

Type.... Outlet Input Data
Name.... Outlet 1

Page 5.03

File.... C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP
1\5508RPI4-100 YR-BMP 1.PFW

OUTLET STRUCTURE INPUT DATA

Structure ID = CV
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 3.0000 ft
Upstream Invert = 400.00 ft
Dnstream Invert = 399.00 ft
Horiz. Length = 72.35 ft
Barrel Length = 72.36 ft
Barrel Slope = .01382 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0130
Ke = .5000 (forward entrance loss)
Kb = .007228 (per ft of full flow)
Kr = .5000 (reverse entrance loss)
HW Convergence = .001 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0078
Inlet Control M = 2.0000
Inlet Control c = .03790
Inlet Control Y = .6900
T1 ratio (HW/D) = 1.129
T2 ratio (HW/D) = 1.289
Slope Factor = -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...

At T1 Elev = 403.39 ft ---> Flow = 42.85 cfs
At T2 Elev = 403.87 ft ---> Flow = 48.97 cfs

S/N:

PondPack Ver:

Compute Time:

Date:

Type.... Outlet Input Data
Name.... Outlet 1

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File.... C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP
1\5508RPL4-100 YR-BMP 1.PPW

OUTLET STRUCTURE INPUT DATA

Structure ID = TW
Structure Type = TW SETUP, DS Channel

USE DOWNSTREAM CHANNEL NORMAL DEPTH FOR TW...
Channel Type: Chn-Circular
Channel ID: Chn-Cir - 1

CONVERGENCE TOLERANCES...
Maximum Iterations= 40
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .00 cfs
Max. Q tolerance = .00 cfs

S/N:

PondPack Ver:

Compute Time:

Date:

Type.... Outlet Input Data
Name.... Outlet 1

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File.... C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP
1\5508RPL4-100 YR-BMP 1.PPW

USE DOWNSTREAM CHANNEL NORMAL DEPTH FOR TW...
Channel Type: Chn-Circular
Channel ID: Chn-Cir - 1

Solution to Mannings Open Channel Flow Equation
(Computed values are based on normal depth.)

CIRCULAR CROSS SECTION

Slope = .005000 ft/ft
Mannings n = 0.01300
Invert Elev. = 397.00 ft
Top of Channel = 400.00 ft
Diameter = 3.0000 ft

S/N:
PondPack Ver:

Compute Time:

Date:

Type.... Pond E-V-Q Table
 Name.... DETENTION 1
 File.... C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submital\Pond Pack\BMP
 1\5508RPL4-100 YR-BMP 1.PPW

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LEVEL POOL ROUTING DATA

HYG Dir = C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submital\Pond
 Pack\BMP 1\

Inflow HYG file = NONE STORED - DETENTION 1 IN 100
 Outflow HYG file = NONE STORED - DETENTION 1 OUT 100

Pond Node Data = DETENTION 1
 Pond Volume Data = DETENTION 1
 Pond Outlet Data = Outlet 1

Infiltration = .1000 in/hr

INITIAL CONDITIONS

 Starting WS Elev = 405.00 ft
 Starting Volume = 0 cu.ft
 Starting Outflow = .00 cfs
 Starting Infiltr. = .00 cfs
 Starting Total Qout= .00 cfs
 Time Increment = 1.00 min

Elevation ft	Outflow cfs	Storage cu.ft	Area sq.ft	Infilt. cfs	Q Total cfs	2S/t + O cfs
405.00	.00	0	33800	.00	.00	.00
405.50	.30	17251	35211	.08	.38	575.42
406.00	.48	35215	36650	.08	.56	1174.41
406.50	.61	53906	38119	.09	.70	1797.58
407.00	.72	73339	39617	.09	.81	2445.45
407.25	.76	83339	40377	.09	.86	2778.80
407.50	1.06	93529	41144	.10	1.16	3118.77
408.00	3.00	114488	42700	.10	3.10	3819.37
408.20	4.17	123091	43320	.10	4.27	4107.29
408.50	14.22	136227	44258	.10	14.32	4555.20
409.00	44.38	158751	45844	.11	44.49	5336.18
409.50	83.30	182076	47458	.11	83.41	6152.59
410.00	102.38	206214	49100	.11	102.49	6976.28

S/N:

PondPack Ver:

Compute Time:

Date:

Type.... Pond Routing Summary
Name.... DETENTION 1 OUT Tag: 100
File.... C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP
1\5508RPL4-100 YR-BMP 1.PPW
Storm... 100 Tag: 100

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Event: 100 yr

LEVEL POOL ROUTING SUMMARY

HYG Dir = C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond
Pack\BMP 1\

Inflow HYG file = NONE STORED - DETENTION 1 IN 100
Outflow HYG file = NONE STORED - DETENTION 1 OUT 100

Pond Node Data = DETENTION 1
Pond Volume Data = DETENTION 1
Pond Outlet Data = Outlet 1

Infiltration = .1000 in/hr

INITIAL CONDITIONS

Starting WS Elev = 405.00 ft
Starting Volume = 0 cu.ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout = .00 cfs
Time Increment = 1.00 min

INFLOW/OUTFLOW HYDROGRAPH SUMMARY

=====

Peak Inflow	=	83.02 cfs	at	255.00 min
Peak Outflow	=	36.61 cfs	at	265.00 min
Peak Infiltration	=	.11 cfs	at	265.00 min

Peak Elevation	=	408.87 ft
Peak Storage	=	152870 cu.ft

=====

MASS BALANCE (cu.ft)

+ Initial Vol	=	0
+ HYG Vol IN	=	224628
- Infiltration	=	10617
- HYG Vol OUT	=	182388
- Retained Vol	=	31565

Unrouted Vol = -58 cu.ft (.026% of Inflow Volume)

WARNING: Outflow hydrograph truncated on right side.

S/N:

PondPack Ver:

Compute Time:

Date:

Type.... Detention Time
Name.... DETENTION 1 OUT Tag: 100
File.... C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP
1\5508RPL4-100 YR-BMP 1.PPW
Storm... 100 Tag: 100

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Event: 100 yr

DETENTION TIMES SUMMARY

HYG Dir = C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond
Pack\BMP 1\

Inflow HYG file = NONE STORED - DETENTION 1 IN 100

Outflow HYG file = NONE STORED - DETENTION 1 OUT 100

Pond Node Data = DETENTION 1

Pond Volume Data = DETENTION 1

Pond Outlet Data = Outlet 1

Infiltration = .1000 in/hr

APPROXIMATE DETENTION TIME

Tp, Outflow + Infilt. = 265.00 min
Tp, Total Inflow = 255.00 min
Peak to Peak = 10.00 min

Qout+Infilt. Centroid = 611.54 min
Inflow Centroid = 221.52 min
Centroid to Centroid = 390.03 min

Weighted Avg. Plug Time = 597.13 min
Max.Plug Vol. Plug Time = 568.76 min
Max.Inflow Plug Volume = 4859 cu.ft (From 254.00 to 255.00 min)

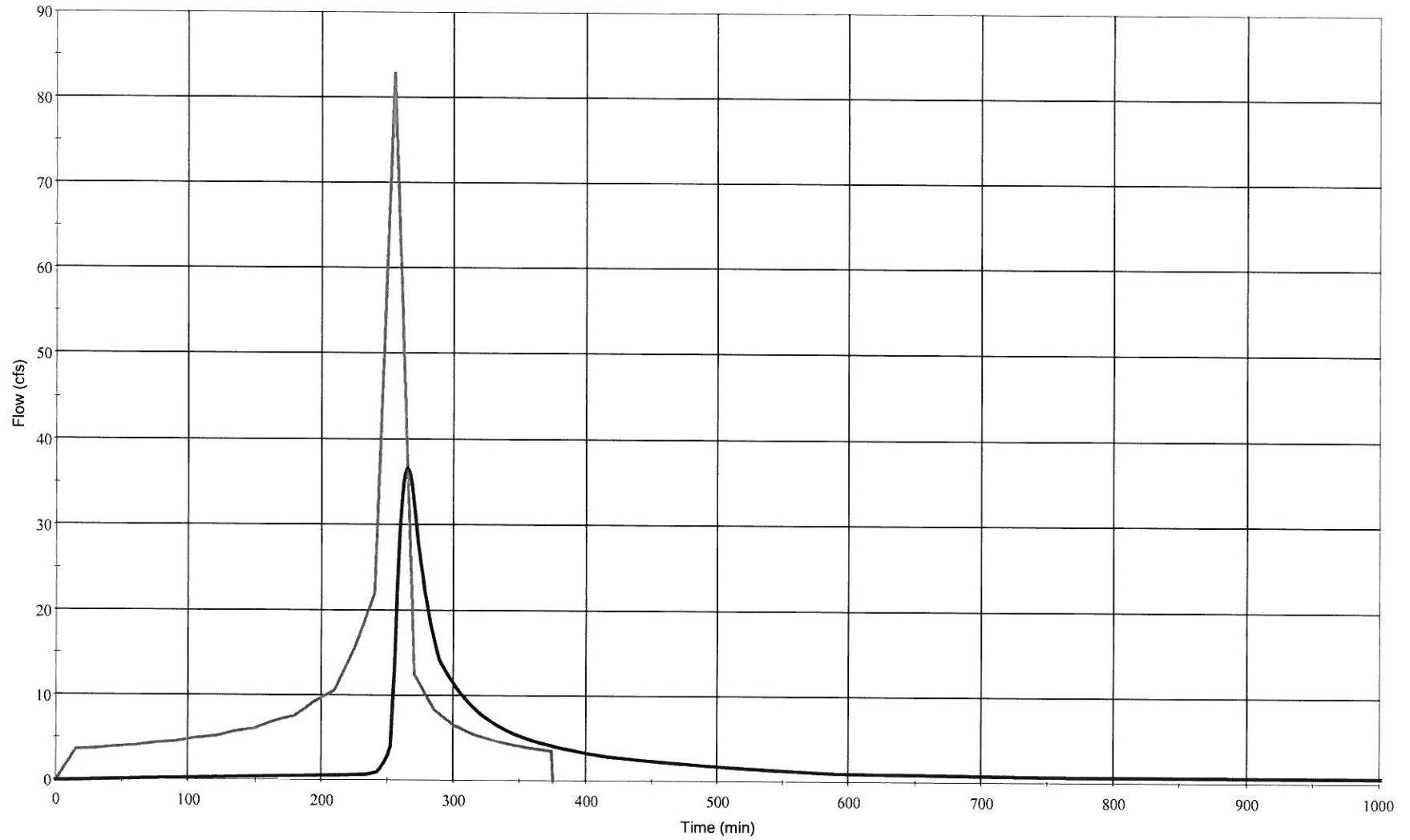
S/N:

PondPack Ver:

Compute Time:

Date:

NODE 21 - DETENTION POND - BMP 1
INFLOW/OUTFLOW HYDROGRAPH
100-YEAR STORM EVENT



— INFLOW= 83.02 CFS — OUTFLOW= 36.61 CFS

Bio-Retention Pond-2

100-year Development Conditions

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c)1991-2003 Version 7.3

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 11/14/12

***** Hydrology Study Control Information *****

WARNER RANCH

100-YEAR STORM EVENT DEVELOPMENT CONDITIONS
BIO-RETENTION POND 2

Shapouri & Associates, Rancho Santa Fe, CA - S/N 968

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:

6 hour, precipitation(inches) = 3.500

24 hour precipitation(inches) = 6.000

P6/P24 = 58.3%

San Diego hydrology manual 'C' values used

Process from Point/Station 26.100 to Point/Station 26.200
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000

[MEDIUM DENSITY RESIDENTIAL]
(4.3 DU/A or Less)

Impervious value, Ai = 0.300

Sub-Area C Value = 0.480

Initial subarea total flow distance = 448.600(Ft.)

Highest elevation = 470.000(Ft.)

Lowest elevation = 460.000(Ft.)

Elevation difference = 10.000(Ft.) Slope = 2.229 %

Top of Initial Area Slope adjusted by User to 1.700 %

Bottom of Initial Area Slope adjusted by User to 5.140 %

INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:

The maximum overland flow distance is 80.00 (Ft)

for the top area slope value of 1.70 %, in a development type of
4.3 DU/A or Less

In Accordance With Figure 3-3

Initial Area Time of Concentration = 8.36 minutes

TC = $[1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5}] / (\% \text{ slope}^{(1/3)})]$

TC = $[1.8 * (1.1 - 0.4800) * (80.000^{.5}) / (1.700^{(1/3)})] = 8.36$

The initial area total distance of 448.60 (Ft.) entered leaves a
remaining distance of 368.60 (Ft.)

Using Figure 3-4, the travel time for this distance is 2.32 minutes

for a distance of 368.60 (Ft.) and a slope of 5.14 %

with an elevation difference of 18.95(Ft.) from the end of the top area

Tt = $[11.9 * \text{length}(\text{Mi})^{.3}] / (\text{elevation change}(\text{Ft.}))^{.385} * 60(\text{min/hr})$

= 2.318 Minutes

Tt = $[(11.9 * 0.0698^{.3}) / (18.95)]^{.385} = 2.32$

Total initial area Ti = 8.36 minutes from Figure 3-3 formula plus
2.32 minutes from the Figure 3-4 formula = 10.68 minutes

Rainfall intensity (I) = 5.652(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.480

Subarea runoff = 3.201(CFS)

Total initial stream area = 1.180(Ac.)

Process from Point/Station 26.200 to Point/Station 22.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 455.000(Ft.)

Downstream point/station elevation = 454.500(Ft.)

Pipe length = 11.90(Ft.) Manning's N = 0.015

No. of pipes = 1 Required pipe flow = 3.201(CFS)

Given pipe size = 24.00(In.)

Calculated individual pipe flow = 3.201(CFS)

Normal flow depth in pipe = 4.58(In.)

Flow top width inside pipe = 18.87(In.)

Critical Depth = 7.50(In.)

Pipe flow velocity = 7.65(Ft/s)

Travel time through pipe = 0.03 min.

Time of concentration (TC) = 10.71 min.

Process from Point/Station 22.000 to Point/Station 23.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 454.500(Ft.)

Downstream point/station elevation = 436.500(Ft.)

Pipe length = 266.25(Ft.) Manning's N = 0.015

No. of pipes = 1 Required pipe flow = 3.201(CFS)

Given pipe size = 24.00(In.)

Calculated individual pipe flow = 3.201(CFS)

Normal flow depth in pipe = 4.08(In.)

Flow top width inside pipe = 18.03(In.)

Critical Depth = 7.50(In.)

Pipe flow velocity = 9.05(Ft/s)

Travel time through pipe = 0.49 min.

Time of concentration (TC) = 11.20 min.

Process from Point/Station 22.000 to Point/Station 23.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 1.180(Ac.)

Runoff from this stream = 3.201(CFS)

Time of concentration = 11.20 min.

Rainfall intensity = 5.482(In/Hr)

Program is now starting with Main Stream No. 2

 Process from Point/Station 27.100 to Point/Station 27.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL
 (4.3 DU/A or Less)]
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Initial subarea total flow distance = 598.000(Ft.)
 Highest elevation = 470.000(Ft.)
 Lowest elevation = 442.000(Ft.)
 Elevation difference = 28.000(Ft.) Slope = 4.682 %
 Top of Initial Area Slope adjusted by User to 1.830 %
 Bottom of Initial Area Slope adjusted by User to 6.910 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 80.00 (Ft)
 for the top area slope value of 1.83 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 8.16 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4800) * (80.000^{.5})] / (1.830^{(1/3)}) = 8.16$
 The initial area total distance of 598.00 (Ft.) entered leaves a
 remaining distance of 518.00 (Ft.)
 Using Figure 3-4, the travel time for this distance is 2.69 minutes
 for a distance of 518.00 (Ft.) and a slope of 6.91 %
 with an elevation difference of 35.79(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60 (min/hr)$
 $= 2.688 \text{ Minutes}$
 $Tt = [(11.9 * 0.0981^3) / (35.79)]^{.385} = 2.69$
 Total initial area Ti = 8.16 minutes from Figure 3-3 formula plus
 2.69 minutes from the Figure 3-4 formula = 10.85 minutes
 Rainfall intensity (I) = 5.595(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.480
 Subarea runoff = 3.706(CFS)
 Total initial stream area = 1.380 (Ac.)

 Process from Point/Station 27.200 to Point/Station 23.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 437.000(Ft.)
 Downstream point/station elevation = 436.500(Ft.)
 Pipe length = 26.25(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 3.706(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 3.706(CFS)
 Normal flow depth in pipe = 6.00(In.)
 Flow top width inside pipe = 20.78(In.)
 Critical Depth = 8.08(In.)
 Pipe flow velocity = 6.03(Ft/s)
 Travel time through pipe = 0.07 min.
 Time of concentration (TC) = 10.92 min.

 Process from Point/Station 27.200 to Point/Station 23.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 2
 Stream flow area = 1.380(Ac.)
 Runoff from this stream = 3.706(CFS)
 Time of concentration = 10.92 min.
 Rainfall intensity = 5.571(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	3.201	11.20	5.482
2	3.706	10.92	5.571
Qmax(1) =			
	1.000 *	1.000 *	3.201) +
	0.984 *	1.000 *	3.706) + = 6.848
Qmax(2) =			
	1.000 *	0.975 *	3.201) +
	1.000 *	1.000 *	3.706) + = 6.828

Total of 2 main streams to confluence:
 Flow rates before confluence point:
 3.201 3.706
 Maximum flow rates at confluence using above data:
 6.848 6.828
 Area of streams before confluence:
 1.180 1.380

Results of confluence:
 Total flow rate = 6.848(CFS)
 Time of concentration = 11.198 min.
 Effective stream area after confluence = 2.560 (Ac.)

 Process from Point/Station 23.000 to Point/Station 24.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 436.500(Ft.)
 Downstream point/station elevation = 426.500(Ft.)
 Pipe length = 131.00(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 6.848(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 6.848(CFS)
 Normal flow depth in pipe = 5.76(In.)
 Flow top width inside pipe = 20.50(In.)
 Critical Depth = 11.14(In.)
 Pipe flow velocity = 11.80(Ft/s)
 Travel time through pipe = 0.19 min.
 Time of concentration (TC) = 11.38 min.

 Process from Point/Station 23.000 to Point/Station 24.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 2.560(Ac.)
 Runoff from this stream = 6.848(CFS)
 Time of concentration = 11.38 min.
 Rainfall intensity = 5.424(In/Hr)
 Program is now starting with Main Stream No. 2

 Process from Point/Station 28.100 to Point/Station 28.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 1.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.480
 Initial subarea total flow distance = 471.780(Ft.)
 Highest elevation = 460.000(Ft.)
 Lowest elevation = 433.000(Ft.)
 Elevation difference = 27.000(Ft.) Slope = 5.723 %
 Top of Initial Area Slope adjusted by User to 2.000 %
 Bottom of Initial Area Slope adjusted by User to 7.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 80.00 (Ft)
 for the top area slope value of 2.00 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 7.92 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4800) * (80.000^{.5}) / (2.000^{(1/3)})] = 7.92$
 The initial area total distance of 471.78 (Ft.) entered leaves a
 remaining distance of 391.78 (Ft.)
 Using Figure 3-4, the travel time for this distance is 2.16 minutes
 for a distance of 391.78 (Ft.) and a slope of 7.00 %
 with an elevation difference of 27.42(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60(min/hr)$
 $= 2.157 Minutes$
 $Tt = [(11.9 * 0.0742^3) / (27.42)]^{.385} = 2.16$
 Total initial area Ti = 7.92 minutes from Figure 3-3 formula plus
 2.16 minutes from the Figure 3-4 formula = 10.08 minutes
 Rainfall intensity (I) = 5.867(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.480
 Subarea runoff = 3.295(CFS)
 Total initial stream area = 1.170(Ac.)

 Process from Point/Station 28.200 to Point/Station 24.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 428.000(Ft.)
 Downstream point/station elevation = 426.500(Ft.)
 Pipe length = 9.00(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 3.295(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 3.295(CFS)
 Normal flow depth in pipe = 3.32(In.)
 Flow top width inside pipe = 16.58(In.)
 Critical Depth = 7.61(In.)
 Pipe flow velocity = 12.53(Ft/s)
 Travel time through pipe = 0.01 min.
 Time of concentration (TC) = 10.09 min.

 Process from Point/Station 28.200 to Point/Station 24.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 1.170(Ac.)
 Runoff from this stream = 3.295(CFS)
 Time of concentration = 10.09 min.
 Rainfall intensity = 5.863(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	6.848	11.38	5.424
2	3.295	10.09	5.863
Qmax(1) =	1.000 * 0.925	1.000 * 1.000	6.848) + 3.295) + = 9.897
Qmax(2) =	1.000 * 1.000	0.887 * 1.000	6.848) + 3.295) + = 9.366

Total of 2 main streams to confluence:

Flow rates before confluence point:

6.848 3.295

Maximum flow rates at confluence using above data:

9.897 9.366

Area of streams before confluence:

2.560 1.170

Results of confluence:

Total flow rate = 9.897(CFS)

Time of concentration = 11.383 min.

Effective stream area after confluence = 3.730(Ac.)

 Process from Point/Station 24.000 to Point/Station 25.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 426.500(Ft.)
 Downstream point/station elevation = 404.500(Ft.)
 Pipe length = 288.15(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 9.897(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 9.897(CFS)
 Normal flow depth in pipe = 6.95(In.)
 Flow top width inside pipe = 21.77(In.)
 Critical Depth = 13.50(In.)
 Pipe flow velocity = 13.12(Ft/s)
 Travel time through pipe = 0.37 min.
 Time of concentration (TC) = 11.75 min.

 Process from Point/Station 24.000 to Point/Station 25.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 1
 Stream flow area = 3.730(Ac.)
 Runoff from this stream = 9.897(CFS)
 Time of concentration = 11.75 min.
 Rainfall intensity = 5.315(In/Hr)
 Program is now starting with Main Stream No. 2

 Process from Point/Station 29.100 to Point/Station 29.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.100
 Decimal fraction soil group C = 0.900
 Decimal fraction soil group D = 0.000
 (MEDIUM DENSITY RESIDENTIAL)
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.477
 Initial subarea total flow distance = 359.770(Ft.)
 Highest elevation = 432.000(Ft.)
 Lowest elevation = 411.000(Ft.)
 Elevation difference = 21.000(Ft.) Slope = 5.837 %
 Top of Initial Area Slope adjusted by User to 2.000 %
 Bottom of Initial Area Slope adjusted by User to 8.940 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 80.00 (Ft)
 for the top area slope value of 2.00 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 7.96 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})$
 $TC = [1.8 * (1.1 - 0.4770) * (80.000^{.5})] / (2.000^{(1/3)}) = 7.96$
 The initial area total distance of 359.77 (Ft.) entered leaves a

remaining distance of 279.77 (Ft.)
 Using Figure 3-4, the travel time for this distance is 1.51 minutes
 for a distance of 279.77 (Ft.) and a slope of 8.94 %
 with an elevation difference of 25.01(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60(min/hr)$
 $= 1.515 Minutes$
 $Tt = [(11.9 * 0.0530^3) / (25.01)]^{.385} = 1.51$
 Total initial area Ti = 7.96 minutes from Figure 3-3 formula plus
 1.51 minutes from the Figure 3-4 formula = 9.48 minutes
 Rainfall intensity (I) = 6.106(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.477
 Subarea runoff = 2.359(CFS)
 Total initial stream area = 0.810(Ac.)

 Process from Point/Station 29.200 to Point/Station 25.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 406.000(Ft.)
 Downstream point/station elevation = 404.500(Ft.)
 Pipe length = 9.92(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 2.359(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 2.359(CFS)
 Normal flow depth in pipe = 2.90(In.)
 Flow top width inside pipe = 15.64(In.)
 Critical Depth = 6.39(In.)
 Pipe flow velocity = 10.96(Ft/s)
 Travel time through pipe = 0.02 min.
 Time of concentration (TC) = 9.49 min.

 Process from Point/Station 29.200 to Point/Station 25.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 2
 Stream flow area = 0.810(Ac.)
 Runoff from this stream = 2.359(CFS)
 Time of concentration = 9.49 min.
 Rainfall intensity = 6.099(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	9.897	11.75	5.315
2	2.359	9.49	6.099
Qmax(1) =	1.000 * 0.871 *	1.000 * 1.000 *	9.897) + 2.359) + = 11.952
Qmax(2) =	1.000 * 1.000 *	0.808 * 1.000 *	9.897) + 2.359) + = 10.354

Total of 2 main streams to confluence:

Flow rates before confluence point:

9.897 2.359

Maximum flow rates at confluence using above data:

11.952 10.354

Area of streams before confluence:

3.730 0.810

Results of confluence:

Total flow rate = 11.952(CFS)

Time of concentration = 11.749 min.

Effective stream area after confluence = 4.540(Ac.)

Process from Point/Station 25.000 to Point/Station 28.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 404.500(Ft.)
Downstream point/station elevation = 386.500(Ft.)
Pipe length = 201.65(Ft.) Manning's N = 0.015
No. of pipes = 1 Required pipe flow = 11.952(CFS)
Given pipe size = 24.00(In.)
Calculated individual pipe flow = 11.952(CFS)
Normal flow depth in pipe = 7.35(In.)
Flow top width inside pipe = 22.13(In.)
Critical Depth = 14.91(In.)
Pipe flow velocity = 14.64(Ft/s)
Travel time through pipe = 0.23 min.
Time of concentration (TC) = 11.98 min.

Process from Point/Station 25.000 to Point/Station 28.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 4.540(Ac.)

Runoff from this stream = 11.952(CFS)

Time of concentration = 11.98 min.

Rainfall intensity = 5.249(In/Hr)

Program is now starting with Main Stream No. 2

Process from Point/Station 30.100 to Point/Station 30.200
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 1.000
Decimal fraction soil group D = 0.000
[MEDIUM DENSITY RESIDENTIAL
(4.3 DU/A or Less)]

Impervious value, Ai = 0.300

Sub-Area C Value = 0.480

Initial subarea total flow distance = 531.500(Ft.)

Highest elevation = 478.000(Ft.)

Lowest elevation = 448.000(Ft.)

Elevation difference = 30.000(Ft.) Slope = 5.644 %

Top of Initial Area Slope adjusted by User to 1.370 %

Bottom of Initial Area Slope adjusted by User to 10.000 %

INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:

The maximum overland flow distance is 70.00 (Ft)

for the top area slope value of 1.37 %, in a development type of
4.3 DU/A or Less

In Accordance With Figure 3-3

Initial Area Time of Concentration = 8.41 minutes

TC = $[1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5}] / \{\% \text{slope}^{(1/3)}\}$

TC = $[1.8 * (1.1 - 0.4800) * (70.000^{.5})] / (1.370^{(1/3)}) = 8.41$

The initial area total distance of 531.50 (Ft.) entered leaves a
remaining distance of 461.50 (Ft.)

Using Figure 3-4, the travel time for this distance is 2.13 minutes

for a distance of 461.50 (Ft.) and a slope of 10.00 %

with an elevation difference of 46.15(Ft.) from the end of the top area

Tt = $[11.9 * \text{length}(\text{Mi})^3 / (\text{elevation change}(\text{Ft.}))]^{.385} * 60(\text{min/hr})$
= 2.133 Minutes

Tt = $[(11.9 * 0.0874^3) / (46.15)]^{.385} = 2.13$

Total initial area Ti = 8.41 minutes from Figure 3-3 formula plus

2.13 minutes from the Figure 3-4 formula = 10.54 minutes

Rainfall intensity (I) = 5.700(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.480

Subarea runoff = 3.749(CFS)

Total initial stream area = 1.370(Ac.)

Process from Point/Station 30.200 to Point/Station 26.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 443.000(Ft.)
Downstream point/station elevation = 439.500(Ft.)
Pipe length = 37.34(Ft.) Manning's N = 0.015
No. of pipes = 1 Required pipe flow = 3.749(CFS)
Given pipe size = 24.00(In.)
Calculated individual pipe flow = 3.749(CFS)
Normal flow depth in pipe = 4.07(In.)
Flow top width inside pipe = 18.01(In.)
Critical Depth = 8.14(In.)
Pipe flow velocity = 10.64(Ft/s)
Travel time through pipe = 0.06 min.
Time of concentration (TC) = 10.60 min.

 Process from Point/Station 30.200 to Point/Station 26.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 1.370(Ac.)
 Runoff from this stream = 3.749(CFS)
 Time of concentration = 10.60 min.
 Rainfall intensity = 5.680(In/Hr)

 Process from Point/Station 31.100 to Point/Station 31.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.100
 Decimal fraction soil group C = 0.900
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.477
 Initial subarea total flow distance = 553.370(Ft.)
 Highest elevation = 480.000(Ft.)
 Lowest elevation = 446.000(Ft.)
 Elevation difference = 34.000(Ft.) Slope = 6.144 %
 Top of Initial Area Slope adjusted by User to 1.330 %
 Bottom of Initial Area Slope adjusted by User to 10.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 70.00 (Ft)
 for the top area slope value of 1.33 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 8.53 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})$
 $TC = [1.8 * (1.1 - 0.4770) * (70.000^{.5})] / (1.330^{(1/3)}) = 8.53$
 The initial area total distance of 553.37 (Ft.) entered leaves a
 remaining distance of 483.37 (Ft.)
 Using Figure 3-4, the travel time for this distance is 2.21 minutes
 for a distance of 483.37 (Ft.) and a slope of 10.00 %
 with an elevation difference of 48.34(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60(min/hr)$
 $= 2.210 Minutes$
 $Tt = [(11.9 * 0.0915^3) / (48.34)]^{.385} = 2.21$
 Total initial area Ti = 8.53 minutes from Figure 3-3 formula plus
 2.21 minutes from the Figure 3-4 formula = 10.74 minutes
 Rainfall intensity (I) = 5.631(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.477
 Subarea runoff = 6.017(CFS)
 Total initial stream area = 2.240(Ac.)

 Process from Point/Station 31.200 to Point/Station 26.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 441.000(Ft.)
 Downstream point/station elevation = 439.500(Ft.)
 Pipe length = 11.90(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 6.017(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 6.017(CFS)
 Normal flow depth in pipe = 4.77(In.)
 Flow top width inside pipe = 19.15(In.)
 Critical Depth = 10.41(In.)
 Pipe flow velocity = 13.58(Ft/s)
 Travel time through pipe = 0.01 min.
 Time of concentration (TC) = 10.76 min.

 Process from Point/Station 31.200 to Point/Station 26.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 2.240(Ac.)
 Runoff from this stream = 6.017(CFS)
 Time of concentration = 10.76 min.
 Rainfall intensity = 5.626(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	3.749	10.60	5.680
2	6.017	10.76	5.626
Qmax(1) =	1.000 *	1.000 *	3.749) +
	1.000 *	0.985 *	6.017) + = 9.677
Qmax(2) =	0.990 *	1.000 *	3.749) +
	1.000 *	1.000 *	6.017) + = 9.730

Total of 2 streams to confluence:
 Flow rates before confluence point:
 3.749 6.017
 Maximum flow rates at confluence using above data:
 9.677 9.730
 Area of streams before confluence:
 1.370 2.240
 Results of confluence:
 Total flow rate = 9.730(CFS)
 Time of concentration = 10.757 min.
 Effective stream area after confluence = 3.610(Ac.)

 Process from Point/Station 26.000 to Point/Station 27.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 439.500(Ft.)
 Downstream point/station elevation = 392.500(Ft.)
 Pipe length = 497.00(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 9.730(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 9.730(CFS)
 Normal flow depth in pipe = 6.52(In.)
 Flow top width inside pipe = 21.35(In.)
 Critical Depth = 13.37(In.)
 Pipe flow velocity = 14.10(Ft/s)
 Travel time through pipe = 0.59 min.
 Time of concentration (TC) = 11.34 min.

 Process from Point/Station 26.000 to Point/Station 27.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 3.610(Ac.)
 Runoff from this stream = 9.730(CFS)
 Time of concentration = 11.34 min.
 Rainfall intensity = 5.436(In/Hr)

 Process from Point/Station 32.100 to Point/Station 32.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.800
 Decimal fraction soil group C = 0.200
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.456
 Initial subarea total flow distance = 590.500(Ft.)
 Highest elevation = 448.000(Ft.)
 Lowest elevation = 398.000(Ft.)
 Elevation difference = 50.000(Ft.) Slope = 8.467 %
 Top of Initial Area Slope adjusted by User to 2.000 %
 Bottom of Initial Area Slope adjusted by User to 10.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 80.00 (Ft)
 for the top area slope value of 2.00 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 8.23 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4560) * (80.000^{.5})] / (2.000^{(1/3)}) = 8.23$
 The initial area total distance of 590.50 (Ft.) entered leaves a
 remaining distance of 510.50 (Ft.)

Using Figure 3-4, the travel time for this distance is 2.31 minutes
 for a distance of 510.50 (Ft.) and a slope of 10.00 %
 with an elevation difference of 51.05(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation\ change(Ft.))^{.385} * 60(min/hr)$
 = 2.305 Minutes
 $Tt = [(11.9 * 0.0967^3) / (51.05)]^{.385} = 2.31$
 Total initial area Ti = 8.23 minutes from Figure 3-3 formula plus
 2.31 minutes from the Figure 3-4 formula = 10.53 minutes
 Rainfall intensity (I) = 5.702(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.456
 Subarea runoff = 3.016(CFS)
 Total initial stream area = 1.160(Ac.)

 Process from Point/Station 32.200 to Point/Station 27.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 393.000(Ft.)
 Downstream point/station elevation = 392.500(Ft.)
 Pipe length = 25.96(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 3.016(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 3.016(CFS)
 Normal flow depth in pipe = 5.40(In.)
 Flow top width inside pipe = 20.04(In.)
 Critical Depth = 7.26(In.)
 Pipe flow velocity = 5.71(Ft/s)
 Travel time through pipe = 0.08 min.
 Time of concentration (TC) = 10.61 min.

 Process from Point/Station 32.200 to Point/Station 27.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 1.160(Ac.)
 Runoff from this stream = 3.016(CFS)
 Time of concentration = 10.61 min.
 Rainfall intensity = 5.676(In/Hr)

 Process from Point/Station 33.100 to Point/Station 33.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.900
 Decimal fraction soil group C = 0.100
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.453
 Initial subarea total flow distance = 493.500(Ft.)
 Highest elevation = 446.000(Ft.)

Lowest elevation = 398.000(Ft.)
 Elevation difference = 48.000(Ft.) Slope = 9.726 %
 Top of Initial Area Slope adjusted by User to 10.000 %
 Bottom of Initial Area Slope adjusted by User to 10.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 10.00 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.41 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4530) * (100.000^{.5})] / (10.000^{(1/3)}) = 5.41$
 The initial area total distance of 493.50 (Ft.) entered leaves a
 remaining distance of 393.50 (Ft.)
 Using Figure 3-4, the travel time for this distance is 1.89 minutes
 for a distance of 393.50 (Ft.) and a slope of 10.00 %
 with an elevation difference of 39.35(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60 (min/hr)$
 $= 1.887 \text{ Minutes}$
 $Tt = [11.9 * 0.0745^3] / (39.35)^{.385} = 1.89$
 Total initial area Ti = 5.41 minutes from Figure 3-3 formula plus
 1.89 minutes from the Figure 3-4 formula = 7.29 minutes
 Rainfall intensity (I) = 7.229(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.453
 Subarea runoff = 7.237(CFS)
 Total initial stream area = 2.210(Ac.)

 Process from Point/Station 33.200 to Point/Station 27.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 393.000(Ft.)
 Downstream point/station elevation = 392.500(Ft.)
 Pipe length = 9.90(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 7.237(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 7.237(CFS)
 Normal flow depth in pipe = 6.58(In.)
 Flow top width inside pipe = 21.41(In.)
 Critical Depth = 11.46(In.)
 Pipe flow velocity = 10.35(Ft/s)
 Travel time through pipe = 0.02 min.
 Time of concentration (TC) = 7.31 min.

 Process from Point/Station 33.200 to Point/Station 27.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 3
 Stream flow area = 2.210(Ac.)
 Runoff from this stream = 7.237(CFS)
 Time of concentration = 7.31 min.
 Rainfall intensity = 7.219(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	9.730	11.34	5.436
2	3.016	10.61	5.676
3	7.237	7.31	7.219
Qmax(1) =	1.000 *	1.000 *	9.730) +
	0.958 *	1.000 *	3.016) +
	0.753 *	1.000 *	7.237) + =
Qmax(2) =	1.000 *	0.935 *	9.730) +
	1.000 *	1.000 *	3.016) +
	0.786 *	1.000 *	7.237) + =
Qmax(3) =	1.000 *	0.644 *	9.730) +
	1.000 *	0.689 *	3.016) +
	1.000 *	1.000 *	7.237) + =

Total of 3 streams to confluence:
 Flow rates before confluence point:
 9.730 3.016 7.237
 Maximum flow rates at confluence using above data:
 18.069 17.807 15.583
 Area of streams before confluence:
 3.610 1.160 2.210
 Results of confluence:
 Total flow rate = 18.069(CFS)
 Time of concentration = 11.344 min.
 Effective stream area after confluence = 6.980(Ac.)

 Process from Point/Station 27.000 to Point/Station 28.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 392.500(Ft.)
 Downstream point/station elevation = 386.500(Ft.)
 Pipe length = 73.04(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 18.069(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 18.069(CFS)
 Normal flow depth in pipe = 9.36(In.)
 Flow top width inside pipe = 23.41(In.)
 Critical Depth = 18.38(In.)
 Pipe flow velocity = 15.93(Ft/s)
 Travel time through pipe = 0.08 min.
 Time of concentration (TC) = 11.42 min.

 Process from Point/Station 27.000 to Point/Station 28.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 6.980(Ac.)
 Runoff from this stream = 18.069(CFS)
 Time of concentration = 11.42 min.
 Rainfall intensity = 5.413(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
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1	11.952	11.98	5.249
2	18.069	11.42	5.413

Qmax(1) =

1.000 *	1.000 *	11.952) +	
0.970 *	1.000 *	18.069) + =	29.474

Qmax(2) =

1.000 *	0.953 *	11.952) +	
1.000 *	1.000 *	18.069) + =	29.465

Total of 2 main streams to confluence:

Flow rates before confluence point:

11.952	18.069
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Maximum flow rates at confluence using above data:

29.474	29.465
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Area of streams before confluence:

4.540	6.980
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Results of confluence:

Total flow rate = 29.474(CFS)

Time of concentration = 11.978 min.

Effective stream area after confluence = 11.520(Ac.)

 Process from Point/Station 28.000 to Point/Station 35.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 386.500(Ft.)
 Downstream point/station elevation = 372.000(Ft.)
 Pipe length = 204.90(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 29.474(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 29.474(CFS)
 Normal flow depth in pipe = 12.91(In.)
 Flow top width inside pipe = 23.93(In.)
 Critical Depth = 22.26(In.)
 Pipe flow velocity = 17.11(Ft/s)
 Travel time through pipe = 0.20 min.
 Time of concentration (TC) = 12.18 min.

 Process from Point/Station 28.000 to Point/Station 35.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
 Stream flow area = 11.520(Ac.)
 Runoff from this stream = 29.474(CFS)
 Time of concentration = 12.18 min.
 Rainfall intensity = 5.193(In/Hr)
 Program is now starting with Main Stream No. 2

 Process from Point/Station 36.100 to Point/Station 36.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.700

Decimal fraction soil group C = 0.000

Decimal fraction soil group D = 0.300

[HIGH DENSITY RESIDENTIAL]

(24.0 DU/A or Less)

Impervious value, Ai = 0.650

Sub-Area C Value = 0.682

Initial subarea total flow distance = 584.900(Ft.)

Highest elevation = 412.500(Ft.)

Lowest elevation = 394.000(Ft.)

Elevation difference = 18.500(Ft.) Slope = 3.163 %

Top of Initial Area Slope adjusted by User to 3.600 %

Bottom of Initial Area Slope adjusted by User to 2.310 %

INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:

The maximum overland flow distance is 90.00 (Ft)

for the top area slope value of 3.60 %, in a development type of 24.0 DU/A or Less

In Accordance With Figure 3-3

Initial Area Time of Concentration = 4.66 minutes

TC = $[1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5}] / (\% \text{ slope}^{(1/3)})]$

TC = $[1.8 * (1.1 - 0.6820) * (90.000^{.5})] / (3.600^{(1/3)}) = 4.66$

The initial area total distance of 584.90 (Ft.) entered leaves a remaining distance of 494.90 (Ft.)

Using Figure 3-4, the travel time for this distance is 3.96 minutes

for a distance of 494.90 (Ft.) and a slope of 2.31 %

with an elevation difference of 11.43(Ft.) from the end of the top area

Tt = $[11.9 * \text{length}(\text{Mi})^3] / (\text{elevation change}(\text{Ft.}))^{.385} * 60(\text{min/hr})$

= 3.957 Minutes

Tt = $[(11.9 * 0.0937^3) / (11.43)]^{.385} = 3.96$

Total initial area Ti = 4.66 minutes from Figure 3-3 formula plus

3.96 minutes from the Figure 3-4 formula = 8.61 minutes

Rainfall intensity (I) = 6.493(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.682

Subarea runoff = 11.955(CFS)

Total initial stream area = 2.700(Ac.)

 Process from Point/Station 36.200 to Point/Station 30.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 389.000(Ft.)
 Downstream point/station elevation = 383.500(Ft.)
 Pipe length = 239.43(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 11.955(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 11.955(CFS)
 Normal flow depth in pipe = 10.59(In.)
 Flow top width inside pipe = 23.83(In.)
 Critical Depth = 14.91(In.)
 Pipe flow velocity = 8.95(Ft/s)
 Travel time through pipe = 0.45 min.
 Time of concentration (TC) = 9.06 min.

 Process from Point/Station 36.200 to Point/Station 30.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 2.700(Ac.)
 Runoff from this stream = 11.955(CFS)
 Time of concentration = 9.06 min.
 Rainfall intensity = 6.285(In/Hr)

 Process from Point/Station 37.100 to Point/Station 37.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.600
 Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.400
 [HIGH DENSITY RESIDENTIAL]
 (24.0 DU/A or Less)
 Impervious value, Ai = 0.650
 Sub-Area C Value = 0.686
 Initial subarea total flow distance = 450.690(Ft.)
 Highest elevation = 395.500(Ft.)
 Lowest elevation = 390.000(Ft.)
 Elevation difference = 5.500(Ft.) Slope = 1.220 %
 Top of Initial Area Slope adjusted by User to 1.200 %
 Bottom of Initial Area Slope adjusted by User to 1.810 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 65.00 (Ft)
 for the top area slope value of 1.20 %, in a development type of
 24.0 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.65 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance} (Ft.)^{.5}] / (\% \text{ slope}^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.6860) * (65.000^{.5})] / (1.200^{(1/3)}) = 5.65$
 The initial area total distance of 450.69 (Ft.) entered leaves a
 remaining distance of 385.69 (Ft.)

Using Figure 3-4, the travel time for this distance is 3.59 minutes
 for a distance of 385.69 (Ft.) and a slope of 1.81 %
 with an elevation difference of 6.98(Ft.) from the end of the top area
 $Tt = [11.9 * \text{length} (Mi)^3] / (\text{elevation change} (Ft.))^{.385} * 60 (\text{min/hr})$
 = 3.588 Minutes
 $Tt = [(11.9 * 0.0730^3) / (6.98)]^{.385} = 3.59$
 Total initial area Ti = 5.65 minutes from Figure 3-3 formula plus
 3.59 minutes from the Figure 3-4 formula = 9.24 minutes
 Rainfall intensity (I) = 6.205(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.686
 Subarea runoff = 7.321(CFS)
 Total initial stream area = 1.720(Ac.)

 Process from Point/Station 37.200 to Point/Station 30.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 385.000(Ft.)
 Downstream point/station elevation = 383.500(Ft.)
 Pipe length = 9.90(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 7.321(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 7.321(CFS)
 Normal flow depth in pipe = 5.02(In.)
 Flow top width inside pipe = 19.52(In.)
 Critical Depth = 11.53(In.)
 Pipe flow velocity = 15.35(Ft/s)
 Travel time through pipe = 0.01 min.
 Time of concentration (TC) = 9.25 min.

 Process from Point/Station 37.200 to Point/Station 30.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 1.720(Ac.)
 Runoff from this stream = 7.321(CFS)
 Time of concentration = 9.25 min.
 Rainfall intensity = 6.200(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	11.955	9.06	6.285
2	7.321	9.25	6.200
Qmax(1) =	1.000 *	1.000 *	11.955) +
	1.000 *	0.979 *	7.321) + =
Qmax(2) =	0.987 *	1.000 *	11.955) +
	1.000 *	1.000 *	7.321) + =

Total of 2 streams to confluence:

Flow rates before confluence point:

11.955 7.321

Maximum flow rates at confluence using above data:

19.125 19.117

Area of streams before confluence:

2.700 1.720

Results of confluence:

Total flow rate = 19.125(CFS)

Time of concentration = 9.060 min.

Effective stream area after confluence = 4.420(Ac.)

Process from Point/Station 30.000 to Point/Station 31.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 383.500(Ft.)
Downstream point/station elevation = 380.000(Ft.)
Pipe length = 246.76(Ft.) Manning's N = 0.015
No. of pipes = 1 Required pipe flow = 19.125(CFS)
Given pipe size = 24.00(In.)
Calculated individual pipe flow = 19.125(CFS)
Normal flow depth in pipe = 16.52(In.)
Flow top width inside pipe = 22.23(In.)
Critical Depth = 18.88(In.)
Pipe flow velocity = 8.29(Ft/s)
Travel time through pipe = 0.50 min.
Time of concentration (TC) = 9.56 min.

Process from Point/Station 30.000 to Point/Station 31.000
**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
Stream flow area = 4.420(Ac.)
Runoff from this stream = 19.125(CFS)
Time of concentration = 9.56 min.
Rainfall intensity = 6.072(In/Hr)

Process from Point/Station 38.100 to Point/Station 38.200
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.100
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.900
[HIGH DENSITY RESIDENTIAL]
(24.0 DU/A or Less)
Impervious value, Ai = 0.650
Sub-Area C Value = 0.706
Initial subarea total flow distance = 788.000(Ft.)
Highest elevation = 402.000(Ft.)
Lowest elevation = 385.500(Ft.)
Elevation difference = 16.500(Ft.) Slope = 2.094 %

Top of Initial Area Slope adjusted by User to 3.000 %

Bottom of Initial Area Slope adjusted by User to 1.810 %

INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:

The maximum overland flow distance is 90.00 (Ft)

for the top area slope value of 3.00 %, in a development type of
24.0 DU/A or Less

In Accordance With Figure 3-3

Initial Area Time of Concentration = 4.66 minutes

TC = $[1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5}] / (\% \text{ slope}^{(1/3)})]$

TC = $[1.8 * (1.1 - 0.7060) * (90.000^{.5})] / (3.000^{(1/3)}) = 4.66$

The initial area total distance of 788.00 (Ft.) entered leaves a
remaining distance of 698.00 (Ft.)

Using Figure 3-4, the travel time for this distance is 5.66 minutes
for a distance of 698.00 (Ft.) and a slope of 1.81 %

with an elevation difference of 12.63(Ft.) from the end of the top area
Tt = $[11.9 * \text{length}(\text{Mi})^{.3}] / (\text{elevation change}(\text{Ft.}))^{.385} * 60(\text{min/hr})$

= 5.664 Minutes

Tt = $[(11.9 * 0.1322^{.3}) / (12.63)]^{.385} = 5.66$

Total initial area Ti = 4.66 minutes from Figure 3-3 formula plus
5.66 minutes from the Figure 3-4 formula = 10.33 minutes

Rainfall intensity (I) = 5.775(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.706

Subarea runoff = 8.644(CFS)

Total initial stream area = 2.120(Ac.)

Process from Point/Station 38.200 to Point/Station 31.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 380.500(Ft.)
Downstream point/station elevation = 380.000(Ft.)
Pipe length = 27.00(Ft.) Manning's N = 0.015
No. of pipes = 1 Required pipe flow = 8.644(CFS)
Given pipe size = 24.00(In.)
Calculated individual pipe flow = 8.644(CFS)
Normal flow depth in pipe = 9.40(In.)
Flow top width inside pipe = 23.43(In.)
Critical Depth = 12.58(In.)
Pipe flow velocity = 7.58(Ft/s)
Travel time through pipe = 0.06 min.
Time of concentration (TC) = 10.39 min.

Process from Point/Station 38.200 to Point/Station 31.000
**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
Stream flow area = 2.120(Ac.)
Runoff from this stream = 8.644(CFS)
Time of concentration = 10.39 min.
Rainfall intensity = 5.754(In/Hr)
Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	19.125	9.56	6.072
2	8.644	10.39	5.754
Qmax(1) =			
	1.000 *	1.000 *	19.125) +
	1.000 *	0.920 *	8.644) + =
			27.077
Qmax(2) =			
	0.948 *	1.000 *	19.125) +
	1.000 *	1.000 *	8.644) + =
			26.766

Total of 2 streams to confluence:

Flow rates before confluence point:

19.125 8.644

Maximum flow rates at confluence using above data:

27.077 26.766

Area of streams before confluence:

4.420 2.120

Results of confluence:

Total flow rate = 27.077(CFS)

Time of concentration = 9.556 min.

Effective stream area after confluence = 6.540 (Ac.)

Process from Point/Station 31.000 to Point/Station 32.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 380.000(Ft.)
Downstream point/station elevation = 377.500(Ft.)
Pipe length = 84.47(Ft.) Manning's N = 0.015
No. of pipes = 1 Required pipe flow = 27.077(CFS)
Given pipe size = 24.00(In.)
Calculated individual pipe flow = 27.077(CFS)
Normal flow depth in pipe = 16.29(In.)
Flow top width inside pipe = 22.41(In.)
Critical Depth = 21.73(In.)
Pipe flow velocity = 11.94(Ft/s)
Travel time through pipe = 0.12 min.
Time of concentration (TC) = 9.67 min.

Process from Point/Station 31.000 to Point/Station 32.000
**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
Stream flow area = 6.540(Ac.)
Runoff from this stream = 27.077(CFS)
Time of concentration = 9.67 min.
Rainfall intensity = 6.024(In/Hr)

Process from Point/Station 29.000 to Point/Station 32.000
**** USER DEFINED FLOW INFORMATION AT A POINT ****

Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.500
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.500
[HIGH DENSITY RESIDENTIAL]
(24.0 DU/A or Less)
Impervious value, Ai = 0.650
Sub-Area C Value = 0.690
Rainfall intensity (I) = 5.811(In/Hr) for a 100.0 year storm
User specified values are as follows:
TC = 10.23 min. Rain intensity = 5.81(In/Hr)
Total area = 1.540(Ac.) Total runoff = 6.766(CFS)

Process from Point/Station 29.000 to Point/Station 32.000
**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2

Stream flow area = 1.540(Ac.)
Runoff from this stream = 6.766(CFS)
Time of concentration = 10.23 min.
Rainfall intensity = 5.811(In/Hr)
Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	27.077	9.67	6.024
2	6.766	10.23	5.811
Qmax(1) =			
	1.000 *	1.000 *	27.077) +
	1.000 *	0.946 *	6.766) + =
			33.475
Qmax(2) =			
	0.965 *	1.000 *	27.077) +
	1.000 *	1.000 *	6.766) + =
			32.885

Total of 2 streams to confluence:

Flow rates before confluence point:

27.077 6.766

Maximum flow rates at confluence using above data:

33.475 32.885

Area of streams before confluence:

6.540 1.540

Results of confluence:

Total flow rate = 33.475(CFS)

Time of concentration = 9.674 min.

Effective stream area after confluence = 8.080 (Ac.)

 Process from Point/Station 32.000 to Point/Station 33.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 377.500(Ft.)
 Downstream point/station elevation = 375.500(Ft.)
 Pipe length = 275.13(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 33.475(CFS)
 Given pipe size = 36.00(In.)
 Calculated individual pipe flow = 33.475(CFS)
 Normal flow depth in pipe = 21.75(In.)
 Flow top width inside pipe = 35.21(In.)
 Critical Depth = 22.53(In.)
 Pipe flow velocity = 7.49(Ft/s)
 Travel time through pipe = 0.61 min.
 Time of concentration (TC) = 10.29 min.

 Process from Point/Station 32.000 to Point/Station 33.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 8.080(Ac.)
 Runoff from this stream = 33.475(CFS)
 Time of concentration = 10.29 min.
 Rainfall intensity = 5.791(In/Hr)

 Process from Point/Station 39.100 to Point/Station 39.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.800
 Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.200
 [HIGH DENSITY RESIDENTIAL]
 (24.0 DU/A or Less)
 Impervious value, Ai = 0.650
 Sub-Area C Value = 0.678
 Initial subarea total flow distance = 574.000(Ft.)
 Highest elevation = 390.000(Ft.)
 Lowest elevation = 382.000(Ft.)
 Elevation difference = 8.000(Ft.) Slope = 1.394 %
 Top of Initial Area Slope adjusted by User to 1.730 %
 Bottom of Initial Area Slope adjusted by User to 1.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 75.00 (Ft)
 for the top area slope value of 1.73 %, in a development type of
 24.0 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.48 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})$
 $TC = [1.8 * (1.1 - 0.6780) * (75.000^{.5})] / (1.730^{(1/3)}) = 5.48$
 The initial area total distance of 574.00 (Ft.) entered leaves a
 remaining distance of 499.00 (Ft.)

Using Figure 3-4, the travel time for this distance is 5.50 minutes
 for a distance of 499.00 (Ft.) and a slope of 1.00 %
 with an elevation difference of 4.99(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60(min/hr)$
 = 5.497 Minutes
 $Tt = [11.9 * 0.0945^3] / (4.99)^{.385} = 5.50$
 Total initial area Ti = 5.48 minutes from Figure 3-3 formula plus
 5.50 minutes from the Figure 3-4 formula = 10.98 minutes
 Rainfall intensity (I) = 5.553(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.678
 Subarea runoff = 8.170(CFS)
 Total initial stream area = 2.170(Ac.)

 Process from Point/Station 39.200 to Point/Station 33.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 377.000(Ft.)
 Downstream point/station elevation = 375.500(Ft.)
 Pipe length = 9.45(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 8.170(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 8.170(CFS)
 Normal flow depth in pipe = 5.24(In.)
 Flow top width inside pipe = 19.83(In.)
 Critical Depth = 12.21(In.)
 Pipe flow velocity = 16.11(Ft/s)
 Travel time through pipe = 0.01 min.
 Time of concentration (TC) = 10.99 min.

 Process from Point/Station 39.200 to Point/Station 33.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 2.170(Ac.)
 Runoff from this stream = 8.170(CFS)
 Time of concentration = 10.99 min.
 Rainfall intensity = 5.550(In/Hr)

 Process from Point/Station 40.100 to Point/Station 40.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.400
 Decimal fraction soil group B = 0.600
 Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.434
 Initial subarea total flow distance = 350.000(Ft.)
 Highest elevation = 384.500(Ft.)

Lowest elevation = 382.000(Ft.)
 Elevation difference = 2.500(Ft.) Slope = 0.714 %
 Top of Initial Area Slope adjusted by User to 1.000 %
 Bottom of Initial Area Slope adjusted by User to 1.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 70.00 (Ft)
 for the top area slope value of 1.00 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 10.03 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})$
 $TC = [1.8 * (1.1 - 0.4340) * (70.000^{.5})] / (1.000^{(1/3)}) = 10.03$
 The initial area total distance of 350.00 (Ft.) entered leaves a
 remaining distance of 280.00 (Ft.)
 Using Figure 3-4, the travel time for this distance is 3.52 minutes
 for a distance of 280.00 (Ft.) and a slope of 1.00 %
 with an elevation difference of 2.80(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60(min/hr)$
 $= 3.523 \text{ Minutes}$
 $Tt = [(11.9 * 0.0530^3) / (2.80)]^{.385} = 3.52$
 Total initial area $Ti = 10.03$ minutes from Figure 3-3 formula plus
 3.52 minutes from the Figure 3-4 formula = 13.55 minutes
 Rainfall intensity (I) = 4.847(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.434
 Subarea runoff = 1.304(CFS)
 Total initial stream area = 0.620(Ac.)

 Process from Point/Station 40.200 to Point/Station 33.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 377.000(Ft.)
 Downstream point/station elevation = 375.500(Ft.)
 Pipe length = 29.23(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 1.304(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 1.304(CFS)
 Normal flow depth in pipe = 2.82(In.)
 Flow top width inside pipe = 15.47(In.)
 Critical depth could not be calculated.
 Pipe flow velocity = 6.28(Ft/s)
 Travel time through pipe = 0.08 min.
 Time of concentration (TC) = 13.63 min.

 Process from Point/Station 40.200 to Point/Station 33.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 3
 Stream flow area = 0.620(Ac.)
 Runoff from this stream = 1.304(CFS)
 Time of concentration = 13.63 min.
 Rainfall intensity = 4.829(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	33.475	10.29	5.791
2	8.170	10.99	5.550
3	1.304	13.63	4.829
Qmax(1) =			
	1.000 *	1.000 *	33.475) +
	1.000 *	0.936 *	8.170) +
	1.000 *	0.755 *	1.304) + =
Qmax(2) =			
	0.958 *	1.000 *	33.475) +
	1.000 *	1.000 *	8.170) +
	1.000 *	0.806 *	1.304) + =
Qmax(3) =			
	0.834 *	1.000 *	33.475) +
	0.870 *	1.000 *	8.170) +
	1.000 *	1.000 *	1.304) + =

Total of 3 streams to confluence:
 Flow rates before confluence point:
 33.475 8.170 1.304
 Maximum flow rates at confluence using above data:
 42.109 41.304 36.331
 Area of streams before confluence:
 8.080 2.170 0.620
 Results of confluence:
 Total flow rate = 42.109(CFS)
 Time of concentration = 10.286 min.
 Effective stream area after confluence = 10.870(Ac.)

 Process from Point/Station 33.000 to Point/Station 34.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 375.500(Ft.)
 Downstream point/station elevation = 373.500(Ft.)
 Pipe length = 251.22(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 42.109(CFS)
 Given pipe size = 36.00(In.)
 Calculated individual pipe flow = 42.109(CFS)
 Normal flow depth in pipe = 24.73(In.)
 Flow top width inside pipe = 33.39(In.)
 Critical Depth = 25.34(In.)
 Pipe flow velocity = 8.14(Ft/s)
 Travel time through pipe = 0.51 min.
 Time of concentration (TC) = 10.80 min.

 Process from Point/Station 33.000 to Point/Station 34.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 10.870(Ac.)
 Runoff from this stream = 42.109(CFS)
 Time of concentration = 10.80 min.
 Rainfall intensity = 5.611(In/Hr)

 Process from Point/Station 41.100 to Point/Station 41.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000
 Decimal fraction soil group B = 0.950
 Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.050
 [HIGH DENSITY RESIDENTIAL]
 (24.0 DU/A or Less)
 Impervious value, Ai = 0.650
 Sub-Area C Value = 0.672
 Initial subarea total flow distance = 527.700(Ft.)
 Highest elevation = 408.000(Ft.)
 Lowest elevation = 380.000(Ft.)
 Elevation difference = 28.000(Ft.) Slope = 5.306 %
 Top of Initial Area Slope adjusted by User to 5.000 %
 Bottom of Initial Area Slope adjusted by User to 1.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 95.00 (Ft)
 for the top area slope value of 5.00 %, in a development type of
 24.0 DU/A or Less

In Accordance With Figure 3-3
 Initial Area Time of Concentration = 4.39 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.6720) * (95.000^{.5})] / (5.000^{(1/3)}) = 4.39$
 The initial area total distance of 527.70 (Ft.) entered leaves a
 remaining distance of 432.70 (Ft.)
 Using Figure 3-4, the travel time for this distance is 4.93 minutes
 for a distance of 432.70 (Ft.) and a slope of 1.00 %
 with an elevation difference of 4.33(Ft.) from the end of the top area
 $Tt = [(11.9 * length(Mi)^3) / (elevation change(Ft.))]^{.385} * 60(min/hr)$
 $= 4.926$ Minutes
 $Tt = [(11.9 * 0.0820^3) / (4.33)]^{.385} = 4.93$
 Total initial area Ti = 4.39 minutes from Figure 3-3 formula plus
 4.93 minutes from the Figure 3-4 formula = 9.32 minutes
 Rainfall intensity (I) = 6.173(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.672
 Subarea runoff = 5.185(CFS)
 Total initial stream area = 1.250(Ac.)

 Process from Point/Station 41.200 to Point/Station 34.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 375.000(Ft.)
 Downstream point/station elevation = 373.500(Ft.)
 Pipe length = 21.70(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 5.185(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 5.185(CFS)
 Normal flow depth in pipe = 5.14(In.)
 Flow top width inside pipe = 19.69(In.)
 Critical Depth = 9.62(In.)
 Pipe flow velocity = 10.51(Ft/s)
 Travel time through pipe = 0.03 min.
 Time of concentration (TC) = 9.35 min.

 Process from Point/Station 41.200 to Point/Station 34.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 1.250(Ac.)
 Runoff from this stream = 5.185(CFS)
 Time of concentration = 9.35 min.
 Rainfall intensity = 6.158(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	42.109	10.80	5.611
2	5.185	9.35	6.158
Qmax(1) =	1.000 * 0.911 *	1.000 * 1.000 *	42.109) + 5.185) + = 46.833
Qmax(2) =	1.000 * 1.000 *	0.866 * 1.000 *	42.109) + 5.185) + = 41.642

Total of 2 streams to confluence:
 Flow rates before confluence point:
 42.109 5.185
 Maximum flow rates at confluence using above data:
 46.833 41.642
 Area of streams before confluence:
 10.870 1.250
 Results of confluence:
 Total flow rate = 46.833(CFS)
 Time of concentration = 10.801 min.
 Effective stream area after confluence = 12.120(Ac.)

 Process from Point/Station 34.000 to Point/Station 35.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 373.500(Ft.)
 Downstream point/station elevation = 372.000(Ft.)
 Pipe length = 183.85(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 46.833(CFS)
 Given pipe size = 36.00(In.)
 Calculated individual pipe flow = 46.833(CFS)
 Normal flow depth in pipe = 26.63(In.)
 Flow top width inside pipe = 31.60(In.)
 Critical Depth = 26.75(In.)
 Pipe flow velocity = 8.36(Ft/s)
 Travel time through pipe = 0.37 min.
 Time of concentration (TC) = 11.17 min.

 Process from Point/Station 34.000 to Point/Station 35.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 2
 Stream flow area = 12.120(Ac.)
 Runoff from this stream = 46.833(CFS)
 Time of concentration = 11.17 min.
 Rainfall intensity = 5.492(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	29.474	12.18	5.193
2	46.833	11.17	5.492
Qmax(1) =	1.000 * 29.474 + 0.946 * 46.833	1.000 * 12.18 + 1.000 * 11.17	29.474 * 5.193 + 46.833 * 5.492
Qmax(2) =	1.000 * 29.474 + 1.000 * 46.833	0.917 * 12.18 + 1.000 * 11.17	29.474 * 5.193 + 46.833 * 5.492

Total of 2 main streams to confluence:
 Flow rates before confluence point:
 29.474 46.833
 Maximum flow rates at confluence using above data:
 73.762 73.862
 Area of streams before confluence:
 11.520 12.120

Results of confluence:
 Total flow rate = 73.862(CFS)
 Time of concentration = 11.168 min.
 Effective stream area after confluence = 23.640(Ac.)

 Process from Point/Station 35.000 to Point/Station 36.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 372.000(Ft.)
 Downstream point/station elevation = 368.500(Ft.)
 Pipe length = 171.00(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 73.862(CFS)
 Given pipe size = 36.00(In.)
 Calculated individual pipe flow = 73.862(CFS)
 Normal flow depth in pipe = 26.53(In.)
 Flow top width inside pipe = 31.70(In.)
 Critical Depth = 32.48(In.)
 Pipe flow velocity = 13.23(Ft/s)
 Travel time through pipe = 0.22 min.
 Time of concentration (TC) = 11.38 min.

 Process from Point/Station 35.000 to Point/Station 36.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 1
 Stream flow area = 23.640(Ac.)
 Runoff from this stream = 73.862(CFS)
 Time of concentration = 11.38 min.
 Rainfall intensity = 5.424(In/Hr)
 Program is now starting with Main Stream No. 2

 Process from Point/Station 42.100 to Point/Station 42.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.200
 Decimal fraction soil group B = 0.500
 Decimal fraction soil group C = 0.300
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)
 Impervious value, Ai = 0.300
 Sub-Area C Value = 0.451
 Initial subarea total flow distance = 740.000(Ft.)
 Highest elevation = 422.000(Ft.)
 Lowest elevation = 374.000(Ft.)
 Elevation difference = 48.000(Ft.) Slope = 6.486 %
 Top of Initial Area Slope adjusted by User to 8.370 %
 Bottom of Initial Area Slope adjusted by User to 3.500 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 100.00 (Ft)
 for the top area slope value of 8.37 %, in a development type of 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 5.75 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5} / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.4510) * (100.000^{.5}) / (8.370^{(1/3)})] = 5.75$

The initial area total distance of 740.00 (Ft.) entered leaves a remaining distance of 640.00 (Ft.)
 Using Figure 3-4, the travel time for this distance is 4.11 minutes for a distance of 640.00 (Ft.) and a slope of 3.50 %
 with an elevation difference of 22.40(Ft.) from the end of the top area
 $Tt = [11.9 * \text{length}(\text{Mi})^3 / (\text{elevation change}(\text{Ft.}))]^{\wedge}.385 * 60(\text{min/hr})$
 $= 4.110 \text{ Minutes}$
 $Tt = [(11.9 * 0.1212^3) / (22.40)]^{\wedge}.385 = 4.11$
 Total initial area $Ti = 5.75$ minutes from Figure 3-3 formula plus
 4.11 minutes from the Figure 3-4 formula = 9.86 minutes
 Rainfall intensity (I) = 5.949(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is $C = 0.451$
 Subarea runoff = 4.508(CFS)
 Total initial stream area = 1.680(Ac.)

 Process from Point/Station 42.200 to Point/Station 36.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 369.000(Ft.)
 Downstream point/station elevation = 368.500(Ft.)
 Pipe length = 20.00(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 4.508(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 4.508(CFS)
 Normal flow depth in pipe = 6.18(In.)
 Flow top width inside pipe = 20.99(In.)
 Critical Depth = 8.94(In.)
 Pipe flow velocity = 7.03(Ft/s)
 Travel time through pipe = 0.05 min.
 Time of concentration (TC) = 9.91 min.

 Process from Point/Station 42.200 to Point/Station 36.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 2
 Stream flow area = 1.680(Ac.)
 Runoff from this stream = 4.508(CFS)
 Time of concentration = 9.91 min.
 Rainfall intensity = 5.931(In/Hr)
 Program is now starting with Main Stream No. 3

 Process from Point/Station 43.100 to Point/Station 43.200
 **** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.100
 Decimal fraction soil group B = 0.700
 Decimal fraction soil group C = 0.200
 Decimal fraction soil group D = 0.000
 [MEDIUM DENSITY RESIDENTIAL]
 (4.3 DU/A or Less)

Impervious value, $Ai = 0.300$
 Sub-Area C Value = 0.452
 Initial subarea total flow distance = 667.700(Ft.)
 Highest elevation = 410.000(Ft.)
 Lowest elevation = 373.000(Ft.)
 Elevation difference = 37.000(Ft.) Slope = 5.541 %
 Top of Initial Area Slope adjusted by User to 2.000 %
 Bottom of Initial Area Slope adjusted by User to 3.840 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 80.00 (Ft)
 for the top area slope value of 2.00 %, in a development type of
 4.3 DU/A or Less
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 8.28 minutes
 $TC = [1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{\wedge}.5] / (\% \text{ slope}^{\wedge}(1/3))$
 $TC = [1.8 * (1.1 - 0.4520) * (80.000^{\wedge}.5)] / (2.000^{\wedge}(1/3)) = 8.28$
 The initial area total distance of 667.70 (Ft.) entered leaves a remaining distance of 587.70 (Ft.)
 Using Figure 3-4, the travel time for this distance is 3.71 minutes for a distance of 587.70 (Ft.) and a slope of 3.84 %
 with an elevation difference of 22.57(Ft.) from the end of the top area
 $Tt = [11.9 * \text{length}(\text{Mi})^3 / (\text{elevation change}(\text{Ft.}))]^{\wedge}.385 * 60(\text{min/hr})$
 $= 3.714 \text{ Minutes}$
 $Tt = [(11.9 * 0.1113^3) / (22.57)]^{\wedge}.385 = 3.71$
 Total initial area $Ti = 8.28$ minutes from Figure 3-3 formula plus
 3.71 minutes from the Figure 3-4 formula = 11.99 minutes
 Rainfall intensity (I) = 5.244(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is $C = 0.452$
 Subarea runoff = 1.896(CFS)
 Total initial stream area = 0.800(Ac.)

 Process from Point/Station 43.200 to Point/Station 36.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 369.000(Ft.)
 Downstream point/station elevation = 368.500(Ft.)
 Pipe length = 33.32(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 1.896(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 1.896(CFS)
 Normal flow depth in pipe = 4.56(In.)
 Flow top width inside pipe = 18.83(In.)
 Critical Depth = 5.72(In.)
 Pipe flow velocity = 4.56(Ft/s)
 Travel time through pipe = 0.12 min.
 Time of concentration (TC) = 12.12 min.

 Process from Point/Station 43.200 to Point/Station 36.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 3
 Stream flow area = 0.800 (Ac.)
 Runoff from this stream = 1.896 (CFS)
 Time of concentration = 12.12 min.
 Rainfall intensity = 5.210 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	73.862	11.38	5.424
2	4.508	9.91	5.931
3	1.896	12.12	5.210

Qmax(1) =
 1.000 * 1.000 * 73.862) +
 0.915 * 1.000 * 4.508) +
 1.000 * 0.939 * 1.896) + = 79.766

Qmax(2) =
 1.000 * 0.871 * 73.862) +
 1.000 * 1.000 * 4.508) +
 1.000 * 0.818 * 1.896) + = 70.372

Qmax(3) =
 0.961 * 1.000 * 73.862) +
 0.878 * 1.000 * 4.508) +
 1.000 * 1.000 * 1.896) + = 76.803

Total of 3 main streams to confluence:

Flow rates before confluence point:
 73.862 4.508 1.896

Maximum flow rates at confluence using above data:

79.766 70.372 76.803

Area of streams before confluence:

23.640 1.680 0.800

Results of confluence:

Total flow rate = 79.766 (CFS)

Time of concentration = 11.383 min.

Effective stream area after confluence = 26.120 (Ac.)

 Process from Point/Station 36.000 to Point/Station 37.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 368.500 (Ft.)
 Downstream point/station elevation = 368.000 (Ft.)
 Pipe length = 66.44 (Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 79.766 (CFS)
 Given pipe size = 42.00 (In.)
 NOTE: Normal flow is pressure flow in user selected pipe size.
 The approximate hydraulic grade line above the pipe invert is
 0.589 (Ft.) at the headworks or inlet of the pipe(s)
 Pipe friction loss = 0.556 (Ft.)
 Minor friction loss = 0.534 (Ft.) K-factor = 0.50
 Pipe flow velocity = 8.29 (Ft/s)
 Travel time through pipe = 0.13 min.
 Time of concentration (TC) = 11.52 min.
 End of computations, total study area = 26.120 (Ac.)

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c)1991-2003 Version 7.3

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 11/08/12

***** Hydrology Study Control Information *****

WARNER RANCH

100-YEAR STORM EVENT DEVELOPMENT CONDITIONS

BIO-RETENTION POND 2

Shapouri & Associates, Rancho Santa Fe, CA - S/N 968

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:

6 hour, precipitation(inches) = 3.500

24 hour precipitation(inches) = 6.000

P6/P24 = 58.3%

San Diego hydrology manual 'C' values used

Process from Point/Station 34.100 to Point/Station 34.200
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.600

Decimal fraction soil group C = 0.000

Decimal fraction soil group D = 0.400

[HIGH DENSITY RESIDENTIAL
(24.0 DU/A or Less)]

Impervious value, Ai = 0.650

Sub-Area C Value = 0.686

Initial subarea total flow distance = 339.000(Ft.)

Highest elevation = 388.000(Ft.)

Lowest elevation = 384.000(Ft.)

Elevation difference = 4.000(Ft.) Slope = 1.180 %

Top of Initial Area Slope adjusted by User to 1.000 %

Bottom of Initial Area Slope adjusted by User to 1.000 %

INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:

The maximum overland flow distance is 65.00 (Ft)

for the top area slope value of 1.00 %, in a development type of
24.0 DU/A or Less

In Accordance With Figure 3-3

Initial Area Time of Concentration = 6.01 minutes

TC = $[1.8 * (1.1 - C) * \text{distance}(\text{Ft.})^{.5}] / (\% \text{ slope}^{(1/3)})]$

TC = $[1.8 * (1.1 - 0.6860) * (65.000^{.5})] / (1.000^{(1/3)})] = 6.01$

The initial area total distance of 339.00 (Ft.) entered leaves a
remaining distance of 274.00 (Ft.)

Using Figure 3-4, the travel time for this distance is 3.46 minutes

for a distance of 274.00 (Ft.) and a slope of 1.00 %

with an elevation difference of 2.74(Ft.) from the end of the top area

Tt = $[11.9 * \text{length}(\text{Mi})^3 / (\text{elevation change}(\text{Ft.}))^{.385}] * 60(\text{min/hr})$

= 3.465 Minutes

Tt = $[(11.9 * 0.0519^3) / (2.74)]^{.385} = 3.46$

Total initial area Ti = 6.01 minutes from Figure 3-3 formula plus
3.46 minutes from the Figure 3-4 formula = 9.47 minutes

Rainfall intensity (I) = 6.107(In/Hr) for a 100.0 year storm

Effective runoff coefficient used for area (Q=KCIA) is C = 0.686

Subarea runoff = 5.320(CFS)

Total initial stream area = 1.270(Ac.)

Process from Point/Station 34.200 to Point/Station 29.000
**** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 379.000(Ft.)

Downstream point/station elevation = 378.500(Ft.)

Pipe length = 7.00(Ft.) Manning's N = 0.015

No. of pipes = 1 Required pipe flow = 5.320(CFS)

Given pipe size = 24.00(In.)

Calculated individual pipe flow = 5.320(CFS)

Normal flow depth in pipe = 5.17(In.)

Flow top width inside pipe = 19.73(In.)

Critical Depth = 9.75(In.)

Pipe flow velocity = 10.71(Ft/s)

Travel time through pipe = 0.01 min.

Time of concentration (TC) = 9.48 min.

Process from Point/Station 34.200 to Point/Station 29.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 1.270(Ac.)

Runoff from this stream = 5.320(CFS)

Time of concentration = 9.48 min.

Rainfall intensity = 6.102(In/Hr)

Program is now starting with Main Stream No. 2

Process from Point/Station 35.100 to Point/Station 35.200
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 0.000

Decimal fraction soil group B = 0.500

Decimal fraction soil group C = 0.000

Decimal fraction soil group D = 0.500

[Street and Roads]

(Paved)

Impervious value, Ai = 0.950

Sub-Area C Value = 0.870

Initial subarea total flow distance = 357.550(Ft.)

Highest elevation = 387.000(Ft.)

Lowest elevation = 384.000(Ft.)

Elevation difference = 3.000(Ft.) Slope = 0.839 %
 Top of Initial Area Slope adjusted by User to 1.000 %
 Bottom of Initial Area Slope adjusted by User to 1.000 %
 INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
 The maximum overland flow distance is 60.00 (Ft)
 for the top area slope value of 1.00 %, in a development type of
 General Industrial
 In Accordance With Figure 3-3
 Initial Area Time of Concentration = 3.21 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})$
 $TC = [1.8 * (1.1 - 0.8700) * (60.000^{.5})] / (1.000^{(1/3)}) = 3.21$
 The initial area total distance of 357.55 (Ft.) entered leaves a
 remaining distance of 297.55 (Ft.)
 Using Figure 3-4, the travel time for this distance is 3.69 minutes
 for a distance of 297.55 (Ft.) and a slope of 1.00 %
 with an elevation difference of 2.98(Ft.) from the end of the top area
 $Tt = [11.9 * length(Mi)^3] / (elevation change(Ft.))^{.385} * 60 (min/hr)$
 = 3.692 Minutes
 $Tt = [(11.9 * 0.0564^3) / (2.98)]^{.385} = 3.69$
 Total initial area $Ti = 3.21$ minutes from Figure 3-3 formula plus
 3.69 minutes from the Figure 3-4 formula = 6.90 minutes
 Rainfall intensity (I) = 7.493(In/Hr) for a 100.0 year storm
 Effective runoff coefficient used for area (Q=KCIA) is C = 0.870
 Subarea runoff = 1.760(CFS)
 Total initial stream area = 0.270 (Ac.)

 Process from Point/Station 35.200 to Point/Station 29.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 379.000(Ft.)
 Downstream point/station elevation = 378.500(Ft.)
 Pipe length = 26.24(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 1.760(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 1.760(CFS)
 Normal flow depth in pipe = 4.15(In.)
 Flow top width inside pipe = 18.15(In.)
 Critical Depth = 5.51(In.)
 Pipe flow velocity = 4.85(Ft/s)
 Travel time through pipe = 0.09 min.
 Time of concentration (TC) = 6.99 min.

 Process from Point/Station 35.200 to Point/Station 29.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:
 In Main Stream number: 2
 Stream flow area = 0.270 (Ac.)
 Runoff from this stream = 1.760 (CFS)
 Time of concentration = 6.99 min.
 Rainfall intensity = 7.430 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	5.320	9.48	6.102
2	1.760	6.99	7.430
Qmax(1) =			
	1.000 *	1.000 *	5.320) +
	0.821 *	1.000 *	1.760) + =
Qmax(2) =			
	1.000 *	0.737 *	5.320) +
	1.000 *	1.000 *	1.760) + =

Total of 2 main streams to confluence:

Flow rates before confluence point:
 5.320 1.760
 Maximum flow rates at confluence using above data:
 6.766 5.681
 Area of streams before confluence:
 1.270 0.270

Results of confluence:
 Total flow rate = 6.766(CFS)
 Time of concentration = 9.484 min.
 Effective stream area after confluence = 1.540 (Ac.)

 Process from Point/Station 29.000 to Point/Station 32.000
 **** PIPEFLOW TRAVEL TIME (User specified size) ****

Upstream point/station elevation = 378.500(Ft.)
 Downstream point/station elevation = 377.500(Ft.)
 Pipe length = 198.28(Ft.) Manning's N = 0.015
 No. of pipes = 1 Required pipe flow = 6.766(CFS)
 Given pipe size = 24.00(In.)
 Calculated individual pipe flow = 6.766(CFS)
 Normal flow depth in pipe = 11.80(In.)
 Flow top width inside pipe = 24.00(In.)
 Critical Depth = 11.07(In.)
 Pipe flow velocity = 4.40(Ft/s)
 Travel time through pipe = 0.75 min.
 Time of concentration (TC) = 10.23 min.
 End of computations, total study area = 1.540 (Ac.)

San Diego County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c)1991-2003 Version 7.3

Rational method hydrology program based on
San Diego County Flood Control Division 2003 hydrology manual
Rational Hydrology Study Date: 11/14/12

***** Hydrology Study Control Information *****

WARNER RANCH

100-YEAR STORM EVENT DEVELOPMENT CONDITIONS
BIO-RETENTION POND 2

Shapouri & Associates, Rancho Santa Fe, CA - S/N 968

Rational hydrology study storm event year is 100.0
English (in-lb) input data Units used

Map data precipitation entered:

6 hour, precipitation(inches) = 3.500
24 hour precipitation(inches) = 6.000
P6/P24 = 58.3%

San Diego hydrology manual 'C' values used

Process from Point/Station 44.100 to Point/Station 38.000
**** INITIAL AREA EVALUATION ****

Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
[UNDISTURBED NATURAL TERRAIN]
(Permanent Open Space)
Impervious value, Ai = 0.000
Sub-Area C Value = 0.200
Initial subarea total flow distance = 423.586(Ft.)
Highest elevation = 374.000(Ft.)
Lowest elevation = 367.000(Ft.)
Elevation difference = 7.000(Ft.) Slope = 1.653 %
Top of Initial Area Slope adjusted by User to 6.330 %
Bottom of Initial Area Slope adjusted by User to 0.300 %
INITIAL AREA TIME OF CONCENTRATION CALCULATIONS:
The maximum overland flow distance is 100.00 (Ft)
for the top area slope value of 6.33 %, in a development type of
Permanent Open Space
In Accordance With Figure 3-3
Initial Area Time of Concentration = 8.76 minutes
 $TC = [1.8 * (1.1 - C) * distance(Ft.)^{.5}] / (\% slope^{(1/3)})]$
 $TC = [1.8 * (1.1 - 0.2000) * (100.000^{.5}) / (6.330^{(1/3)})] = 8.76$
The initial area total distance of 423.59 (Ft.) entered leaves a
remaining distance of 323.59 (Ft.)
Using Figure 3-4, the travel time for this distance is 6.26 minutes
for a distance of 323.59 (Ft.) and a slope of 0.30 %
with an elevation difference of 0.97(Ft.) from the end of the top area

$Tt = [11.9 * length(Mi)^3] / (elevation\ change(Ft.))^{.385} * 60(min/hr)$
= 6.260 Minutes
 $Tt = [(11.9 * 0.0613^3) / (0.97)]^{.385} = 6.26$
Total initial area Ti = 8.76 minutes from Figure 3-3 formula plus
6.26 minutes from the Figure 3-4 formula = 15.02 minutes
Rainfall intensity (I) = 4.537(In/Hr) for a 100.0 year storm
Effective runoff coefficient used for area (Q=KCIA) is C = 0.200
Subarea runoff = 2.922(CFS)
Total initial stream area = 3.220(Ac.)

Process from Point/Station 44.100 to Point/Station 38.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
Stream flow area = 3.220(Ac.)
Runoff from this stream = 2.922(CFS)
Time of concentration = 15.02 min.
Rainfall intensity = 4.537(In/Hr)
Program is now starting with Main Stream No. 2

Process from Point/Station 37.000 to Point/Station 38.000
**** USER DEFINED FLOW INFORMATION AT A POINT ****

Decimal fraction soil group A = 0.050
Decimal fraction soil group B = 0.460
Decimal fraction soil group C = 0.330
Decimal fraction soil group D = 0.160
[MEDIUM DENSITY RESIDENTIAL]
(4.3 DU/A or Less)
Impervious value, Ai = 0.300
Sub-Area C Value = 0.469
Rainfall intensity (I) = 5.383(In/Hr) for a 100.0 year storm
User specified values are as follows:
TC = 11.52 min. Rain intensity = 5.38(In/Hr)
Total area = 26.120(Ac.) Total runoff = 79.766(CFS)

 Process from Point/Station 37.000 to Point/Station 38.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 26.120 (Ac.)
 Runoff from this stream = 79.766 (CFS)
 Time of concentration = 11.52 min.
 Rainfall intensity = 5.383 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	2.922	15.02	4.537
2	79.766	11.52	5.383

Qmax(1) =
 1.000 * 1.000 * 2.922) +
 0.843 * 1.000 * 79.766) + = 70.149

Qmax(2) =
 1.000 * 0.767 * 2.922) +
 1.000 * 1.000 * 79.766) + = 82.007

Total of 2 main streams to confluence:

Flow rates before confluence point:

2.922 79.766

Maximum flow rates at confluence using above data:

70.149 82.007

Area of streams before confluence:

3.220 26.120

Results of confluence:

Total flow rate = 82.007 (CFS)

Time of concentration = 11.520 min.

Effective stream area after confluence = 29.340 (Ac.)

 Process from Point/Station 37.000 to Point/Station 38.000
 **** 6 HOUR HYDROGRAPH ****

 Hydrograph Data - Section 6, San Diego County Hydrology manual, June 2003

Time (Min)	Discharge (CFS)
0	0.000
11	2.688
22	2.743
33	2.862
44	2.926
55	3.067
66	3.144
77	3.314
88	3.408
99	3.619
110	3.738
121	4.008
132	4.163
143	4.525
154	4.739
165	5.256
176	5.574
187	6.389
198	6.928
209	8.468
220	9.645
231	14.161
242	19.953
253	82.007
264	11.358
275	7.599
286	5.946
297	4.980
308	4.335
319	3.867
330	3.510
341	3.226
352	2.994
363	2.801

Job File: C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP
2\5508RPL4-100 YR-BMP 2.PPW
Rain Dir: C:\Shap_Ass\Active Projects\Warner Ranch\3rd Submittal\Pond Pack\BMP 2\

=====
JOB TITLE
=====

Project Date: 11/23/2012
Project Engineer: Shapouri & Associates
Project Title: 5508rpl4
Project Comments: NODE 38
5508rpl4 - Detention Pond - BMP 2

S/N:
PondPack Ver: Compute Time: Date:

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