001	0				0.01
31.833	0.00	0.00	0.001	0				0.01
31.850	0.00	0.00	0.001	0				0.01
31.867	0.00	0.00	0.001	0				0.01
31.883	0.00	0.00	0.001	0				0.01
31.900	0.00	0.00	0.001	0				0.01
31.917	0.00	0.00	0.001	0				0.01
31.933	0.00	0.00	0.001	0				0.01
31.950	0.00	0.00	0.001	0				0.01
31.967	0.00	0.00	0.001	0				0.01
31.983	0.00	0.00	0.001	0				0.01
32.000	0.00	0.00	0.001	0				0.01
32.017	0.00	0.00	0.001	0				0.01
32.033	0.00	0.00	0.001	0				0.01
32.050	0.00	0.00	0.001	0				0.01
32.067	0.00	0.00	0.001	0				0.01
32.083	0.00	0.00	0.001	0				0.01
32.100	0.00	0.00	0.001	0				0.01

32.117	0.00	0.00	0.001	0					0.01
32.133	0.00	0.00	0.001	0					0.01
32.150	0.00	0.00	0.001	0					0.01
32.167	0.00	0.00	0.001	0					0.01
32.183	0.00	0.00	0.001	0					0.01
32.200	0.00	0.00	0.001	0					0.01
32.217	0.00	0.00	0.001	0					0.01
32.233	0.00	0.00	0.001	0					0.01
32.250	0.00	0.00	0.001	0					0.01
32.267	0.00	0.00	0.001	0					0.01
32.283	0.00	0.00	0.001	0					0.01
32.300	0.00	0.00	0.001	0					0.01
32.317	0.00	0.00	0.001	0					0.01
32.333	0.00	0.00	0.001	0					0.01
32.350	0.00	0.00	0.001	0					0.01
32.367	0.00	0.00	0.001	0					0.01
32.383	0.00	0.00	0.001	0					0.01
32.400	0.00	0.00	0.001	0					0.01
32.417	0.00	0.00	0.001	0					0.01
32.433	0.00	0.00	0.001	0					0.01
32.450	0.00	0.00	0.000	0					0.01
32.467	0.00	0.00	0.000	0					0.01
32.483	0.00	0.00	0.000	0					0.01
32.500	0.00	0.00	0.000	0					0.01
32.517	0.00	0.00	0.000	0					0.01
32.533	0.00	0.00	0.000	0					0.01
32.550	0.00	0.00	0.000	0					0.01
32.567	0.00	0.00	0.000	0					0.01
32.583	0.00	0.00	0.000	0					0.01
32.600	0.00	0.00	0.000	0					0.01
32.617	0.00	0.00	0.000	0					0.01
32.633	0.00	0.00	0.000	0					0.01
32.650	0.00	0.00	0.000	0					0.01
32.667	0.00	0.00	0.000	0					0.01
32.683	0.00	0.00	0.000	0					0.01
32.700	0.00	0.00	0.000	0					0.01
32.717	0.00	0.00	0.000	0					0.01
32.733	0.00	0.00	0.000	0					0.01
32.750	0.00	0.00	0.000	0					0.01
32.767	0.00	0.00	0.000	0					0.01
32.783	0.00	0.00	0.000	0					0.01
32.800	0.00	0.00	0.000	0					0.01
32.817	0.00	0.00	0.000	0					0.01
32.833	0.00	0.00	0.000	0					0.01
32.850	0.00	0.00	0.000	0					0.01
32.867	0.00	0.00	0.000	0					0.01
32.883	0.00	0.00	0.000	0					0.01
32.900	0.00	0.00	0.000	0					0.01
32.917	0.00	0.00	0.000	0					0.01
32.933	0.00	0.00	0.000	0					0.01

32.950	0.00	0.00	0.000	0					0.01
32.967	0.00	0.00	0.000	0					0.01
32.983	0.00	0.00	0.000	0					0.01
33.000	0.00	0.00	0.000	0					0.01
33.017	0.00	0.00	0.000	0					0.01
33.033	0.00	0.00	0.000	0					0.01
33.050	0.00	0.00	0.000	0					0.01
33.067	0.00	0.00	0.000	0					0.01
33.083	0.00	0.00	0.000	0					0.01
33.100	0.00	0.00	0.000	0					0.01
33.117	0.00	0.00	0.000	0					0.01
33.133	0.00	0.00	0.000	0					0.00
33.150	0.00	0.00	0.000	0					0.00
33.167	0.00	0.00	0.000	0					0.00
33.183	0.00	0.00	0.000	0					0.00
33.200	0.00	0.00	0.000	0					0.00
33.217	0.00	0.00	0.000	0					0.00
33.233	0.00	0.00	0.000	0					0.00
33.250	0.00	0.00	0.000	0					0.00
33.267	0.00	0.00	0.000	0					0.00
33.283	0.00	0.00	0.000	0					0.00
33.300	0.00	0.00	0.000	0					0.00
33.317	0.00	0.00	0.000	0					0.00
33.333	0.00	0.00	0.000	0					0.00
33.350	0.00	0.00	0.000	0					0.00
33.367	0.00	0.00	0.000	0					0.00
33.383	0.00	0.00	0.000	0					0.00
33.400	0.00	0.00	0.000	0					0.00
33.417	0.00	0.00	0.000	0					0.00
33.433	0.00	0.00	0.000	0					0.00
33.450	0.00	0.00	0.000	0					0.00
33.467	0.00	0.00	0.000	0					0.00
33.483	0.00	0.00	0.000	0					0.00
33.500	0.00	0.00	0.000	0					0.00
33.517	0.00	0.00	0.000	0					0.00
33.533	0.00	0.00	0.000	0					0.00
33.550	0.00	0.00	0.000	0					0.00
33.567	0.00	0.00	0.000	0					0.00
33.583	0.00	0.00	0.000	0					0.00
33.600	0.00	0.00	0.000	0					0.00
33.617	0.00	0.00	0.000	0					0.00
33.633	0.00	0.00	0.000	0					0.00
33.650	0.00	0.00	0.000	0					0.00
33.667	0.00	0.00	0.000	0					0.00
33.683	0.00	0.00	0.000	0					0.00
33.700	0.00	0.00	0.000	0					0.00
33.717	0.00	0.00	0.000	0					0.00
33.733	0.00	0.00	0.000	0					0.00
33.750	0.00	0.00	0.000	0					0.00
33.767	0.00	0.00	0.000	0					0.00

33.783	0.00	0.00	0.000	0					0.00
33.800	0.00	0.00	0.000	0					0.00
33.817	0.00	0.00	0.000	0					0.00
33.833	0.00	0.00	0.000	0					0.00
33.850	0.00	0.00	0.000	0					0.00
33.867	0.00	0.00	0.000	0					0.00
33.883	0.00	0.00	0.000	0					0.00
33.900	0.00	0.00	0.000	0					0.00
33.917	0.00	0.00	0.000	0					0.00
33.933	0.00	0.00	0.000	0					0.00
33.950	0.00	0.00	0.000	0					0.00
33.967	0.00	0.00	0.000	0					0.00
33.983	0.00	0.00	0.000	0					0.00
34.000	0.00	0.00	0.000	0					0.00
34.017	0.00	0.00	0.000	0					0.00
34.033	0.00	0.00	0.000	0					0.00
34.050	0.00	0.00	0.000	0					0.00
34.067	0.00	0.00	0.000	0					0.00
34.083	0.00	0.00	0.000	0					0.00
34.100	0.00	0.00	0.000	0					0.00
34.117	0.00	0.00	0.000	0					0.00
34.133	0.00	0.00	0.000	0					0.00
34.150	0.00	0.00	0.000	0					0.00
34.167	0.00	0.00	0.000	0					0.00
34.183	0.00	0.00	0.000	0					0.00
34.200	0.00	0.00	0.000	0					0.00
34.217	0.00	0.00	0.000	0					0.00
34.233	0.00	0.00	0.000	0					0.00
34.250	0.00	0.00	0.000	0					0.00
34.267	0.00	0.00	0.000	0					0.00
34.283	0.00	0.00	0.000	0					0.00
34.300	0.00	0.00	0.000	0					0.00
34.317	0.00	0.00	0.000	0					0.00
34.333	0.00	0.00	0.000	0					0.00
34.350	0.00	0.00	0.000	0					0.00
34.367	0.00	0.00	0.000	0					0.00
34.383	0.00	0.00	0.000	0					0.00
34.400	0.00	0.00	0.000	0					0.00
34.417	0.00	0.00	0.000	0					0.00
34.433	0.00	0.00	0.000	0					0.00
34.450	0.00	0.00	0.000	0					0.00
34.467	0.00	0.00	0.000	0					0.00
34.483	0.00	0.00	0.000	0					0.00
34.500	0.00	0.00	0.000	0					0.00
34.517	0.00	0.00	0.000	0					0.00
34.533	0.00	0.00	0.000	0					0.00
34.550	0.00	0.00	0.000	0					0.00

*****HYDROGRAPH DATA*****

Number of intervals = 2073

```

Time interval = 1.0 (Min.)
Maximum/Peak flow rate = 16.022 (CFS)
Total volume = 2.141 (Ac.Ft)
Status of hydrographs being held in storage
Stream 1 Stream 2 Stream 3 Stream 4 Stream 5
Peak (CFS) 0.000 0.000 0.000 0.000 0.000
Vol (Ac.Ft) 0.000 0.000 0.000 0.000 0.000
*****

```

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 06/21/21

108-001 Idaho (Escondido Estates)
Onsite Proposed Condition 501-507
Flow through Drainage Course
Extrapolated storm from County Isopluvials

***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:
6 hour, precipitation(inches) = 3.300
24 hour precipitation(inches) = 7.300
P6/P24 = 45.2%
San Diego hydrology manual 'C' values used

+++++
Process from Point/Station 501.000 to Point/Station 502.000
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[COMMERCIAL area type]
(Neighborhood Commercial)
Impervious value, Ai = 0.800
Sub-Area C Value = 0.780
Initial subarea total flow distance = 85.000(Ft.)
Highest elevation = 710.100(Ft.)
Lowest elevation = 707.000(Ft.)
Elevation difference = 3.100(Ft.) Slope = 3.647 %
INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:

The maximum overland flow distance is 85.00 (Ft)
for the top area slope value of 3.65 %, in a development type of
Neighborhood Commercial

In Accordance With Figure 3-3

Initial Area Time of Concentration = 3.45 minutes

$TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$

$TC = [1.8 * (1.1 - 0.7800) * (85.000^{.5}) / (3.647^{(1/3)})] = 3.45$

Calculated TC of 3.450 minutes is less than 5 minutes,

resetting TC to 5.0 minutes for rainfall intensity calculations

Rainfall intensity (I) = 8.695(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.780

Subarea runoff = 0.203(CFS)

Total initial stream area = 0.030(Ac.)

+++++
Process from Point/Station 502.000 to Point/Station 503.000
**** STREET FLOW TRAVEL TIME + SUBAREA FLOW ADDITION ****

Top of street segment elevation = 707.000(Ft.)

End of street segment elevation = 704.000(Ft.)

Length of street segment = 212.000(Ft.)

Height of curb above gutter flowline = 6.0(In.)

Width of half street (curb to crown) = 21.500(Ft.)

Distance from crown to crossfall grade break = 20.000(Ft.)

Slope from gutter to grade break (v/hz) = 0.020

Slope from grade break to crown (v/hz) = 0.020

Street flow is on [2] side(s) of the street

Distance from curb to property line = 10.000(Ft.)

Slope from curb to property line (v/hz) = 0.020

Gutter width = 1.500(Ft.)

Gutter hike from flowline = 1.500(In.)

Manning's N in gutter = 0.0120

Manning's N from gutter to grade break = 0.0120

Manning's N from grade break to crown = 0.0120

Estimated mean flow rate at midpoint of street = 1.047(CFS)

Depth of flow = 0.178(Ft.), Average velocity = 2.159(Ft/s)

Streetflow hydraulics at midpoint of street travel:

Halfstreet flow width = 4.138(Ft.)

Flow velocity = 2.16(Ft/s)

Travel time = 1.64 min. TC = 5.09 min.

Adding area flow to street

Rainfall intensity (I) = 8.599(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[COMMERCIAL area type]

(Neighborhood Commercial)

Impervious value, Ai = 0.800

Sub-Area C Value = 0.780
 Rainfall intensity = 8.599(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.780 CA = 0.234
 Subarea runoff = 1.809(CFS) for 0.270(Ac.)
 Total runoff = 2.012(CFS) Total area = 0.300(Ac.)
 Street flow at end of street = 2.012(CFS)
 Half street flow at end of street = 1.006(CFS)
 Depth of flow = 0.211(Ft.), Average velocity = 2.452(Ft/s)
 Flow width (from curb towards crown)= 5.822(Ft.)

++++++
 Process from Point/Station 503.000 to Point/Station 504.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 700.000(Ft.)
 Downstream point/station elevation = 697.500(Ft.)
 Pipe length = 53.00(Ft.) Slope = 0.0472 Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 2.012(CFS)
 Nearest computed pipe diameter = 9.00(In.)
 Calculated individual pipe flow = 2.012(CFS)
 Normal flow depth in pipe = 4.82(In.)
 Flow top width inside pipe = 8.98(In.)
 Critical Depth = 7.73(In.)
 Pipe flow velocity = 8.36(Ft/s)
 Travel time through pipe = 0.11 min.
 Time of concentration (TC) = 5.19 min.

++++++
 Process from Point/Station 503.000 to Point/Station 504.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 0.300(Ac.)
 Runoff from this stream = 2.012(CFS)
 Time of concentration = 5.19 min.
 Rainfall intensity = 8.486(In/Hr)
 Program is now starting with Main Stream No. 2

++++++
 Process from Point/Station 504.000 to Point/Station 504.000
 **** USER DEFINED FLOW INFORMATION AT A POINT ****

User specified 'C' value of 0.300 given for subarea
 Rainfall intensity (I) = 3.943(In/Hr) for a 100.0 year storm
 User specified values are as follows:

TC = 17.04 min. Rain intensity = 3.94(In/Hr)
 Total area = 57.330(Ac.) Total runoff = 40.932(CFS)

+++++
 Process from Point/Station 504.000 to Point/Station 504.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 57.330(Ac.)
 Runoff from this stream = 40.932(CFS)
 Time of concentration = 17.04 min.
 Rainfall intensity = 3.943(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	2.012	5.19	8.486
2	40.932	17.04	3.943

Qmax(1) =
 1.000 * 1.000 * 2.012) +
 1.000 * 0.305 * 40.932) + = 14.484

Qmax(2) =
 0.465 * 1.000 * 2.012) +
 1.000 * 1.000 * 40.932) + = 41.867

Total of 2 main streams to confluence:

Flow rates before confluence point:

2.012 40.932

Maximum flow rates at confluence using above data:

14.484 41.867

Area of streams before confluence:

0.300 57.330

Results of confluence:

Total flow rate = 41.867(CFS)

Time of concentration = 17.040 min.

Effective stream area after confluence = 57.630(Ac.)

+++++
 Process from Point/Station 504.000 to Point/Station 506.000
 **** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

Estimated mean flow rate at midpoint of channel = 53.289(CFS)

Depth of flow = 0.866(Ft.), Average velocity = 2.843(Ft/s)

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	25.00	0.00
3	50.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow	=	53.289(CFS)
'	'	flow top width = 43.294(Ft.)
'	'	velocity= 2.843(Ft/s)
'	'	area = 18.744(Sq.Ft)
'	'	Froude number = 0.761

Upstream point elevation = 697.500(Ft.)

Downstream point elevation = 690.000(Ft.)

Flow length = 419.000(Ft.)

Travel time = 2.46 min.

Time of concentration = 19.50 min.

Depth of flow = 0.866(Ft.)

Average velocity = 2.843(Ft/s)

Total irregular channel flow = 53.289(CFS)

Irregular channel normal depth above invert elev. = 0.866(Ft.)

Average velocity of channel(s) = 2.843(Ft/s)

Adding area flow to channel

Rainfall intensity (I) = 3.615(In/Hr) for a 100.0 year storm

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 1.000

Decimal fraction soil group D = 0.000

[UNDISTURBED NATURAL TERRAIN]

(Permanent Open Space)

Impervious value, Ai = 0.000

Sub-Area C Value = 0.300

Rainfall intensity = 3.615(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for total area

(Q=KCIA) is C = 0.302 CA = 17.928

Subarea runoff = 22.937(CFS) for 1.650(Ac.)

Total runoff = 64.804(CFS) Total area = 59.280(Ac.)

Depth of flow = 0.932(Ft.), Average velocity = 2.985(Ft/s)

+++++

Process from Point/Station 504.000 to Point/Station 506.000

**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 59.280(Ac.)

Runoff from this stream = 64.804(CFS)
 Time of concentration = 19.50 min.
 Rainfall intensity = 3.615(In/Hr)
 Program is now starting with Main Stream No. 2

+++++
 Process from Point/Station 501.000 to Point/Station 505.000
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [COMMERCIAL area type]
 (Neighborhood Commercial)
 Impervious value, Ai = 0.800
 Sub-Area C Value = 0.780
 Initial subarea total flow distance = 93.000(Ft.)
 Highest elevation = 710.100(Ft.)
 Lowest elevation = 706.000(Ft.)
 Elevation difference = 4.100(Ft.) Slope = 4.409 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 95.00 (Ft)
 for the top area slope value of 4.41 %, in a development type of
 Neighborhood Commercial
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 3.42 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5} / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.7800) * (95.000^{.5}) / (4.409^{(1/3)})] = 3.42$
 Calculated TC of 3.424 minutes is less than 5 minutes,
 resetting TC to 5.0 minutes for rainfall intensity calculations
 Rainfall intensity (I) = 8.695(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.780
 Subarea runoff = 0.407(CFS)
 Total initial stream area = 0.060(Ac.)

+++++
 Process from Point/Station 505.000 to Point/Station 506.000
 **** STREET FLOW TRAVEL TIME + SUBAREA FLOW ADDITION ****

Top of street segment elevation = 706.000(Ft.)
 End of street segment elevation = 690.000(Ft.)
 Length of street segment = 461.000(Ft.)
 Height of curb above gutter flowline = 6.0(In.)
 Width of half street (curb to crown) = 21.500(Ft.)
 Distance from crown to crossfall grade break = 20.000(Ft.)
 Slope from gutter to grade break (v/hz) = 0.020
 Slope from grade break to crown (v/hz) = 0.020

Street flow is on [2] side(s) of the street
 Distance from curb to property line = 10.000(Ft.)
 Slope from curb to property line (v/hz) = 0.020
 Gutter width = 1.500(Ft.)
 Gutter hike from flowline = 1.500(In.)
 Manning's N in gutter = 0.0120
 Manning's N from gutter to grade break = 0.0120
 Manning's N from grade break to crown = 0.0120
 Estimated mean flow rate at midpoint of street = 2.449(CFS)
 Depth of flow = 0.198(Ft.), Average velocity = 3.645(Ft/s)
 Streetflow hydraulics at midpoint of street travel:
 Halfstreet flow width = 5.144(Ft.)
 Flow velocity = 3.64(Ft/s)
 Travel time = 2.11 min. TC = 5.53 min.
 Adding area flow to street
 Rainfall intensity (I) = 8.146(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [COMMERCIAL area type]
 (Neighborhood Commercial)
 Impervious value, Ai = 0.800
 Sub-Area C Value = 0.780
 Rainfall intensity = 8.146(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.780 CA = 0.569
 Subarea runoff = 4.231(CFS) for 0.670(Ac.)
 Total runoff = 4.638(CFS) Total area = 0.730(Ac.)
 Street flow at end of street = 4.638(CFS)
 Half street flow at end of street = 2.319(CFS)
 Depth of flow = 0.234(Ft.), Average velocity = 4.180(Ft/s)
 Flow width (from curb towards crown)= 6.954(Ft.)

++++++
 Process from Point/Station 505.000 to Point/Station 506.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 0.730(Ac.)
 Runoff from this stream = 4.638(CFS)
 Time of concentration = 5.53 min.
 Rainfall intensity = 8.146(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	64.804	19.50	3.615
2	4.638	5.53	8.146

Qmax(1) =

1.000 *	1.000 *	64.804) +	
0.444 *	1.000 *	4.638) + =	66.862

Qmax(2) =

1.000 *	0.284 *	64.804) +	
1.000 *	1.000 *	4.638) + =	23.025

Total of 2 main streams to confluence:

Flow rates before confluence point:

64.804	4.638
--------	-------

Maximum flow rates at confluence using above data:

66.862	23.025
--------	--------

Area of streams before confluence:

59.280	0.730
--------	-------

Results of confluence:

Total flow rate = 66.862(CFS)

Time of concentration = 19.496 min.

Effective stream area after confluence = 60.010(Ac.)

+++++

Process from Point/Station 506.000 to Point/Station 507.000

**** IRREGULAR CHANNEL FLOW TRAVEL TIME ****

!!Warning: Water is above left or right bank elevations

!!Warning: Water is above left or right bank elevations

!!Warning: Water is above left or right bank elevations

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Estimated mean flow rate at midpoint of channel = 66.910(CFS)

Depth of flow = 1.028(Ft.), Average velocity = 3.167(Ft/s)

!!Warning: Water is above left or right bank elevations

***** Irregular Channel Data *****

Information entered for subchannel number 1 :

Point number	'X' coordinate	'Y' coordinate
1	0.00	1.00
2	20.00	0.00
3	40.00	1.00

Manning's 'N' friction factor = 0.040

Sub-Channel flow = 66.911(CFS)

'	'	flow top width =	40.000(Ft.)
'	'	velocity=	3.167(Ft/s)
'	'	area =	21.129(Sq.Ft)
'	'	Froude number =	0.768

Upstream point elevation = 690.000(Ft.)
 Downstream point elevation = 687.000(Ft.)
 Flow length = 176.000(Ft.)
 Travel time = 0.93 min.
 Time of concentration = 20.42 min.
 Depth of flow = 1.028(Ft.)
 Average velocity = 3.167(Ft/s)
 Total irregular channel flow = 66.910(CFS)
 Irregular channel normal depth above invert elev. = 1.028(Ft.)
 Average velocity of channel(s) = 3.167(Ft/s)
 !!Warning: Water is above left or right bank elevations
 Adding area flow to channel
 Rainfall intensity (I) = 3.508(In/Hr) for a 100.0 year storm
 Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [UNDISTURBED NATURAL TERRAIN]
 (Permanent Open Space)
 Impervious value, Ai = 0.000
 Sub-Area C Value = 0.300
 The area added to the existing stream causes a
 a lower flow rate of Q = 65.626(CFS)
 therefore the upstream flow rate of Q = 66.862(CFS) is being used
 Rainfall intensity = 3.508(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for total area
 (Q=KCIA) is C = 0.308 CA = 18.707
 Subarea runoff = 0.000(CFS) for 0.700(Ac.)
 Total runoff = 66.862(CFS) Total area = 60.710(Ac.)
 Depth of flow = 1.028(Ft.), Average velocity = 3.166(Ft/s)
 !!Warning: Water is above left or right bank elevations

++++++
 Process from Point/Station 506.000 to Point/Station 507.000
 **** 6 HOUR HYDROGRAPH ****

++++++
 Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 20.42
 Basin Area = 60.71 Acres
 6 Hour Rainfall = 3.300 Inches
 Runoff Coefficient = 0.308
 Peak Discharge = 66.86 CFS

Time (Min)	Discharge (CFS)
0	0.000
20	3.728

40	3.872
60	4.209
80	4.408
100	4.888
120	5.184
140	5.942
160	6.443
180	7.875
200	8.970
220	13.171
240	18.557
260	66.862
280	10.564
300	7.068
320	5.530
340	4.632
360	4.032
380	3.597

+++++

6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	16.7	33.4	50.1	66.9
0+ 0	0.0000		0.00	Q				
0+ 1	0.0003		0.19	Q				
0+ 2	0.0008		0.37	Q				
0+ 3	0.0015		0.56	Q				
0+ 4	0.0026		0.75	Q				
0+ 5	0.0039		0.93	Q				
0+ 6	0.0054		1.12	Q				
0+ 7	0.0072		1.30	Q				
0+ 8	0.0092		1.49	Q				
0+ 9	0.0116		1.68	VQ				
0+10	0.0141		1.86	VQ				
0+11	0.0169		2.05	VQ				
0+12	0.0200		2.24	VQ				
0+13	0.0234		2.42	VQ				
0+14	0.0270		2.61	VQ				
0+15	0.0308		2.80	VQ				
0+16	0.0349		2.98	VQ				
0+17	0.0393		3.17	VQ				
0+18	0.0439		3.36	V Q				
0+19	0.0488		3.54	V Q				
0+20	0.0539		3.73	V Q				
0+21	0.0591		3.74	V Q				
0+22	0.0642		3.74	V Q				

0+23	0.0694	3.75	V Q				
0+24	0.0746	3.76	V Q				
0+25	0.0797	3.76	V Q				
0+26	0.0849	3.77	V Q				
0+27	0.0901	3.78	V Q				
0+28	0.0954	3.79	V Q				
0+29	0.1006	3.79	V Q				
0+30	0.1058	3.80	V Q				
0+31	0.1111	3.81	V Q				
0+32	0.1163	3.81	V Q				
0+33	0.1216	3.82	V Q				
0+34	0.1268	3.83	V Q				
0+35	0.1321	3.84	VQ				
0+36	0.1374	3.84	VQ				
0+37	0.1427	3.85	VQ				
0+38	0.1480	3.86	VQ				
0+39	0.1534	3.86	VQ				
0+40	0.1587	3.87	VQ				
0+41	0.1641	3.89	VQ				
0+42	0.1694	3.91	VQ				
0+43	0.1748	3.92	VQ				
0+44	0.1803	3.94	VQ				
0+45	0.1857	3.96	VQ				
0+46	0.1912	3.97	VQ				
0+47	0.1967	3.99	VQ				
0+48	0.2022	4.01	VQ				
0+49	0.2077	4.02	VQ				
0+50	0.2133	4.04	VQ				
0+51	0.2189	4.06	VQ				
0+52	0.2245	4.07	VQ				
0+53	0.2301	4.09	VQ				
0+54	0.2358	4.11	VQ				
0+55	0.2415	4.12	VQ				
0+56	0.2472	4.14	VQ				
0+57	0.2529	4.16	VQ				
0+58	0.2587	4.18	VQ				
0+59	0.2644	4.19	Q				
1+ 0	0.2702	4.21	Q				
1+ 1	0.2761	4.22	Q				
1+ 2	0.2819	4.23	Q				
1+ 3	0.2877	4.24	Q				
1+ 4	0.2936	4.25	Q				
1+ 5	0.2994	4.26	Q				
1+ 6	0.3053	4.27	Q				
1+ 7	0.3112	4.28	Q				
1+ 8	0.3171	4.29	Q				
1+ 9	0.3230	4.30	Q				
1+10	0.3290	4.31	Q				
1+11	0.3349	4.32	Q				
1+12	0.3409	4.33	Q				

1+13	0.3469	4.34	Q				
1+14	0.3528	4.35	Q				
1+15	0.3588	4.36	Q				
1+16	0.3649	4.37	Q				
1+17	0.3709	4.38	Q				
1+18	0.3769	4.39	Q				
1+19	0.3830	4.40	Q				
1+20	0.3891	4.41	QV				
1+21	0.3952	4.43	QV				
1+22	0.4013	4.46	QV				
1+23	0.4075	4.48	QV				
1+24	0.4137	4.50	QV				
1+25	0.4199	4.53	QV				
1+26	0.4262	4.55	QV				
1+27	0.4325	4.58	QV				
1+28	0.4388	4.60	QV				
1+29	0.4452	4.62	QV				
1+30	0.4516	4.65	QV				
1+31	0.4580	4.67	QV				
1+32	0.4645	4.70	QV				
1+33	0.4710	4.72	QV				
1+34	0.4775	4.74	QV				
1+35	0.4841	4.77	QV				
1+36	0.4907	4.79	QV				
1+37	0.4973	4.82	QV				
1+38	0.5040	4.84	QV				
1+39	0.5107	4.86	QV				
1+40	0.5174	4.89	Q V				
1+41	0.5242	4.90	Q V				
1+42	0.5310	4.92	Q V				
1+43	0.5378	4.93	Q V				
1+44	0.5446	4.95	Q V				
1+45	0.5514	4.96	Q V				
1+46	0.5583	4.98	Q V				
1+47	0.5651	4.99	Q V				
1+48	0.5720	5.01	Q V				
1+49	0.5790	5.02	QV				
1+50	0.5859	5.04	QV				
1+51	0.5928	5.05	QV				
1+52	0.5998	5.07	QV				
1+53	0.6068	5.08	QV				
1+54	0.6138	5.10	QV				
1+55	0.6209	5.11	QV				
1+56	0.6279	5.12	QV				
1+57	0.6350	5.14	QV				
1+58	0.6421	5.15	QV				
1+59	0.6492	5.17	Q V				
2+ 0	0.6564	5.18	Q V				
2+ 1	0.6636	5.22	Q V				
2+ 2	0.6708	5.26	Q V				

2+ 3	0.6781	5.30	Q V				
2+ 4	0.6855	5.34	Q V				
2+ 5	0.6929	5.37	Q V				
2+ 6	0.7003	5.41	Q V				
2+ 7	0.7078	5.45	Q V				
2+ 8	0.7154	5.49	Q V				
2+ 9	0.7230	5.53	Q V				
2+10	0.7307	5.56	Q V				
2+11	0.7384	5.60	Q V				
2+12	0.7461	5.64	Q V				
2+13	0.7540	5.68	Q V				
2+14	0.7618	5.71	Q V				
2+15	0.7698	5.75	Q V				
2+16	0.7777	5.79	Q V				
2+17	0.7858	5.83	Q V				
2+18	0.7938	5.87	Q V				
2+19	0.8020	5.90	Q V				
2+20	0.8102	5.94	Q V				
2+21	0.8184	5.97	Q V				
2+22	0.8266	5.99	Q V				
2+23	0.8349	6.02	Q V				
2+24	0.8432	6.04	Q V				
2+25	0.8516	6.07	Q V				
2+26	0.8600	6.09	Q V				
2+27	0.8684	6.12	Q V				
2+28	0.8769	6.14	Q V				
2+29	0.8854	6.17	Q V				
2+30	0.8939	6.19	Q V				
2+31	0.9025	6.22	Q V				
2+32	0.9111	6.24	Q V				
2+33	0.9197	6.27	Q V				
2+34	0.9284	6.29	Q V				
2+35	0.9371	6.32	Q V				
2+36	0.9458	6.34	Q V				
2+37	0.9546	6.37	Q V				
2+38	0.9634	6.39	Q V				
2+39	0.9722	6.42	Q V				
2+40	0.9811	6.44	Q V				
2+41	0.9901	6.51	Q V				
2+42	0.9991	6.59	Q V				
2+43	1.0083	6.66	Q V				
2+44	1.0176	6.73	Q V				
2+45	1.0270	6.80	Q V				
2+46	1.0364	6.87	Q V				
2+47	1.0460	6.94	Q V				
2+48	1.0557	7.02	Q V				
2+49	1.0654	7.09	Q V				
2+50	1.0753	7.16	Q V				
2+51	1.0852	7.23	Q V				
2+52	1.0953	7.30	Q V				

2+53	1.1054	7.37	Q	V				
2+54	1.1157	7.45	Q	V				
2+55	1.1261	7.52	Q	V				
2+56	1.1365	7.59	Q	V				
2+57	1.1471	7.66	Q	V				
2+58	1.1577	7.73	Q	V				
2+59	1.1685	7.80	Q	V				
3+ 0	1.1793	7.88	Q	V				
3+ 1	1.1902	7.93	Q	V				
3+ 2	1.2012	7.98	Q	V				
3+ 3	1.2123	8.04	Q	V				
3+ 4	1.2235	8.09	Q	V				
3+ 5	1.2347	8.15	Q	V				
3+ 6	1.2460	8.20	Q	V				
3+ 7	1.2574	8.26	Q	V				
3+ 8	1.2688	8.31	Q	V				
3+ 9	1.2803	8.37	Q	V				
3+10	1.2919	8.42	Q	V				
3+11	1.3036	8.48	Q	V				
3+12	1.3154	8.53	Q	V				
3+13	1.3272	8.59	Q	V				
3+14	1.3391	8.64	Q	V				
3+15	1.3511	8.70	Q	V				
3+16	1.3631	8.75	Q	V				
3+17	1.3753	8.81	Q	V				
3+18	1.3875	8.86	Q	V				
3+19	1.3997	8.92	Q	V				
3+20	1.4121	8.97	Q	V				
3+21	1.4247	9.18	Q	V				
3+22	1.4377	9.39	Q	V				
3+23	1.4509	9.60	Q	V				
3+24	1.4644	9.81	Q	V				
3+25	1.4782	10.02	Q	V				
3+26	1.4923	10.23	Q	V				
3+27	1.5067	10.44	Q	V				
3+28	1.5214	10.65	Q	V				
3+29	1.5363	10.86	Q	V				
3+30	1.5516	11.07	Q	V				
3+31	1.5671	11.28	Q	V				
3+32	1.5829	11.49	Q	V				
3+33	1.5990	11.70	Q	V				
3+34	1.6154	11.91	Q	V				
3+35	1.6321	12.12	Q	V				
3+36	1.6491	12.33	Q	V				
3+37	1.6664	12.54	Q	V				
3+38	1.6840	12.75	Q	V				
3+39	1.7018	12.96	Q	V				
3+40	1.7200	13.17	Q	V				
3+41	1.7385	13.44	Q	V				
3+42	1.7574	13.71	Q	V				

3+43	1.7766	13.98	Q	V				
3+44	1.7962	14.25	Q	V				
3+45	1.8162	14.52	Q	V				
3+46	1.8366	14.79	Q	V				
3+47	1.8573	15.06	Q	V				
3+48	1.8784	15.33	Q	V				
3+49	1.8999	15.59	Q	V				
3+50	1.9218	15.86	Q	V				
3+51	1.9440	16.13	Q	V				
3+52	1.9666	16.40	Q	V				
3+53	1.9896	16.67	Q	V				
3+54	2.0129	16.94	Q	V				
3+55	2.0366	17.21	Q	V				
3+56	2.0607	17.48	Q	V				
3+57	2.0851	17.75	Q	V				
3+58	2.1099	18.02	Q	V				
3+59	2.1351	18.29	Q	V				
4+ 0	2.1607	18.56	Q	V				
4+ 1	2.1896	20.97	Q	V				
4+ 2	2.2218	23.39	Q	V				
4+ 3	2.2573	25.80	Q	V				
4+ 4	2.2962	28.22	Q	V				
4+ 5	2.3384	30.63	Q	V				
4+ 6	2.3839	33.05	Q	V				
4+ 7	2.4328	35.46	Q	V				
4+ 8	2.4850	37.88	Q	V				
4+ 9	2.5405	40.29	Q	V				
4+10	2.5993	42.71	Q	V				
4+11	2.6614	45.12	Q	V				
4+12	2.7269	47.54	Q	V				
4+13	2.7957	49.96	Q	V				
4+14	2.8679	52.37	Q	V				
4+15	2.9433	54.79	Q	V				
4+16	3.0221	57.20	Q	V				
4+17	3.1042	59.62	Q	V				
4+18	3.1897	62.03	Q	V				
4+19	3.2784	64.45	Q	V				
4+20	3.3705	66.86	Q	V				
4+21	3.4588	64.05	Q	V				
4+22	3.5431	61.23	Q	V				
4+23	3.6236	58.42	Q	V				
4+24	3.7001	55.60	Q	V				
4+25	3.7729	52.79	Q	V				
4+26	3.8417	49.97	Q	V				
4+27	3.9066	47.16	Q	V				
4+28	3.9677	44.34	Q	V				
4+29	4.0249	41.53	Q	V				
4+30	4.0782	38.71	Q	V				
4+31	4.1277	35.90	Q	V				
4+32	4.1733	33.08	Q	V				

4+33	4.2150	30.27			Q	V
4+34	4.2528	27.45			Q	V
4+35	4.2867	24.64			Q	V
4+36	4.3168	21.82			Q	V
4+37	4.3429	19.01		Q		V
4+38	4.3653	16.19		Q		V
4+39	4.3837	13.38		Q		V
4+40	4.3982	10.56	Q			V
4+41	4.4125	10.39	Q			V
4+42	4.4266	10.21	Q			V
4+43	4.4404	10.04	Q			V
4+44	4.4540	9.86	Q			V
4+45	4.4674	9.69	Q			V
4+46	4.4805	9.52	Q			V
4+47	4.4933	9.34	Q			V
4+48	4.5060	9.17	Q			V
4+49	4.5184	8.99	Q			V
4+50	4.5305	8.82	Q			V
4+51	4.5424	8.64	Q			V
4+52	4.5541	8.47	Q			V
4+53	4.5655	8.29	Q			V
4+54	4.5767	8.12	Q			V
4+55	4.5876	7.94	Q			V
4+56	4.5983	7.77	Q			V
4+57	4.6088	7.59	Q			V
4+58	4.6190	7.42	Q			V
4+59	4.6290	7.24	Q			V
5+ 0	4.6387	7.07	Q			V
5+ 1	4.6483	6.99	Q			V
5+ 2	4.6578	6.91	Q			V
5+ 3	4.6673	6.84	Q			V
5+ 4	4.6766	6.76	Q			V
5+ 5	4.6858	6.68	Q			V
5+ 6	4.6949	6.61	Q			V
5+ 7	4.7039	6.53	Q			V
5+ 8	4.7128	6.45	Q			V
5+ 9	4.7215	6.38	Q			V
5+10	4.7302	6.30	Q			V
5+11	4.7388	6.22	Q			V
5+12	4.7473	6.15	Q			V
5+13	4.7556	6.07	Q			V
5+14	4.7639	5.99	Q			V
5+15	4.7720	5.91	Q			V
5+16	4.7801	5.84	Q			V
5+17	4.7880	5.76	Q			V
5+18	4.7958	5.68	Q			V
5+19	4.8035	5.61	Q			V
5+20	4.8112	5.53	Q			V
5+21	4.8187	5.49	Q			V
5+22	4.8262	5.44	Q			V

5+23	4.8336	5.40	Q				V
5+24	4.8410	5.35	Q				V
5+25	4.8483	5.31	Q				V
5+26	4.8556	5.26	Q				V
5+27	4.8627	5.22	Q				V
5+28	4.8699	5.17	Q				V
5+29	4.8769	5.13	Q				V
5+30	4.8839	5.08	Q				V
5+31	4.8909	5.04	Q				V
5+32	4.8977	4.99	Q				V
5+33	4.9046	4.95	Q				V
5+34	4.9113	4.90	Q				V
5+35	4.9180	4.86	Q				V
5+36	4.9246	4.81	Q				V
5+37	4.9312	4.77	Q				V
5+38	4.9377	4.72	Q				V
5+39	4.9441	4.68	Q				V
5+40	4.9505	4.63	Q				V
5+41	4.9569	4.60	Q				V
5+42	4.9632	4.57	Q				V
5+43	4.9694	4.54	Q				V
5+44	4.9756	4.51	Q				V
5+45	4.9818	4.48	Q				V
5+46	4.9879	4.45	Q				V
5+47	4.9940	4.42	Q				V
5+48	5.0001	4.39	Q				V
5+49	5.0061	4.36	Q				V
5+50	5.0120	4.33	Q				V
5+51	5.0180	4.30	Q				V
5+52	5.0239	4.27	Q				V
5+53	5.0297	4.24	Q				V
5+54	5.0355	4.21	Q				V
5+55	5.0413	4.18	Q				V
5+56	5.0470	4.15	Q				V
5+57	5.0527	4.12	Q				V
5+58	5.0583	4.09	Q				V
5+59	5.0639	4.06	Q				V
6+ 0	5.0694	4.03	Q				V
6+ 1	5.0750	4.01	Q				V
6+ 2	5.0805	3.99	Q				V
6+ 3	5.0859	3.97	Q				V
6+ 4	5.0913	3.94	Q				V
6+ 5	5.0968	3.92	Q				V
6+ 6	5.1021	3.90	Q				V
6+ 7	5.1075	3.88	Q				V
6+ 8	5.1128	3.86	Q				V
6+ 9	5.1181	3.84	Q				V
6+10	5.1233	3.81	Q				V
6+11	5.1285	3.79	Q				V
6+12	5.1337	3.77	Q				V

6+13	5.1389	3.75	Q				V
6+14	5.1440	3.73	Q				V
6+15	5.1491	3.71	Q				V
6+16	5.1542	3.68	Q				V
6+17	5.1593	3.66	Q				V
6+18	5.1643	3.64	Q				V
6+19	5.1693	3.62	Q				V
6+20	5.1742	3.60	Q				V

End of computations, total study area = 60.710 (Ac.)

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software,(c)1991-2018 Version 9.0

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 06/21/21

108-001 Idaho (Escondido Estates)
Proposed Discharge at Existing CalTrans Culvert
Extrapolated Storm from County Isopluvials
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***** Hydrology Study Control Information *****

Program License Serial Number 6440

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:
6 hour, precipitation(inches) = 3.300
24 hour precipitation(inches) = 7.300
P6/P24 = 45.2%
San Diego hydrology manual 'C' values used

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Process from Point/Station 403.000 to Point/Station 507.000
**** USER DEFINED FLOW INFORMATION AT A POINT ****

User specified 'C' value of 0.527 given for subarea
Rainfall intensity (I) = 7.064(In/Hr) for a 100.0 year storm
User specified values are as follows:
TC = 6.90 min. Rain intensity = 7.06(In/Hr)
Total area = 13.270(Ac.) Total runoff = 16.022(CFS)

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Process from Point/Station 507.000 to Point/Station 507.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
In Main Stream number: 1
Stream flow area = 13.270(Ac.)

Runoff from this stream = 16.022(CFS)
 Time of concentration = 6.90 min.
 Rainfall intensity = 7.064(In/Hr)
 Program is now starting with Main Stream No. 2

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 Process from Point/Station 507.000 to Point/Station 507.000
 **** USER DEFINED FLOW INFORMATION AT A POINT ****

User specified 'C' value of 0.308 given for subarea
 Rainfall intensity (I) = 3.508(In/Hr) for a 100.0 year storm
 User specified values are as follows:
 TC = 20.42 min. Rain intensity = 3.51(In/Hr)
 Total area = 60.710(Ac.) Total runoff = 66.860(CFS)

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 Process from Point/Station 507.000 to Point/Station 507.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 60.710(Ac.)
 Runoff from this stream = 66.860(CFS)
 Time of concentration = 20.42 min.
 Rainfall intensity = 3.508(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	16.022	6.90	7.064
2	66.860	20.42	3.508

Qmax(1) =

1.000 *	1.000 *	16.022)	+	
1.000 *	0.338 *	66.860)	+	38.614

Qmax(2) =

0.497 *	1.000 *	16.022)	+	
1.000 *	1.000 *	66.860)	+	74.818

Total of 2 main streams to confluence:

Flow rates before confluence point:

16.022 66.860

Maximum flow rates at confluence using above data:

38.614 74.818

Area of streams before confluence:

13.270 60.710

Results of confluence:

Total flow rate = 74.818(CFS)

Time of concentration = 20.420 min.

Effective stream area after confluence = 73.980(Ac.)

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Process from Point/Station 507.000 to Point/Station 507.000

**** 6 HOUR HYDROGRAPH ****

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Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time of Concentration = 20.42

Basin Area = 73.98 Acres

6 Hour Rainfall = 3.300 Inches

Runoff Coefficient = 0.347

Peak Discharge = 74.82 CFS

Time (Min)	Discharge (CFS)
0	0.000
20	5.120
40	5.318
60	5.780
80	6.053
100	6.713
120	7.120
140	8.161
160	8.849
180	10.816
200	12.319
220	18.088
240	25.486
260	74.818
280	14.508
300	9.707
320	7.595
340	6.362
360	5.537
380	4.939

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6 - H O U R S T O R M

R u n o f f H y d r o g r a p h

Hydrograph in 1 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	18.7	37.4	56.1	74.8

0+ 0	0.0000	0.00	Q				
0+ 1	0.0004	0.26	Q				
0+ 2	0.0011	0.51	Q				
0+ 3	0.0021	0.77	Q				
0+ 4	0.0035	1.02	Q				
0+ 5	0.0053	1.28	Q				
0+ 6	0.0074	1.54	Q				
0+ 7	0.0099	1.79	Q				
0+ 8	0.0127	2.05	VQ				
0+ 9	0.0159	2.30	VQ				
0+10	0.0194	2.56	VQ				
0+11	0.0233	2.82	VQ				
0+12	0.0275	3.07	VQ				
0+13	0.0321	3.33	VQ				
0+14	0.0370	3.58	VQ				
0+15	0.0423	3.84	V Q				
0+16	0.0480	4.10	V Q				
0+17	0.0539	4.35	V Q				
0+18	0.0603	4.61	V Q				
0+19	0.0670	4.86	V Q				
0+20	0.0740	5.12	V Q				
0+21	0.0811	5.13	V Q				
0+22	0.0882	5.14	V Q				
0+23	0.0953	5.15	V Q				
0+24	0.1024	5.16	V Q				
0+25	0.1095	5.17	V Q				
0+26	0.1166	5.18	V Q				
0+27	0.1238	5.19	V Q				
0+28	0.1310	5.20	V Q				
0+29	0.1381	5.21	V Q				
0+30	0.1453	5.22	V Q				
0+31	0.1525	5.23	V Q				
0+32	0.1597	5.24	V Q				
0+33	0.1670	5.25	VQ				
0+34	0.1742	5.26	VQ				
0+35	0.1815	5.27	VQ				
0+36	0.1887	5.28	VQ				
0+37	0.1960	5.29	VQ				
0+38	0.2033	5.30	VQ				
0+39	0.2106	5.31	VQ				
0+40	0.2180	5.32	VQ				
0+41	0.2253	5.34	VQ				
0+42	0.2327	5.36	VQ				
0+43	0.2401	5.39	VQ				
0+44	0.2476	5.41	VQ				
0+45	0.2551	5.43	VQ				
0+46	0.2626	5.46	VQ				
0+47	0.2701	5.48	VQ				
0+48	0.2777	5.50	VQ				
0+49	0.2853	5.53	VQ				

0+50	0.2930	5.55	VQ				
0+51	0.3006	5.57	VQ				
0+52	0.3083	5.60	VQ				
0+53	0.3161	5.62	V Q				
0+54	0.3238	5.64	V Q				
0+55	0.3316	5.66	V Q				
0+56	0.3395	5.69	VQ				
0+57	0.3474	5.71	VQ				
0+58	0.3552	5.73	VQ				
0+59	0.3632	5.76	VQ				
1+ 0	0.3711	5.78	VQ				
1+ 1	0.3791	5.79	VQ				
1+ 2	0.3871	5.81	VQ				
1+ 3	0.3951	5.82	VQ				
1+ 4	0.4032	5.83	VQ				
1+ 5	0.4112	5.85	VQ				
1+ 6	0.4193	5.86	VQ				
1+ 7	0.4274	5.88	VQ				
1+ 8	0.4355	5.89	VQ				
1+ 9	0.4436	5.90	VQ				
1+10	0.4518	5.92	VQ				
1+11	0.4600	5.93	VQ				
1+12	0.4682	5.94	VQ				
1+13	0.4764	5.96	VQ				
1+14	0.4846	5.97	VQ				
1+15	0.4928	5.99	VQ				
1+16	0.5011	6.00	Q				
1+17	0.5094	6.01	Q				
1+18	0.5177	6.03	Q				
1+19	0.5260	6.04	Q				
1+20	0.5343	6.05	Q				
1+21	0.5427	6.09	Q				
1+22	0.5511	6.12	Q				
1+23	0.5596	6.15	Q				
1+24	0.5681	6.19	Q				
1+25	0.5767	6.22	Q				
1+26	0.5853	6.25	Q				
1+27	0.5940	6.28	Q				
1+28	0.6027	6.32	Q				
1+29	0.6114	6.35	Q				
1+30	0.6202	6.38	Q				
1+31	0.6290	6.42	Q				
1+32	0.6379	6.45	Q				
1+33	0.6469	6.48	Q				
1+34	0.6558	6.52	Q				
1+35	0.6648	6.55	QV				
1+36	0.6739	6.58	QV				
1+37	0.6830	6.61	QV				
1+38	0.6922	6.65	QV				
1+39	0.7014	6.68	QV				

1+40	0.7106	6.71	QV				
1+41	0.7199	6.73	QV				
1+42	0.7292	6.75	QV				
1+43	0.7385	6.77	QV				
1+44	0.7479	6.79	QV				
1+45	0.7573	6.81	QV				
1+46	0.7667	6.84	QV				
1+47	0.7761	6.86	QV				
1+48	0.7856	6.88	QV				
1+49	0.7951	6.90	QV				
1+50	0.8046	6.92	QV				
1+51	0.8142	6.94	QV				
1+52	0.8238	6.96	QV				
1+53	0.8334	6.98	Q V				
1+54	0.8430	7.00	Q V				
1+55	0.8527	7.02	Q V				
1+56	0.8624	7.04	Q V				
1+57	0.8721	7.06	Q V				
1+58	0.8819	7.08	Q V				
1+59	0.8916	7.10	Q V				
2+ 0	0.9014	7.12	Q V				
2+ 1	0.9113	7.17	Q V				
2+ 2	0.9213	7.22	Q V				
2+ 3	0.9313	7.28	Q V				
2+ 4	0.9414	7.33	Q V				
2+ 5	0.9516	7.38	Q V				
2+ 6	0.9618	7.43	Q V				
2+ 7	0.9721	7.48	QV				
2+ 8	0.9825	7.54	QV				
2+ 9	0.9929	7.59	QV				
2+10	1.0035	7.64	Q V				
2+11	1.0141	7.69	Q V				
2+12	1.0247	7.74	Q V				
2+13	1.0355	7.80	Q V				
2+14	1.0463	7.85	Q V				
2+15	1.0572	7.90	Q V				
2+16	1.0681	7.95	Q V				
2+17	1.0791	8.00	Q V				
2+18	1.0902	8.06	Q V				
2+19	1.1014	8.11	Q V				
2+20	1.1126	8.16	Q V				
2+21	1.1239	8.20	Q V				
2+22	1.1353	8.23	Q V				
2+23	1.1466	8.26	Q V				
2+24	1.1581	8.30	Q V				
2+25	1.1696	8.33	Q V				
2+26	1.1811	8.37	Q V				
2+27	1.1927	8.40	Q V				
2+28	1.2043	8.44	Q V				
2+29	1.2159	8.47	Q V				

2+30	1.2277	8.50	Q	V				
2+31	1.2394	8.54	Q	V				
2+32	1.2512	8.57	Q	V				
2+33	1.2631	8.61	Q	V				
2+34	1.2750	8.64	Q	V				
2+35	1.2869	8.68	Q	V				
2+36	1.2989	8.71	Q	V				
2+37	1.3110	8.75	Q	V				
2+38	1.3231	8.78	Q	V				
2+39	1.3352	8.81	Q	V				
2+40	1.3474	8.85	Q	V				
2+41	1.3597	8.95	Q	V				
2+42	1.3722	9.05	Q	V				
2+43	1.3848	9.14	Q	V				
2+44	1.3975	9.24	Q	V				
2+45	1.4104	9.34	Q	V				
2+46	1.4234	9.44	Q	V				
2+47	1.4365	9.54	Q	V				
2+48	1.4498	9.64	Q	V				
2+49	1.4632	9.73	Q	V				
2+50	1.4767	9.83	Q	V				
2+51	1.4904	9.93	Q	V				
2+52	1.5042	10.03	Q	V				
2+53	1.5182	10.13	Q	V				
2+54	1.5323	10.23	Q	V				
2+55	1.5465	10.32	Q	V				
2+56	1.5608	10.42	Q	V				
2+57	1.5753	10.52	Q	V				
2+58	1.5900	10.62	Q	V				
2+59	1.6047	10.72	Q	V				
3+ 0	1.6196	10.82	Q	V				
3+ 1	1.6346	10.89	Q	V				
3+ 2	1.6497	10.97	Q	V				
3+ 3	1.6649	11.04	Q	V				
3+ 4	1.6802	11.12	Q	V				
3+ 5	1.6957	11.19	Q	V				
3+ 6	1.7112	11.27	Q	V				
3+ 7	1.7268	11.34	Q	V				
3+ 8	1.7425	11.42	Q	V				
3+ 9	1.7584	11.49	Q	V				
3+10	1.7743	11.57	Q	V				
3+11	1.7903	11.64	Q	V				
3+12	1.8065	11.72	Q	V				
3+13	1.8227	11.79	Q	V				
3+14	1.8391	11.87	Q	V				
3+15	1.8555	11.94	Q	V				
3+16	1.8721	12.02	Q	V				
3+17	1.8887	12.09	Q	V				
3+18	1.9055	12.17	Q	V				
3+19	1.9223	12.24	Q	V				

3+20	1.9393	12.32	Q	V				
3+21	1.9567	12.61	Q	V				
3+22	1.9744	12.90	Q	V				
3+23	1.9926	13.18	Q	V				
3+24	2.0112	13.47	Q	V				
3+25	2.0301	13.76	Q	V				
3+26	2.0495	14.05	Q	V				
3+27	2.0692	14.34	Q	V				
3+28	2.0894	14.63	Q	V				
3+29	2.1099	14.92	Q	V				
3+30	2.1309	15.20	Q	V				
3+31	2.1522	15.49	Q	V				
3+32	2.1739	15.78	Q	V				
3+33	2.1961	16.07	Q	V				
3+34	2.2186	16.36	Q	V				
3+35	2.2415	16.65	Q	V				
3+36	2.2648	16.93	Q	V				
3+37	2.2886	17.22	Q	V				
3+38	2.3127	17.51	Q	V				
3+39	2.3372	17.80	Q	V				
3+40	2.3621	18.09	Q	V				
3+41	2.3875	18.46	Q	V				
3+42	2.4135	18.83	Q	V				
3+43	2.4399	19.20	Q	V				
3+44	2.4669	19.57	Q	V				
3+45	2.4943	19.94	Q	V				
3+46	2.5223	20.31	Q	V				
3+47	2.5508	20.68	Q	V				
3+48	2.5798	21.05	Q	V				
3+49	2.6093	21.42	Q	V				
3+50	2.6393	21.79	Q	V				
3+51	2.6698	22.16	Q	V				
3+52	2.7008	22.53	Q	V				
3+53	2.7324	22.90	Q	V				
3+54	2.7644	23.27	Q	V				
3+55	2.7970	23.64	Q	V				
3+56	2.8301	24.01	Q	V				
3+57	2.8636	24.38	Q	V				
3+58	2.8977	24.75	Q	V				
3+59	2.9323	25.12	Q	V				
4+ 0	2.9674	25.49	Q	V				
4+ 1	3.0059	27.95	Q	V				
4+ 2	3.0478	30.42	Q	V				
4+ 3	3.0931	32.89	Q	V				
4+ 4	3.1418	35.35	Q	V				
4+ 5	3.1939	37.82	Q	V				
4+ 6	3.2494	40.29	Q	V				
4+ 7	3.3083	42.75	Q	V				
4+ 8	3.3706	45.22	Q	V				
4+ 9	3.4362	47.69	Q	V				

4+10	3.5053	50.15			V	Q		
4+11	3.5778	52.62			V	Q		
4+12	3.6537	55.09			V	Q		
4+13	3.7329	57.55			V	Q		
4+14	3.8156	60.02			V	Q	Q	
4+15	3.9017	62.48			V	Q	Q	
4+16	3.9911	64.95			V	Q	Q	
4+17	4.0840	67.42			V	Q	Q	
4+18	4.1803	69.88			V	Q	Q	
4+19	4.2799	72.35			V	Q	Q	
4+20	4.3830	74.82			V	Q	Q	
4+21	4.4819	71.80			V	Q	Q	
4+22	4.5766	68.79			V	Q	Q	
4+23	4.6672	65.77			V	Q	Q	
4+24	4.7537	62.76			V	Q	Q	
4+25	4.8360	59.74			V	Q	Q	
4+26	4.9141	56.72			V	Q	Q	
4+27	4.9881	53.71			V	Q	Q	
4+28	5.0579	50.69			V	Q	Q	
4+29	5.1236	47.68			V	Q	Q	
4+30	5.1851	44.66			V	Q	Q	
4+31	5.2424	41.65			V	Q	Q	
4+32	5.2957	38.63			V	Q	Q	
4+33	5.3447	35.62			V	Q	Q	
4+34	5.3896	32.60			V	Q	Q	
4+35	5.4304	29.59			V	Q	Q	
4+36	5.4670	26.57			V	Q	Q	
4+37	5.4994	23.55			V	Q	Q	
4+38	5.5277	20.54			V	Q	Q	
4+39	5.5518	17.52			V	Q	Q	
4+40	5.5718	14.51	Q		V	Q	Q	
4+41	5.5915	14.27	Q		V	Q	Q	
4+42	5.6108	14.03	Q		V	Q	Q	
4+43	5.6298	13.79	Q		V	Q	Q	
4+44	5.6485	13.55	Q		V	Q	Q	
4+45	5.6668	13.31	Q		V	Q	Q	
4+46	5.6848	13.07	Q		V	Q	Q	
4+47	5.7025	12.83	Q		V	Q	Q	
4+48	5.7198	12.59	Q		V	Q	Q	
4+49	5.7368	12.35	Q		V	Q	Q	
4+50	5.7535	12.11	Q		V	Q	Q	
4+51	5.7698	11.87	Q		V	Q	Q	
4+52	5.7858	11.63	Q		V	Q	Q	
4+53	5.8015	11.39	Q		V	Q	Q	
4+54	5.8169	11.15	Q		V	Q	Q	
4+55	5.8319	10.91	Q		V	Q	Q	
4+56	5.8466	10.67	Q		V	Q	Q	
4+57	5.8610	10.43	Q		V	Q	Q	
4+58	5.8750	10.19	Q		V	Q	Q	
4+59	5.8887	9.95	Q		V	Q	Q	

5+ 0	5.9021	9.71	Q			V
5+ 1	5.9153	9.60	Q			V
5+ 2	5.9284	9.50	Q			V
5+ 3	5.9413	9.39	Q			V
5+ 4	5.9541	9.28	Q			V
5+ 5	5.9667	9.18	Q			V
5+ 6	5.9792	9.07	Q			V
5+ 7	5.9916	8.97	Q			V
5+ 8	6.0038	8.86	Q			V
5+ 9	6.0158	8.76	Q			V
5+10	6.0278	8.65	Q			V
5+11	6.0395	8.55	Q			V
5+12	6.0512	8.44	Q			V
5+13	6.0626	8.33	Q			V
5+14	6.0740	8.23	Q			V
5+15	6.0852	8.12	Q			V
5+16	6.0962	8.02	Q			V
5+17	6.1071	7.91	Q			V
5+18	6.1178	7.81	Q			V
5+19	6.1285	7.70	Q			V
5+20	6.1389	7.60	Q			V
5+21	6.1493	7.53	Q			V
5+22	6.1596	7.47	Q			V
5+23	6.1698	7.41	Q			V
5+24	6.1799	7.35	Q			V
5+25	6.1900	7.29	Q			V
5+26	6.1999	7.22	Q			V
5+27	6.2098	7.16	Q			V
5+28	6.2196	7.10	Q			V
5+29	6.2292	7.04	Q			V
5+30	6.2389	6.98	Q			V
5+31	6.2484	6.92	Q			V
5+32	6.2578	6.85	Q			V
5+33	6.2672	6.79	Q			V
5+34	6.2765	6.73	Q			V
5+35	6.2856	6.67	Q			V
5+36	6.2947	6.61	Q			V
5+37	6.3038	6.55	Q			V
5+38	6.3127	6.48	Q			V
5+39	6.3215	6.42	Q			V
5+40	6.3303	6.36	Q			V
5+41	6.3390	6.32	Q			V
5+42	6.3477	6.28	Q			V
5+43	6.3563	6.24	Q			V
5+44	6.3648	6.20	Q			V
5+45	6.3733	6.16	Q			V
5+46	6.3817	6.11	Q			V
5+47	6.3901	6.07	Q			V
5+48	6.3984	6.03	Q			V
5+49	6.4066	5.99	Q			V

5+50	6.4148	5.95	Q				V
5+51	6.4229	5.91	Q				V
5+52	6.4310	5.87	Q				V
5+53	6.4391	5.83	Q				V
5+54	6.4470	5.78	Q				V
5+55	6.4549	5.74	Q				V
5+56	6.4628	5.70	Q				V
5+57	6.4706	5.66	Q				V
5+58	6.4783	5.62	Q				V
5+59	6.4860	5.58	Q				V
6+ 0	6.4936	5.54	Q				V
6+ 1	6.5012	5.51	Q				V
6+ 2	6.5088	5.48	Q				V
6+ 3	6.5163	5.45	Q				V
6+ 4	6.5237	5.42	Q				V
6+ 5	6.5311	5.39	Q				V
6+ 6	6.5385	5.36	Q				V
6+ 7	6.5459	5.33	Q				V
6+ 8	6.5532	5.30	Q				V
6+ 9	6.5604	5.27	Q				V
6+10	6.5676	5.24	Q				V
6+11	6.5748	5.21	Q				V
6+12	6.5819	5.18	Q				V
6+13	6.5890	5.15	Q				V
6+14	6.5961	5.12	Q				V
6+15	6.6031	5.09	Q				V
6+16	6.6101	5.06	Q				V
6+17	6.6170	5.03	Q				V
6+18	6.6239	5.00	Q				V
6+19	6.6307	4.97	Q				V
6+20	6.6375	4.94	Q				V

End of computations, total study area = 73.980 (Ac.)

Offsite North Idaho Culvert Existing Condition
Stage-Storage-Discharge

Description	Increment	Elevation (ft)	End Area (sf)	Stage Depth (ft)	Basin Depth (ft)	Average End Area (sf)	Incremental Volume (cf)	Cumulative Volume (cf)	Cumulative Volume (ac-ft)	Head for Orifice (ft)	Q through orifice (cfs)	Head for weir (ft)	Q over weir (cfs)	Total Q (cfs)
Invert of Culvert	1	698.4	0	0	0		0	0	0.00	0	0	0	0	0
Soffit of culvert	2	700.9	15,639	2.5	2.5	7,820	19,549	19,549	0.45	1.25	27.31	0.00	0.00	27.31
Elevation 701	3	701	16,626	0.1	2.6	16,133	1,613	21,162	0.49	1.35	28.38	0.00	0.00	28.38
Crown of road	4	701.8	23,952	0.8	3.4	20,289	16,231	37,393	0.86	2.15	35.81	0.00	0.00	35.81
Elevation 702	5	702	26,379	0.2	3.6	25,166	5,033	42,426	0.97	2.35	37.44	0.20	17.64	55.08
Elevation 702.75	6	702.75	34,115	0.75	4.35	30,247	22,685	65,112	1.49	3.1	43.00	0.95	182.64	225.64
Elevation 703	7	703	36,668	0.25	4.6	35,392	8,848	73,959	1.70	3.35	44.70	1.20	259.29	303.99

Orifice parameters

D 2.5
A 4.91

Weir parameters

B 75

Offsite North Idaho Culvert Proposed Condition
Stage-Storage-Discharge

Description	Increment	Elevation (ft)	End Area (sf)	Stage Depth (ft)	Basin Depth (ft)	Average End Area (sf)	Incremental Volume (cf)	Cumulative Volume (cf)	Cumulative Volume (ac-ft)	Head for Orifice (ft)	Q through orifice (cfs)
Invert of Culvert	1	698.4	0	0	0		0	0	0.00	0	0
Soffit of culvert	2	700.9	15,639	2.5	2.5	7,820	19,549	19,549	0.45	1.25	27.31
Elevation 701	3	701	16,626	0.1	2.6	16,133	1,613	21,162	0.49	1.35	28.38
Crown of road	4	701.8	23,952	0.8	3.4	20,289	16,231	37,393	0.86	2.15	35.81
Elevation 702	5	702	26,379	0.2	3.6	25,166	5,033	42,426	0.97	2.35	37.44
Elevation 702.75	6	702.75	34,115	0.75	4.35	30,247	22,685	65,112	1.49	3.1	43.00

Orifice parameters

D 2.5
A 4.91

Weir parameters

B 75

Idaho Detention Basin Proposed Condition
Stage-Storage-Discharge

Description	Depth for SSD	Elevation	Depth to Orifice springline (ft)	Depth to Weir Outflow (ft)	End Area (sf)	Average End Area (sf)	Storage Volume (cu. ft)	Storage Volume (AC-ft)	Cumulative Storage Volume (AC-ft)	Weir Outflow (cfs)	Outflow as Orifice (cfs)	Total Outflow (cfs)
invert of orifice	0	689.25			8857			0	0.00			
springline of orifice	0.125	689.375	0		8857	8857	442.85	0.010166437	0.01	0.04	0.00	0.04
top of orifice	0.25	689.5	0.125		8857	8857	442.85	0.010166437	0.02		0.09	0.09
top of gravel	0.75	690	0.625		8857	8857	1771.4	0.040665748	0.06		0.19	0.19
top of media	2.25	691.5	2.125		8857	8857	3985.65	0.091497934	0.15		0.36	0.36
	2.75	692	2.625		9488	9172.5	4586.25	0.105285813	0.26		0.40	0.40
top of ponding	3.25	692.5	3.125	0	10133	9810.5	4905.25	0.112609045	0.37		0.43	0.43
	3.75	693	3.625	0.5	10792	10462.5	5231.25	0.120092975	0.49	2.35	0.47	2.82
	4.75	694	4.625	1.5	12152	11472	11472	0.263360882	0.75	12.24	0.53	12.76
top of peak storm	5.25	694.5	5.125	2	12853	12502.5	6251.25	0.143508953	0.90	18.84	0.55	19.39

weir equation C B
 $Q = C \times B \times H^{3/2}$ 3.33 2

Orifice Equation C A
 $Q = C \times A \times (2gh)^{1/2}$ 0.62 0.05

APPENDIX C



MEMORANDUM

DATE: February 26, 2020

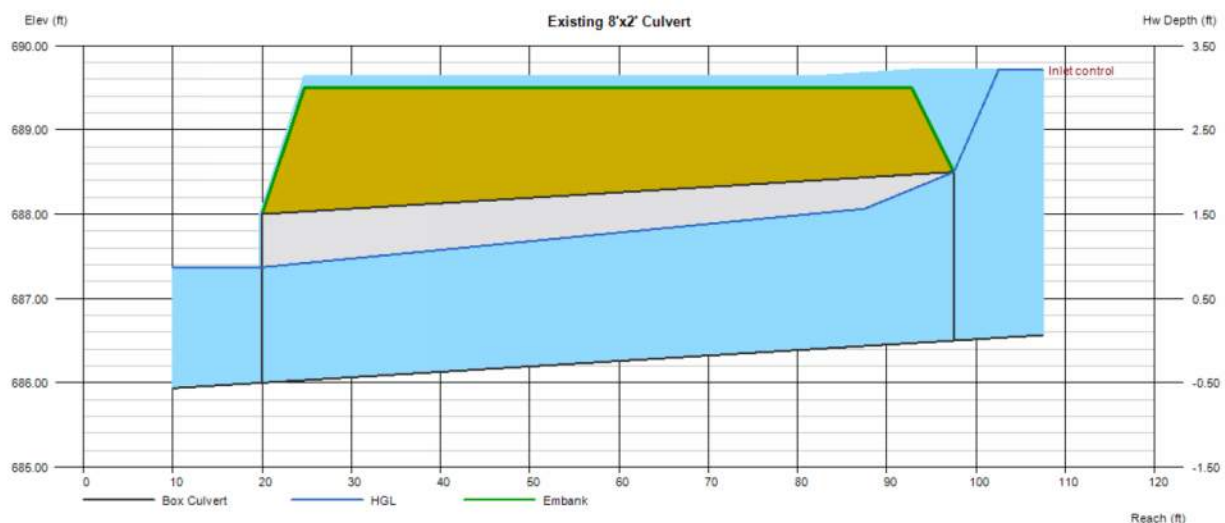
TO: Oscar Uranga

FROM: Tina Fransson, PE

SUBJECT: Escondido Estates – Flood Limits Assessment

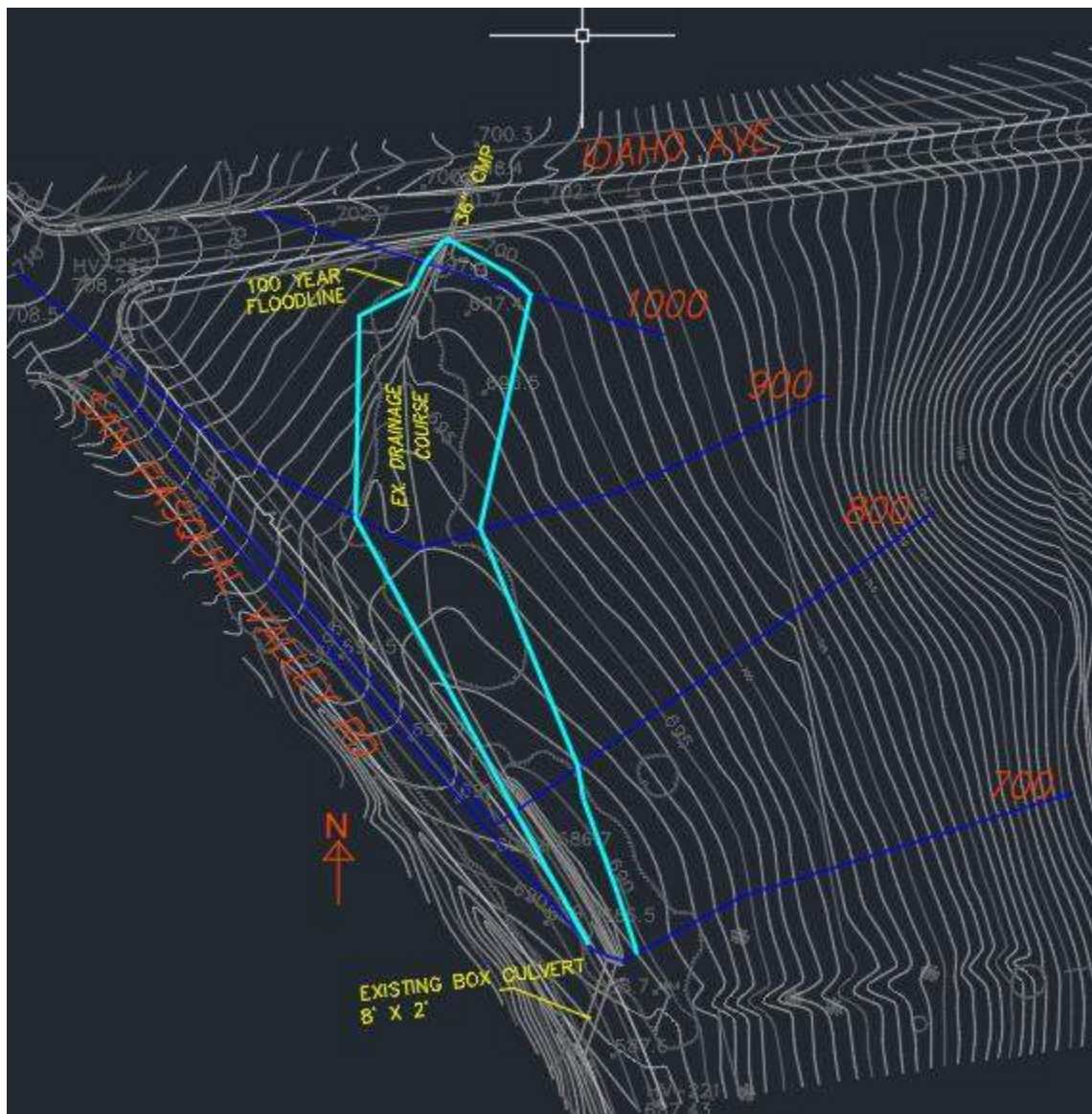
This memo includes the findings of the preliminary flood limits assessment for the Escondido Estates project wetlands. At the upstream end of the wetlands lies an existing 36-inch corrugated metal pipe (CMP), which crosses Idaho Avenue. An existing 8-foot x 2-foot reinforced concrete box (RCB) lies at the downstream end of the wetlands, crosses westerly beneath San Pasqual Valley Road, and releases runoff into an existing channel that flows towards the south.

The 100-year flow rate at the existing 8-foot x 2-foot RCB inlet was taken from the X-Engineering January 2020 hydrology report and is reported to be 111.5 cfs. This flow rate was used to calculate a starting water surface elevation at the RCB inlet for the HEC-RAS model. Both normal depth and submerged tailwater conditions were modeled using Hydraflow Express, however, both methods resulted in the same upstream hydraulic grade line (HGL) of 689.71 feet (see figure below). This HGL was then used as the starting water surface elevation in the HEC-RAS model. The culvert calculation shows that the 8-foot x 2-foot RCB is under capacity for the 100-year storm.



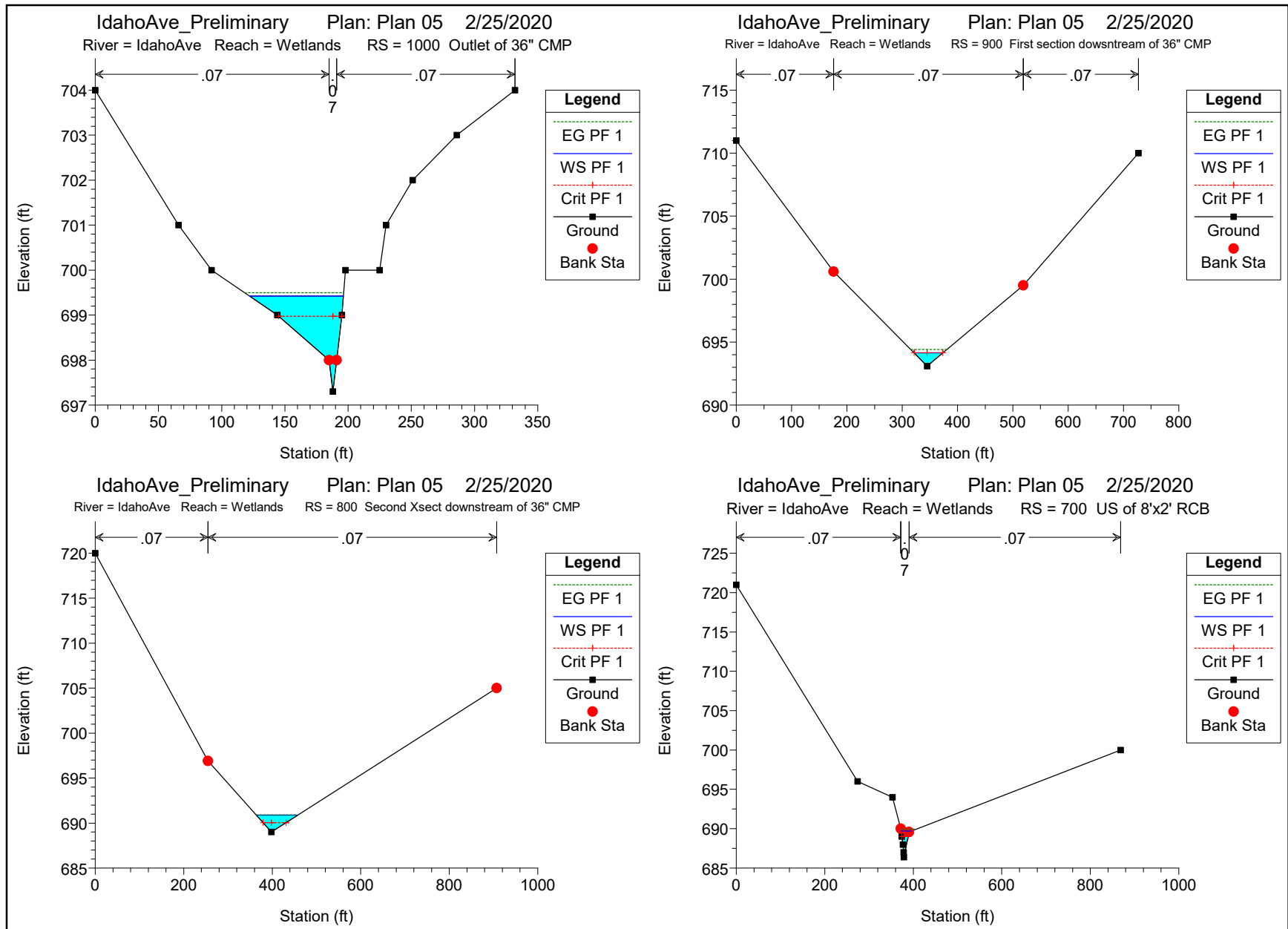
The HEC-RAS model used existing topography provided by X-Engineering. Four cross sections were cut along the wetlands, beginning at the outlet of the existing 36-inch CMP, two cross sections along the wetlands, and a last cross section immediately upstream of the 8-foot x 2-foot RCB. The HEC-RAS model produced the results in the table below, and the floodlines that encompass the existing wetland are shown in the figure below.

HEC-RAS Station	Water Surface Elevation (ft)	Minimum Channel Elevation (ft)
1000	699.42	697.3
900	694.14	693.1
800	690.90	689.0
700	689.71	686.4



HEC-RAS Plan: Mixed River: IdahoAve Reach: Wetlands Profile: PF 1

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Wetlands	1000	PF 1	111.50	697.30	699.42	698.97	699.50	0.010543	3.14	57.06	74.24	0.42
Wetlands	900	PF 1	111.50	693.10	694.14	694.14	694.41	0.091711	4.15	26.85	51.67	1.02
Wetlands	800	PF 1	111.50	689.00	690.90	690.04	690.92	0.003687	1.24	89.70	94.63	0.23
Wetlands	700	PF 1	111.50	686.40	689.71	689.01	689.94	0.018833	3.85	29.20	22.78	0.53



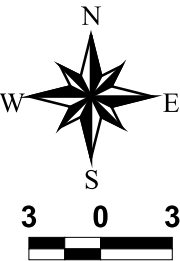
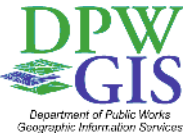
County of San Diego Hydrology Manual



Rainfall Isopluvials

100 Year Rainfall Event - 6 Hours

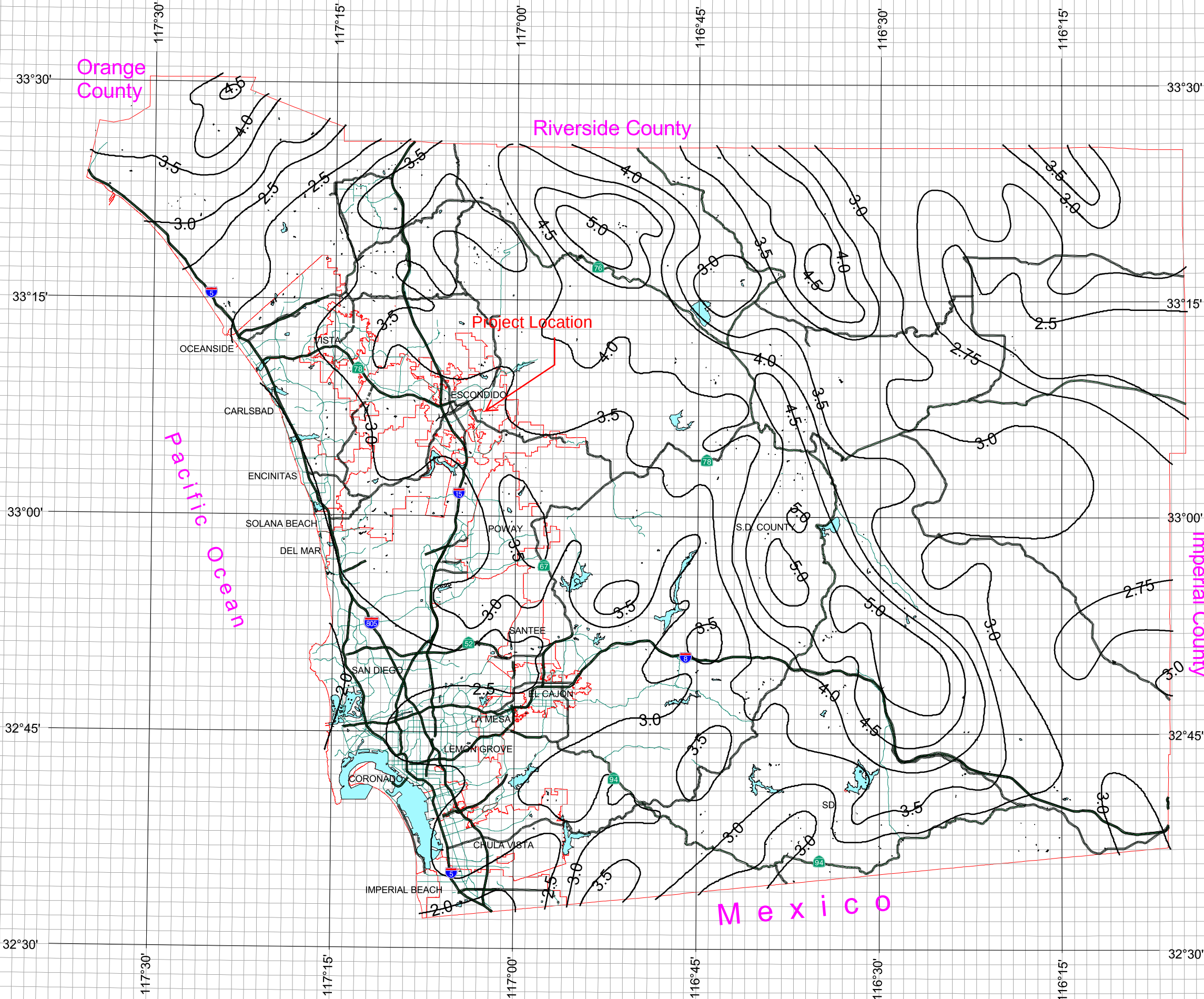
— Isopluvial (inches)



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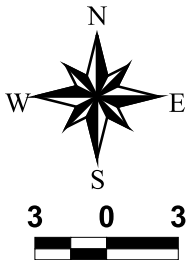
County of San Diego Hydrology Manual



Rainfall Isophuvials

100 Year Rainfall Event - 24 Hours

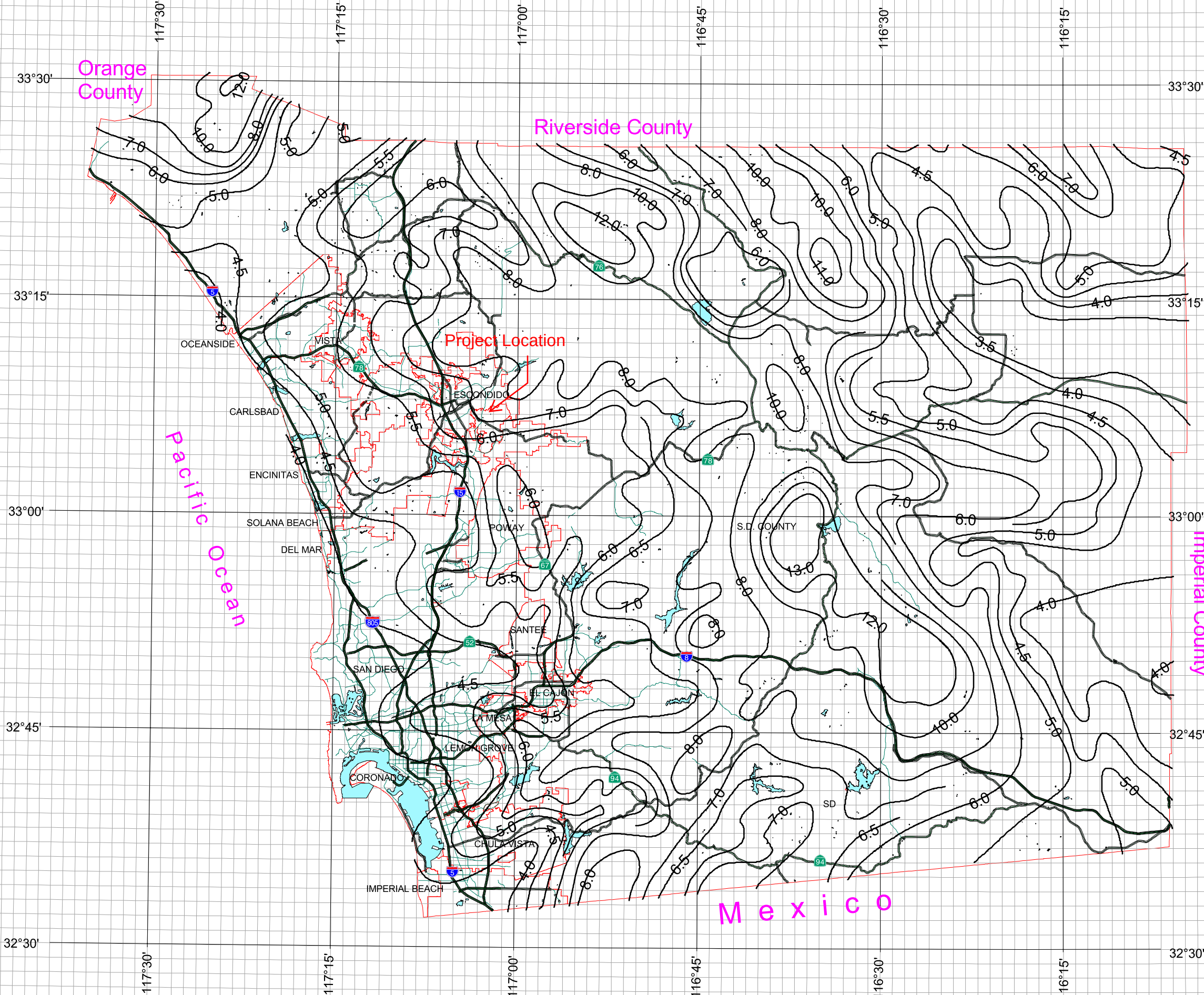
— Isopluvial (inches)



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APPENDIX D

PARCEL LINE INFORMATION PROVIDED BY:
SAN DIEGO DATA PROCESSING CORPORATION
ACCORDING TO THE BASE MAP SUB LICENSE
AGREEMENT WITH THE CITY OF ESCONDIDO

TOPOGRAPHY COMPILED BY:
GEONEX NORTH AMERICAN OPERATIONS, INC.
DATED NOVEMBER, 1992

LEGEND

EXISTING CONTOUR ELEVATION
EXISTING NATURAL FLOW
EXISTING IMPROVED CHANNEL
EXISTING STORM DRAIN
PROPOSED FACILITY
EXISTING LAKES

LEGEND

MAJOR BASIN BOUNDARY
MINOR BASIN BOUNDARY
GENERAL PLAN BOUNDARY
BASIN DESIGNATION
EXIST. / PROP. DESILT BASIN
FACILITY IDENTIFIER



SEE TILE 19 (SHEET 12)



SEE TILE 7 (SHEET 10)

Date	App'd	REVISIONS	By	Date

Checked By	Drawn by	Designed By
DWM-END	DAM	JAW-LBR-END
Plans Prepared Under Supervision Of: For Masson & Assoc. Inc.		
EDWARD N. DOMINGUE	R.C.E. 34131	DATE:

SCALE
Horizontal
1" = 400'
DATE: 02-06-95
Drawn By: DAM

Submitted	Approved
By	By
Asst. City Engineer	Asst. Director of Public Works

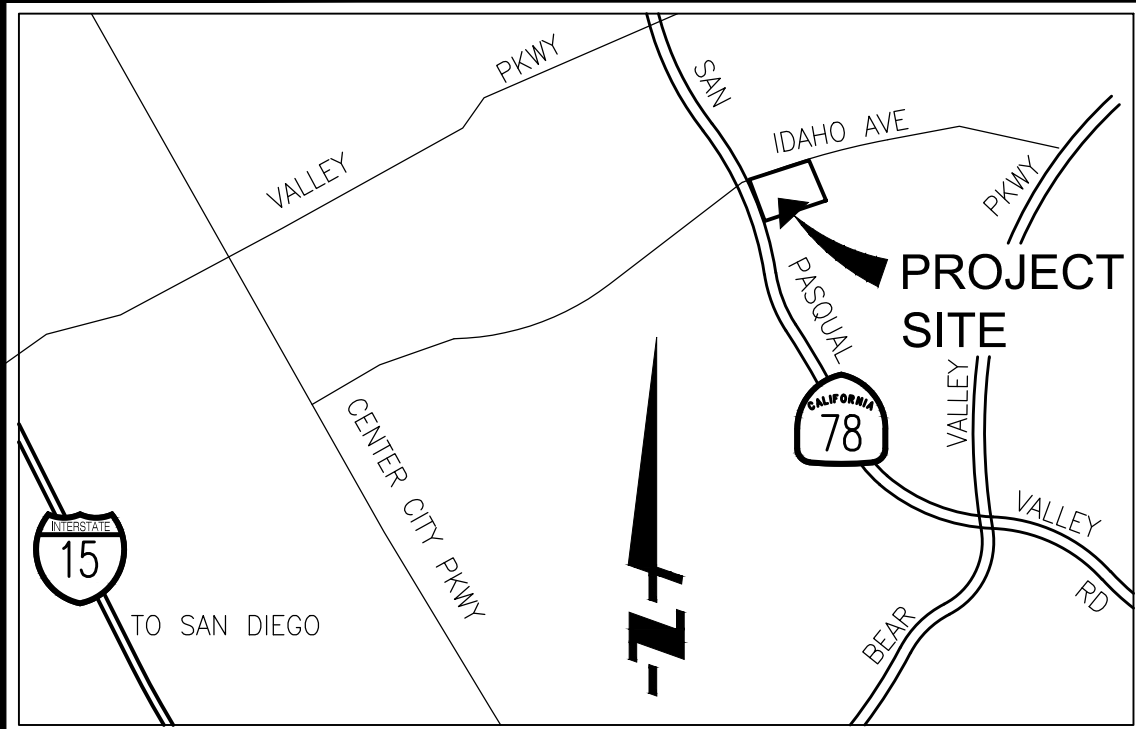


TABLE 3.1
EXISTING FACILITIES

GEOMETRY: 1 = TRAP. OPEN CHANNEL, 2 = IRREGULAR CHANNEL, 3 = BOX CHANNEL, 4 = PIPE, 5 = RECT. OPEN CHANNEL
MATERIALS: 1 = R.C.P., 2 = C.I.P.P., 3 = R.C.B., 4 = C.M.P.A., 5 = C.M.P., 6 = NAT. CHNL., 7 = CONC. CHNL., 8 = SPIRAL RIBBED
Qc= EXISTING CAPACITY

ID #	SYSTEM	GEOMETRY	WIDTH (IN)	HEIGHT (IN)	RATIO	MATERIAL	DRAWING NO.	LENGTH (FEET)	SLOPE	Qc (CFS)
851	E-62	1	72	24	1.5	7	P-1636-10	300	1.30%	158
852	E-62	4	48			1	P-1636-2	56	1.30%	164
853	E-62	4	42			1	P-1636-2	33	2.93%	172
854	E-162	4	36			5	P-1604-13	156	1.00%	36
855	E-171	4	36			1	1003-8	366	0.25%	33
856	E-171	4	36			1	1003-8	546	0.25%	33
857	E-162	1	24	36	1.25	7	1365	460	0.12%	77
858	KC-22	1	12	24	1.5	7	11-095024-1	1050	2.40%	121
859	L.H.	4	42			1	P-1746-6	91	22.34%	476
860	NOT USED									
861	E-221	3	60	24		3	P-1724-2	332	0.12%	39
862	E-221	3	60	24		3	P-1724-2	393	0.38%	70
863	E-221	3	72	18		3	P-1724-2	72	0.50%	52
864	E-221	3	72	24		3	P-1724-2	40	0.50%	84
865	E-201	3	72	24		3	P-1348-2	570	0.20%	53
866	E-201	3	72	24		3	P-1348-6	70	0.20%	53
867	L.H.	4	36			4	1667	453	0.40%	42
868	R-81	4	66	36		5	SFP40	60	0.80%	145
869	R-12	4	65	40		4	P-1604-23	120	1.00%	213
870	E-242	4	48		2	1	P-1804-3	580	0.43%	188
871	E-242	4	48		2	1	P-1806	280	0.75%	249
872	E-132	4	58	36		4	P-1623/P-2145	650	0.76%	136
873	E-221	4	36	22	2	4	N.A.	260	0.90%	84
874	KC-42	4	228	76		4	P-2113	64	0.43%	1479
875	KC-42	4	48		3	5	P-2113	52	1.00%	233
876	L.H.	4	54			5	N.A.	100	1.00%	197
877	KC-21	4	48			1	N.A.	60	1.00%	144
878	L.H.	4	42			5	N.A.	140	10.00%	318
879	L.H.	4	48			5	N.A.	90	5.00%	321
880	KC-21	3	72	60		3	N.A.	80	5.00%	1015
881	KC-23	4	48		2	5	N.A.	90	2.00%	220
882	KC-23	4	54			5	N.A.	110	1.50%	130
883	KC-23	3	120	48		3	N.A.	160	1.00%	662
884	KC-23	4	36			5	N.A.	200	5.00%	81

APPENDIX E



LEGEND

---	EXISTING PROPERTY LINE
---	STREET CENTERLINE
---	PROPOSED RIGHT OF WAY
---	PROPOSED LOT LINE
---	EXISTING EASEMENT
---	PROPOSED EASEMENT
---	EXISTING PARCELS
---	RELINQUISHED ABUTTERS RIGHTS

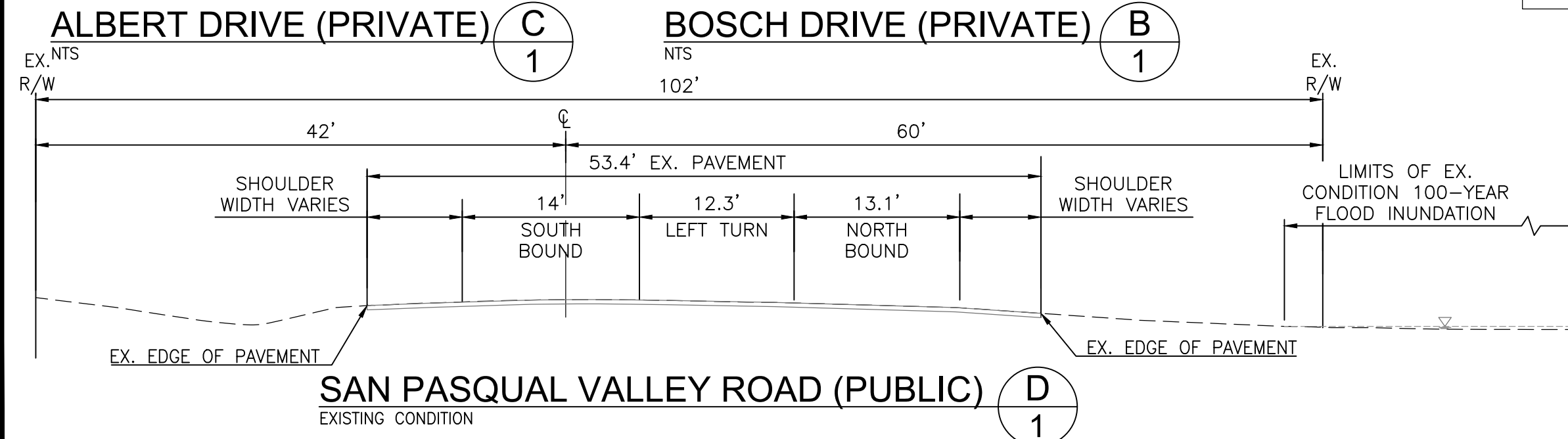
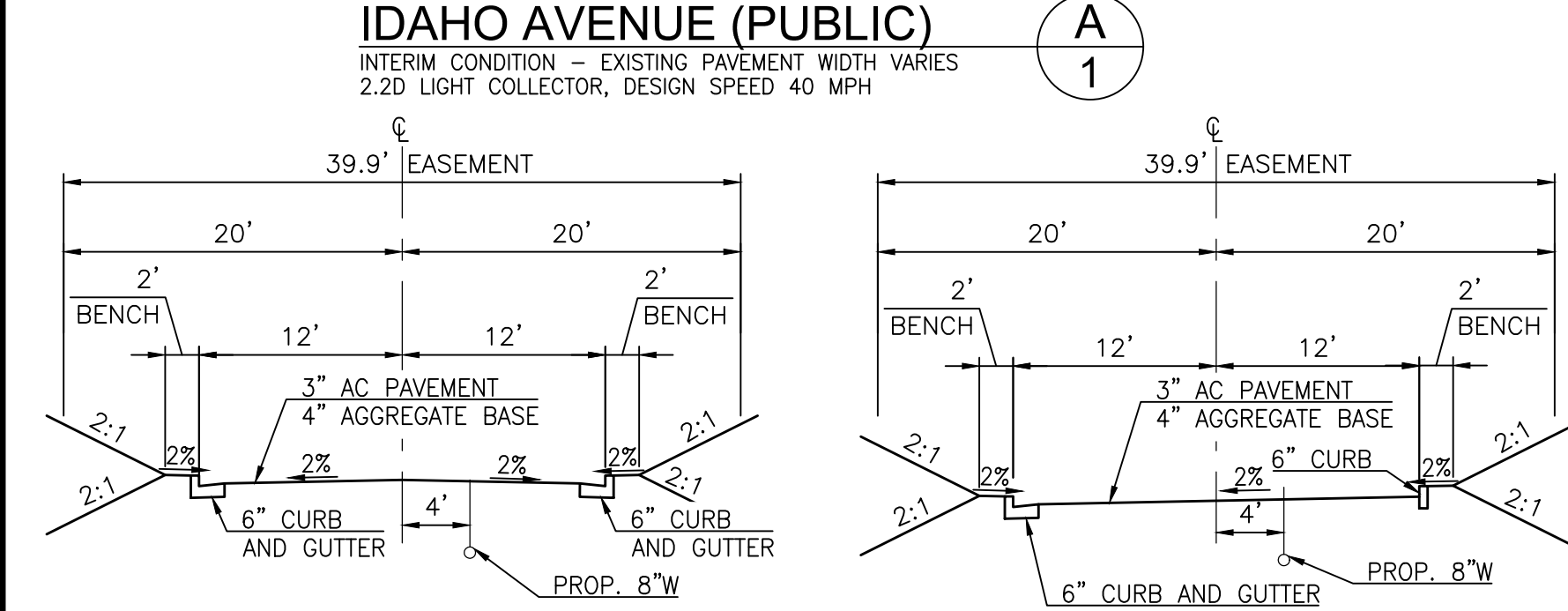
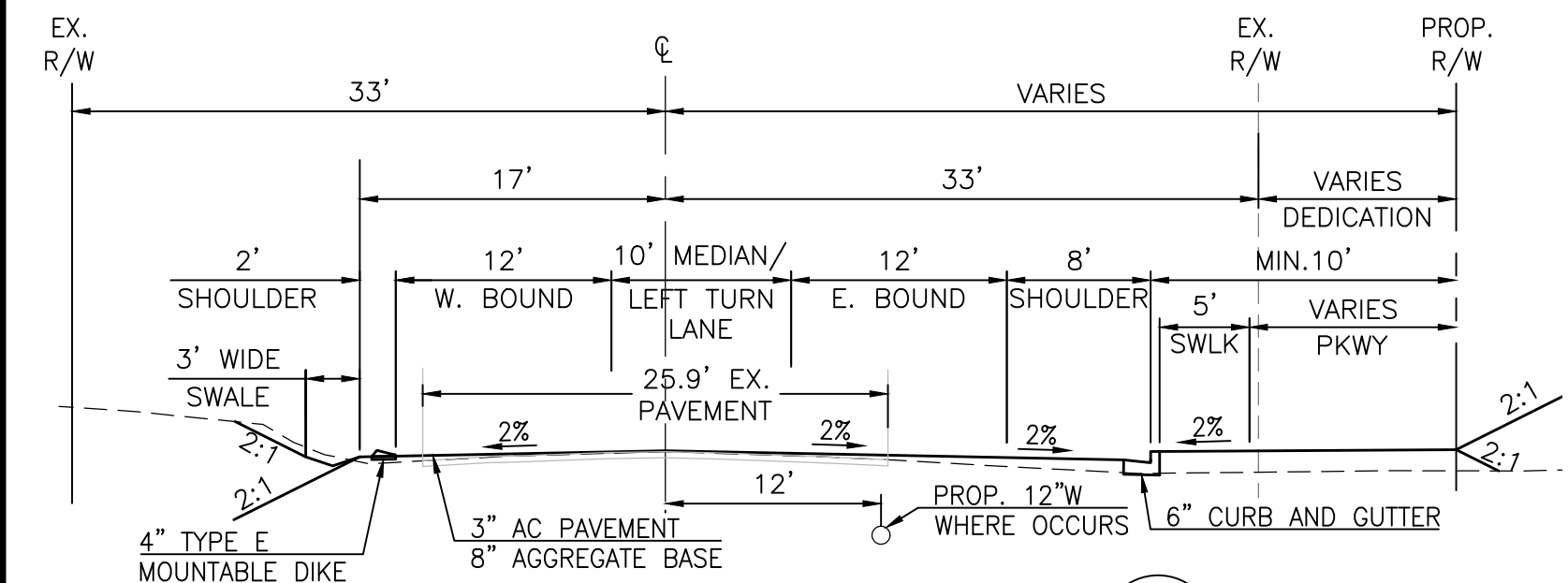
PRELIMINARY GRADING PLAN NOTE

THIS PLAN IS PROVIDED TO ALLOW FOR FULL AND ADEQUATE DISCRETIONARY REVIEW OF A PROPOSED DEVELOPMENT PROJECT. THE PROPERTY OWNER ACKNOWLEDGES THAT ACCEPTANCE OR APPROVAL OF THIS PLAN DOES NOT CONSTITUTE AN APPROVAL TO PERFORM ANY GRADING SHOWN HEREON AND AGREES TO OBTAIN VALID GRADING PERMISSIONS BEFORE COMMENCING SUCH ACTIVITY.

NOTICE

THE SUBJECT PROPERTY CONTAINS WETLANDS, A LAKE, A STREAM, AND/OR WATERS OF THE U.S. AND/OR STATE WHICH MAY BE SUBJECT TO REGULATION BY STATE AND/OR FEDERAL AGENCIES, INCLUDING, BUT NOT LIMITED TO, THE REGIONAL WATER QUALITY CONTROL BOARD, U.S. ARMY CORPS OF ENGINEERS AND THE CALIFORNIA DEPARTMENT OF FISH AND WILDLIFE. IT IS THE APPLICANT'S RESPONSIBILITY TO CONSULT WITH EACH AGENCY TO DETERMINE IF A PERMIT, AGREEMENT OR OTHER APPROVAL IS REQUIRED AND TO OBTAIN ALL NECESSARY PERMITS, AGREEMENTS OR APPROVALS BEFORE COMMENCING ANY ACTIVITY WHICH COULD IMPACT THE WETLANDS, LAKE, STREAM, AND/OR WATERS OF THE STATE OR U.S. ON THE SUBJECT PROPERTY. THE AGENCY CONTACT INFORMATION IS PROVIDED BELOW.

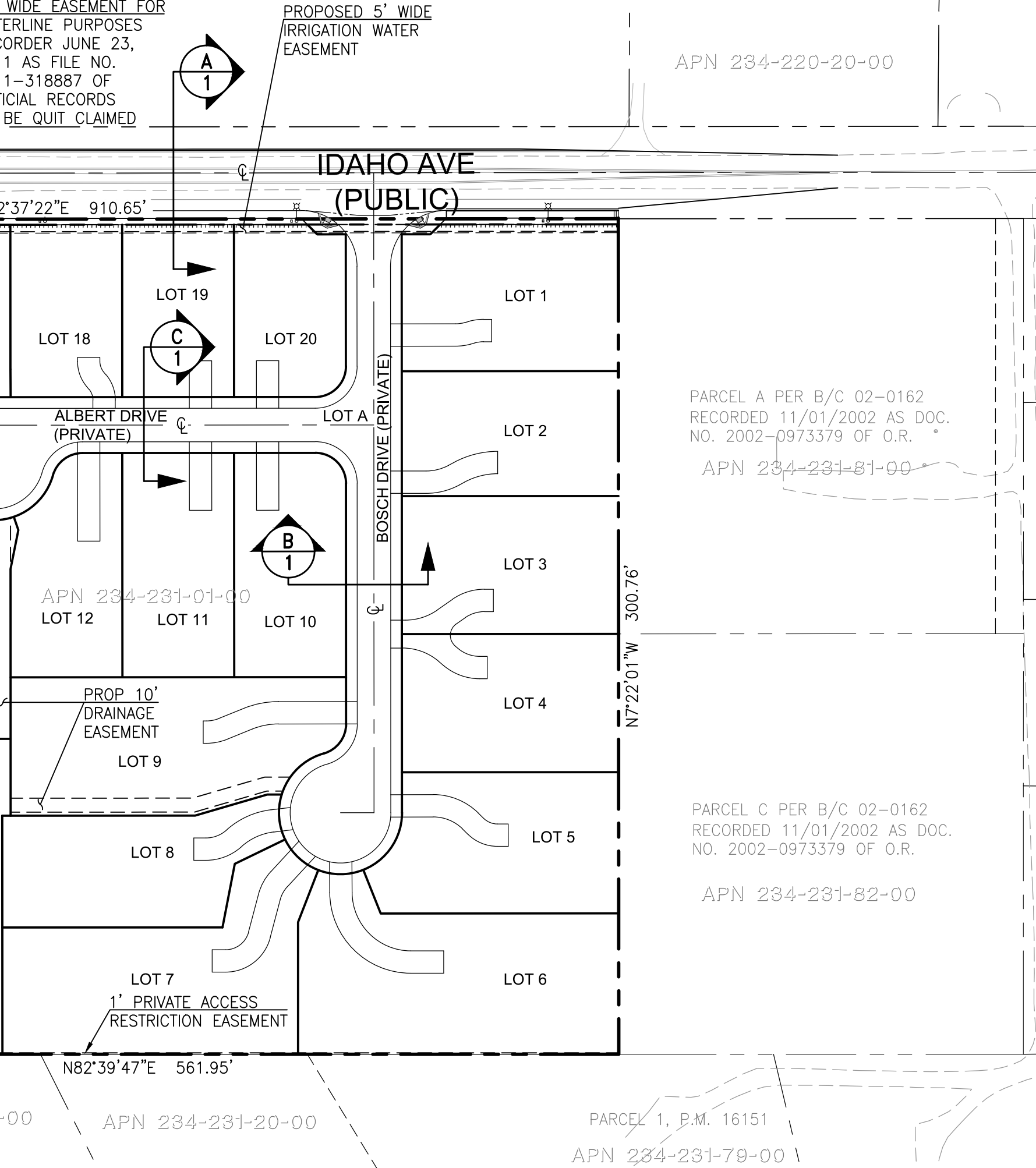
U.S. ARMY CORPS OF ENGINEERS: 915 WILSHIRE BLVD., SUITE 1101, LOS ANGELES, CA 90017; (213) 452-3333; [HTTP://WWW.USACE.ARMY.MIL/](http://www.usace.army.mil/)
REGIONAL WATER QUALITY CONTROL BOARD: 2375 NORTHSIDE DRIVE, SUITE 100, SAN DIEGO, CA 92108; RB9_DREDGEFILL@WATERBOARDS.CA.GOV ; [HTTP://WWW.WATERBOARDS.CA.GOV/SANDIEGO/CALIFORNIA](http://www.waterboards.ca.gov/sandiego/california)
DEPARTMENT OF FISH AND WILDLIFE: 3883 RUFFIN RD., SAN DIEGO, CA 92123; (858) 636-3160; ASKR5@WILDLIFE.CA.GOV
[HTTP://WWW.DFG.CA.GOV/](http://www.dfg.ca.gov/)



COUNTY OF SAN DIEGO

TENTATIVE TRACT No. 5639

PRELIMINARY GRADING PLAN



NUMBERED LOT SUMMARY

LOT #	AREA (SF)	AREA (AC)	TYPE	PAD ELEV
LOT 1	16,107	0.37	RESIDENTIAL	733'
LOT 2	13,861	0.32	RESIDENTIAL	741'
LOT 3	15,295	0.35	RESIDENTIAL	741'
LOT 4	15,330	0.35	RESIDENTIAL	741'
LOT 5	16,637	0.38	RESIDENTIAL	741'
LOT 6	24,442	0.56	RESIDENTIAL	734'
LOT 7	22,197	0.51	RESIDENTIAL	713'
LOT 8	14,513	0.33	RESIDENTIAL	718'
LOT 9	21,377	0.49	RESIDENTIAL	721'
LOT 10	12,731	0.29	RESIDENTIAL	724'

NUMBERED LOT SUMMARY

LOT #	AREA (SF)	AREA (AC)	TYPE	PAD ELEV
LOT 11	12,835	0.29	RESIDENTIAL	719'
LOT 12	11,524	0.26	RESIDENTIAL	714'
LOT 13	12,863	0.30	RESIDENTIAL	707'
LOT 14	12,945	0.30	RESIDENTIAL	703'
LOT 15	16,840	0.39	RESIDENTIAL	703'
LOT 16	15,210	0.35	RESIDENTIAL	703'
LOT 17	10,589	0.24	RESIDENTIAL	707'
LOT 18	9,870	0.23	RESIDENTIAL	712'
LOT 19	9,873	0.23	RESIDENTIAL	717'
LOT 20	9,602	0.22	RESIDENTIAL	722'

LETTERED LOT SUMMARY

LOT	AREA (SF)	AREA (AC)	TYPE	OWNERSHIP	MAINTENANCE RESPONSIBILITY
LOT 'A'	37,071	0.85	PRIVATE STREET	HOA	HOA
LOT 'B'	102,915	2.36	OPEN SPACE/DRAINAGE	HOA	HOA

EXISTING SITE INFORMATION

APN 234-231-01-00

EXISTING ZONING	
USE REGULATIONS	A70
ANIMAL REGULATIONS	L
DEVELOPMENT REGULATIONS	
DENSITY	
LOT SIZE	0.5 ACRES
BUILDING TYPE	C
MAXIMUM FLOOR AREA	
FLOOR AREA RATIO	
HEIGHT	G
LOT COVERAGE	
SETBACKS	C
OPEN SPACE	
SPECIAL AREA REGULATION	

ESTIMATED GRADING QUANTITIES

	CUT (CY)	FILL (CY)
RAW	14,005	25,695
ADDITIONAL EXCAVATION	37,000	37,000
SHRINKAGE (15%)*	—	7,651
SUBSIDENCE (0.10)*		560
BULKING (10%)*	3,700	
TOTALS	54,705	70,906
16,201 CY NET IMPORT		

*TO BE CONFIRMED IN GEOTECHNICAL REVIEW OF GRADING PLANS.

BASIS OF BEARINGS

THE BASIS OF BEARING FOR THIS SURVEY IS A PORTION OF THE NORTHEASTERLY SIDELINE OF SAN PASQUAL ROAD PER MS 855. I.E. S 37°38'19"E

LEGAL DESCRIPTION

ALL THAT PORTION OF LOT 9 IN BLOCK 321 OF RANCHO RINCON DEL DIABLO RESURVEY ACCORDING TO MAP THEREOF NO. 725 FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY AUGUST 13, 1892, LYING WITHIN THE COUNTY OF SAN DIEGO, STATE OF CALIFORNIA.

ABBREVIATIONS

BLDG	BUILDING	MPT	MULTI-PURPOSE TRAIL
BW	BACK OF WALK	P	PAD
C	CURB	PKWY	PARKWAY
C&G	CURB & GUTTER	PL	PROPERTY LINE
DW	DOMESTIC WATER	POC	POINT OF CONNECTION
CL	CENTERLINE	PP	POWER POLE
ESMT	EASEMENT	PUE	PUBLIC UTILITY EASEMENT
EX	EXISTING	R/W	RIGHT OF WAY
FG	FINISHED GRADE	RW	RECLAIMED WATER
FL	FLOWLINE	S	SANITARY SEWER
FS	FINISHED SURFACE	SD	STORM DRAIN
LBZ	LIMITED BUILDING ZONE	SWLK	SIDEWALK
LL	LOT LINE	TC	TOP OF CURB
MG	MASS GRADING	TEMP	TEMPORARY

LAND DIVISION STATEMENT

I HEREBY CERTIFY THAT I AM THE RECORD OWNER, AS SHOWN ON THE LATEST EQUALIZED COUNTY ASSESSMENT, OF THE PROPERTY SHOWN ON THE TENTATIVE MAP. ALL OF MY CONTIGUOUS OWNERSHIP WITHIN AND BEYOND THE BOUNDARIES OF THE TENTATIVE MAP IS SHOWN. THE BASIS OF CREATION OF THE LOTS IN MY OWNERSHIP (E.G. PARCEL MAP, FINAL MAP, CERTIFICATE OF COMPLIANCE, RECORDED DEED BEFORE 2-1-72) IS CONSIDERED CONTIGUOUS EVEN IF IT IS SEPARATED BY ROADS, STREETS, UTILITY EASEMENTS OR RAILROAD RIGHTS-OF-WAY. "FREEWAY" AS DEFINED IN SECTION 23.5 OF THE STREETS AND HIGHWAYS CODE, SHALL NOT BE CONSIDERED AS ROADS OR STREETS.

I FURTHER CERTIFY THAT I WILL NOT, BY THIS APPLICATION, CREATE, OR CAUSE TO BE CREATED, OR WILL NOT HAVE PARTICIPATED IN THE CREATION OF MORE THAN FOUR PARCELS ON CONTIGUOUS PROPERTY UNLESS SUCH CONTIGUOUS PARCELS WERE CREATED BY MAJOR SUBDIVISION. FOR PURPOSES OF THIS CERTIFICATION, THE TERM PARTICIPANT MEANS HAVING COOPERATED WITH OR ACTED IN A PLANNING, COORDINATING, OR DECISION-MAKING CAPACITY IN ANY FORMAL OR INFORMAL ASSOCIATION OR PARTNERSHIP FOR THE PURPOSE OF DIVIDING REAL PROPERTY.

I CERTIFY UNDER PENALTY OF PERJURY THAT THE FOREGOING IS TRUE AND CORRECT.

OWNER/APPLICANT

OSCAR URANGA, ESCONDIDO ESTATES, LLC
19782 MACARTHUR BLD SUITE 300
IRVINE, CA 92707
TEL: 949-933-4103

I HEREBY CERTIFY THAT I AM THE RECORD OWNER OF THE PROPERTY SHOWN ON THE TENTATIVE SUBDIVISION MAP AND THAT SAID MAP SHOWS ALL MY CONTIGUOUS OWNERSHIP IN WHICH I HAVE ANY DEED OR TRUST INTEREST. I UNDERSTAND THAT MY (OUR) PROPERTY IS CONSIDERED CONTIGUOUS EVEN IF IT IS SEPARATED BY ROADS, STREETS, EASEMENTS, OR RAILROAD RIGHTS-OF-WAY.

OSCAR URANGA

GEOTECHNICAL ENGINEER

VINJE & MIDDLETON ENGINEERING, INC.
2450 AUTO PARK WAY
ESCONDIDO, CA 92029
TEL: 760-743-1214

CONTACT: RALPH M. VINJE, CEG #863

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SITE INFORMATION

- PROJECT ADDRESS:
1125 IDAHO AVE, ESCONDIDO, CA, 92027-4302
- ASSESSORS PARCEL No. (S): 234-231-01-00
- No. OF EXISTING LOTS: 1
- No. OF PROPOSED LOTS: 23
- SITE GROSS AREA: 10.28 AC
- SITE NET AREA: 9.98 AC
- ASSESSOR TAX RATE AREA: 74019

GENERAL NOTES

- TOTAL RESIDENTIAL PROPOSED: 20
- MINIMUM LOT SIZE BEING CREATED: 9,602 SQ.FT
- AVERAGE RESIDENTIAL LOT BEING CREATED:
14,627 SQ.FT
- GENERAL PLAN LAND USE DESIGNATION:
VILLAGE RESIDENTIAL (VR-2)
- ZONING: A70 (LIMITED AGRICULTURAL)
- COMMUNITY PLAN AREA: NORTH COUNTY
METRO-COUNTY OF ESCONDIDO
- SCHOOL DISTRICT: GEN ELEM ESCONDIDO
- ASSOCIATED PERMITS:
PDS2020-TM-5639
- ADMINISTRATIVE PERMIT:
PDS2020-AD-20-007
- OCCUPANCY CLASSIFICATION
(PER CA BUILDING CODE)
- SOLAR ACCESS STATEMENT:
ALL LOTS WITHIN THE SUBDIVISION WILL HAVE UNOBSTRUCTED ACCESS TO SUNLIGHT TO AN AREA OF NOT LESS THAN 100 SQUARE FEET, FALLING IN A HORIZONTAL PLANE 10 FEET ABOVE GRADE OF BUILDING AREA OF THE LOT BETWEEN AZIMUTHS OF THE SUN AT 45 DEGREES TO THE EAST AND 45 DEGREES TO THE WEST OF TRUE SOUTH, WHEN MEASURED ON THE WINTER SOLSTICE.
- STREET LIGHT STATEMENT: AS REQUIRED BY COUNTY STANDARDS & DARK SKY POLICY.

TOPOGRAPHICAL SOURCE

THE TOPOGRAPHY SHOWN HEREON IS BASED ON PHOTOGAMMETRIC COMPUTER AIDED DRAFTING TECHNIQUES. IN OPEN, UNOBSTRUCTED AREAS THIS MAP COMPLIES WITH NATIONAL STANDARDS FOR MAP ACCURACY. FIELD SURVEY CONTROL BY BRAIN POLLEY LAND SURVEYING DATED JUNE 23, 2004 JOB NO. 10611-101

SLOPE AVERAGE ANALYSIS

AREA= 10.28 AC, CONTOUR INTERVAL= 1FT,
L= 39,666 FT
AVERAGE SLOPE, $S = \frac{1}{10.28} \times \frac{39,666}{100} \times 100 = 8.82\%$
10.28 AC x 43,560

FEMA FLOOD ZONE

THE SITE IS LOCATED IN FLOOD ZONE X, AREA DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN, PER FIRM 06073C1081G.

UTILITY PURVEYORS

GAS & ELECTRIC:	SAN DIEGO GAS AND ELECTRIC 800-411-7343
WATER:	CITY OF ESCONDIDO 760-839-4657
SEWER:	SEPTIC SYSTEM (COUNTY OF SAN DIEGO)
STORM DRAIN:	CITY OF ESCONDIDO 760-839-4657
FIRE:	CITY OF ESCONDIDO 760-839-5400
CABLE:	COX CABLE 760-878-5222
TELEPHONE:	PACIFIC BELL 760-489-3187

CIVIL ENGINEER

X ENGINEERING & CONSULTING, INC.
6 HUTTON CENTRE DR., SUITE 650,
SANTA ANA, CA 92707
TEL: 949-522-7100

CONTACT: ERIC LISSNER, P.E.

TITLE SHEET

ESCONDIDO ESTATES

TM 5639

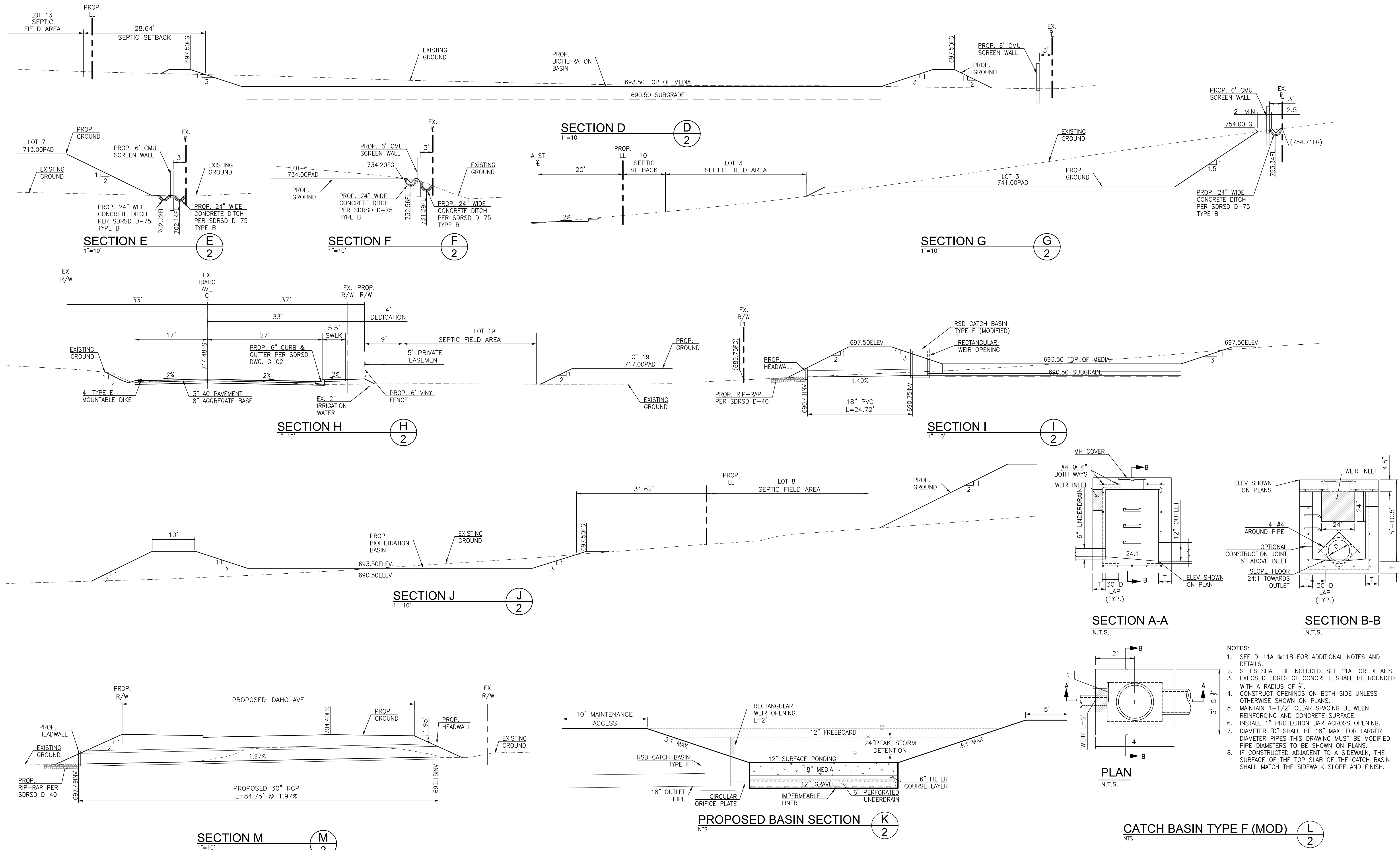
COUNTY OF SAN DIEGO, CALIFORNIA

SHEET

1

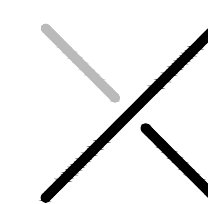
OF

4



△	BY	REVISIONS	DATE

PREPARED BY:



X ENGINEERING & CONSULTING, INC.
 6 Hutton Centre Drive, Suite 650
 Santa Ana, California 92707
 949.522.7100 | xengineeringinc.com



SECTIONS & DETAILS
ESCONDIDO ESTATES
TM 5639
 COUNTY OF SAN DIEGO, CALIFORNIA

SHEET

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